Project Details

Project Name: Food Farm

Location: 2612 Co Rd 1, Wrenshall, MN 55797, USA

Unique ID: 5P-19.75-6TOP-SD-24-L-3Hx16W-D9DB

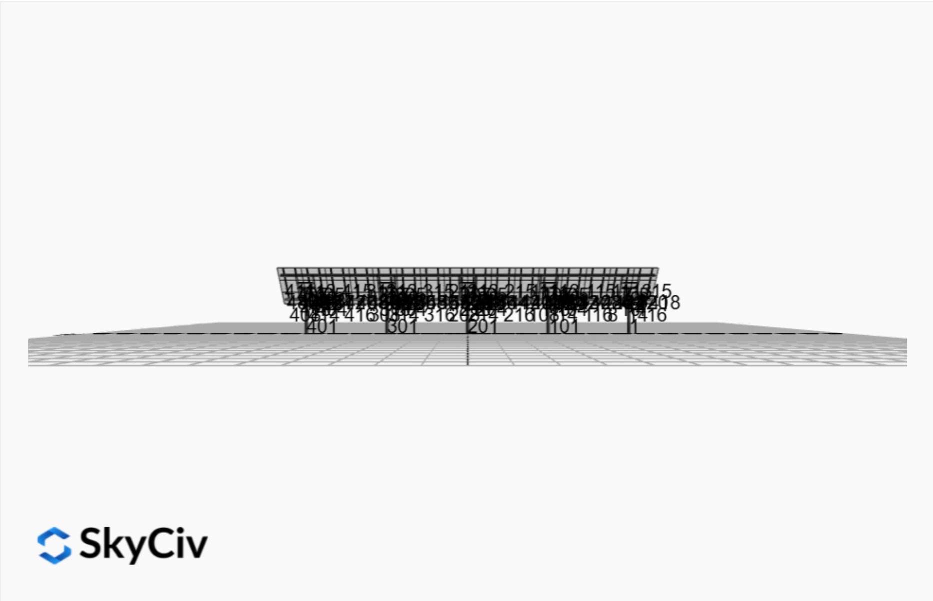
Dealer: ______________________________

Date: Sun Sep 14 2025

Number of Modules: 48

Number of Poles: 5

Date Sold: ______________________________



Array Dimensions N/S	11.30 ft
Array Dimensions E/W	91.73 ft
Winter Tilt Angle	50
Front Edge Clearance	7 ft

MT Solar Bill of Materials (5P-19.75-6TOP-SD-24-L-3Hx16W-D9DB)

Part	Short Description	BOM Qty
MTS-PC-6	6IN Pole Cap Assembly	5
MTS-HF-SD	H-Frame Assembly-SD	5
MTS-SD-Wing-24	24IN SD Wing	4
MTS-SD-Splice-90	90IN SD Splice	8
MTS-SD-Splice-57	57IN SD Splice	8
MTS-CLAMP-HOOK-4PK	Hook Clamp	16

Rail Bill of Materials

Part	Qty
Rails (136in)	32
Rail Attachment	64
Module Mid Clamp	64
Module End Clamp	64
Ground Lug	16

Site Details:

Site Address: 2612 Co Rd 1, Wrenshall, MN 55797, USA

Array Specification

Duty Classification:	SD
Module Width:	44.70 in
Module Length:	67.80in
Number of Rows:	3
Number of Columns:	16
Total Number of Modules:	48
Winter Tilt Angle:	50
Front Edge Clearance:	7
Total Array Height at Tilt:	15.66 ft
Total Frame Length:	90.50 ft
Module Info/Notes:	
Array Dimensions N/S:	11.30 ft
Array Dimensions E/W:	91.73 ft
Rail Length:	135.60 in
Rail Spacing:	2.87 ft

Support Specifications

Pole Size:	6in Pipe Sch 80
Pole Length above Grade:	11.33 ft
Number of Poles:	5
Pole Spacing:	19.75 ft

Foundation Specifications

Foundation Type:	Square
Foundation Dimensions:	48 x 48 in
Foundation Depth (below grade):	Pile 1: 5.75 ft Pile 2: 6.25 ft Pile 3: 6.25 ft Pile 4: 6.25 ft Pile 5: 5.75 ft
Foundation Volume:	17.926 y^3

Site Info

Risk Category:	I
Exposure:	C
Soil Classification:	sand
Site Location:	2612 Co Rd 1, Wrenshall, MN 55797, USA
Wind Speed:	100 mph

Snow Load:	60 psf

Design Disclaimer

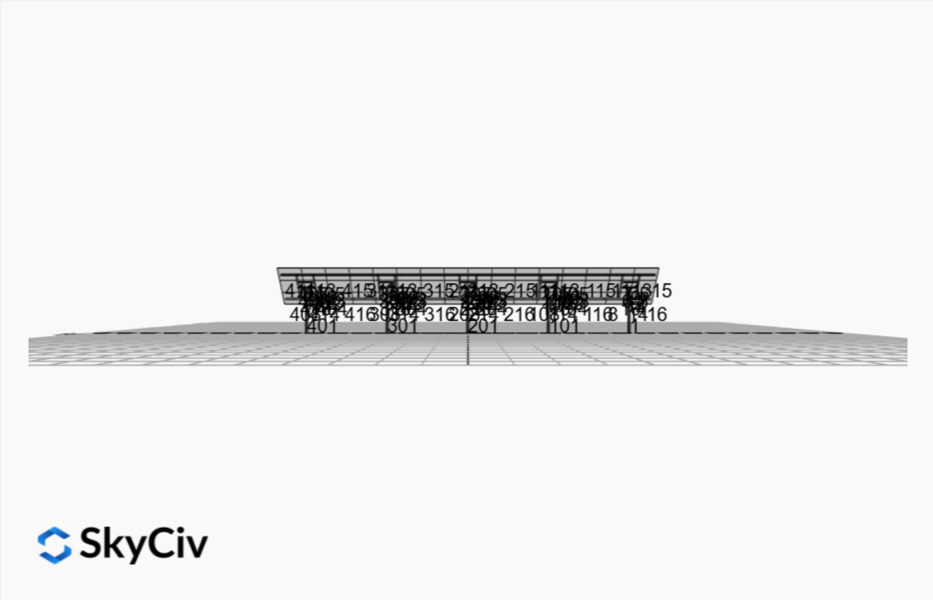
This software should be used for preliminary designs and should not be used as a final design unless reviewed, verified and designed by a qualified structural engineer.

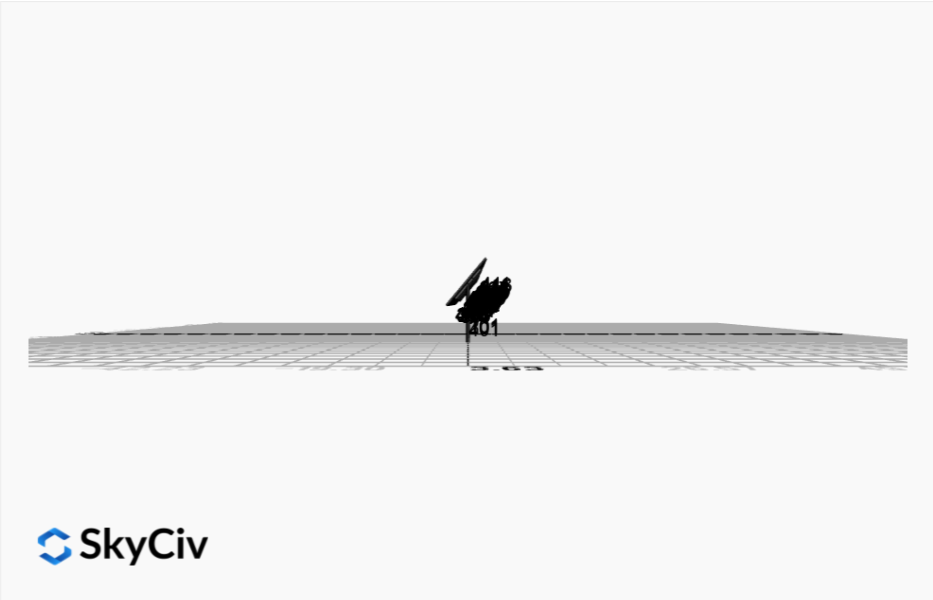
AutoDesigner Input

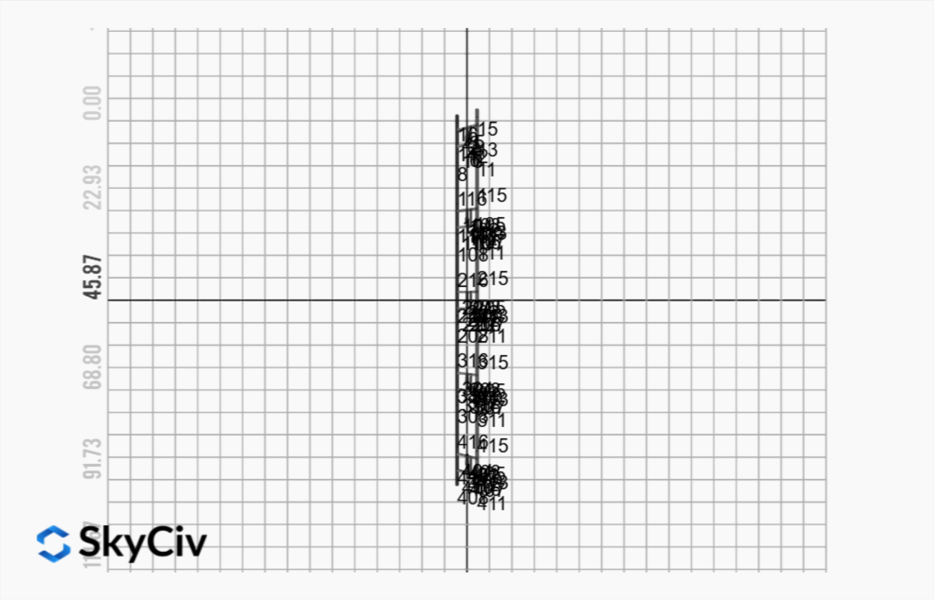
{"wind_speed_override":null,"snow_load_override":null,"direct_snow_load":false,"add_angle_brace":false,"product_type":"Beam","designer_name":"","designer_email":"john@star-power-electric.com","designer_phone":"","project_id":"Food Farm","site_address":"2612 Co Rd 1, Wrenshall, MN 55797,
USA","module_info":"","module_width":44.7,"module_length":67.8,"number_rows":"3","number_columns":16,"pole_mount_section":"4_40","core_pipe_width":65,"core_pipe_section":"2_40","adjuster_section":"2_40","core_beam_height":65,"core_beam_section":"HSS3x2x1/8","main_pipe_section":"2_12GA","pole_spacing":"15","tilt_angle":50,"ground_clearance":7,"risk_category":"I","exposure_category":"C","frame_duty_override":"auto","pole_override":"auto","soil_type":"sand","customer_foundation_override":"48_Square","foundation_type":"Square","foundation_size":48,"check_rails":false}

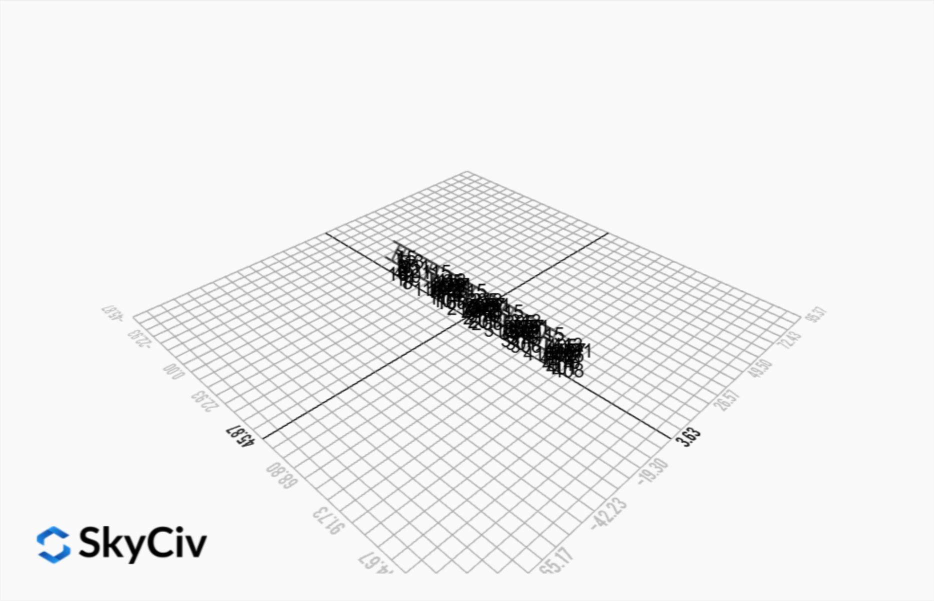
Design Notes:

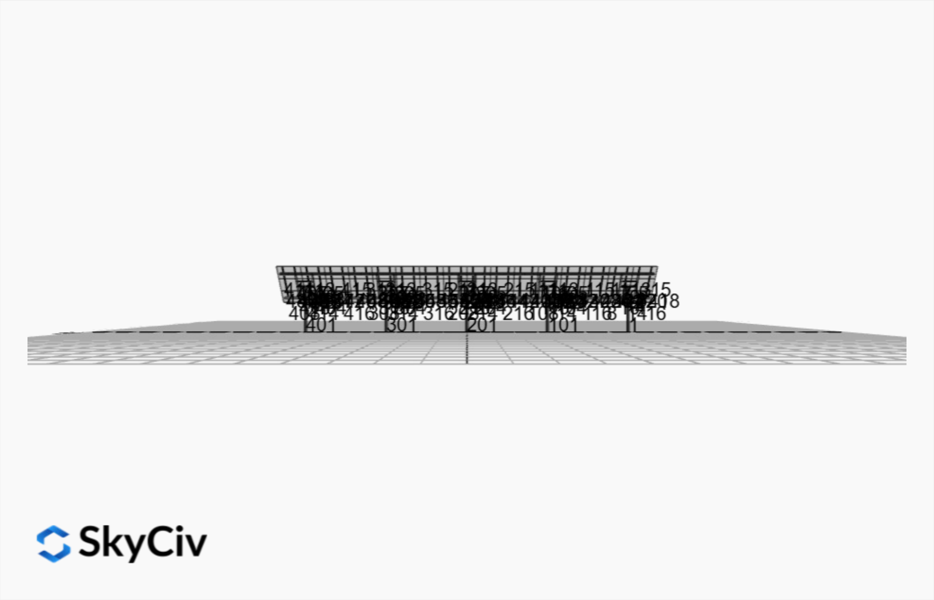
- AISC Deflection checks are set to L/1 due to structure design intent
- Foundation Soil Parameters used in this Autodesign are all estimates, proper geotechnical reports are required to confirm soil profiles
- Wind speeds, snow loads and other site specific results are based on ASCE 7 2016
- Steel frame design checks are based on AISC 360 2016 (LRFD)
- Foundation Design and Sizing is approximate only



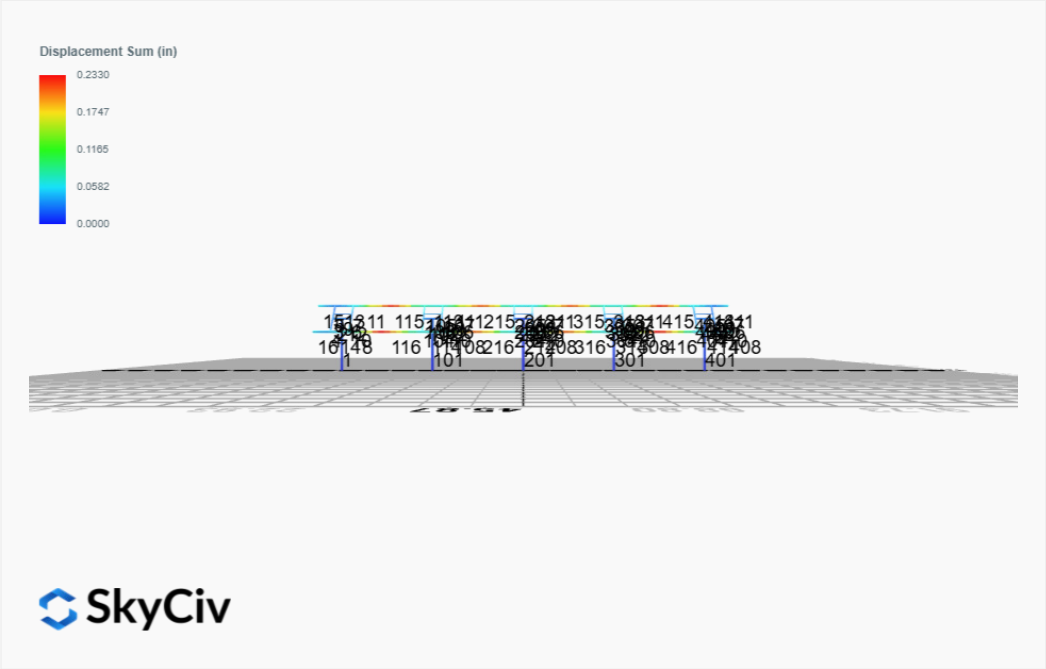


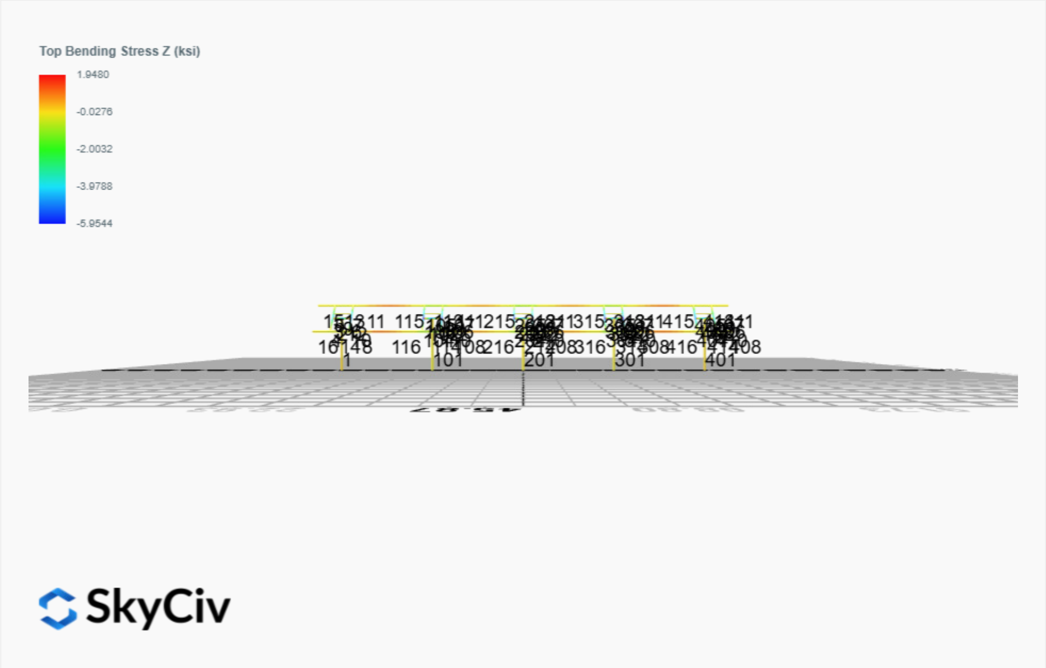

0.00
22.93
45.87
68.80
91.73

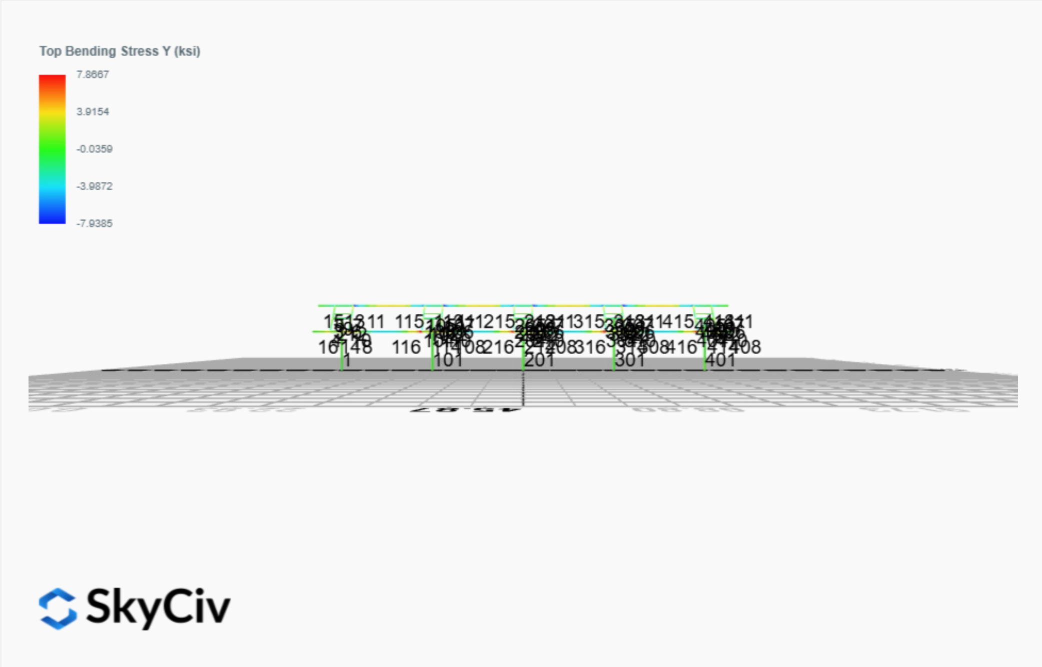

45.87
68.80
91.73
3.63
-19.30
-42.23
26.57
49.50

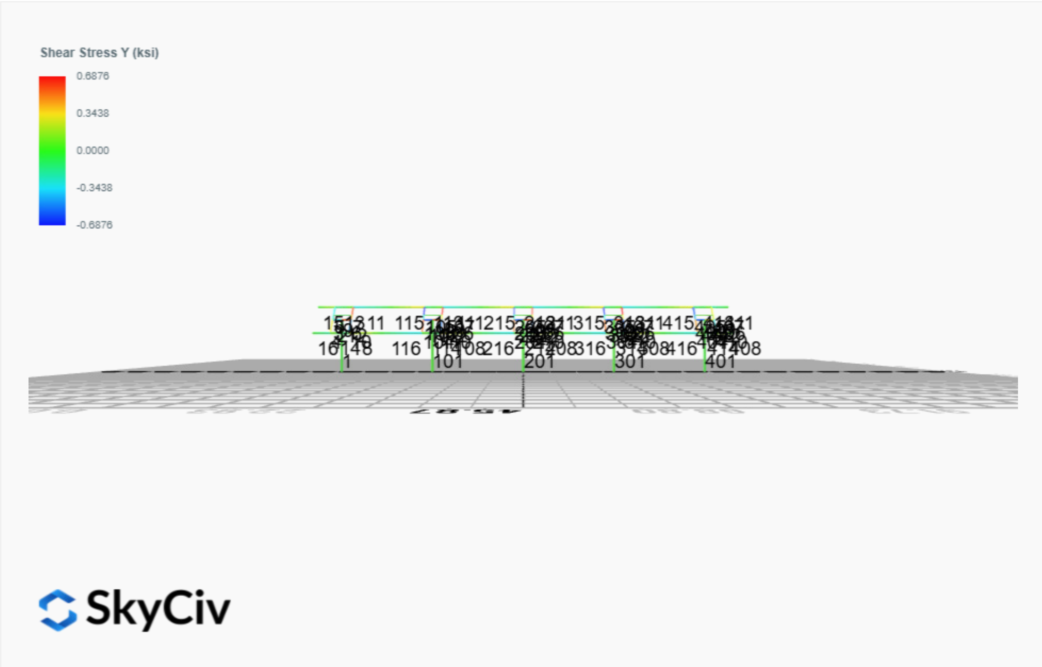


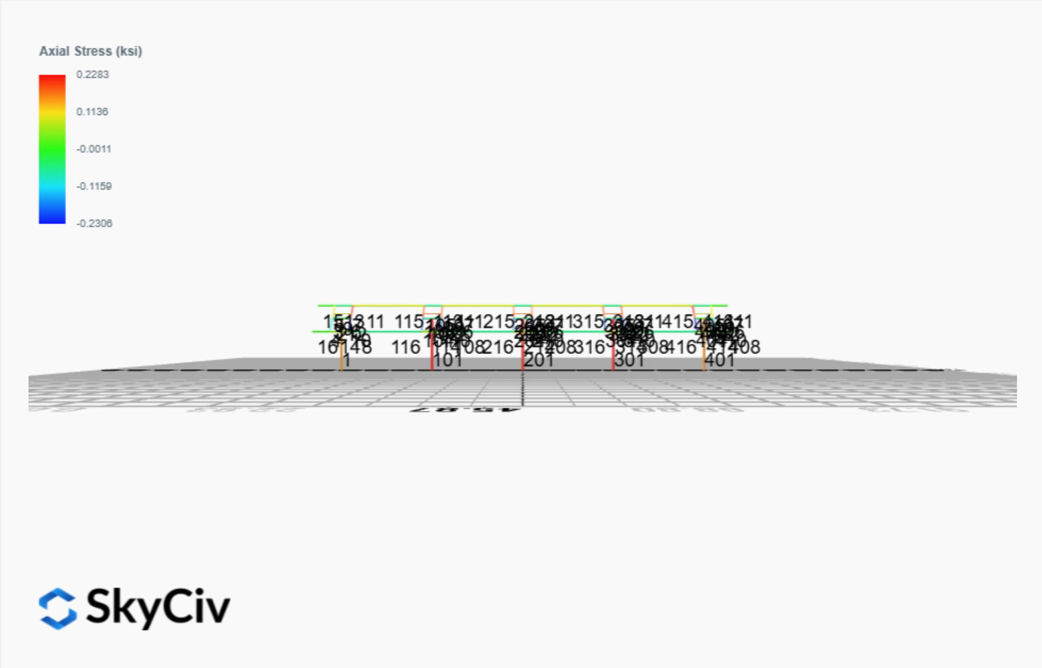
FEM Results (Envelope Worst Case for each member)






Top Bending Stress Y (ksi)
7.8667
3.9154
-0.0359
-3.9872
-7.9385


Shear Stress Y (ksi)
0.6876
0.3438
0.0000
-0.3438
-0.6876


Axial Stress (ksi)
0.2283
0.1136
-0.0011
-0.1159
-0.2306
SkyCiv

Reaction Forces for Foundation 1 (Node ID#1), (kip, kip-ft)

ASD Load Combination Results

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 1. D	0.0186	1.6193	0.0620	0.2164	-0.0815	-0.1828
ULS: 2. D + L	0.0186	1.6193	0.0620	0.2164	-0.0815	-0.1828
ULS: 3. D + (S or Lr or R)	0.0425	3.0918	0.1417	0.4950	-0.1867	-0.4335
ULS: 3. D + (S or Lr or R)	0.0186	1.6193	0.0620	0.2164	-0.0815	-0.1828
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	0.0365	2.7236	0.1218	0.4254	-0.1604	-0.3709
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	0.0186	1.6193	0.0620	0.2164	-0.0815	-0.1828
ULS: 5b. D + 0.7E	0.0186	1.6193	0.0620	0.2164	-0.0815	-0.1828
ULS: 6b. D + 0.75L + 0.75(0.7)E + 0.75S	0.0365	2.7236	0.1218	0.4254	-0.1604	-0.3709
ULS: 8. 0.6D + 0.7E	0.0112	0.9716	0.0372	0.1298	-0.0489	-0.1097
ULS: 5a. D + 0.6W_Wind downforce Case A only	-1.5733	2.9125	0.2112	0.7135	-0.7402	18.2126
ULS: 5a. D + 0.6W_Wind downforce Case B only	0.0186	1.6193	0.0620	0.2164	-0.0815	-0.1828
ULS: 5a. D + 0.6W_Wind uplift Case A only	1.6078	0.3267	-0.0842	-0.2698	0.5634	-18.1238
ULS: 5a. D + 0.6W_Wind uplift Case B only	0.0186	1.6193	0.0620	0.2164	-0.0815	-0.1828
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-1.1574	3.6936	0.2337	0.7982	-0.6545	13.4257
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	0.0365	2.7236	0.1218	0.4254	-0.1604	-0.3709
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	1.2284	1.7542	0.0121	0.0607	0.3232	-13.8266
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	0.0365	2.7236	0.1218	0.4254	-0.1604	-0.3709
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-1.1753	2.5892	0.1739	0.5892	-0.5756	13.6137
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	0.0186	1.6193	0.0620	0.2164	-0.0815	-0.1828
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	1.2105	0.6498	-0.0477	-0.1483	0.4021	-13.6386
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	0.0186	1.6193	0.0620	0.2164	-0.0815	-0.1828
ULS: 7. 0.6D + 0.6W_Wind downforce Case A only	-1.5808	2.2648	0.1864	0.6269	-0.7076	18.2857
ULS: 7. 0.6D + 0.6W_Wind downforce Case B only	0.0112	0.9716	0.0372	0.1298	-0.0489	-0.1097
ULS: 7. 0.6D + 0.6W_Wind uplift Case A only	1.6003	-0.3210	-0.1090	-0.3564	0.5960	-18.0507
ULS: 7. 0.6D + 0.6W_Wind uplift Case B only	0.0112	0.9716	0.0372	0.1298	-0.0489	-0.1097

Worst Case Reactions LRFD

These calculations are taken directly from the FEA via SkyCiv and are used in the Concrete Checks of the Foundatino Module.
Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	5.3768
Shear X	-2.6851
Shear Z	0.3657
Moment X	1.2377
Moment Y (Twist)	1.2602
Moment Z	30.8216

Worst Case Reactions ASD

These results are taken from the worst case values in the above table and are used in the Soil Checks in the Foundation Module.
Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	3.6936
Shear X	-1.6078
Shear Z	0.2337
Moment X	0.7982
Moment Y (Twist)	0.7402
Moment Z	18.2857

Reaction Forces for Foundation 2 (Node ID#101), (kip, kip-ft)

ASD Load Combination Results

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 1. D	-0.0180	1.9668	-0.0043	-0.0155	0.0171	0.1976
ULS: 2. D + L	-0.0180	1.9668	-0.0043	-0.0155	0.0171	0.1976
ULS: 3. D + (S or Lr or R)	-0.0412	3.8856	-0.0098	-0.0353	0.0390	0.4382
ULS: 3. D + (S or Lr or R)	-0.0180	1.9668	-0.0043	-0.0155	0.0171	0.1976
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	-0.0354	3.4059	-0.0084	-0.0303	0.0336	0.3781

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	-0.0180	1.9668	-0.0043	-0.0155	0.0171	0.1976
ULS: 5b. D + 0.7E	-0.0180	1.9668	-0.0043	-0.0155	0.0171	0.1976
ULS: 6b. D + 0.75L + 0.75(0.7)E + 0.75S	-0.0354	3.4059	-0.0084	-0.0303	0.0336	0.3781
ULS: 8. 0.6D + 0.7E	-0.0108	1.1801	-0.0026	-0.0093	0.0103	0.1186
ULS: 5a. D + 0.6W_Wind downforce Case A only	-2.0492	3.7102	0.0060	0.0149	-0.0748	23.3794
ULS: 5a. D + 0.6W_Wind downforce Case B only	-0.0180	1.9668	-0.0043	-0.0155	0.0171	0.1976
ULS: 5a. D + 0.6W_Wind uplift Case A only	2.0160	0.2223	-0.0127	-0.0396	0.0989	-22.3333
ULS: 5a. D + 0.6W_Wind uplift Case B only	-0.0180	1.9668	-0.0043	-0.0155	0.0171	0.1976
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-1.5588	4.7134	-0.0007	-0.0076	-0.0354	17.7644
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-0.0354	3.4059	-0.0084	-0.0303	0.0336	0.3781
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	1.4901	2.0975	-0.0147	-0.0484	0.0949	-16.5201
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	-0.0354	3.4059	-0.0084	-0.0303	0.0336	0.3781
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-1.5414	3.2743	0.0034	0.0073	-0.0518	17.5840
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-0.0180	1.9668	-0.0043	-0.0155	0.0171	0.1976
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	1.5075	0.6584	-0.0106	-0.0336	0.0784	-16.7006
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	-0.0180	1.9668	-0.0043	-0.0155	0.0171	0.1976
ULS: 7. 0.6D + 0.6W_Wind downforce Case A only	-2.0419	2.9234	0.0077	0.0211	-0.0816	23.3004
ULS: 7. 0.6D + 0.6W_Wind downforce Case B only	-0.0108	1.1801	-0.0026	-0.0093	0.0103	0.1186
ULS: 7. 0.6D + 0.6W_Wind uplift Case A only	2.0232	-0.5644	-0.0109	-0.0334	0.0920	-22.4123
ULS: 7. 0.6D + 0.6W_Wind uplift Case B only	-0.0108	1.1801	-0.0026	-0.0093	0.0103	0.1186

Worst Case Reactions LRFD

These calculations are taken directly from the FEA via SkyCiv and are used in the Concrete Checks of the Foundatino Module.
Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	6.8825
Shear X	-3.4155
Shear Z	-0.0233
Moment X	-0.0756
Moment Y (Twist)	0.1762
Moment Z	39.7051

Worst Case Reactions ASD

These results are taken from the worst case values in the above table and are used in the Soil Checks in the Foundation Module.
Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	4.7134
Shear X	-2.0492
Shear Z	-0.0147
Moment X	-0.0484
Moment Y (Twist)	0.0989
Moment Z	23.3794

Reaction Forces for Foundation 3 (Node ID#201), (kip, kip-ft)

ASD Load Combination Results

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 1. D	-0.0011	1.9456	0.0000	-0.0000	0.0000	0.0489
ULS: 2. D + L	-0.0011	1.9456	0.0000	-0.0000	0.0000	0.0489
ULS: 3. D + (S or Lr or R)	-0.0026	3.8370	0.0000	-0.0000	0.0000	0.0983
ULS: 3. D + (S or Lr or R)	-0.0011	1.9456	0.0000	-0.0000	0.0000	0.0489
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	-0.0022	3.3642	0.0000	-0.0000	0.0000	0.0860
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	-0.0011	1.9456	0.0000	-0.0000	0.0000	0.0489
ULS: 5b. D + 0.7E	-0.0011	1.9456	0.0000	-0.0000	0.0000	0.0489
ULS: 6b. D + 0.75L + 0.75(0.7)E + 0.75S	-0.0022	3.3642	0.0000	-0.0000	0.0000	0.0860
ULS: 8. 0.6D + 0.7E	-0.0007	1.1673	0.0000	-0.0000	0.0000	0.0293
ULS: 5a. D + 0.6W_Wind downforce Case A only	-2.0366	3.6605	0.0000	-0.0000	0.0000	23.5151
ULS: 5a. D + 0.6W_Wind downforce Case B only	-0.0011	1.9456	0.0000	-0.0000	0.0000	0.0489
ULS: 5a. D + 0.6W_Wind uplift Case A only	2.0340	0.2317	0.0000	-0.0000	0.0000	-22.7204
ULS: 5a. D + 0.6W_Wind uplift Case B only	-0.0011	1.9456	0.0000	-0.0000	0.0000	0.0489

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-1.5288	4.6503	0.0000	-0.0000	0.0000	17.6856
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-0.0022	3.3642	0.0000	-0.0000	0.0000	0.0860
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	1.5241	2.0788	0.0000	-0.0000	0.0000	-16.9910
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	-0.0022	3.3642	0.0000	-0.0000	0.0000	0.0860
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-1.5277	3.2317	0.0000	-0.0000	0.0000	17.6485
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-0.0011	1.9456	0.0000	-0.0000	0.0000	0.0489
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	1.5252	0.6602	0.0000	-0.0000	0.0000	-17.0280
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	-0.0011	1.9456	0.0000	-0.0000	0.0000	0.0489
ULS: 7. 0.6D + 0.6W_Wind downforce Case A only	-2.0361	2.8822	0.0000	-0.0000	0.0000	23.4955
ULS: 7. 0.6D + 0.6W_Wind downforce Case B only	-0.0007	1.1673	0.0000	-0.0000	0.0000	0.0293
ULS: 7. 0.6D + 0.6W_Wind uplift Case A only	2.0345	-0.5465	0.0000	-0.0000	0.0000	-22.7399
ULS: 7. 0.6D + 0.6W_Wind uplift Case B only	-0.0007	1.1673	0.0000	-0.0000	0.0000	0.0293

Worst Case Reactions LRFD

These calculations are taken directly from the FEA via SkyCiv and are used in the Concrete Checks of the Foundatino Module.
Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	6.7914
Shear X	-3.3951
Shear Z	0.0000
Moment X	-0.0001
Moment Y (Twist)	0.0001
Moment Z	39.9538

Worst Case Reactions ASD

These results are taken from the worst case values in the above table and are used in the Soil Checks in the Foundation Module.
Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	4.6503
Shear X	-2.0366
Shear Z	0.0000
Moment X	-0.0000
Moment Y (Twist)	0.0000
Moment Z	23.5151

Reaction Forces for Foundation 4 (Node ID#301), (kip, kip-ft)

ASD Load Combination Results

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 1. D	-0.0180	1.9668	0.0043	0.0155	-0.0171	0.1976
ULS: 2. D + L	-0.0180	1.9668	0.0043	0.0155	-0.0171	0.1976
ULS: 3. D + (S or Lr or R)	-0.0412	3.8856	0.0098	0.0353	-0.0390	0.4382
ULS: 3. D + (S or Lr or R)	-0.0180	1.9668	0.0043	0.0155	-0.0171	0.1976
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	-0.0354	3.4059	0.0084	0.0303	-0.0335	0.3781
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	-0.0180	1.9668	0.0043	0.0155	-0.0171	0.1976
ULS: 5b. D + 0.7E	-0.0180	1.9668	0.0043	0.0155	-0.0171	0.1976
ULS: 6b. D + 0.75L + 0.75(0.7)E + 0.75S	-0.0354	3.4059	0.0084	0.0303	-0.0335	0.3781
ULS: 8. 0.6D + 0.7E	-0.0108	1.1801	0.0026	0.0093	-0.0103	0.1186
ULS: 5a. D + 0.6W_Wind downforce Case A only	-2.0492	3.7102	-0.0060	-0.0149	0.0748	23.3794
ULS: 5a. D + 0.6W_Wind downforce Case B only	-0.0180	1.9668	0.0043	0.0155	-0.0171	0.1976
ULS: 5a. D + 0.6W_Wind uplift Case A only	2.0160	0.2223	0.0127	0.0396	-0.0988	-22.3333
ULS: 5a. D + 0.6W_Wind uplift Case B only	-0.0180	1.9668	0.0043	0.0155	-0.0171	0.1976
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-1.5588	4.7134	0.0007	0.0075	0.0354	17.7644
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-0.0354	3.4059	0.0084	0.0303	-0.0335	0.3781
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	1.4901	2.0975	0.0147	0.0484	-0.0948	-16.5201
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	-0.0354	3.4059	0.0084	0.0303	-0.0335	0.3781
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-1.5414	3.2743	-0.0034	-0.0073	0.0518	17.5840
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-0.0180	1.9668	0.0043	0.0155	-0.0171	0.1976
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	1.5075	0.6584	0.0106	0.0336	-0.0784	-16.7006
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	-0.0180	1.9668	0.0043	0.0155	-0.0171	0.1976

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 7. 0.6D + 0.6W_Wind downforce Case A only	-2.0419	2.9234	-0.0077	-0.0211	0.0816	23.3004
ULS: 7. 0.6D + 0.6W_Wind downforce Case B only	-0.0108	1.1801	0.0026	0.0093	-0.0103	0.1186
ULS: 7. 0.6D + 0.6W_Wind uplift Case A only	2.0232	-0.5644	0.0109	0.0334	-0.0920	-22.4123
ULS: 7. 0.6D + 0.6W_Wind uplift Case B only	-0.0108	1.1801	0.0026	0.0093	-0.0103	0.1186

Worst Case Reactions LRFD

These calculations are taken directly from the FEA via SkyCiv and are used in the Concrete Checks of the Foundatino Module.
Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	6.8825
Shear X	-3.4155
Shear Z	0.0233
Moment X	0.0756
Moment Y (Twist)	0.1761
Moment Z	39.7052

Worst Case Reactions ASD

These results are taken from the worst case values in the above table and are used in the Soil Checks in the Foundation Module.
Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	4.7134
Shear X	-2.0492
Shear Z	0.0147
Moment X	0.0484
Moment Y (Twist)	0.0988
Moment Z	23.3794

Reaction Forces for Foundation 5 (Node ID#401), (kip, kip-ft)

ASD Load Combination Results

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 1. D	0.0186	1.6193	-0.0620	-0.2164	0.0815	-0.1828
ULS: 2. D + L	0.0186	1.6193	-0.0620	-0.2164	0.0815	-0.1828
ULS: 3. D + (S or Lr or R)	0.0425	3.0918	-0.1417	-0.4951	0.1868	-0.4335
ULS: 3. D + (S or Lr or R)	0.0186	1.6193	-0.0620	-0.2164	0.0815	-0.1828
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	0.0365	2.7236	-0.1218	-0.4254	0.1605	-0.3708
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	0.0186	1.6193	-0.0620	-0.2164	0.0815	-0.1828
ULS: 5b. D + 0.7E	0.0186	1.6193	-0.0620	-0.2164	0.0815	-0.1828
ULS: 6b. D + 0.75L + 0.75(0.7)E + 0.75S	0.0365	2.7236	-0.1218	-0.4254	0.1605	-0.3708
ULS: 8. 0.6D + 0.7E	0.0112	0.9716	-0.0372	-0.1298	0.0489	-0.1097
ULS: 5a. D + 0.6W_Wind downforce Case A only	-1.5733	2.9125	-0.2112	-0.7135	0.7402	18.2126
ULS: 5a. D + 0.6W_Wind downforce Case B only	0.0186	1.6193	-0.0620	-0.2164	0.0815	-0.1828
ULS: 5a. D + 0.6W_Wind uplift Case A only	1.6077	0.3267	0.0842	0.2698	-0.5634	-18.1238
ULS: 5a. D + 0.6W_Wind uplift Case B only	0.0186	1.6193	-0.0620	-0.2164	0.0815	-0.1828
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-1.1574	3.6936	-0.2337	-0.7982	0.6545	13.4257
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	0.0365	2.7236	-0.1218	-0.4254	0.1605	-0.3708
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	1.2284	1.7542	-0.0121	-0.0608	-0.3232	-13.8266
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	0.0365	2.7236	-0.1218	-0.4254	0.1605	-0.3708
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-1.1753	2.5892	-0.1739	-0.5892	0.5756	13.6138
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	0.0186	1.6193	-0.0620	-0.2164	0.0815	-0.1828
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	1.2105	0.6498	0.0477	0.1483	-0.4021	-13.6385
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	0.0186	1.6193	-0.0620	-0.2164	0.0815	-0.1828
ULS: 7. 0.6D + 0.6W_Wind downforce Case A only	-1.5808	2.2648	-0.1864	-0.6269	0.7076	18.2857
ULS: 7. 0.6D + 0.6W_Wind downforce Case B only	0.0112	0.9716	-0.0372	-0.1298	0.0489	-0.1097
ULS: 7. 0.6D + 0.6W_Wind uplift Case A only	1.6003	-0.3210	0.1090	0.3564	-0.5960	-18.0507
ULS: 7. 0.6D + 0.6W_Wind uplift Case B only	0.0112	0.9716	-0.0372	-0.1298	0.0489	-0.1097

Worst Case Reactions LRFD

These calculations are taken directly from the FEA via SkyCiv and are used in the Concrete Checks of the Foundatino Module.
Note: Worst case values are assumed as downforce wind load cases.

Worst Case Reactions ASD

These results are taken from the worst case values in the above table and are used in the Soil Checks in the Foundation Module.
Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	5.3768
Shear X	-2.6851
Shear Z	-0.3657
Moment X	-1.2379
Moment Y (Twist)	1.2603
Moment Z	30.8222

Result	Value (kip, kip-ft)
Axial	3.6936
Shear X	-1.6077
Shear Z	-0.2337
Moment X	-0.7982
Moment Y (Twist)	0.7402
Moment Z	18.2857

Project Details

Design Code:	AISC 360-16 LRFD
Provision:	LRFD
Country:	United States
User Name:	sales@mtsolar.us
Project Name:	Food Farm
Unit System:	imperial

Design Input Information

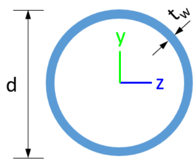
Design Factors

Φ_t	Φ_c	Φ_b	Φ_v
0.9	0.9	0.9	0.9

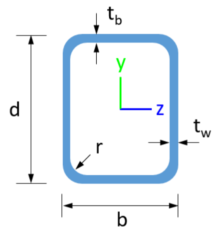
Design Materials

ID	E (ksi)	F_y (ksi)	F_u (ksi)
1	29000	50	65

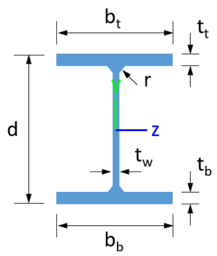
Section Dimensions



ID	Name	d (in)	t_w (in)					
1	2in Pipe Sch 40	2.38	0.15					
4	4in Pipe Sch 40	4.50	0.24					
8	6in Pipe Sch 80	6.63	0.43					



ID	Name	d (in)	b (in)	t_w (in)	t_b (in)	r (in)		
15	HSS5x3x1/8	5.00	3.00	0.12	0.12	0.12		



ID	Name	d (in)	t_w (in)	b_t (in)	b_b (in)	t_t (in)	t_b (in)	r (in)
18	W6x9	5.90	0.17	3.94	3.94	0.21	0.21	0.25

Section Properties

ID	Name	A (in^2)	J (in^4)	I_{yp} (in^4)	I_{zp} (in^4)	I_w (in^6)	S_{yp} (in^3)	S_{zp} (in^3)

1	2in Pipe Sch 40	1.07	1.33	0.67	0.67	0.00	0.76	0.76
4	4in Pipe Sch 40	3.17	14.47	7.23	7.23	0.00	4.31	4.31
8	6in Pipe Sch 80	8.40	80.98	40.49	40.49	0.00	16.60	16.60
15	HSS5x3x1/8	1.77	6.02	2.75	6.03	0.51	2.07	2.93
18	W6x9	2.68	0.04	2.20	16.40	17.70	1.72	6.23

Member Properties								
Member ID	Section ID	K_zL (ft)	K_yL (ft)	L_b(ft)	C_b	LST	LSC	LD
1	8	23.79	23.79	11.33	-	300	200	1
2	4	1.30	1.30	2.00	-	300	200	1
3	15	0.92	0.92	1.42	1.18,1.18,1.18,1.17,1.18,1.18,1.17,1.18,1.16,1.18,1.17,1.18,1.16,1.18,1.17,1.17,1.19,1.17,1.17,1.18,1.15,1.18,1.17,1.18,1.16,1.18	300	200	1
4	15	2.44	2.44	3.75	1.69,1.68,1.69,1.68,1.69,1.69,1.67,1.68,1.66,1.68,1.67,1.69,1.66,1.69,1.67,1.68,1.70,1.68,1.67,1.69,1.64,1.69,1.67,1.69,1.66,1.69	300	200	1
5	15	1.52	1.52	2.33	1.68,1.67,1.68,1.67,1.68,1.68,1.67,1.67,1.66,1.67,1.67,1.68,1.66,1.68,1.67,1.67,1.68,1.67,1.67,1.68,1.65,1.68,1.67,1.68,1.66,1.68	300	200	1
6	15	0.92	0.92	1.42	1.19,1.19,1.19,1.19,1.19,1.19,1.19,1.19,1.18,1.19,1.19,1.19,1.18,1.19,1.19,1.19,1.19,1.19,1.19,1.19,1.18,1.19,1.19,1.19,1.18,1.19	300	200	1
7	15	1.52	1.52	2.33	1.68,1.67,1.68,1.67,1.67,1.68,1.67,1.67,1.66,1.67,1.67,1.68,1.66,1.68,1.67,1.67,1.68,1.67,1.67,1.68,1.66,1.68,1.67,1.68,1.67,1.68	300	200	1
8	18	1.33	1.33	2.05	1.34,1.34,1.34,1.34,1.34,1.34,1.32,1.34,1.31,1.34,1.32,1.34,1.32,1.34,1.33,1.34,1.39,1.34,1.33,1.34,1.31,1.34,1.32,1.34,1.32,1.34	300	200	1
9	1	2.60	2.60	4.00	-	300	200	1
10	15	2.44	2.44	3.75	1.68,1.68,1.68,1.67,1.68,1.68,1.67,1.68,1.66,1.68,1.67,1.68,1.66,1.68,1.67,1.67,1.69,1.67,1.67,1.68,1.65,1.68,1.67,1.68,1.66,1.68	300	200	1
11	18	1.33	1.33	2.05	1.37,1.37,1.37,1.37,1.37,1.37,1.46,1.37,1.57,1.37,1.46,1.37,1.52,1.37,1.42,1.37,1.19,1.37,1.45,1.37,1.65,1.37,1.47,1.37,1.51,1.37	300	200	1
12	4	1.30	1.30	2.00	-	300	200	1
13	18	4.88	4.00	7.50	1.14,1.15,1.14,1.15,1.15,1.14,1.11,1.15,1.20,1.15,1.12,1.14,1.17,1.14,1.12,1.15,1.46,1.15,1.11,1.14,1.23,1.14,1.12,1.14,1.16,1.14	300	200	1
14	18	4.88	4.00	7.50	1.15,1.15,1.15,1.15,1.15,1.15,1.14,1.15,1.12,1.15,1.14,1.15,1.13,1.15,1.14,1.15,1.18,1.15,1.14,1.15,1.12,1.15,1.14,1.15,1.13,1.15	300	200	1
15	18	4.20	4.20	2.00	2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33	300	200	1
16	18	4.20	4.20	2.00	2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33	300	200	1
101	8	23.79	23.79	11.33	-	300	200	1
102	4	1.30	1.30	2.00	-	300	200	1
103	15	0.92	0.92	1.42	1.19,1.18,1.19,1.18,1.18,1.19,1.18,1.18,1.17,1.18,1.18,1.19,1.17,1.19,1.18,1.18,1.19,1.18,1.18,1.19,1.16,1.19,1.18,1.19,1.17,1.19	300	200	1
104	15	2.44	2.44	3.75	1.68,1.68,1.68,1.67,1.68,1.68,1.67,1.68,1.66,1.68,1.67,1.68,1.66,1.68,1.67,1.67,1.69,1.67,1.67,1.68,1.65,1.68,1.67,1.68,1.66,1.68	300	200	1
105	15	1.52	1.52	2.33	1.68,1.67,1.68,1.67,1.67,1.68,1.67,1.67,1.66,1.67,1.67,1.68,1.66,1.68,1.67,1.67,1.68,1.67,1.67,1.68,1.66,1.68,1.67,1.68,1.66,1.68	300	200	1
106	15	0.92	0.92	1.42	1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.17,1.18,1.18,1.18,1.18,1.18	300	200	1
107	15	1.52	1.52	2.33	1.68,1.67,1.68,1.67,1.67,1.68,1.67,1.67,1.66,1.67,1.67,1.68,1.66,1.68,1.67,1.67,1.68,1.67,1.67,1.68,1.66,1.68,1.67,1.68,1.66,1.68	300	200	1
108	18	1.33	1.33	2.05	2.11,2.11,2.11,2.11,2.11,2.11,2.09,2.11,2.07,2.11,2.09,2.11,2.08,2.11,2.10,2.11,2.31,2.11,2.09,2.11,2.07,2.11,2.09,2.11,2.08,2.11	300	200	1
109	1	2.60	2.60	4.00	-	300	200	1
110	15	2.44	2.44	3.75	1.68,1.68,1.68,1.67,1.68,1.68,1.67,1.68,1.66,1.68,1.67,1.68,1.66,1.68,1.67,1.67,1.69,1.67,1.67,1.68,1.65,1.68,1.67,1.68,1.66,1.68	300	200	1
111	18	1.33	1.33	2.05	2.08,2.08,2.08,2.08,2.08,2.08,1.99,2.08,1.67,2.08,1.95,2.08,1.76,2.08,2.06,2.08,1.96,2.08,2.03,2.08,1.58,2.08,1.93,2.08,1.80,2.08	300	200	1
112	4	1.30	1.30	2.00	-	300	200	1

113	18	4.88	4.00	7.50	1.05,1.05,1.05,1.05,1.05,1.05,1.08,1.05,1.13,1.05,1.09,1.05,1.11,1.05,1.07,1.05,1.09,1.05,1.08,1.05,1.16,1.05,1.09,1.05,1.10,1.05	300	200	1
114	18	4.88	4.00	7.50	1.05,1.05,1.05,1.05,1.05,1.06,1.05,1.08,1.05,1.06,1.05,1.07,1.05,1.06,1.05,1.05,1.05,1.06,1.05,1.09,1.05,1.06,1.05,1.07,1.05	300	200	1
115	18	6.63	6.63	10.20	1.15,1.15,1.15,1.15,1.15,1.15,1.12,1.15,1.09,1.15,1.12,1.15,1.10,1.15,1.13,1.15,1.33,1.15,1.12,1.15,1.08,1.15,1.11,1.15,1.10,1.15	300	200	1
116	18	6.63	6.63	10.20	1.16,1.16,1.16,1.16,1.16,1.16,1.15,1.16,1.15,1.16,1.15,1.16,1.15,1.16,1.16,1.16,1.18,1.16,1.15,1.16,1.14,1.16,1.15,1.16,1.15,1.16	300	200	1
201	8	23.79	23.79	11.33	-	300	200	1
202	4	1.30	1.30	2.00	-	300	200	1
203	15	0.92	0.92	1.42	1.19,1.18,1.19,1.18,1.18,1.19,1.18,1.18,1.17,1.18,1.18,1.19,1.18,1.19,1.18,1.18,1.19,1.18,1.18,1.19,1.17,1.19,1.18,1.19,1.18,1.19	300	200	1
204	15	2.44	2.44	3.75	1.68,1.68,1.68,1.67,1.68,1.68,1.67,1.68,1.66,1.68,1.67,1.68,1.66,1.68,1.67,1.67,1.69,1.67,1.67,1.68,1.65,1.68,1.67,1.68,1.66,1.68	300	200	1
205	15	1.52	1.52	2.33	1.68,1.67,1.68,1.67,1.67,1.68,1.67,1.67,1.66,1.67,1.67,1.68,1.66,1.68,1.67,1.67,1.68,1.67,1.67,1.68,1.66,1.68,1.67,1.68,1.66,1.68	300	200	1
206	15	0.92	0.92	1.42	1.19,1.18,1.19,1.18,1.18,1.19,1.18,1.18,1.17,1.18,1.18,1.19,1.18,1.19,1.18,1.18,1.19,1.18,1.18,1.19,1.17,1.19,1.18,1.19,1.18,1.19	300	200	1
207	15	1.52	1.52	2.33	1.68,1.67,1.68,1.67,1.67,1.68,1.67,1.67,1.66,1.67,1.67,1.68,1.66,1.68,1.67,1.67,1.68,1.67,1.67,1.68,1.66,1.68,1.67,1.68,1.66,1.68	300	200	1
208	18	1.33	1.33	2.05	2.09,2.09,2.09,2.09,2.09,2.09,2.09,2.09,2.09,2.09,2.09,2.09,2.09,2.09,2.09,2.09,2.09,2.09,2.09,2.09,2.09,2.09,2.09,2.09,2.09,2.09	300	200	1
209	1	2.60	2.60	4.00	-	300	200	1
210	15	2.44	2.44	3.75	1.68,1.68,1.68,1.67,1.68,1.68,1.67,1.68,1.66,1.68,1.67,1.68,1.66,1.68,1.67,1.67,1.69,1.67,1.67,1.68,1.65,1.68,1.67,1.68,1.66,1.68	300	200	1
211	18	1.33	1.33	2.05	2.11,2.11,2.11,2.11,2.11,2.11,2.08,2.11,2.01,2.11,2.08,2.11,2.06,2.11,2.09,2.11,2.18,2.11,2.08,2.11,1.89,2.11,2.07,2.11,2.07,2.11	300	200	1
212	4	1.30	1.30	2.00	-	300	200	1
213	18	4.88	4.00	7.50	1.04,1.04,1.04,1.04,1.04,1.04,1.04,1.04,1.04,1.04,1.04,1.04,1.04,1.04,1.04,1.04,1.03,1.04,1.04,1.04,1.04,1.04,1.04,1.04,1.04,1.04	300	200	1
214	18	4.88	4.00	7.50	1.04,1.04,1.04,1.04,1.04,1.04,1.04,1.04,1.04,1.04,1.04,1.04,1.04,1.04,1.04,1.04,1.04,1.04,1.04,1.04,1.04,1.04,1.04,1.04,1.04,1.04	300	200	1
215	18	6.63	6.63	10.20	1.17,1.17,1.17,1.17,1.17,1.17,1.15,1.17,1.14,1.17,1.15,1.17,1.15,1.17,1.16,1.17,1.23,1.17,1.16,1.17,1.14,1.17,1.15,1.17,1.15,1.17	300	200	1
216	18	6.63	6.63	10.20	1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.15,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16	300	200	1
301	8	23.79	23.79	11.33	-	300	200	1
302	4	1.30	1.30	2.00	-	300	200	1
303	15	0.92	0.92	1.42	1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.17,1.18,1.18,1.18,1.18,1.18	300	200	1
304	15	2.44	2.44	3.75	1.68,1.68,1.68,1.67,1.68,1.68,1.67,1.68,1.66,1.68,1.67,1.68,1.66,1.68,1.67,1.67,1.69,1.67,1.67,1.68,1.65,1.68,1.67,1.68,1.66,1.68	300	200	1
305	15	1.52	1.52	2.33	1.68,1.67,1.68,1.67,1.67,1.68,1.67,1.67,1.66,1.67,1.67,1.68,1.66,1.68,1.67,1.67,1.68,1.67,1.67,1.68,1.66,1.68,1.67,1.68,1.66,1.68	300	200	1
306	15	0.92	0.92	1.42	1.19,1.18,1.19,1.18,1.18,1.19,1.18,1.18,1.17,1.18,1.18,1.19,1.17,1.19,1.18,1.18,1.19,1.18,1.18,1.19,1.16,1.19,1.18,1.19,1.17,1.19	300	200	1
307	15	1.52	1.52	2.33	1.68,1.67,1.68,1.67,1.67,1.68,1.67,1.67,1.66,1.67,1.67,1.68,1.66,1.68,1.67,1.67,1.68,1.67,1.67,1.68,1.66,1.68,1.67,1.68,1.66,1.68	300	200	1
308	18	1.33	1.33	2.05	2.07,2.07,2.07,2.08,2.07,2.07,2.03,2.07,1.88,2.07,2.02,2.07,1.93,2.07,2.07,2.08,2.15,2.08,2.05,2.07,1.83,2.07,2.01,2.07,1.95,2.07	300	200	1
309	1	2.60	2.60	4.00	-	300	200	1
310	15	2.44	2.44	3.75	1.68,1.68,1.68,1.67,1.68,1.68,1.67,1.68,1.66,1.68,1.67,1.68,1.66,1.68,1.67,1.67,1.69,1.67,1.67,1.68,1.65,1.68,1.67,1.68,1.66,1.68	300	200	1
311	18	1.33	1.33	2.05	1.93,1.93,1.93,1.93,1.93,1.94,1.56,1.93,1.37,1.93,1.54,1.93,1.43,1.93,1.68,1.93,1.90,1.93,1.59,1.93,1.31,1.93,1.53,1.94,1.45,1.94	300	200	1
312	4	1.30	1.30	2.00	-	300	200	1
313	18	4.88	4.00	7.50	1.05,1.05,1.05,1.05,1.05,1.05,1.08,1.05,1.13,1.05,1.09,1.05,1.11,1.05,1.07,1.05,1.09,1.05,1.08,1.05,1.16,1.05,1.09,1.05,1.10,1.05	300	200	1
314	18	4.88	4.00	7.5	1.05,1.05,1.05,1.05,1.05,1.05,1.06,1.05,1.08,1.05,1.06,1.05,1.07,1.05,1.06,1.05,1.05,1.05,1.06,1.05,1.0	30	20	1

314	18	4.88	4.00	0	9,1.05,1.06,1.05,1.07,1.05	0	0	1
315	18	6.63	6.63	10.20	1.16,1.16,1.16,1.16,1.16,1.16,1.14,1.16,1.12,1.16,1.14,1.16,1.13,1.16,1.14,1.16,1.26,1.16,1.14,1.16,1.11,1.16,1.13,1.16,1.13,1.16	300	200	1
316	18	6.63	6.63	10.20	1.17,1.17,1.17,1.17,1.17,1.17,1.16,1.17,1.15,1.17,1.16,1.17,1.15,1.17,1.16,1.17,1.20,1.17,1.16,1.17,1.15,1.17,1.16,1.17,1.15,1.17	300	200	1
401	8	23.79	23.79	11.33	-	300	200	1
402	4	1.30	1.30	2.00	-	300	200	1
403	15	0.92	0.92	1.42	1.19,1.19,1.19,1.19,1.19,1.19,1.19,1.19,1.18,1.19,1.19,1.19,1.18,1.19,1.19,1.19,1.19,1.19,1.19,1.19,1.18,1.19,1.19,1.19,1.18,1.19	300	200	1
404	15	2.44	2.44	3.75	1.68,1.68,1.68,1.67,1.68,1.68,1.67,1.68,1.66,1.68,1.67,1.68,1.66,1.68,1.67,1.67,1.69,1.67,1.67,1.68,1.65,1.68,1.67,1.68,1.66,1.68	300	200	1
405	15	1.52	1.52	2.33	1.68,1.67,1.68,1.67,1.68,1.68,1.67,1.67,1.66,1.67,1.67,1.68,1.66,1.68,1.67,1.67,1.68,1.67,1.67,1.68,1.66,1.68,1.67,1.68,1.67,1.68	300	200	1
406	15	0.92	0.92	1.42	1.18,1.18,1.18,1.17,1.18,1.18,1.17,1.18,1.16,1.18,1.17,1.18,1.16,1.18,1.17,1.17,1.19,1.17,1.17,1.18,1.15,1.18,1.17,1.18,1.16,1.18	300	200	1
407	15	1.52	1.52	2.33	1.68,1.67,1.68,1.67,1.68,1.68,1.67,1.67,1.66,1.67,1.67,1.68,1.66,1.68,1.67,1.67,1.68,1.67,1.67,1.68,1.65,1.68,1.67,1.68,1.66,1.68	300	200	1
408	18	4.20	4.20	2.00	2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33	300	200	1
409	1	2.60	2.60	4.00	-	300	200	1
410	15	2.44	2.44	3.75	1.69,1.68,1.69,1.68,1.69,1.69,1.67,1.68,1.66,1.68,1.67,1.69,1.66,1.69,1.67,1.68,1.70,1.68,1.67,1.69,1.64,1.69,1.67,1.69,1.66,1.69	300	200	1
411	18	4.20	4.20	2.00	2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33	300	200	1
412	4	1.30	1.30	2.00	-	300	200	1
413	18	4.88	4.00	7.50	1.14,1.15,1.14,1.15,1.15,1.14,1.11,1.15,1.20,1.15,1.12,1.14,1.17,1.14,1.12,1.15,1.46,1.15,1.11,1.14,1.23,1.14,1.12,1.14,1.16,1.14	300	200	1
414	18	4.88	4.00	7.50	1.15,1.15,1.15,1.15,1.15,1.15,1.14,1.15,1.13,1.15,1.14,1.15,1.13,1.15,1.14,1.15,1.18,1.15,1.14,1.15,1.12,1.15,1.14,1.15,1.13,1.15	300	200	1
415	18	6.63	6.63	10.20	1.09,1.09,1.09,1.09,1.09,1.09,1.10,1.09,1.12,1.09,1.10,1.09,1.11,1.09,1.10,1.09,1.10,1.09,1.10,1.09,1.13,1.09,1.11,1.09,1.11,1.09	300	200	1
416	18	6.63	6.63	10.20	1.09,1.09,1.09,1.09,1.09,1.09,1.09,1.08,1.09,1.09,1.09,1.09,1.09,1.09,1.09,1.10,1.09,1.09,1.09,1.08,1.09,1.09,1.09,1.09,1.09	300	200	1

Member Design Capacity

Member ID	$\Phi_t P_n$ (kip)	$\Phi_c P_n$ (kip)	$\Phi_b M_{zn}$ (k-ft)	$\Phi_b M_{yn}$ (k-ft)	$\Phi_v V_{yn}$ (kip)	$\Phi_v V_{zn}$ (kip)
1	378.22	112.25	62.23	62.23	113.47	113.47
2	142.83	141.72	16.17	16.17	42.85	42.85
3	79.65	74.89	10.99	6.26	29.14	16.61
4	79.65	72.84	10.99	6.26	29.14	16.61
5	79.65	74.30	10.99	6.26	29.14	16.61
6	79.65	74.89	10.99	6.26	29.14	16.61
7	79.65	74.30	10.99	6.26	29.14	16.61
8	120.60	115.40	23.36	6.45	30.09	45.74
9	48.35	43.11	2.85	2.85	14.51	14.51
10	79.65	72.84	10.99	6.26	29.14	16.61
11	120.60	115.40	23.36	6.45	30.09	45.74
12	142.83	141.72	16.17	16.17	42.85	42.85
13	120.60	84.03	19.46	6.45	30.09	45.74
14	120.60	84.03	19.68	6.45	30.09	45.74
15	120.60	96.18	23.36	6.45	30.09	45.74
16	120.60	96.18	23.36	6.45	30.09	45.74
101	378.22	112.25	62.23	62.23	113.47	113.47
102	142.83	141.72	16.17	16.17	42.85	42.85

103	79.65	74.89	10.99	6.26	29.14	16.61
104	79.65	72.84	10.99	6.26	29.14	16.61
105	79.65	74.30	10.99	6.26	29.14	16.61
106	79.65	74.89	10.99	6.26	29.14	16.61
107	79.65	74.30	10.99	6.26	29.14	16.61
108	120.60	115.40	23.36	6.45	30.09	45.74
109	48.35	43.11	2.85	2.85	14.51	14.51
110	79.65	72.84	10.99	6.26	29.14	16.61
111	120.60	115.40	23.36	6.45	30.09	45.74
112	142.83	141.72	16.17	16.17	42.85	42.85
113	120.60	84.03	18.55	6.45	30.09	45.74
114	120.60	84.03	18.43	6.45	30.09	45.74
115	120.60	68.63	14.76	6.45	30.09	45.74
116	120.60	68.63	15.62	6.45	30.09	45.74
201	378.22	112.25	62.23	62.23	113.47	113.47
202	142.83	141.72	16.17	16.17	42.85	42.85
203	79.65	74.89	10.99	6.26	29.14	16.61
204	79.65	72.84	10.99	6.26	29.14	16.61
205	79.65	74.30	10.99	6.26	29.14	16.61
206	79.65	74.89	10.99	6.26	29.14	16.61
207	79.65	74.30	10.99	6.26	29.14	16.61
208	120.60	115.40	23.36	6.45	30.09	45.74
209	48.35	43.11	2.85	2.85	14.51	14.51
210	79.65	72.84	10.99	6.26	29.14	16.61
211	120.60	115.40	23.36	6.45	30.09	45.74
212	142.83	141.72	16.17	16.17	42.85	42.85
213	120.60	84.03	18.16	6.45	30.09	45.74
214	120.60	84.03	18.25	6.45	30.09	45.74
215	120.60	68.63	15.57	6.45	30.09	45.74
216	120.60	68.63	15.76	6.45	30.09	45.74
301	378.22	112.25	62.23	62.23	113.47	113.47
302	142.83	141.72	16.17	16.17	42.85	42.85
303	79.65	74.89	10.99	6.26	29.14	16.61
304	79.65	72.84	10.99	6.26	29.14	16.61
305	79.65	74.30	10.99	6.26	29.14	16.61
306	79.65	74.89	10.99	6.26	29.14	16.61
307	79.65	74.30	10.99	6.26	29.14	16.61
308	120.60	115.40	23.36	6.45	30.09	45.74
309	48.35	43.11	2.85	2.85	14.51	14.51
310	79.65	72.84	10.99	6.26	29.14	16.61
311	120.60	115.40	23.36	6.45	30.09	45.74
312	142.83	141.72	16.17	16.17	42.85	42.85
313	120.60	84.03	18.55	6.45	30.09	45.74
314	120.60	84.03	18.43	6.45	30.09	45.74
315	120.60	68.63	15.20	6.45	30.09	45.74
316	120.60	68.63	15.66	6.45	30.09	45.74
401	378.22	112.25	62.23	62.23	113.47	113.47
402	142.83	141.72	16.17	16.17	42.85	42.85
403	79.65	74.89	10.99	6.26	29.14	16.61
404	79.65	72.84	10.99	6.26	29.14	16.61
405	79.65	74.30	10.99	6.26	29.14	16.61
406	79.65	74.89	10.99	6.26	29.14	16.61

407	79.65	74.30	10.99	6.26	29.14	16.61
408	120.60	96.18	23.36	6.45	30.09	45.74
409	48.35	43.11	2.85	2.85	14.51	14.51
410	79.65	72.84	10.99	6.26	29.14	16.61
411	120.60	96.18	23.36	6.45	30.09	45.74
412	142.83	141.72	16.17	16.17	42.85	42.85
413	120.60	84.03	19.46	6.45	30.09	45.74
414	120.60	84.03	19.70	6.45	30.09	45.74
415	120.60	68.63	14.89	6.45	30.09	45.74
416	120.60	68.63	14.80	6.45	30.09	45.74

Design Ratio

Member ID	P	M_z	M_y	V_y	V_z	(P,M_z,M_y)	Worst LC	KL/r	δ	Status
1	0.048	0.495	0.047	0.024	0.003	0.537	#13	0.650	Not Required	Pass
2	0.002	0.155	0.123	0.042	0.026	0.256	#13	0.052	Not Required	Pass
3	0.007	0.322	0.030	0.031	0.004	0.344	#13	0.044	Not Required	Pass
4	0.006	0.325	0.098	0.033	0.019	0.408	#13	0.078	Not Required	Pass
5	0.007	0.199	0.067	0.032	0.013	0.201	#13	0.073	Not Required	Pass
6	0.013	0.517	0.135	0.054	0.030	0.601	#13	0.044	Not Required	Pass
7	0.014	0.321	0.221	0.052	0.043	0.347	#13	0.073	Not Required	Pass
8	0.004	0.090	0.113	0.029	0.013	0.137	#13	0.088	Not Required	Pass
9	0.005	0.049	0.072	0.004	0.004	0.105	#13	0.198	Not Required	Pass
10	0.014	0.484	0.213	0.049	0.037	0.523	#21	0.078	Not Required	Pass
11	0.004	0.079	0.116	0.031	0.013	0.127	#21	0.088	Not Required	Pass
12	0.001	0.315	0.201	0.075	0.040	0.513	#13	0.052	Not Required	Pass
13	0.006	0.082	0.295	0.040	0.016	0.331	#21	0.265	Not Required	Pass
14	0.004	0.070	0.292	0.038	0.016	0.304	#21	0.177	Not Required	Pass
15	0.000	0.013	0.029	0.010	0.004	0.040	#21	Not Required	Not Required	Pass
16	0.000	0.013	0.029	0.010	0.004	0.040	#21	Not Required	Not Required	Pass
101	0.061	0.638	0.003	0.030	0.000	0.666	#13	0.650	Not Required	Pass
102	0.003	0.309	0.213	0.076	0.038	0.503	#13	0.034	Not Required	Pass
103	0.013	0.525	0.096	0.052	0.013	0.589	#13	0.044	Not Required	Pass
104	0.012	0.551	0.221	0.055	0.038	0.654	#13	0.078	Not Required	Pass
105	0.013	0.325	0.232	0.052	0.046	0.361	#13	0.073	Not Required	Pass
106	0.013	0.546	0.095	0.055	0.014	0.594	#13	0.044	Not Required	Pass
107	0.013	0.339	0.216	0.054	0.043	0.372	#13	0.073	Not Required	Pass
108	0.004	0.040	0.107	0.032	0.013	0.132	#21	0.088	Not Required	Pass
109	0.012	0.039	0.049	0.001	0.000	0.089	#13	0.198	Not Required	Pass
110	0.012	0.546	0.208	0.055	0.036	0.627	#13	0.078	Not Required	Pass
111	0.005	0.050	0.110	0.031	0.013	0.129	#21	0.088	Not Required	Pass
112	0.003	0.307	0.221	0.075	0.041	0.517	#13	0.034	Not Required	Pass
113	0.006	0.128	0.308	0.041	0.016	0.409	#21	0.265	Not Required	Pass
114	0.007	0.156	0.306	0.044	0.016	0.428	#21	0.265	Not Required	Pass
115	0.007	0.201	0.165	0.032	0.013	0.337	#21	0.439	Not Required	Pass
116	0.004	0.193	0.165	0.035	0.013	0.329	#21	0.439	Not Required	Pass
201	0.061	0.642	0.000	0.030	0.000	0.669	#13	0.650	Not Required	Pass
202	0.003	0.304	0.216	0.075	0.040	0.506	#13	0.034	Not Required	Pass
203	0.013	0.541	0.093	0.054	0.013	0.598	#13	0.044	Not Required	Pass
204	0.012	0.532	0.204	0.053	0.035	0.617	#13	0.078	Not Required	Pass
205	0.013	0.335	0.214	0.054	0.042	0.366	#13	0.073	Not Required	Pass

205	0.013	0.335	0.214	0.054	0.042	0.366	#13	0.073	Not Required	Pass
206	0.013	0.541	0.093	0.054	0.013	0.598	#13	0.044	Not Required	Pass
207	0.013	0.335	0.214	0.054	0.042	0.366	#13	0.073	Not Required	Pass
208	0.004	0.040	0.105	0.032	0.012	0.130	#21	0.088	Not Required	Pass
209	0.011	0.034	0.048	0.001	0.000	0.085	#13	0.198	Not Required	Pass
210	0.012	0.532	0.204	0.053	0.035	0.617	#13	0.078	Not Required	Pass
211	0.005	0.044	0.107	0.032	0.012	0.134	#21	0.088	Not Required	Pass
212	0.003	0.304	0.216	0.075	0.040	0.506	#13	0.034	Not Required	Pass
213	0.006	0.150	0.285	0.041	0.016	0.410	#21	0.265	Not Required	Pass
214	0.007	0.155	0.282	0.041	0.016	0.404	#21	0.265	Not Required	Pass
215	0.008	0.152	0.165	0.032	0.012	0.293	#21	0.439	Not Required	Pass
216	0.005	0.141	0.165	0.032	0.012	0.285	#21	0.439	Not Required	Pass
301	0.061	0.638	0.003	0.030	0.000	0.666	#13	0.650	Not Required	Pass
302	0.003	0.307	0.221	0.075	0.041	0.517	#13	0.034	Not Required	Pass
303	0.013	0.546	0.095	0.055	0.014	0.594	#13	0.044	Not Required	Pass
304	0.012	0.546	0.208	0.055	0.036	0.627	#13	0.078	Not Required	Pass
305	0.013	0.339	0.216	0.054	0.043	0.372	#13	0.073	Not Required	Pass
306	0.013	0.525	0.096	0.052	0.013	0.589	#13	0.044	Not Required	Pass
307	0.013	0.325	0.232	0.052	0.046	0.361	#13	0.073	Not Required	Pass
308	0.004	0.057	0.124	0.035	0.013	0.144	#21	0.088	Not Required	Pass
309	0.012	0.039	0.049	0.001	0.000	0.089	#13	0.198	Not Required	Pass
310	0.012	0.551	0.221	0.055	0.038	0.654	#13	0.078	Not Required	Pass
311	0.004	0.073	0.126	0.032	0.013	0.134	#21	0.088	Not Required	Pass
312	0.003	0.309	0.212	0.076	0.038	0.503	#13	0.034	Not Required	Pass
313	0.006	0.128	0.308	0.041	0.016	0.409	#21	0.265	Not Required	Pass
314	0.007	0.156	0.306	0.044	0.016	0.428	#21	0.265	Not Required	Pass
315	0.008	0.154	0.165	0.031	0.013	0.295	#21	0.439	Not Required	Pass
316	0.005	0.141	0.165	0.032	0.013	0.285	#21	0.439	Not Required	Pass
401	0.048	0.495	0.047	0.024	0.003	0.537	#13	0.650	Not Required	Pass
402	0.001	0.315	0.201	0.075	0.040	0.513	#13	0.052	Not Required	Pass
403	0.013	0.517	0.135	0.054	0.030	0.601	#13	0.044	Not Required	Pass
404	0.014	0.484	0.213	0.049	0.037	0.523	#21	0.078	Not Required	Pass
405	0.014	0.321	0.221	0.052	0.043	0.346	#13	0.073	Not Required	Pass
406	0.007	0.322	0.030	0.031	0.004	0.344	#13	0.044	Not Required	Pass
407	0.007	0.199	0.067	0.032	0.013	0.201	#13	0.073	Not Required	Pass
408	0.000	0.013	0.029	0.010	0.004	0.040	#21	Not Required	Not Required	Pass
409	0.005	0.049	0.072	0.004	0.004	0.105	#13	0.198	Not Required	Pass
410	0.006	0.325	0.098	0.033	0.019	0.408	#13	0.078	Not Required	Pass
411	0.000	0.013	0.029	0.010	0.004	0.040	#21	Not Required	Not Required	Pass
412	0.002	0.155	0.123	0.042	0.026	0.256	#13	0.052	Not Required	Pass
413	0.006	0.082	0.295	0.040	0.016	0.331	#21	0.177	Not Required	Pass
414	0.004	0.070	0.292	0.038	0.016	0.304	#21	0.265	Not Required	Pass
415	0.007	0.203	0.165	0.031	0.013	0.339	#21	0.439	Not Required	Pass
416	0.004	0.204	0.164	0.029	0.013	0.335	#21	0.439	Not Required	Pass

Definitions

Φ_t	Safety factor for tensile
Φ_c	Safety factor for compression
Φ_b	Safety factor for flexure
Φ_v	Safety factor for shear
E	Modulus of elasticity
F_y	Specified minimum yield stress

F_u	Specified minimum tensile strength
A	Cross-sectional area
J	Torsional constant
I_{yp}	Moment of inertia about the Y axes
I_{zp}	Moment of inertia about the Z axes
I_w	Warping constant
S_{yp}	Plastic section modulus about the Y axis
S_{zp}	Plastic section modulus about the Z axis
KL	Effective length
C_b	Buckling modification factor (from all load combinations)
L_b	Length between braced points
LST	Limited slenderness for tension
LSC	Limited slenderness for compression
LD	Limited deflection
P_n	Nominal axial strength (tension/comression)
M_n	Nominal flexural strength (about Z/Y axis)
V_n	Nominal shear strength (along Z/Y axis)
P	Design ratio in case of axial force
M_z	Design ratio in case of bending about Z axis
M_y	Design ratio in case of bending about Y axis
V_y	Design ratio in case of shear along Y axis
V_z	Design ratio in case of shear along Z axis
(P,M_z,M_y)	Design ratio in case of axial force and bending action
KL/r	Design ratio in case of section slenderness
δ	Design ratio in case of member deflection
OK	Capacity is provided
NG	Capacity is not provided

REFERENCES	CALCULATIONS	RESULTS

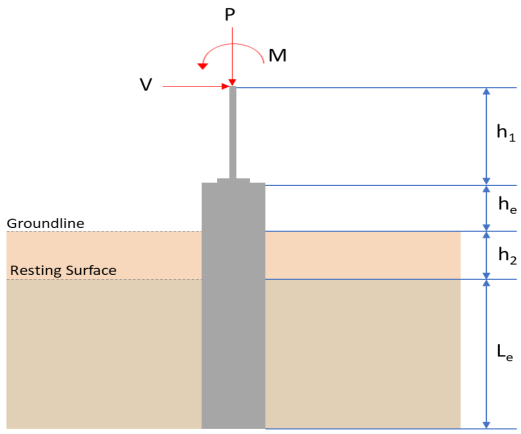
SkyCiv Foundation Design

Pile Foundation

Design Information :

Design code : IBC 2021 (International Building Code)
Unit System : Imperial

Pile Input



Geometry

Pile shape: rectangular
$b = 48\ \mathrm{in}$ - Pile width
$D = 48\ \mathrm{in}$ - Pile depth
$L = 5.75\ \mathrm{ft}$ - Total pile length
$h_1 = 0\ \mathrm{ft}$ - Lateral load height from the top of the pile,
$h_2 = 0\ \mathrm{ft}$ - Depth to resisting surface
$h_e = 0\ \mathrm{ft}$ - Length of pile above the ground

Tabulation of Soil Parameters

Layer	Label	Allowable Bearing Pressure (q_a) (psf)	Allowable Lateral Pressure (R) (psf/ft)
1	Sand, silty sand, clayey sand, silty gravel & clayey gravel	2000.000	150.000

Tabulation of Loads

Load Component	ASD	LRFD
P (kip)	3.694	5.377
V_x (kip)	-1.608	-2.685
V_z (kip)	0.234	0.366
M_x (kipft)	0.798	1.238
M_z (kipft)	18.286	30.822

Material Properties

$f'_{ck} = 2.5\ \mathrm{ksi}$ - Concrete strength,

Required depth to resist lateral loads (ASD)

H - Point of application of the lateral load

$$H = h_1 + h_2 + h_e$$

$$H = (0\ \mathrm{ft}) + (0\ \mathrm{ft}) + (0\ \mathrm{ft})$$

$$H = 0\ \mathrm{ft}$$

Considering x-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\ D}$$

$$H_o = \frac{(-1.608\ \mathrm{kip})}{1.57 \times (48\ \mathrm{in})}$$

$$H_o = -0.25605\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\, H)}{1.57\, D}$$

$$M_o = \frac{(18.286 \text{ kipft}) + ((-1.608 \text{ kip}) \times (0 \text{ ft}))}{1.57 \times (48 \text{ in})}$$

$$M_o = 2.9118 \text{ kipft/ft}$$

Required depth of embedment in earth:

$$L_x^3 - \left(14.14 \times \frac{H_o \times L_x}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,x} = 5.3264 \text{ ft}$ - Required depth in x-direction,

Considering z-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\, b}$$

$$H_o = \frac{(0.234 \text{ kip})}{1.57 \times (48 \text{ in})}$$

$$H_o = 0.037261 \text{ kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\, H)}{1.57\, b}$$

$$M_o = \frac{(0.798 \text{ kipft}) + ((0.234 \text{ kip}) \times (0 \text{ ft}))}{1.57 \times (48 \text{ in})}$$

$$M_o = 0.12707 \text{ kipft/ft}$$

Required depth of embedment in earth:

$$L_z^3 - \left(14.14 \times \frac{H_o \times L_z}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,z} = 2.5078 \text{ ft}$ - Required depth in z-direction,

Minimum embedded depth required:

$L_{e,req}$ - Depth of pile required,

$$L_{e,req} = MAX[L_{e,x}, L_{e,z}]$$

$$L_{e,req} = MAX[(5.3264 \text{ ft}), (2.5078 \text{ ft})]$$

$$L_{e,req} = 5.326 \text{ ft}$$

L_e - Actual embedded length of pile,

$$L_e = L - h_e - h_2$$

$$L_e = (5.75 \text{ ft}) - (0 \text{ ft}) - (0 \text{ ft})$$

$$L_e = 5.75 \text{ ft}$$

$Ratio$ - Embedded depth

$$Ratio = \frac{L_{e,req}}{L_e}$$

$$Ratio = \frac{(5.326 \text{ ft})}{(5.75 \text{ ft})}$$

$$Ratio = 0.92626$$

Status: **PASS**
Ratio: **0.930**

End-bearing Capacity (ASD)

A - Pile cross-section area

$$A = b\, D$$

$$A = (48 \text{ in}) \times (48 \text{ in})$$

$$A = 16 \text{ ft}^2$$

q - End-bearing pressure

$$q = \frac{P_v}{A}$$

$$q = \frac{(3.694\ \mathrm{kip})}{(16\ \mathrm{ft}^2)}$$

$$q = 0.23088\ \mathrm{kip/ft^2}$$

Check bearing capacity ratio:

$Ratio$ - Capacity

$$Ratio = \frac{q}{q_a}$$

$$Ratio = \frac{\left(0.23088\ \mathrm{kip/ft^2}\right)}{(2000\ \mathrm{psf})}$$

$$Ratio = 0.11544$$

Status: **PASS**
Ratio: **0.120**

Czerniak

Lateral Soil Pressure (ASD):

L/D - Length to least lateral dimension ratio,

$$L/D = \frac{L}{D}$$

$$L/D = \frac{(5.75\ \mathrm{ft})}{(48\ \mathrm{in})}$$

$$L/D = 1.4375$$

Since L/D $\leq$ 10,

Pile is short.

Considering x-direction:

$H_o = -0.25605\ \mathrm{kip/ft}$ - Lateral force per length of pile,
$M_o = 2.9118\ \mathrm{kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\, M_o\, L_e) + \left(3\, H_o\, L_e^2\right)}{(6\, M_o) + (4\, H_o\, L_e)}$$

$$a = \frac{(4 \times (2.9118\ \mathrm{kipft/ft}) \times (5.75\ \mathrm{ft})) + \left(3 \times (-0.25605\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft})^2\right)}{(6 \times (2.9118\ \mathrm{kipft/ft})) + (4 \times (-0.25605\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))}$$

$$a = 3.9541\ \mathrm{ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\,[(4\, M_o) + (3\, H_o\, L_e)]^2}{L_e^2\,[(3\, M_o) + (2\, H_o\, L_e)]}$$

$$p = \frac{0.75 \times [(4 \times (2.9118\ \mathrm{kipft/ft})) + (3 \times (-0.25605\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))]^{\ 2}}{(5.75\ \mathrm{ft})^2 \times [(3 \times (2.9118\ \mathrm{kipft/ft})) + (2 \times (-0.25605\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))]}$$

$$p = 0.20478\ \mathrm{kip/ft^2}$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\,[(2\, M_o) + (H_o\, L_e)]}{L_e^2}$$

$$s = \frac{6 \times [(2 \times (2.9118\ \mathrm{kipft/ft})) + ((-0.25605\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))]}{(5.75\ \mathrm{ft})^2}$$

$$s = 0.78965\ \mathrm{kip/ft^2}$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\,\frac{a}{2}$$

$$p_a = (150\ \mathrm{psf/ft}) \times \frac{(3.9541\ \mathrm{ft})}{2}$$

$$p_a = 0.29656\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{(0.20478 \text{ kip/ft}^2)}{(0.29656 \text{ kip/ft}^2)}$$

$$Ratio = 0.69053$$

Status: **PASS**
Ratio: **0.690**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\,L_e$$

$$p_s = (150 \text{ psf/ft}) \times (5.75 \text{ ft})$$

$$p_s = 0.8625 \text{ kip/ft}^2$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{(0.78965 \text{ kip/ft}^2)}{(0.8625 \text{ kip/ft}^2)}$$

$$Ratio = 0.91553$$

Status: **PASS**
Ratio: **0.920**

Considering z-direction:

$H_o = 0.037261 \text{ kip/ft}$ - Lateral force per length of pile,
$M_o = 0.12707 \text{ kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + (3\,H_o\,L_e^2)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (0.12707 \text{ kipft/ft}) \times (5.75 \text{ ft})) + \left(3 \times (0.037261 \text{ kip/ft}) \times (5.75 \text{ ft})^2\right)}{(6 \times (0.12707 \text{ kipft/ft})) + (4 \times (0.037261 \text{ kip/ft}) \times (5.75 \text{ ft}))}$$

$$a = 4.0869 \text{ ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\,[(4\,M_o) + (3\,H_o\,L_e)]^2}{L_e^2\,[(3\,M_o) + (2\,H_o\,L_e)]}$$

$$p = \frac{0.75 \times [(4 \times (0.12707 \text{ kipft/ft})) + (3 \times (0.037261 \text{ kip/ft}) \times (5.75 \text{ ft}))]^{\;2}}{(5.75 \text{ ft})^2 \times [(3 \times (0.12707 \text{ kipft/ft})) + (2 \times (0.037261 \text{ kip/ft}) \times (5.75 \text{ ft}))]}$$

$$p = 0.037117 \text{ kip/ft}^2$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\,[(2\,M_o) + (H_o\,L_e)]}{L_e^2}$$

$$s = \frac{6 \times [(2 \times (0.12707 \text{ kipft/ft})) + ((0.037261 \text{ kip/ft}) \times (5.75 \text{ ft}))]}{(5.75 \text{ ft})^2}$$

$$s = 0.085001 \text{ kip/ft}^2$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\,\frac{a}{2}$$

$$p_a = (150 \text{ psf/ft}) \times \frac{(4.0869 \text{ ft})}{2}$$

$$p_a = 0.30652 \text{ kip/ft}^2$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{(0.037117 \text{ kip/ft}^2)}{(0.30652 \text{ kip/ft}^2)}$$

$$Ratio = 0.12109$$

Status: **PASS**
Ratio: **0.120**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\, L_e$$

$$p_s = (150\ \mathrm{psf/ft}) \times (5.75\ \mathrm{ft})$$

$$p_s = 0.8625\ \mathrm{kip/ft^2}$$

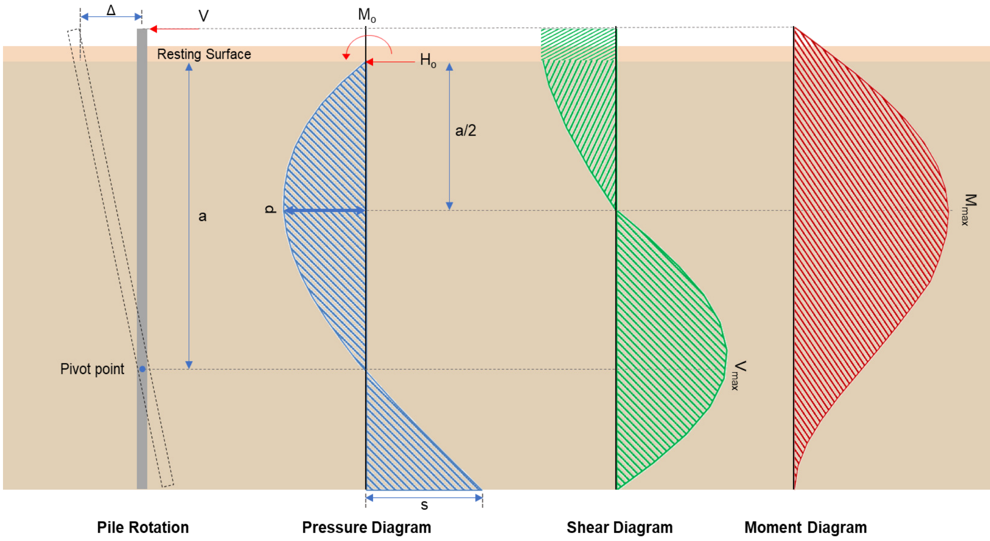
$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{(0.085001\ \mathrm{kip/ft^2})}{(0.8625\ \mathrm{kip/ft^2})}$$

$$Ratio = 0.098552$$

Status: **PASS**
Ratio: **0.100**



Shear force and Bending moment (x-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\, D}$$

$$H_o = \frac{(-2.685\ \mathrm{kip})}{1.57 \times (48\ \mathrm{in})}$$

$$H_o = -0.42755\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\, H)}{1.57\, D}$$

$$M_o = \frac{(30.822\ \mathrm{kipft}) + ((-2.685\ \mathrm{kip}) \times (0\ \mathrm{ft}))}{1.57 \times (48\ \mathrm{in})}$$

$$M_o = 4.908\ \mathrm{kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(4.908\ \mathrm{kipft/ft})}{(-0.42755\ \mathrm{kip/ft})}$$

$$E = 11.479\ \mathrm{ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\, M_o\, L_e) + \left(3\, H_o\, L_e^2\right)}{(6\, M_o) + (4\, H_o\, L_e)}$$

$$a = \frac{(4 \times (4.908\ \mathrm{kipft/ft}) \times (5.75\ \mathrm{ft})) + \left(3 \times (-0.42755\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft})^2\right)}{}$$

$$a = \frac{}{(6 \times (4.908 \text{ kipft/ft})) + (4 \times (-0.42755 \text{ kip/ft}) \times (5.75 \text{ ft}))}$$

$$a = 3.9533 \text{ ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\, D) \left[1 - \left[3 \left(\frac{4\,E}{L_e} + 3\right) \left(\frac{a}{L_e}\right)^2\right] + \left[4 \left(\frac{3\,E}{L_e} + 2\right) \left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((-0.42755 \text{ kip/ft}) \times (48 \text{ in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (11.479 \text{ ft})}{(5.75 \text{ ft})} + 3\right) \times \left(\frac{(3.9533 \text{ ft})}{(5.75 \text{ ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (11.479 \text{ ft})}{(5.75 \text{ ft})} + 2\right) \times \left(\frac{(3.9533 \text{ ft})}{(5.75 \text{ ft})}\right)^3\right]\right]$$

$$V_{max} = 7.1706 \text{ kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, D\, L_e) \left[\left(\frac{E}{L_e} + \frac{a}{2\,L_e}\right) - \left[\left(\frac{4\,E}{L_e} + 3\right) \left(\frac{a}{2\,L_e}\right)^3\right] + \left[\left(\frac{3\,E}{L_e} + 2\right) \left(\frac{a}{2\,L_e}\right)^4\right]\right]$$

$$M_{max} = ((-0.42755 \text{ kip/ft}) \times (48 \text{ in}) \times (5.75 \text{ ft})) \times \left[\left(\frac{(11.479 \text{ ft})}{(5.75 \text{ ft})} + \frac{(3.9533 \text{ ft})}{2 \times (5.75 \text{ ft})}\right) - \left[\left(\frac{4 \times (11.479 \text{ ft})}{(5.75 \text{ ft})} + 3\right) \times \left(\frac{(3.9533 \text{ ft})}{2 \times (5.75 \text{ ft})}\right)^3\right] + \left[\left(\frac{3 \times (11.479 \text{ ft})}{(5.75 \text{ ft})} + 2\right) \times \left(\frac{(3.9533 \text{ ft})}{2 \times (5.75 \text{ ft})}\right)^4\right]\right]$$

$$M_{max} = 19.721 \text{ kipft}$$

Shear force and Bending moment (z-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\, b}$$

$$H_o = \frac{(0.366 \text{ kip})}{1.57 \times (48 \text{ in})}$$

$$H_o = 0.05828 \text{ kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\, H)}{1.57\, b}$$

$$M_o = \frac{(1.238 \text{ kipft}) + ((0.366 \text{ kip}) \times (0 \text{ ft}))}{1.57 \times (48 \text{ in})}$$

$$M_o = 0.19713 \text{ kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(0.19713 \text{ kipft/ft})}{(0.05828 \text{ kip/ft})}$$

$$E = 3.3825 \text{ ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\, M_o\, L_e) + \left(3\, H_o\, L_e^2\right)}{(6\, M_o) + (4\, H_o\, L_e)}$$

$$a = \frac{(4 \times (0.19713 \text{ kipft/ft}) \times (5.75 \text{ ft})) + \left(3 \times (0.05828 \text{ kip/ft}) \times (5.75 \text{ ft})^2\right)}{(6 \times (0.19713 \text{ kipft/ft})) + (4 \times (0.05828 \text{ kip/ft}) \times (5.75 \text{ ft}))}$$

$$a = 4.0879 \text{ ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\, b) \left[1 - \left[3 \left(\frac{4\,E}{L_e} + 3\right) \left(\frac{a}{L_e}\right)^2\right] + \left[4 \left(\frac{3\,E}{L_e} + 2\right) \left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((0.05828 \text{ kip/ft}) \times (48 \text{ in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (3.3825 \text{ ft})}{(5.75 \text{ ft})} + 3\right) \times \left(\frac{(4.0879 \text{ ft})}{(5.75 \text{ ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (3.3825 \text{ ft})}{(5.75 \text{ ft})} + 2\right) \times \left(\frac{(4.0879 \text{ ft})}{(5.75 \text{ ft})}\right)^3\right]\right]$$

$$V_{max} = 0.39761 \text{ kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, b\, L_e) \left[\left(\frac{E}{L_e} + \frac{a}{2\, L_e}\right) - \left[\left(\frac{4\, E}{L_e} + 3\right)\left(\frac{a}{2\, L_e}\right)^3\right] + \left[\left(\frac{3\, E}{L_e} + 2\right)\left(\frac{a}{2\, L_e}\right)^4\right]\right]$$

$$M_{max} = ((0.05828 \text{ kip/ft}) \times (48 \text{ in}) \times (5.75 \text{ ft})) \times \left[\left(\frac{(3.3825 \text{ ft})}{(5.75 \text{ ft})} + \frac{(4.0879 \text{ ft})}{2 \times (5.75 \text{ ft})}\right) - \left[\left(\frac{4 \times (3.3825 \text{ ft})}{(5.75 \text{ ft})} + 3\right) \times \left(\frac{(4.0879 \text{ ft})}{2 \times (5.75 \text{ ft})}\right)^3\right] + \left[\left(\frac{3 \times (3.3825 \text{ ft})}{(5.75 \text{ ft})} + 2\right) \times \left(\frac{(4.0879 \text{ ft})}{2 \times (5.75 \text{ ft})}\right)^4\right]\right]$$

$$M_{max} = 1.0233 \text{ kipft}$$

Minimum Reinforcement Check (LRFD)

Parameters:

$f'_{ck} = 2.5 \text{ ksi}$ - Concrete strength,
$f_{yk} = 60 \text{ ksi}$ - Longitudinal reinforcement strength,
$\phi = 0.65$ - Reduction factor for axial strength,
Table 22.4.2.1 $\alpha = 0.8$ - Alpha factor for axial strength,
$A_g = 2304 \text{ in}^2$ - Gross area of concrete,

Longitudinal reinforcement:

Required reinforcement due to axial load, $A_{st,required}$
22.4.2.2, 10.6.1.1 $A_{st,required}$

$$A_{st,required} = Min\left[\frac{\frac{P}{\phi\, \alpha} - (0.85\, f'_{ck}\, A_g)}{f_{yk} - (0.85\, f'_{ck})}, (0.08\, A_g)\right]$$

$$A_{st,required} = Min\left[\frac{\frac{(5.377 \text{ kip})}{(0.65)\times(0.8)} - (0.85 \times (2.5 \text{ ksi}) \times (2304 \text{ in}^2))}{(60 \text{ ksi}) - (0.85 \times (2.5 \text{ ksi}))}, (0.08 \times (2304 \text{ in}^2))\right]$$

$$A_{st,required} = -84.417 \text{ in}^2$$

A_{min} - Governing minimum reinforcement area,

$$A_{min} = Max[A_{st,required}, (0.0018\, A_g)]$$

$$A_{min} = Max[(-84.417 \text{ in}^2), (0.0018 \times (2304 \text{ in}^2))]$$

$$A_{min} = 4.1472 \text{ in}^2$$

n_{rebar} - Required number of reinforcement,

$$n_{rebar} = \frac{A_{min}}{A_{rebar}}$$

$$n_{rebar} = \frac{(4.1472 \text{ in}^2)}{(0.3068 \text{ in}^2)}$$

$$n_{rebar} = 14$$

A_{st} - Actual total reinforcement area,

$$A_{st} = n_{rebar}\, \frac{\pi\, d_{bar}^2}{4}$$

$$A_{st} = (14) \times \frac{\pi \times (0.625 \text{ in})^2}{4}$$

$$A_{st} = 4.2951 \text{ in}^2$$

$Ratio$ - Capacity

$$Ratio = \frac{A_{min}}{A_{st}}$$

$$Ratio = \frac{(4.1472 \text{ in}^2)}{(4.2951 \text{ in}^2)}$$

$$Ratio = 0.96556$$

Status: PASS
Ratio: 0.970

25.2.3 s_{rebar} - Minimum spacing of reinforcement,

$$s_{rebar} = Max\left[1.5, \left(1.5\ d_{bar}\right)\right]$$

$$s_{rebar} = Max\left[1.5, \left(1.5 \times (0.625\ \mathrm{in})\right)\right]$$

$$s_{rebar} = 1.5\ \mathrm{in}$$

Ties:

25.7.2.2 Since longitudinal reinforcement is $\leq$ No. 10ø: Use #3(0.375 in)

25.7.2.1 s_{ties} - Maximum spacing of ties,

$$s_{ties} = Min\left[\left(16\ d_{bar}\right), \left(48\ d_{ties}\right), Min\left(D, b\right)\right]$$

$$s_{ties} = Min\left[\left(16 \times (0.625\ \mathrm{in})\right), \left(48 \times (0.375\ \mathrm{in})\right), Min\left((48\ \mathrm{in}), (48\ \mathrm{in})\right)\right]$$

$$s_{ties} = 10\ \mathrm{in}$$

Summary:

Main reinforcement: **14 - #5 (0.625 in)**
Ties: **#3(0.375 in) - 10 in**

Axial Compression Strength (ACI 318-19, LRFD)

22.4.2.2 ϕP_N - Allowable axial compressive strength

$$\phi P_N = \phi\ 0.80\ \left[\left(0.85\ f'_{ck}\ \left[A_g - A_{st}\right]\right) + \left(f_{yk}\ A_{st}\right)\right]$$

$$\phi P_N = (0.65) \times 0.80 \times \left[\left(0.85 \times (2.5\ \mathrm{ksi}) \times \left[\left(2304\ \mathrm{in}^2\right) - \left(4.2951\ \mathrm{in}^2\right)\right]\right) + \left((60\ \mathrm{ksi}) \times \left(4.2951\ \mathrm{in}^2\right)\right)\right]$$

$$\phi P_N = 2675.2\ \mathrm{kip}$$

$Ratio$ - Capacity

$$Ratio = \frac{P}{\phi P_N}$$

$$Ratio = \frac{(5.377\ \mathrm{kip})}{(2675.2\ \mathrm{kip})}$$

$$Ratio = 0.00201$$

Status: PASS
Ratio: 0.000

Shear Strength (ACI 318-19, LRFD)

Parameters:

$b_w = 48\ \mathrm{in}$ - Effective width,

22.5.2.2 d - Effective depth

$$d = 0.80\ D$$

$$d = 0.80 \times (48\ \mathrm{in})$$

$$d = 38.4\ \mathrm{in}$$

22.5.5.1.3 λ_s - size effect modification factor

$$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{d}{10}}}, 1\right]$$

$$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{(38.4\ \mathrm{in})}{10}}}, 1\right]$$

$$\lambda_s = 0.64282$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5\ \mathrm{ksi} \rightarrow 2500\ \mathrm{psi}$,

22.5.5.1.1 $V_{c,max}$ - Max shear strength of concrete

$$V_{c,max} = 5\ \lambda_s\ \sqrt{f'_{ck}}\ b_w\ d$$

$$V_{c,max} = 5 \times (0.64282) \times \sqrt{(2500\ \mathrm{psi})} \times (48\ \mathrm{in}) \times (38.4\ \mathrm{in})$$

$$V_{c,max} = 296.21 \text{ kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$, $P = 5.377 \text{ kip} \rightarrow 5377 \text{ lbf}$,

22.5.5.1.1(a) $V_{c,a}$ - Shear strength of concrete (a)

$$V_{c,a} = \left[2\,\lambda_s\,\sqrt{f'_{ck}} + \frac{P}{6\,A_g}\right]\,b_w\,d$$

$$V_{c,a} = \left[2 \times (0.64282) \times \sqrt{(2500 \text{ psi})} + \frac{(5377 \text{ lbf})}{6 \times (2304 \text{ in}^2)}\right] \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{c,a} = 119.2 \text{ kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$,

22.5.5.1.2 $V_{c,b}$ - Shear strength of concrete (b)

$$V_{c,b} = \left[2\,\lambda_s\,\sqrt{f'_{ck}} + \left(0.05\,f'_{ck}\right)\right]\,b_w\,d$$

$$V_{c,b} = \left[2 \times (0.64282) \times \sqrt{(2500 \text{ psi})} + (0.05 \times (2500 \text{ psi}))\right] \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{c,b} = 348.89 \text{ kip}$$

V_c - Governing shear strength of concrete

$$V_c = Min\left[V_{c,max}, V_{c,a}, V_{c,b}\right]$$

$$V_c = Min\left[(296.21 \text{ kip}), (119.2 \text{ kip}), (348.89 \text{ kip})\right]$$

$$V_c = 119.2 \text{ kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$,

22.5.1.2 $V_{s,a}$ - Shear strength of steel (a)

$$V_{s,a} = 8\,\sqrt{f'_{ck}}\,b_w\,d$$

$$V_{s,a} = 8 \times \sqrt{(2500 \text{ psi})} \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{s,a} = 737.28 \text{ kip}$$

A_v - Ties rebar area,

$$A_v = \frac{\pi\,d_{ties}^2}{4}$$

$$A_v = \frac{\pi \times (0.375 \text{ in})^2}{4}$$

$$A_v = 0.11045 \text{ in}^2$$

22.5.8.5.3 $V_{s,b}$ - Shear strength of steel (b)

$$V_{s,b} = \frac{2\,A_v\,f_{ywk}\,d}{s_{ties}}$$

$$V_{s,b} = \frac{2 \times \left(0.11045 \text{ in}^2\right) \times (60 \text{ ksi}) \times (38.4 \text{ in})}{(10 \text{ in})}$$

$$V_{s,b} = 50.894 \text{ kip}$$

V_s - Governing shear strength of steel

$$V_s = MIN\left[V_{s,a}, V_{s,b}\right]$$

$$V_s = MIN\left[(737.28 \text{ kip}), (50.894 \text{ kip})\right]$$

$$V_s = 50.894 \text{ kip}$$

22.5.1.1 ϕV_n - Allowable shear strength

$$\phi V_n = \phi\ (V_c + V_s)$$

$$\phi V_n = (0.65) \times ((119.2 \text{ kip}) + (50.894 \text{ kip}))$$

$$\phi V_n = 110.56 \text{ kip}$$

Considering x-direction:

$V_{max} = 7.1706 \text{ kip}$ - Maximum shear force in the x-direction,
$Ratio$ - Capacity

$$Ratio = \frac{V_{max}}{\phi V_n}$$

$$Ratio = \frac{(7.1706 \text{ kip})}{(110.56 \text{ kip})}$$

$$Ratio = 0.064855$$

Status: **PASS**
Ratio: **0.060**

Considering z-direction:

$V_{max} = 0.39761 \text{ kip}$ - Maximum shear force in the z-direction,
$Ratio$ - Capacity

$$Ratio = \frac{V_{max}}{\phi V_n}$$

$$Ratio = \frac{(0.39761 \text{ kip})}{(110.56 \text{ kip})}$$

$$Ratio = 0.0035963$$

Status: **PASS**
Ratio: **0.000**

Flexural Strength (ACI 318-19, LRFD)

S_m - Section modulus

$$S_m = \frac{b\,D^2}{6}$$

$$S_m = \frac{(48 \text{ in}) \times (48 \text{ in})^2}{6}$$

$$S_m = 18432 \text{ in}^3$$

$\lambda = 1$ - Concrete modification factor (Normal concrete),
Allowable flexural strength:
M_n shall be the lesser of:
$\phi M_{n,1}$

$$\phi M_{n,1} = \phi \times 5 \times \lambda \times \sqrt{(f_c')} \times S_m$$

$$\phi M_{n,1} = 0.65 \times 5 \times 1 \times \sqrt{(2.5ksi)} \times 18432.001in^3$$

$$\phi M_{n,1} = 249.600kipft$$

14.5.2.1b $\phi M_{n,2}$

$$\phi M_{n,2} = \phi\, 0.85\, f_{ck}'\, S_m$$

$$\phi M_{n,2} = (0.65) \times 0.85 \times (2.5 \text{ ksi}) \times \left(18432 \text{ in}^3\right)$$

$$\phi M_{n,2} = 2121.6 \text{ kipft}$$

Therefore,
ϕM_n - Allowable flexural strength,

$$\phi M_n = MIN[\phi M_{n,1}, \phi M_{n,2}]$$

$$\phi M_n = MIN[(249.6 \text{ kipft}), (2121.6 \text{ kipft})]$$

$$\phi M_n = 249.6 \text{ kipft}$$

Considering x-direction:

$M_{max} = 19.721 \text{ kipft}$ - Maximum moment in the x-direction,
$Ratio$ - Capacity

$$Ratio = \frac{M_{max}}{\phi M_n}$$

$$Ratio = \frac{(19.721 \text{ kipft})}{(249.6 \text{ kipft})}$$

$$Ratio = 0.07901$$

Status: **PASS**
Ratio: **0.080**

Considering z-direction:

$M_{max} = 1.0233 \text{ kipft}$ - Maximum moment in the z-direction,
$Ratio$ - Capacity

$$Ratio = \frac{M_{max}}{\phi M_n}$$

	$$Ratio = \frac{(1.0233 \text{ kipft})}{(249.6 \text{ kipft})}$$ $$Ratio = 0.0040998$$	Status: **PASS** Ratio: **0.000**

REFERENCES	CALCULATIONS	RESULTS

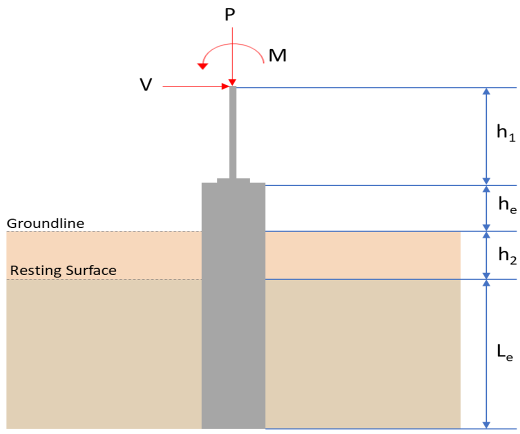
SkyCiv Foundation Design

Pile Foundation

Design Information :

Design code : IBC 2021 (International Building Code)
Unit System : Imperial

Pile Input



Geometry

Pile shape: rectangular
$b = 48$ in - Pile width
$D = 48$ in - Pile depth
$L = 5.75$ ft - Total pile length
$h_1 = 0$ ft - Lateral load height from the top of the pile,
$h_2 = 0$ ft - Depth to resisting surface
$h_e = 0$ ft - Length of pile above the ground

Tabulation of Soil Parameters

Layer	Label	Allowable Bearing Pressure (q_a) (psf)	Allowable Lateral Pressure (R) (psf/ft)
1	Sand, silty sand, clayey sand, silty gravel & clayey gravel	2000.000	150.000

Tabulation of Loads

Load Component	ASD	LRFD
P (kip)	3.694	5.377
V_x (kip)	-1.608	-2.685
V_z (kip)	-0.234	-0.366
M_x (kipft)	-0.798	-1.238
M_z (kipft)	18.286	30.822

Material Properties

$f'_{ck} = 2.5$ ksi - Concrete strength,

Required depth to resist lateral loads (ASD)

H - Point of application of the lateral load

$$H = h_1 + h_2 + h_e$$

$$H = (0\ \text{ft}) + (0\ \text{ft}) + (0\ \text{ft})$$

$$H = 0\ \text{ft}$$

Considering x-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\ D}$$

$$H_o = \frac{(-1.608\ \text{kip})}{1.57 \times (48\ \text{in})}$$

$$H_o = -0.25605\ \text{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\,H)}{1.57\,D}$$

$$M_o = \frac{(18.286\ \text{kipft}) + ((-1.608\ \text{kip}) \times (0\ \text{ft}))}{1.57 \times (48\ \text{in})}$$

$$M_o = 2.9118\ \text{kipft/ft}$$

Required depth of embedment in earth:

$$L_x^3 - \left(14.14 \times \frac{H_o \times L_x}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,x} = 5.3264\ \text{ft}$ - Required depth in x-direction,

Considering z-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\,b}$$

$$H_o = \frac{(-0.234\ \text{kip})}{1.57 \times (48\ \text{in})}$$

$$H_o = -0.037261\ \text{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\,H)}{1.57\,b}$$

$$M_o = \frac{(0.798\ \text{kipft}) + ((-0.234\ \text{kip}) \times (0\ \text{ft}))}{1.57 \times (48\ \text{in})}$$

$$M_o = 0.12707\ \text{kipft/ft}$$

Required depth of embedment in earth:

$$L_z^3 - \left(14.14 \times \frac{H_o \times L_z}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,z} = 1.8256\ \text{ft}$ - Required depth in z-direction,

Minimum embedded depth required:

$L_{e,req}$ - Depth of pile required,

$$L_{e,req} = MAX[L_{e,x}, L_{e,z}]$$

$$L_{e,req} = MAX[(5.3264\ \text{ft}), (1.8256\ \text{ft})]$$

$$L_{e,req} = 5.326\ \text{ft}$$

L_e - Actual embedded length of pile,

$$L_e = L - h_e - h_2$$

$$L_e = (5.75\ \text{ft}) - (0\ \text{ft}) - (0\ \text{ft})$$

$$L_e = 5.75\ \text{ft}$$

$Ratio$ - Embedded depth

$$Ratio = \frac{L_{e,req}}{L_e}$$

$$Ratio = \frac{(5.326\ \text{ft})}{(5.75\ \text{ft})}$$

$$Ratio = 0.92626$$

Status: **PASS**
Ratio: **0.930**

End-bearing Capacity (ASD)

A - Pile cross-section area

$$A = b\,D$$

$$A = (48\ \text{in}) \times (48\ \text{in})$$

$$A = 16\ \text{ft}^2$$

q - End-bearing pressure

$$q = \frac{P_v}{A}$$

$$q = \frac{(3.694\ \mathrm{kip})}{(16\ \mathrm{ft}^2)}$$

$$q = 0.23088\ \mathrm{kip/ft^2}$$

Check bearing capacity ratio:

$Ratio$ - Capacity

$$Ratio = \frac{q}{q_a}$$

$$Ratio = \frac{(0.23088\ \mathrm{kip/ft^2})}{(2000\ \mathrm{psf})}$$

$$Ratio = 0.11544$$

Status: **PASS**
Ratio: **0.120**

Czerniak

Lateral Soil Pressure (ASD):

L/D - Length to least lateral dimension ratio,

$$L/D = \frac{L}{D}$$

$$L/D = \frac{(5.75\ \mathrm{ft})}{(48\ \mathrm{in})}$$

$$L/D = 1.4375$$

Since L/D $\leq$ 10,

Pile is short.

Considering x-direction:

$H_o = -0.25605\ \mathrm{kip/ft}$ - Lateral force per length of pile,
$M_o = 2.9118\ \mathrm{kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\, M_o\, L_e) + \left(3\, H_o\, L_e^2\right)}{(6\, M_o) + (4\, H_o\, L_e)}$$

$$a = \frac{(4 \times (2.9118\ \mathrm{kipft/ft}) \times (5.75\ \mathrm{ft})) + \left(3 \times (-0.25605\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft})^2\right)}{(6 \times (2.9118\ \mathrm{kipft/ft})) + (4 \times (-0.25605\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))}$$

$$a = 3.9541\ \mathrm{ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\, [(4\, M_o) + (3\, H_o\, L_e)]^2}{L_e^2\, [(3\, M_o) + (2\, H_o\, L_e)]}$$

$$p = \frac{0.75 \times [(4 \times (2.9118\ \mathrm{kipft/ft})) + (3 \times (-0.25605\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft})]^{\ 2}}{(5.75\ \mathrm{ft})^2 \times [(3 \times (2.9118\ \mathrm{kipft/ft})) + (2 \times (-0.25605\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))]}$$

$$p = 0.20478\ \mathrm{kip/ft^2}$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\ [(2\, M_o) + (H_o\, L_e)]}{L_e^2}$$

$$s = \frac{6 \times [(2 \times (2.9118\ \mathrm{kipft/ft})) + ((-0.25605\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))]}{(5.75\ \mathrm{ft})^2}$$

$$s = 0.78965\ \mathrm{kip/ft^2}$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\, \frac{a}{2}$$

$$p_a = (150\ \mathrm{psf/ft}) \times \frac{(3.9541\ \mathrm{ft})}{2}$$

$$p_a = 0.29656\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

	$$Ratio = \frac{p}{p_a}$$ $$Ratio = \frac{\left(0.20478\ \mathrm{kip/ft^2}\right)}{\left(0.29656\ \mathrm{kip/ft^2}\right)}$$ $$Ratio = 0.69053$$	Status: **PASS** Ratio: **0.690**
	p_s - Allowable lateral soil pressure at depth L_e, $$p_s = R\,L_e$$ $$p_s = (150\ \mathrm{psf/ft}) \times (5.75\ \mathrm{ft})$$ $$p_s = 0.8625\ \mathrm{kip/ft^2}$$ $Ratio$ - Lateral soil capacity $$Ratio = \frac{s}{p_s}$$ $$Ratio = \frac{\left(0.78965\ \mathrm{kip/ft^2}\right)}{\left(0.8625\ \mathrm{kip/ft^2}\right)}$$ $$Ratio = 0.91553$$	Status: **PASS** Ratio: **0.920**
	Considering z-direction: $H_o = -0.037261\ \mathrm{kip/ft}$ - Lateral force per length of pile, $M_o = 0.12707\ \mathrm{kipft/ft}$ - Overturning moment per length of pile, a - Distance from resting surface to pivot point, $$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$ $$a = \frac{(4 \times (0.12707\ \mathrm{kipft/ft}) \times (5.75\ \mathrm{ft})) + \left(3 \times (-0.037261\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft})^2\right)}{(6 \times (0.12707\ \mathrm{kipft/ft})) + (4 \times (-0.037261\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))}$$ $$a = 4.0869\ \mathrm{ft}$$ p - Earth pressure against the pile at distance a/2 from resting surface, $$p = \frac{0.75\,[(4\,M_o) + (3\,H_o\,L_e)]^2}{L_e^2\,[(3\,M_o) + (2\,H_o\,L_e)]}$$ $$p = \frac{0.75 \times [(4 \times (0.12707\ \mathrm{kipft/ft})) + (3 \times (-0.037261\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))]^2}{(5.75\ \mathrm{ft})^2 \times [(3 \times (0.12707\ \mathrm{kipft/ft})) + (2 \times (-0.037261\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))]}$$ $$p = -0.0086738\ \mathrm{kip/ft^2}$$ s - Earth pressure against the pile at distance L_e, $$s = \frac{6\,[(2\,M_o) + (H_o\,L_e)]}{L_e^2}$$ $$s = \frac{6 \times [(2 \times (0.12707\ \mathrm{kipft/ft})) + ((-0.037261\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))]}{(5.75\ \mathrm{ft})^2}$$ $$s = 0.0072388\ \mathrm{kip/ft^2}$$ **Check lateral soil pressure capacity:** p_a - Allowable lateral soil pressure at depth a/2, $$p_a = R\,\frac{a}{2}$$ $$p_a = (150\ \mathrm{psf/ft}) \times \frac{(4.0869\ \mathrm{ft})}{2}$$ $$p_a = 0.30652\ \mathrm{kip/ft^2}$$ $Ratio$ - Lateral soil capacity $$Ratio = \frac{p}{p_a}$$ $$Ratio = \frac{\left(-0.0086738\ \mathrm{kip/ft^2}\right)}{\left(0.30652\ \mathrm{kip/ft^2}\right)}$$	

$$Ratio = -0.028298$$

Status: **PASS**
Ratio: **-0.030**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\, L_e$$

$$p_s = (150\ \mathrm{psf/ft}) \times (5.75\ \mathrm{ft})$$

$$p_s = 0.8625\ \mathrm{kip/ft^2}$$

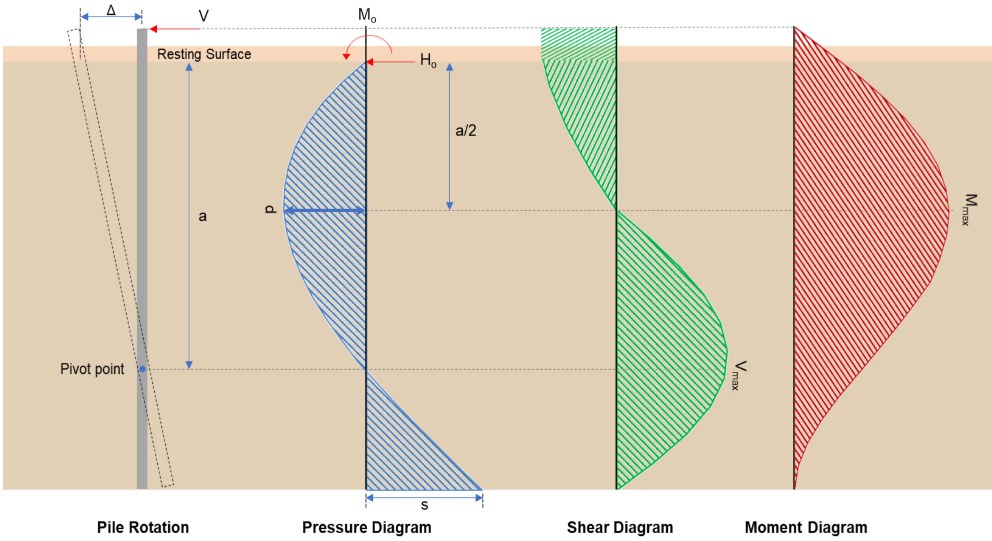
$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{\left(0.0072388\ \mathrm{kip/ft^2}\right)}{\left(0.8625\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = 0.0083928$$

Status: **PASS**
Ratio: **0.010**



Shear force and Bending moment (x-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\, D}$$

$$H_o = \frac{(-2.685\ \mathrm{kip})}{1.57 \times (48\ \mathrm{in})}$$

$$H_o = -0.42755\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\, H)}{1.57\, D}$$

$$M_o = \frac{(30.822\ \mathrm{kipft}) + ((-2.685\ \mathrm{kip}) \times (0\ \mathrm{ft}))}{1.57 \times (48\ \mathrm{in})}$$

$$M_o = 4.908\ \mathrm{kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(4.908\ \mathrm{kipft/ft})}{(-0.42755\ \mathrm{kip/ft})}$$

$$E = 11.479\ \mathrm{ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\, M_o\, L_e) + \left(3\, H_o\, L_e^2\right)}{(6\, M_o) + (4\, H_o\, L_e)}$$

$$a = \frac{(4 \times (4.908\ \mathrm{kipft/ft}) \times (5.75\ \mathrm{ft})) + \left(3 \times (-0.42755\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft})^2\right)}{}$$

$$a = \frac{\quad}{(6 \times (4.908\ \text{kipft/ft})) + (4 \times (-0.42755\ \text{kip/ft}) \times (5.75\ \text{ft}))}$$

$$a = 3.9533\ \text{ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\, D) \left[1 - \left[3 \left(\frac{4\,E}{L_e} + 3\right) \left(\frac{a}{L_e}\right)^2\right] + \left[4 \left(\frac{3\,E}{L_e} + 2\right) \left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((-0.42755\ \text{kip/ft}) \times (48\ \text{in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (11.479\ \text{ft})}{(5.75\ \text{ft})} + 3\right) \times \left(\frac{(3.9533\ \text{ft})}{(5.75\ \text{ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (11.479\ \text{ft})}{(5.75\ \text{ft})} + 2\right) \times \left(\frac{(3.9533\ \text{ft})}{(5.75\ \text{ft})}\right)^3\right]\right]$$

$$V_{max} = 7.1706\ \text{kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, D\, L_e) \left[\left(\frac{E}{L_e} + \frac{a}{2\,L_e}\right) - \left[\left(\frac{4\,E}{L_e} + 3\right) \left(\frac{a}{2\,L_e}\right)^3\right] + \left[\left(\frac{3\,E}{L_e} + 2\right) \left(\frac{a}{2\,L_e}\right)^4\right]\right]$$

$$M_{max} = ((-0.42755\ \text{kip/ft}) \times (48\ \text{in}) \times (5.75\ \text{ft})) \times \left[\left(\frac{(11.479\ \text{ft})}{(5.75\ \text{ft})} + \frac{(3.9533\ \text{ft})}{2 \times (5.75\ \text{ft})}\right) - \left[\left(\frac{4 \times (11.479\ \text{ft})}{(5.75\ \text{ft})} + 3\right) \times \left(\frac{(3.9533\ \text{ft})}{2 \times (5.75\ \text{ft})}\right)^3\right] + \left[\left(\frac{3 \times (11.479\ \text{ft})}{(5.75\ \text{ft})} + 2\right) \times \left(\frac{(3.9533\ \text{ft})}{2 \times (5.75\ \text{ft})}\right)^4\right]\right]$$

$$M_{max} = 19.721\ \text{kipft}$$

Shear force and Bending moment (z-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\,b}$$

$$H_o = \frac{(-0.366\ \text{kip})}{1.57 \times (48\ \text{in})}$$

$$H_o = -0.05828\ \text{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\, H)}{1.57\,b}$$

$$M_o = \frac{(1.238\ \text{kipft}) + ((-0.366\ \text{kip}) \times (0\ \text{ft}))}{1.57 \times (48\ \text{in})}$$

$$M_o = 0.19713\ \text{kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(0.19713\ \text{kipft/ft})}{(-0.05828\ \text{kip/ft})}$$

$$E = 3.3825\ \text{ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (0.19713\ \text{kipft/ft}) \times (5.75\ \text{ft})) + \left(3 \times (-0.05828\ \text{kip/ft}) \times (5.75\ \text{ft})^2\right)}{(6 \times (0.19713\ \text{kipft/ft})) + (4 \times (-0.05828\ \text{kip/ft}) \times (5.75\ \text{ft}))}$$

$$a = 4.0879\ \text{ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\, b) \left[1 - \left[3 \left(\frac{4\,E}{L_e} + 3\right) \left(\frac{a}{L_e}\right)^2\right] + \left[4 \left(\frac{3\,E}{L_e} + 2\right) \left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((-0.05828\ \text{kip/ft}) \times (48\ \text{in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (3.3825\ \text{ft})}{(5.75\ \text{ft})} + 3\right) \times \left(\frac{(4.0879\ \text{ft})}{(5.75\ \text{ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (3.3825\ \text{ft})}{(5.75\ \text{ft})} + 2\right) \times \left(\frac{(4.0879\ \text{ft})}{(5.75\ \text{ft})}\right)^3\right]\right]$$

$$V_{max} = 0.39761\ \text{kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, b\, L_e) \left[\left(\frac{E}{L_e} + \frac{a}{2\, L_e}\right) - \left[\left(\frac{4\, E}{L_e} + 3\right) \left(\frac{a}{2\, L_e}\right)^3\right] + \left[\left(\frac{3\, E}{L_e} + 2\right) \left(\frac{a}{2\, L_e}\right)^4\right]\right]$$

$$M_{max} = ((-0.05828\ \text{kip/ft}) \times (48\ \text{in}) \times (5.75\ \text{ft})) \times \left[\left(\frac{(3.3825\ \text{ft})}{(5.75\ \text{ft})} + \frac{(4.0879\ \text{ft})}{2 \times (5.75\ \text{ft})}\right) - \left[\left(\frac{4 \times (3.3825\ \text{ft})}{(5.75\ \text{ft})} + 3\right) \times \left(\frac{(4.0879\ \text{ft})}{2 \times (5.75\ \text{ft})}\right)^3\right] + \left[\left(\frac{3 \times (3.3825\ \text{ft})}{(5.75\ \text{ft})} + 2\right) \times \left(\frac{(4.0879\ \text{ft})}{2 \times (5.75\ \text{ft})}\right)^4\right]\right]$$

$$M_{max} = 1.0233\ \text{kipft}$$

Minimum Reinforcement Check (LRFD)

Parameters:

$f'_{ck} = 2.5\ \text{ksi}$ - Concrete strength,
$f_{yk} = 60\ \text{ksi}$ - Longitudinal reinforcement strength,
$\phi = 0.65$ - Reduction factor for axial strength,
Table 22.4.2.1 $\alpha = 0.8$ - Alpha factor for axial strength,
$A_g = 2304\ \text{in}^2$ - Gross area of concrete,

Longitudinal reinforcement:

Required reinforcement due to axial load, $A_{st,required}$

22.4.2.2, 10.6.1.1 $A_{st,required}$

$$A_{st,required} = Min\left[\frac{\frac{P}{\phi\, \alpha} - (0.85\, f'_{ck}\, A_g)}{f_{yk} - (0.85\, f'_{ck})}, (0.08\, A_g)\right]$$

$$A_{st,required} = Min\left[\frac{\frac{(5.377\ \text{kip})}{(0.65) \times (0.8)} - (0.85 \times (2.5\ \text{ksi}) \times (2304\ \text{in}^2))}{(60\ \text{ksi}) - (0.85 \times (2.5\ \text{ksi}))}, (0.08 \times (2304\ \text{in}^2))\right]$$

$$A_{st,required} = -84.417\ \text{in}^2$$

A_{min} - Governing minimum reinforcement area,

$$A_{min} = Max\left[A_{st,required}, (0.0018\, A_g)\right]$$

$$A_{min} = Max\left[(-84.417\ \text{in}^2), (0.0018 \times (2304\ \text{in}^2))\right]$$

$$A_{min} = 4.1472\ \text{in}^2$$

n_{rebar} - Required number of reinforcement,

$$n_{rebar} = \frac{A_{min}}{A_{rebar}}$$

$$n_{rebar} = \frac{(4.1472\ \text{in}^2)}{(0.3068\ \text{in}^2)}$$

$$n_{rebar} = 14$$

A_{st} - Actual total reinforcement area,

$$A_{st} = n_{rebar}\, \frac{\pi\, d_{bar}^2}{4}$$

$$A_{st} = (14) \times \frac{\pi \times (0.625\ \text{in})^2}{4}$$

$$A_{st} = 4.2951\ \text{in}^2$$

$Ratio$ - Capacity

$$Ratio = \frac{A_{min}}{A_{st}}$$

$$Ratio = \frac{(4.1472\ \text{in}^2)}{(4.2951\ \text{in}^2)}$$

	$$Ratio = 0.96556$$	Status: **PASS** Ratio: **0.970**
25.2.3	s_{rebar} - Minimum spacing of reinforcement, $$s_{rebar} = Max\left[1.5, \left(1.5\, d_{bar}\right)\right]$$ $$s_{rebar} = Max\left[1.5, \left(1.5 \times (0.625\ \mathrm{in})\right)\right]$$ $$s_{rebar} = 1.5\ \mathrm{in}$$ **Ties:**	
25.7.2.2 25.7.2.1	Since longitudinal reinforcement is $\leq$ No. 10ø: Use #3(0.375 in) s_{ties} - Maximum spacing of ties, $$s_{ties} = Min\left[\left(16\, d_{bar}\right), \left(48\, d_{ties}\right), Min\left(D, b\right)\right]$$ $$s_{ties} = Min\left[\left(16 \times (0.625\ \mathrm{in})\right), \left(48 \times (0.375\ \mathrm{in})\right), Min\left((48\ \mathrm{in}), (48\ \mathrm{in})\right)\right]$$ $$s_{ties} = 10\ \mathrm{in}$$ **Summary:** Main reinforcement: **14 - #5 (0.625 in)** Ties: **#3(0.375 in) - 10 in**	
22.4.2.2	**Axial Compression Strength (ACI 318-19, LRFD)** ϕP_N - Allowable axial compressive strength $$\phi P_N = \phi\, 0.80\, \left[\left(0.85\, f'_{ck}\, \left[A_g - A_{st}\right]\right) + \left(f_{yk}\, A_{st}\right)\right]$$ $$\phi P_N = (0.65) \times 0.80 \times \left[\left(0.85 \times (2.5\ \mathrm{ksi}) \times \left[\left(2304\ \mathrm{in}^2\right) - \left(4.2951\ \mathrm{in}^2\right)\right]\right) + \left((60\ \mathrm{ksi}) \times \left(4.2951\ \mathrm{in}^2\right)\right)\right]$$ $$\phi P_N = 2675.2\ \mathrm{kip}$$ $Ratio$ - Capacity $$Ratio = \frac{P}{\phi P_N}$$ $$Ratio = \frac{(5.377\ \mathrm{kip})}{(2675.2\ \mathrm{kip})}$$ $$Ratio = 0.00201$$	Status: **PASS** Ratio: **0.000**
22.5.2.2 22.5.5.1.3 22.5.5.1.1	**Shear Strength (ACI 318-19, LRFD)** **Parameters:** $b_w = 48\ \mathrm{in}$ - Effective width, d - Effective depth $$d = 0.80\, D$$ $$d = 0.80 \times (48\ \mathrm{in})$$ $$d = 38.4\ \mathrm{in}$$ λ_s - size effect modification factor $$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{d}{10}}}, 1\right]$$ $$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{(38.4\ \mathrm{in})}{10}}}, 1\right]$$ $$\lambda_s = 0.64282$$ The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5\ \mathrm{ksi} \rightarrow 2500\ \mathrm{psi}$, $V_{c,max}$ - Max shear strength of concrete $$V_{c,max} = 5\, \lambda_s\, \sqrt{f'_{ck}}\, b_w\, d$$ $$V_{c,max} = 5 \times (0.64282) \times \sqrt{(2500\ \mathrm{psi})} \times (48\ \mathrm{in}) \times (38.4\ \mathrm{in})$$	

$$V_{c,max} = 296.21 \text{ kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$, $P = 5.377 \text{ kip} \rightarrow 5377 \text{ lbf}$,

22.5.5.1.1(a) $V_{c,a}$ - Shear strength of concrete (a)

$$V_{c,a} = \left[2\,\lambda_s\,\sqrt{f'_{ck}} + \frac{P}{6\,A_g}\right]\,b_w\,d$$

$$V_{c,a} = \left[2 \times (0.64282) \times \sqrt{(2500 \text{ psi})} + \frac{(5377 \text{ lbf})}{6 \times (2304 \text{ in}^2)}\right] \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{c,a} = 119.2 \text{ kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$,

22.5.5.1.2 $V_{c,b}$ - Shear strength of concrete (b)

$$V_{c,b} = \left[2\,\lambda_s\,\sqrt{f'_{ck}} + \left(0.05\,f'_{ck}\right)\right]\,b_w\,d$$

$$V_{c,b} = \left[2 \times (0.64282) \times \sqrt{(2500 \text{ psi})} + (0.05 \times (2500 \text{ psi}))\right] \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{c,b} = 348.89 \text{ kip}$$

V_c - Governing shear strength of concrete

$$V_c = Min\left[V_{c,max}, V_{c,a}, V_{c,b}\right]$$

$$V_c = Min\left[(296.21 \text{ kip}), (119.2 \text{ kip}), (348.89 \text{ kip})\right]$$

$$V_c = 119.2 \text{ kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$,

22.5.1.2 $V_{s,a}$ - Shear strength of steel (a)

$$V_{s,a} = 8\,\sqrt{f'_{ck}}\,b_w\,d$$

$$V_{s,a} = 8 \times \sqrt{(2500 \text{ psi})} \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{s,a} = 737.28 \text{ kip}$$

A_v - Ties rebar area,

$$A_v = \frac{\pi\,d_{ties}^2}{4}$$

$$A_v = \frac{\pi \times (0.375 \text{ in})^2}{4}$$

$$A_v = 0.11045 \text{ in}^2$$

22.5.8.5.3 $V_{s,b}$ - Shear strength of steel (b)

$$V_{s,b} = \frac{2\,A_v\,f_{ywk}\,d}{s_{ties}}$$

$$V_{s,b} = \frac{2 \times \left(0.11045 \text{ in}^2\right) \times (60 \text{ ksi}) \times (38.4 \text{ in})}{(10 \text{ in})}$$

$$V_{s,b} = 50.894 \text{ kip}$$

V_s - Governing shear strength of steel

$$V_s = MIN\left[V_{s,a}, V_{s,b}\right]$$

$$V_s = MIN\left[(737.28 \text{ kip}), (50.894 \text{ kip})\right]$$

$$V_s = 50.894 \text{ kip}$$

22.5.1.1 ϕV_n - Allowable shear strength

$$\phi V_n = \phi\,\left(V_c + V_s\right)$$

$$\phi V_n = (0.65) \times ((119.2 \text{ kip}) + (50.894 \text{ kip}))$$

$$\phi V_n = 110.56 \text{ kip}$$

Considering x-direction:

$V_{max} = 7.1706 \text{ kip}$ - Maximum shear force in the x-direction,
$Ratio$ - Capacity

$$Ratio = \frac{V_{max}}{\phi V_n}$$

$$Ratio = \frac{(7.1706 \text{ kip})}{(110.56 \text{ kip})}$$

$$Ratio = 0.064855$$

Status: **PASS**
Ratio: **0.060**

Considering z-direction:

$V_{max} = 0.39761 \text{ kip}$ - Maximum shear force in the z-direction,
$Ratio$ - Capacity

$$Ratio = \frac{V_{max}}{\phi V_n}$$

$$Ratio = \frac{(0.39761 \text{ kip})}{(110.56 \text{ kip})}$$

$$Ratio = 0.0035963$$

Status: **PASS**
Ratio: **0.000**

Flexural Strength (ACI 318-19, LRFD)

S_m - Section modulus

$$S_m = \frac{b\,D^2}{6}$$

$$S_m = \frac{(48 \text{ in}) \times (48 \text{ in})^2}{6}$$

$$S_m = 18432 \text{ in}^3$$

$\lambda = 1$ - Concrete modification factor (Normal concrete),
Allowable flexural strength:
M_n shall be the lesser of:
$\phi M_{n,1}$

$$\phi M_{n,1} = \phi \times 5 \times \lambda \times \sqrt{(f_c')} \times S_m$$

$$\phi M_{n,1} = 0.65 \times 5 \times 1 \times \sqrt{(2.5ksi)} \times 18432.001in^3$$

$$\phi M_{n,1} = 249.600kipft$$

14.5.2.1b

$\phi M_{n,2}$

$$\phi M_{n,2} = \phi\; 0.85\; f_{ck}'\; S_m$$

$$\phi M_{n,2} = (0.65) \times 0.85 \times (2.5 \text{ ksi}) \times \left(18432 \text{ in}^3\right)$$

$$\phi M_{n,2} = 2121.6 \text{ kipft}$$

Therefore,
ϕM_n - Allowable flexural strength,

$$\phi M_n = MIN[\phi M_{n,1}, \phi M_{n,2}]$$

$$\phi M_n = MIN[(249.6 \text{ kipft}), (2121.6 \text{ kipft})]$$

$$\phi M_n = 249.6 \text{ kipft}$$

Considering x-direction:

$M_{max} = 19.721 \text{ kipft}$ - Maximum moment in the x-direction,
$Ratio$ - Capacity

$$Ratio = \frac{M_{max}}{\phi M_n}$$

$$Ratio = \frac{(19.721 \text{ kipft})}{(249.6 \text{ kipft})}$$

$$Ratio = 0.07901$$

Status: **PASS**
Ratio: **0.080**

Considering z-direction:

$M_{max} = 1.0233 \text{ kipft}$ - Maximum moment in the z-direction,
$Ratio$ - Capacity

$$Ratio = \frac{M_{max}}{\phi M_n}$$

	$$Ratio = \frac{(1.0233 \text{ kipft})}{(249.6 \text{ kipft})}$$ $$Ratio = 0.0040998$$	Status: **PASS** Ratio: **0.000**

REFERENCES	CALCULATIONS	RESULTS

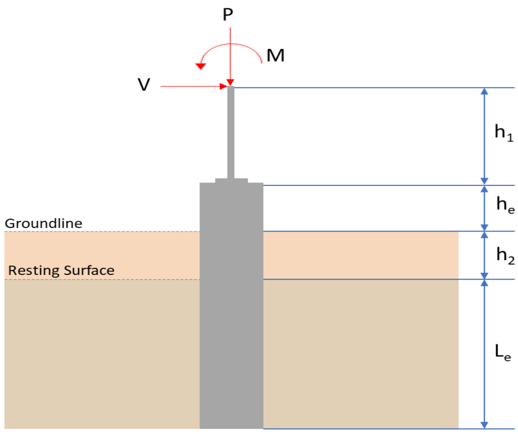
SkyCiv Foundation Design

Pile Foundation

Design Information :

Design code : IBC 2021 (International Building Code)
Unit System : Imperial

Pile Input



Geometry

Pile shape: rectangular
$b = 48\ \text{in}$ - Pile width
$D = 48\ \text{in}$ - Pile depth
$L = 6.25\ \text{ft}$ - Total pile length
$h_1 = 0\ \text{ft}$ - Lateral load height from the top of the pile,
$h_2 = 0\ \text{ft}$ - Depth to resisting surface
$h_e = 0\ \text{ft}$ - Length of pile above the ground

Tabulation of Soil Parameters

Layer	Label	Allowable Bearing Pressure (q_a) (psf)	Allowable Lateral Pressure (R) (psf/ft)
1	Sand, silty sand, clayey sand, silty gravel & clayey gravel	2000.000	150.000

Tabulation of Loads

Load Component	ASD	LRFD
P (kip)	4.713	6.882
V_x (kip)	-2.049	-3.415
V_z (kip)	-0.015	-0.023
M_x (kipft)	-0.048	-0.076
M_z (kipft)	23.379	39.705

Material Properties

$f'_{ck} = 2.5\ \text{ksi}$ - Concrete strength,

Required depth to resist lateral loads (ASD)

H - Point of application of the lateral load

$$H = h_1 + h_2 + h_e$$

$$H = (0\ \text{ft}) + (0\ \text{ft}) + (0\ \text{ft})$$

$$H = 0\ \text{ft}$$

Considering x-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\ D}$$

$$H_o = \frac{(-2.049\ \text{kip})}{1.57 \times (48\ \text{in})}$$

$$H_o = -0.32627\ \text{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\,H)}{1.57\,D}$$

$$M_o = \frac{(23.379\text{ kipft}) + ((-2.049\text{ kip}) \times (0\text{ ft}))}{1.57 \times (48\text{ in})}$$

$$M_o = 3.7228\text{ kipft/ft}$$

Required depth of embedment in earth:

$$L_x^3 - \left(14.14 \times \frac{H_o \times L_x}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,x} = 5.7089\text{ ft}$ - Required depth in x-direction,

Considering z-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\,b}$$

$$H_o = \frac{(-0.015\text{ kip})}{1.57 \times (48\text{ in})}$$

$$H_o = -0.0023885\text{ kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\,H)}{1.57\,b}$$

$$M_o = \frac{(0.048\text{ kipft}) + ((-0.015\text{ kip}) \times (0\text{ ft}))}{1.57 \times (48\text{ in})}$$

$$M_o = 0.0076433\text{ kipft/ft}$$

Required depth of embedment in earth:

$$L_z^3 - \left(14.14 \times \frac{H_o \times L_z}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,z} = 0.79241\text{ ft}$ - Required depth in z-direction,

Minimum embedded depth required:

$L_{e,req}$ - Depth of pile required,

$$L_{e,req} = MAX[L_{e,x}, L_{e,z}]$$

$$L_{e,req} = MAX[(5.7089\text{ ft}), (0.79241\text{ ft})]$$

$$L_{e,req} = 5.709\text{ ft}$$

L_e - Actual embedded length of pile,

$$L_e = L - h_e - h_2$$

$$L_e = (6.25\text{ ft}) - (0\text{ ft}) - (0\text{ ft})$$

$$L_e = 6.25\text{ ft}$$

$Ratio$ - Embedded depth

$$Ratio = \frac{L_{e,req}}{L_e}$$

$$Ratio = \frac{(5.709\text{ ft})}{(6.25\text{ ft})}$$

$$Ratio = 0.91344$$

Status: **PASS**
Ratio: **0.910**

End-bearing Capacity (ASD)

A - Pile cross-section area

$$A = b\,D$$

$$A = (48\text{ in}) \times (48\text{ in})$$

$$A = 16\text{ ft}^2$$

q - End-bearing pressure

$$q = \frac{P_v}{A}$$

$$q = \frac{(4.713 \text{ kip})}{(16 \text{ ft}^2)}$$

$$q = 0.29456 \text{ kip/ft}^2$$

Check bearing capacity ratio:

$Ratio$ - Capacity

$$Ratio = \frac{q}{q_a}$$

$$Ratio = \frac{(0.29456 \text{ kip/ft}^2)}{(2000 \text{ psf})}$$

$$Ratio = 0.14728$$

Status: **PASS**
Ratio: **0.150**

Czerniak

Lateral Soil Pressure (ASD):

L/D - Length to least lateral dimension ratio,

$$L/D = \frac{L}{D}$$

$$L/D = \frac{(6.25 \text{ ft})}{(48 \text{ in})}$$

$$L/D = 1.5625$$

Since L/D $\leq$ 10,

Pile is short.

Considering x-direction:

$H_o = -0.32627 \text{ kip/ft}$ - Lateral force per length of pile,
$M_o = 3.7228 \text{ kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\, M_o\, L_e) + \left(3\, H_o\, L_e^2\right)}{(6\, M_o) + (4\, H_o\, L_e)}$$

$$a = \frac{(4 \times (3.7228 \text{ kipft/ft}) \times (6.25 \text{ ft})) + \left(3 \times (-0.32627 \text{ kip/ft}) \times (6.25 \text{ ft})^2\right)}{(6 \times (3.7228 \text{ kipft/ft})) + (4 \times (-0.32627 \text{ kip/ft}) \times (6.25 \text{ ft}))}$$

$$a = 4.306 \text{ ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\, [(4\, M_o) + (3\, H_o\, L_e)]^2}{L_e^2\, [(3\, M_o) + (2\, H_o\, L_e)]}$$

$$p = \frac{0.75 \times [(4 \times (3.7228 \text{ kipft/ft})) + (3 \times (-0.32627 \text{ kip/ft}) \times (6.25 \text{ ft}))]^2}{(6.25 \text{ ft})^2 \times [(3 \times (3.7228 \text{ kipft/ft})) + (2 \times (-0.32627 \text{ kip/ft}) \times (6.25 \text{ ft}))]}$$

$$p = 0.20845 \text{ kip/ft}^2$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\, [(2\, M_o) + (H_o\, L_e)]}{L_e^2}$$

$$s = \frac{6 \times [(2 \times (3.7228 \text{ kipft/ft})) + ((-0.32627 \text{ kip/ft}) \times (6.25 \text{ ft}))]}{(6.25 \text{ ft})^2}$$

$$s = 0.83041 \text{ kip/ft}^2$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\, \frac{a}{2}$$

$$p_a = (150 \text{ psf/ft}) \times \frac{(4.306 \text{ ft})}{2}$$

$$p_a = 0.32295 \text{ kip/ft}^2$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{\left(0.20845\ \text{kip/ft}^2\right)}{\left(0.32295\ \text{kip/ft}^2\right)}$$

$$Ratio = 0.64546$$

Status: **PASS**
Ratio: **0.650**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\,L_e$$

$$p_s = (150\ \text{psf/ft}) \times (6.25\ \text{ft})$$

$$p_s = 0.9375\ \text{kip/ft}^2$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{\left(0.83041\ \text{kip/ft}^2\right)}{\left(0.9375\ \text{kip/ft}^2\right)}$$

$$Ratio = 0.88577$$

Status: **PASS**
Ratio: **0.890**

Considering z-direction:

$H_o = -0.0023885\ \text{kip/ft}$ - Lateral force per length of pile,
$M_o = 0.0076433\ \text{kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (0.0076433\ \text{kipft/ft}) \times (6.25\ \text{ft})) + \left(3 \times (-0.0023885\ \text{kip/ft}) \times (6.25\ \text{ft})^2\right)}{(6 \times (0.0076433\ \text{kipft/ft})) + (4 \times (-0.0023885\ \text{kip/ft}) \times (6.25\ \text{ft}))}$$

$$a = 4.4613\ \text{ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\,[(4\,M_o) + (3\,H_o\,L_e)]^2}{L_e^2\,[(3\,M_o) + (2\,H_o\,L_e)]}$$

$$p = \frac{0.75 \times [(4 \times (0.0076433\ \text{kipft/ft})) + (3 \times (-0.0023885\ \text{kip/ft}) \times (6.25\ \text{ft}))]^{\ 2}}{(6.25\ \text{ft})^2 \times [(3 \times (0.0076433\ \text{kipft/ft})) + (2 \times (-0.0023885\ \text{kip/ft}) \times (6.25\ \text{ft}))]}$$

$$p = -0.00055985\ \text{kip/ft}^2$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\,[(2\,M_o) + (H_o\,L_e)]}{L_e^2}$$

$$s = \frac{6 \times [(2 \times (0.0076433\ \text{kipft/ft})) + ((-0.0023885\ \text{kip/ft}) \times (6.25\ \text{ft}))]}{(6.25\ \text{ft})^2}$$

$$s = 0.000055032\ \text{kip/ft}^2$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\,\frac{a}{2}$$

$$p_a = (150\ \text{psf/ft}) \times \frac{(4.4613\ \text{ft})}{2}$$

$$p_a = 0.33459\ \text{kip/ft}^2$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{\left(-0.00055985\ \text{kip/ft}^2\right)}{\left(0.33459\ \text{kip/ft}^2\right)}$$

$$Ratio = -0.0016732$$

Status: **PASS**
Ratio: **0.000**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\,L_e$$

$$p_s = (150\ \mathrm{psf/ft}) \times (6.25\ \mathrm{ft})$$

$$p_s = 0.9375\ \mathrm{kip/ft^2}$$

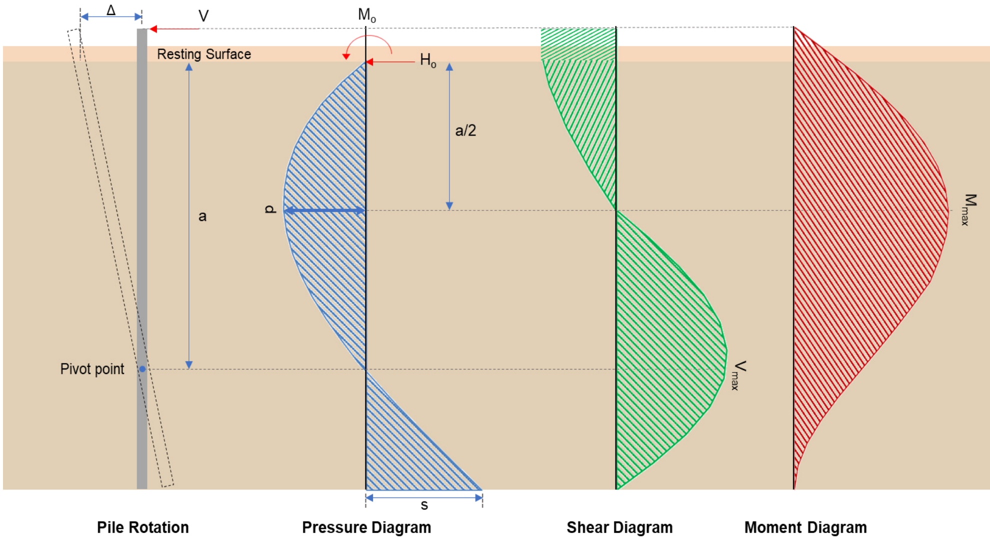
$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{(0.000055032\ \mathrm{kip/ft^2})}{(0.9375\ \mathrm{kip/ft^2})}$$

$$Ratio = 0.000058701$$

Status: **PASS**
Ratio: **0.000**



Shear force and Bending moment (x-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\ D}$$

$$H_o = \frac{(-3.415\ \mathrm{kip})}{1.57 \times (48\ \mathrm{in})}$$

$$H_o = -0.54379\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\ H)}{1.57\ D}$$

$$M_o = \frac{(39.705\ \mathrm{kipft}) + ((-3.415\ \mathrm{kip}) \times (0\ \mathrm{ft}))}{1.57 \times (48\ \mathrm{in})}$$

$$M_o = 6.3225\ \mathrm{kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(6.3225\ \mathrm{kipft/ft})}{(-0.54379\ \mathrm{kip/ft})}$$

$$E = 11.627\ \mathrm{ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\ M_o\ L_e) + \left(3\ H_o\ L_e^2\right)}{(6\ M_o) + (4\ H_o\ L_e)}$$

$$a = \frac{(4 \times (6.3225\ \mathrm{kipft/ft}) \times (6.25\ \mathrm{ft})) + \left(3 \times (-0.54379\ \mathrm{kip/ft}) \times (6.25\ \mathrm{ft})^2\right)}{}$$

$$a = \overline{(6 \times (6.3225\ \text{kipft/ft})) + (4 \times (-0.54379\ \text{kip/ft}) \times (6.25\ \text{ft}))}$$

$$a = 4.3041\ \text{ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\ D) \left[1 - \left[3 \left(\frac{4\ E}{L_e} + 3\right) \left(\frac{a}{L_e}\right)^2\right] + \left[4 \left(\frac{3\ E}{L_e} + 2\right) \left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((-0.54379\ \text{kip/ft}) \times (48\ \text{in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (11.627\ \text{ft})}{(6.25\ \text{ft})} + 3\right) \times \left(\frac{(4.3041\ \text{ft})}{(6.25\ \text{ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (11.627\ \text{ft})}{(6.25\ \text{ft})} + 2\right) \times \left(\frac{(4.3041\ \text{ft})}{(6.25\ \text{ft})}\right)^3\right]\right]$$

$$V_{max} = 8.5953\ \text{kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\ D\ L_e) \left[\left(\frac{E}{L_e} + \frac{a}{2\ L_e}\right) - \left[\left(\frac{4\ E}{L_e} + 3\right) \left(\frac{a}{2\ L_e}\right)^3\right] + \left[\left(\frac{3\ E}{L_e} + 2\right) \left(\frac{a}{2\ L_e}\right)^4\right]\right]$$

$$M_{max} = ((-0.54379\ \text{kip/ft}) \times (48\ \text{in}) \times (6.25\ \text{ft})) \times \left[\left(\frac{(11.627\ \text{ft})}{(6.25\ \text{ft})} + \frac{(4.3041\ \text{ft})}{2 \times (6.25\ \text{ft})}\right) - \left[\left(\frac{4 \times (11.627\ \text{ft})}{(6.25\ \text{ft})} + 3\right) \times \left(\frac{(4.3041\ \text{ft})}{2 \times (6.25\ \text{ft})}\right)^3\right] + \left[\left(\frac{3 \times (11.627\ \text{ft})}{(6.25\ \text{ft})} + 2\right) \times \left(\frac{(4.3041\ \text{ft})}{2 \times (6.25\ \text{ft})}\right)^4\right]\right]$$

$$M_{max} = 25.625\ \text{kipft}$$

Shear force and Bending moment (z-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\ b}$$

$$H_o = \frac{(-0.023\ \text{kip})}{1.57 \times (48\ \text{in})}$$

$$H_o = -0.0036624\ \text{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\ H)}{1.57\ b}$$

$$M_o = \frac{(0.076\ \text{kipft}) + ((-0.023\ \text{kip}) \times (0\ \text{ft}))}{1.57 \times (48\ \text{in})}$$

$$M_o = 0.012102\ \text{kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(0.012102\ \text{kipft/ft})}{(-0.0036624\ \text{kip/ft})}$$

$$E = 3.3043\ \text{ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\ M_o\ L_e) + \left(3\ H_o\ L_e^2\right)}{(6\ M_o) + (4\ H_o\ L_e)}$$

$$a = \frac{(4 \times (0.012102\ \text{kipft/ft}) \times (6.25\ \text{ft})) + \left(3 \times (-0.0036624\ \text{kip/ft}) \times (6.25\ \text{ft})^2\right)}{(6 \times (0.012102\ \text{kipft/ft})) + (4 \times (-0.0036624\ \text{kip/ft}) \times (6.25\ \text{ft}))}$$

$$a = 4.4571\ \text{ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\ b) \left[1 - \left[3 \left(\frac{4\ E}{L_e} + 3\right) \left(\frac{a}{L_e}\right)^2\right] + \left[4 \left(\frac{3\ E}{L_e} + 2\right) \left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((-0.0036624\ \text{kip/ft}) \times (48\ \text{in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (3.3043\ \text{ft})}{(6.25\ \text{ft})} + 3\right) \times \left(\frac{(4.4571\ \text{ft})}{(6.25\ \text{ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (3.3043\ \text{ft})}{(6.25\ \text{ft})} + 2\right) \times \left(\frac{(4.4571\ \text{ft})}{(6.25\ \text{ft})}\right)^3\right]\right]$$

$$V_{max} = 0.023458\ \text{kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, b\, L_e)\left[\left(\frac{E}{L_e} + \frac{a}{2\,L_e}\right) - \left[\left(\frac{4\,E}{L_e} + 3\right)\left(\frac{a}{2\,L_e}\right)^3\right] + \left[\left(\frac{3\,E}{L_e} + 2\right)\left(\frac{a}{2\,L_e}\right)^4\right]\right]$$

$$M_{max} = ((-0.0036624\ \text{kip/ft}) \times (48\ \text{in}) \times (6.25\ \text{ft})) \times \left[\left(\frac{(3.3043\ \text{ft})}{(6.25\ \text{ft})} + \frac{(4.4571\ \text{ft})}{2 \times (6.25\ \text{ft})}\right) - \left[\left(\frac{4 \times (3.3043\ \text{ft})}{(6.25\ \text{ft})} + 3\right) \times \left(\frac{(4.4571\ \text{ft})}{2 \times (6.25\ \text{ft})}\right)^3\right] + \left[\left(\frac{3 \times (3.3043\ \text{ft})}{(6.25\ \text{ft})} + 2\right) \times \left(\frac{(4.4571\ \text{ft})}{2 \times (6.25\ \text{ft})}\right)^4\right]\right]$$

$$M_{max} = 0.065132\ \text{kipft}$$

Minimum Reinforcement Check (LRFD)

Parameters:

$f'_{ck} = 2.5\ \text{ksi}$ - Concrete strength,
$f_{yk} = 60\ \text{ksi}$ - Longitudinal reinforcement strength,
$\phi = 0.65$ - Reduction factor for axial strength,
Table 22.4.2.1 $\alpha = 0.8$ - Alpha factor for axial strength,
$A_g = 2304\ \text{in}^2$ - Gross area of concrete,

Longitudinal reinforcement:

Required reinforcement due to axial load, $A_{st,required}$

22.4.2.2, 10.6.1.1 $A_{st,required}$

$$A_{st,required} = Min\left[\frac{\frac{P}{\phi\,\alpha} - (0.85\, f'_{ck}\, A_g)}{f_{yk} - (0.85\, f'_{ck})}, (0.08\, A_g)\right]$$

$$A_{st,required} = Min\left[\frac{\frac{(6.882\ \text{kip})}{(0.65)\times(0.8)} - (0.85 \times (2.5\ \text{ksi}) \times (2304\ \text{in}^2))}{(60\ \text{ksi}) - (0.85 \times (2.5\ \text{ksi}))}, (0.08 \times (2304\ \text{in}^2))\right]$$

$$A_{st,required} = -84.367\ \text{in}^2$$

A_{min} - Governing minimum reinforcement area,

$$A_{min} = Max\left[A_{st,required}, (0.0018\, A_g)\right]$$

$$A_{min} = Max\left[(-84.367\ \text{in}^2), (0.0018 \times (2304\ \text{in}^2))\right]$$

$$A_{min} = 4.1472\ \text{in}^2$$

n_{rebar} - Required number of reinforcement,

$$n_{rebar} = \frac{A_{min}}{A_{rebar}}$$

$$n_{rebar} = \frac{(4.1472\ \text{in}^2)}{(0.3068\ \text{in}^2)}$$

$$n_{rebar} = 14$$

A_{st} - Actual total reinforcement area,

$$A_{st} = n_{rebar}\, \frac{\pi\, d_{bar}^2}{4}$$

$$A_{st} = (14) \times \frac{\pi \times (0.625\ \text{in})^2}{4}$$

$$A_{st} = 4.2951\ \text{in}^2$$

$Ratio$ - Capacity

$$Ratio = \frac{A_{min}}{A_{st}}$$

$$Ratio = \frac{(4.1472\ \text{in}^2)}{(4.2951\ \text{in}^2)}$$

$$Ratio = 0.96556$$

Status: PASS
Ratio: 0.970

25.2.3 s_{rebar} - Minimum spacing of reinforcement,

$$s_{rebar} = Max\left[1.5, \left(1.5\ d_{bar}\right)\right]$$

$$s_{rebar} = Max\left[1.5, \left(1.5 \times (0.625\ \text{in})\right)\right]$$

$$s_{rebar} = 1.5\ \text{in}$$

Ties:

25.7.2.2 Since longitudinal reinforcement is $\leq$ No. 10ø: Use #3(0.375 in)

25.7.2.1 s_{ties} - Maximum spacing of ties,

$$s_{ties} = Min\left[\left(16\ d_{bar}\right), \left(48\ d_{ties}\right), Min\left(D, b\right)\right]$$

$$s_{ties} = Min\left[\left(16 \times (0.625\ \text{in})\right), \left(48 \times (0.375\ \text{in})\right), Min\left((48\ \text{in}), (48\ \text{in})\right)\right]$$

$$s_{ties} = 10\ \text{in}$$

Summary:

Main reinforcement: **14 - #5 (0.625 in)**
Ties: **#3(0.375 in) - 10 in**

Axial Compression Strength (ACI 318-19, LRFD)

22.4.2.2 ϕP_N - Allowable axial compressive strength

$$\phi P_N = \phi\ 0.80\ \left[\left(0.85\ f'_{ck}\ \left[A_g - A_{st}\right]\right) + \left(f_{yk}\ A_{st}\right)\right]$$

$$\phi P_N = (0.65) \times 0.80 \times \left[\left(0.85 \times (2.5\ \text{ksi}) \times \left[\left(2304\ \text{in}^2\right) - \left(4.2951\ \text{in}^2\right)\right]\right) + \left((60\ \text{ksi}) \times \left(4.2951\ \text{in}^2\right)\right)\right]$$

$$\phi P_N = 2675.2\ \text{kip}$$

$Ratio$ - Capacity

$$Ratio = \frac{P}{\phi P_N}$$

$$Ratio = \frac{(6.882\ \text{kip})}{(2675.2\ \text{kip})}$$

$$Ratio = 0.0025725$$

Status: PASS
Ratio: 0.000

Shear Strength (ACI 318-19, LRFD)

Parameters:

$b_w = 48\ \text{in}$ - Effective width,

22.5.2.2 d - Effective depth

$$d = 0.80\ D$$

$$d = 0.80 \times (48\ \text{in})$$

$$d = 38.4\ \text{in}$$

22.5.5.1.3 λ_s - size effect modification factor

$$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{d}{10}}}, 1\right]$$

$$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{(38.4\ \text{in})}{10}}}, 1\right]$$

$$\lambda_s = 0.64282$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5\ \text{ksi} \rightarrow 2500\ \text{psi}$,

22.5.5.1.1 $V_{c,max}$ - Max shear strength of concrete

$$V_{c,max} = 5\ \lambda_s\ \sqrt{f'_{ck}}\ b_w\ d$$

$$V_{c,max} = 5 \times (0.64282) \times \sqrt{(2500\ \text{psi})} \times (48\ \text{in}) \times (38.4\ \text{in})$$

$$V_{c,max} = 296.21 \text{ kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$, $P = 6.882 \text{ kip} \rightarrow 6882 \text{ lbf}$,

22.5.5.1.1(a) $V_{c,a}$ - Shear strength of concrete (a)

$$V_{c,a} = \left[2\,\lambda_s\,\sqrt{f'_{ck}} + \frac{P}{6\,A_g}\right]\,b_w\,d$$

$$V_{c,a} = \left[2 \times (0.64282) \times \sqrt{(2500 \text{ psi})} + \frac{(6882 \text{ lbf})}{6 \times (2304 \text{ in}^2)}\right] \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{c,a} = 119.4 \text{ kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$,

22.5.5.1.2 $V_{c,b}$ - Shear strength of concrete (b)

$$V_{c,b} = \left[2\,\lambda_s\,\sqrt{f'_{ck}} + \left(0.05\,f'_{ck}\right)\right]\,b_w\,d$$

$$V_{c,b} = \left[2 \times (0.64282) \times \sqrt{(2500 \text{ psi})} + (0.05 \times (2500 \text{ psi}))\right] \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{c,b} = 348.89 \text{ kip}$$

V_c - Governing shear strength of concrete

$$V_c = Min\left[V_{c,max}, V_{c,a}, V_{c,b}\right]$$

$$V_c = Min\left[(296.21 \text{ kip}), (119.4 \text{ kip}), (348.89 \text{ kip})\right]$$

$$V_c = 119.4 \text{ kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$,

22.5.1.2 $V_{s,a}$ - Shear strength of steel (a)

$$V_{s,a} = 8\,\sqrt{f'_{ck}}\,b_w\,d$$

$$V_{s,a} = 8 \times \sqrt{(2500 \text{ psi})} \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{s,a} = 737.28 \text{ kip}$$

A_v - Ties rebar area,

$$A_v = \frac{\pi\,d_{ties}^2}{4}$$

$$A_v = \frac{\pi \times (0.375 \text{ in})^2}{4}$$

$$A_v = 0.11045 \text{ in}^2$$

22.5.8.5.3 $V_{s,b}$ - Shear strength of steel (b)

$$V_{s,b} = \frac{2\,A_v\,f_{ywk}\,d}{s_{ties}}$$

$$V_{s,b} = \frac{2 \times \left(0.11045 \text{ in}^2\right) \times (60 \text{ ksi}) \times (38.4 \text{ in})}{(10 \text{ in})}$$

$$V_{s,b} = 50.894 \text{ kip}$$

V_s - Governing shear strength of steel

$$V_s = MIN\left[V_{s,a}, V_{s,b}\right]$$

$$V_s = MIN\left[(737.28 \text{ kip}), (50.894 \text{ kip})\right]$$

$$V_s = 50.894 \text{ kip}$$

22.5.1.1 ϕV_n - Allowable shear strength

$$\phi V_n = \phi\,\left(V_c + V_s\right)$$

$$\phi V_n = (0.65) \times ((119.4 \text{ kip}) + (50.894 \text{ kip}))$$

$$\phi V_n = 110.69 \text{ kip}$$

Considering x-direction:

$V_{max} = 8.5953 \text{ kip}$ - Maximum shear force in the x-direction,
$Ratio$ - Capacity

$$Ratio = \frac{V_{max}}{\phi V_n}$$

	$$Ratio = \frac{(8.5953\ \mathrm{kip})}{(110.69\ \mathrm{kip})}$$ $$Ratio = 0.07765$$	Status: **PASS** Ratio: **0.080**
	Considering z-direction: $V_{max} = 0.023458\ \mathrm{kip}$ - Maximum shear force in the z-direction, $Ratio$ - Capacity $$Ratio = \frac{V_{max}}{\phi V_n}$$ $$Ratio = \frac{(0.023458\ \mathrm{kip})}{(110.69\ \mathrm{kip})}$$ $$Ratio = 0.00021192$$	Status: **PASS** Ratio: **0.000**
14.5.2.1b	**Flexural Strength (ACI 318-19, LRFD)** S_m - Section modulus $$S_m = \frac{b\,D^2}{6}$$ $$S_m = \frac{(48\ \mathrm{in}) \times (48\ \mathrm{in})^2}{6}$$ $$S_m = 18432\ \mathrm{in}^3$$ $\lambda = 1$ - Concrete modification factor (Normal concrete), Allowable flexural strength: M_n shall be the lesser of: $\phi M_{n,1}$ $$\phi M_{n,1} = \phi \times 5 \times \lambda \times \sqrt{(f_c')} \times S_m$$ $$\phi M_{n,1} = 0.65 \times 5 \times 1 \times \sqrt{(2.5 ksi)} \times 18432.001 in^3$$ $$\phi M_{n,1} = 249.600 kipft$$ $\phi M_{n,2}$ $$\phi M_{n,2} = \phi\, 0.85\, f_{ck}'\, S_m$$ $$\phi M_{n,2} = (0.65) \times 0.85 \times (2.5\ \mathrm{ksi}) \times \left(18432\ \mathrm{in}^3\right)$$ $$\phi M_{n,2} = 2121.6\ \mathrm{kipft}$$ Therefore, ϕM_n - Allowable flexural strength, $$\phi M_n = MIN[\phi M_{n,1}, \phi M_{n,2}]$$ $$\phi M_n = MIN[(249.6\ \mathrm{kipft}), (2121.6\ \mathrm{kipft})]$$ $$\phi M_n = 249.6\ \mathrm{kipft}$$ **Considering x-direction:** $M_{max} = 25.625\ \mathrm{kipft}$ - Maximum moment in the x-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\phi M_n}$$ $$Ratio = \frac{(25.625\ \mathrm{kipft})}{(249.6\ \mathrm{kipft})}$$ $$Ratio = 0.10266$$	Status: **PASS** Ratio: **0.100**
	Considering z-direction: $M_{max} = 0.065132\ \mathrm{kipft}$ - Maximum moment in the z-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\phi M_n}$$	

	$$Ratio = \frac{(0.065132 \text{ kipft})}{(249.6 \text{ kipft})}$$ $$Ratio = 0.00026095$$	Status: **PASS** Ratio: **0.000**

REFERENCES	CALCULATIONS	RESULTS

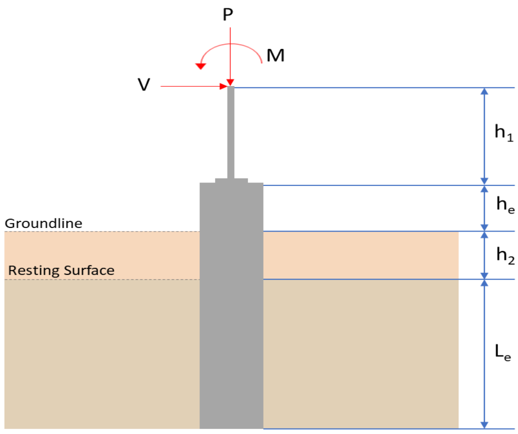
SkyCiv Foundation Design

Pile Foundation

Design Information :

Design code : IBC 2021 (International Building Code)
Unit System : Imperial

Pile Input



Geometry

Pile shape: rectangular
$b = 48\ \text{in}$ - Pile width
$D = 48\ \text{in}$ - Pile depth
$L = 6.25\ \text{ft}$ - Total pile length
$h_1 = 0\ \text{ft}$ - Lateral load height from the top of the pile,
$h_2 = 0\ \text{ft}$ - Depth to resisting surface
$h_e = 0\ \text{ft}$ - Length of pile above the ground

Tabulation of Soil Parameters

Layer	Label	Allowable Bearing Pressure (q_a) (psf)	Allowable Lateral Pressure (R) (psf/ft)
1	Sand, silty sand, clayey sand, silty gravel & clayey gravel	2000.000	150.000

Tabulation of Loads

Load Component	ASD	LRFD
P (kip)	4.650	6.791
V_x (kip)	-2.037	-3.395
V_z (kip)	0.000	0.000
M_x (kipft)	0.000	0.000
M_z (kipft)	23.515	39.954

Material Properties

$f'_{ck} = 2.5\ \text{ksi}$ - Concrete strength,

Required depth to resist lateral loads (ASD)

H - Point of application of the lateral load

$$H = h_1 + h_2 + h_e$$

$$H = (0\ \text{ft}) + (0\ \text{ft}) + (0\ \text{ft})$$

$$H = 0\ \text{ft}$$

Considering x-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\ D}$$

$$H_o = \frac{(-2.037\ \text{kip})}{1.57 \times (48\ \text{in})}$$

$$H_o = -0.32436\ \text{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\,H)}{1.57\,D}$$

$$M_o = \frac{(23.515\text{ kipft}) + ((-2.037\text{ kip}) \times (0\text{ ft}))}{1.57 \times (48\text{ in})}$$

$$M_o = 3.7444\text{ kipft/ft}$$

Required depth of embedment in earth:

$$L_x^3 - \left(14.14 \times \frac{H_o \times L_x}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,x} = 5.7292\text{ ft}$ - Required depth in x-direction,

Considering z-direction:

$L_{e,z} = 0\text{ ft}$ - Required depth in z-direction,

Minimum embedded depth required:

$L_{e,req}$ - Depth of pile required,

$$L_{e,req} = MAX[L_{e,x}, L_{e,z}]$$

$$L_{e,req} = MAX[(5.7292\text{ ft}), (0\text{ ft})]$$

$$L_{e,req} = 5.729\text{ ft}$$

L_e - Actual embedded length of pile,

$$L_e = L - h_e - h_2$$

$$L_e = (6.25\text{ ft}) - (0\text{ ft}) - (0\text{ ft})$$

$$L_e = 6.25\text{ ft}$$

$Ratio$ - Embedded depth

$$Ratio = \frac{L_{e,req}}{L_e}$$

$$Ratio = \frac{(5.729\text{ ft})}{(6.25\text{ ft})}$$

$$Ratio = 0.91664$$

Status: **PASS**
Ratio: **0.920**

End-bearing Capacity (ASD)

A - Pile cross-section area

$$A = b\,D$$

$$A = (48\text{ in}) \times (48\text{ in})$$

$$A = 16\text{ ft}^2$$

q - End-bearing pressure

$$q = \frac{P_v}{A}$$

$$q = \frac{(4.65\text{ kip})}{(16\text{ ft}^2)}$$

$$q = 0.29063\text{ kip/ft}^2$$

Check bearing capacity ratio:

$Ratio$ - Capacity

$$Ratio = \frac{q}{q_a}$$

$$Ratio = \frac{(0.29063\text{ kip/ft}^2)}{(2000\text{ psf})}$$

$$Ratio = 0.14531$$

Status: **PASS**
Ratio: **0.150**

Czerniak

Lateral Soil Pressure (ASD):

L/D - Length to least lateral dimension ratio,

$$L/D = \frac{L}{D}$$

$$L/D = \frac{(6.25 \text{ ft})}{(48 \text{ in})}$$

$$L/D = 1.5625$$

Since L/D $\leq$ 10,

Pile is short.

Considering x-direction:

$H_o = -0.32436 \text{ kip/ft}$ - Lateral force per length of pile,
$M_o = 3.7444 \text{ kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\, M_o\, L_e) + \left(3\, H_o\, L_e^2\right)}{(6\, M_o) + (4\, H_o\, L_e)}$$

$$a = \frac{(4 \times (3.7444 \text{ kipft/ft}) \times (6.25 \text{ ft})) + \left(3 \times (-0.32436 \text{ kip/ft}) \times (6.25 \text{ ft})^2\right)}{(6 \times (3.7444 \text{ kipft/ft})) + (4 \times (-0.32436 \text{ kip/ft}) \times (6.25 \text{ ft}))}$$

$$a = 4.3048 \text{ ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\, [(4\, M_o) + (3\, H_o\, L_e)]^2}{L_e^2\, [(3\, M_o) + (2\, H_o\, L_e)]}$$

$$p = \frac{0.75 \times [(4 \times (3.7444 \text{ kipft/ft})) + (3 \times (-0.32436 \text{ kip/ft}) \times (6.25 \text{ ft}))]^{\,2}}{(6.25 \text{ ft})^2 \times [(3 \times (3.7444 \text{ kipft/ft})) + (2 \times (-0.32436 \text{ kip/ft}) \times (6.25 \text{ ft}))]}$$

$$p = 0.21166 \text{ kip/ft}^2$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\, [(2\, M_o) + (H_o\, L_e)]}{L_e^2}$$

$$s = \frac{6 \times [(2 \times (3.7444 \text{ kipft/ft})) + ((-0.32436 \text{ kip/ft}) \times (6.25 \text{ ft}))]}{(6.25 \text{ ft})^2}$$

$$s = 0.8389 \text{ kip/ft}^2$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\, \frac{a}{2}$$

$$p_a = (150 \text{ psf/ft}) \times \frac{(4.3048 \text{ ft})}{2}$$

$$p_a = 0.32286 \text{ kip/ft}^2$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{\left(0.21166 \text{ kip/ft}^2\right)}{\left(0.32286 \text{ kip/ft}^2\right)}$$

$$Ratio = 0.65557$$

Status: **PASS**
Ratio: **0.660**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\, L_e$$

$$p_s = (150 \text{ psf/ft}) \times (6.25 \text{ ft})$$

$$p_s = 0.9375 \text{ kip/ft}^2$$

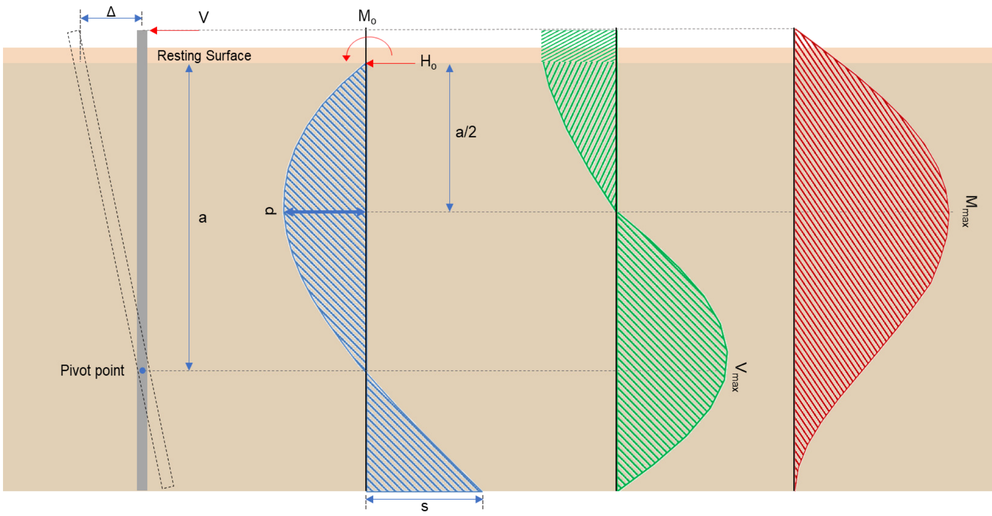
$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{\left(0.8389 \text{ kip/ft}^2\right)}{\left(0.9375 \text{ kip/ft}^2\right)}$$

$$Ratio = 0.89483$$

Status: **PASS**
Ratio: **0.890**



Pile Rotation **Pressure Diagram** **Shear Diagram** **Moment Diagram**

Shear force and Bending moment (x-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\,D}$$

$$H_o = \frac{(-3.395\text{ kip})}{1.57 \times (48\text{ in})}$$

$$H_o = -0.54061\text{ kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\,H)}{1.57\,D}$$

$$M_o = \frac{(39.954\text{ kipft}) + ((-3.395\text{ kip}) \times (0\text{ ft}))}{1.57 \times (48\text{ in})}$$

$$M_o = 6.3621\text{ kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(6.3621\text{ kipft/ft})}{(-0.54061\text{ kip/ft})}$$

$$E = 11.768\text{ ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (6.3621\text{ kipft/ft}) \times (6.25\text{ ft})) + \left(3 \times (-0.54061\text{ kip/ft}) \times (6.25\text{ ft})^2\right)}{(6 \times (6.3621\text{ kipft/ft})) + (4 \times (-0.54061\text{ kip/ft}) \times (6.25\text{ ft}))}$$

$$a = 4.3029\text{ ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\,D)\left[1 - \left[3\left(\frac{4\,E}{L_e} + 3\right)\left(\frac{a}{L_e}\right)^2\right] + \left[4\left(\frac{3\,E}{L_e} + 2\right)\left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((-0.54061\text{ kip/ft}) \times (48\text{ in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (11.768\text{ ft})}{(6.25\text{ ft})} + 3\right) \times \left(\frac{(4.3029\text{ ft})}{(6.25\text{ ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (11.768\text{ ft})}{(6.25\text{ ft})} + 2\right) \times \left(\frac{(4.3029\text{ ft})}{(6.25\text{ ft})}\right)^3\right]\right]$$

$$V_{max} = 8.6310\text{ kip}$$

$$V_{max} = 8.6319 \text{ kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, D\, L_e)\left[\left(\frac{E}{L_e} + \frac{a}{2\,L_e}\right) - \left[\left(\frac{4\,E}{L_e} + 3\right)\left(\frac{a}{2\,L_e}\right)^3\right] + \left[\left(\frac{3\,E}{L_e} + 2\right)\left(\frac{a}{2\,L_e}\right)^4\right]\right]$$

$$M_{max} = ((-0.54061 \text{ kip/ft}) \times (48 \text{ in}) \times (6.25 \text{ ft})) \times \left[\left(\frac{(11.768 \text{ ft})}{(6.25 \text{ ft})} + \frac{(4.3029 \text{ ft})}{2 \times (6.25 \text{ ft})}\right) - \left[\left(\frac{4 \times (11.768 \text{ ft})}{(6.25 \text{ ft})} + 3\right) \times \left(\frac{(4.3029 \text{ ft})}{2 \times (6.25 \text{ ft})}\right)^3\right] + \left[\left(\frac{3 \times (11.768 \text{ ft})}{(6.25 \text{ ft})} + 2\right) \times \left(\frac{(4.3029 \text{ ft})}{2 \times (6.25 \text{ ft})}\right)^4\right]\right]$$

$$M_{max} = 25.746 \text{ kipft}$$

Minimum Reinforcement Check (LRFD)

Parameters:

$f'_{ck} = 2.5 \text{ ksi}$ - Concrete strength,
$f_{yk} = 60 \text{ ksi}$ - Longitudinal reinforcement strength,
$\phi = 0.65$ - Reduction factor for axial strength,
Table 22.4.2.1 $\alpha = 0.8$ - Alpha factor for axial strength,
$A_g = 2304 \text{ in}^2$ - Gross area of concrete,

Longitudinal reinforcement:

Required reinforcement due to axial load, $A_{st,required}$
22.4.2.2, 10.6.1.1 $A_{st,required}$

$$A_{st,required} = Min\left[\frac{\frac{P}{\phi\,\alpha} - \left(0.85\, f'_{ck}\, A_g\right)}{f_{yk} - \left(0.85\, f'_{ck}\right)}, (0.08\, A_g)\right]$$

$$A_{st,required} = Min\left[\frac{\frac{(6.791 \text{ kip})}{(0.65)\times(0.8)} - \left(0.85 \times (2.5 \text{ ksi}) \times \left(2304 \text{ in}^2\right)\right)}{(60 \text{ ksi}) - (0.85 \times (2.5 \text{ ksi}))}, \left(0.08 \times \left(2304 \text{ in}^2\right)\right)\right]$$

$$A_{st,required} = -84.37 \text{ in}^2$$

A_{min} - Governing minimum reinforcement area,

$$A_{min} = Max\left[A_{st,required}, (0.0018\, A_g)\right]$$

$$A_{min} = Max\left[\left(-84.37 \text{ in}^2\right), \left(0.0018 \times \left(2304 \text{ in}^2\right)\right)\right]$$

$$A_{min} = 4.1472 \text{ in}^2$$

n_{rebar} - Required number of reinforcement,

$$n_{rebar} = \frac{A_{min}}{A_{rebar}}$$

$$n_{rebar} = \frac{\left(4.1472 \text{ in}^2\right)}{\left(0.3068 \text{ in}^2\right)}$$

$$n_{rebar} = 14$$

A_{st} - Actual total reinforcement area,

$$A_{st} = n_{rebar}\,\frac{\pi\, d_{bar}^2}{4}$$

$$A_{st} = (14) \times \frac{\pi \times (0.625 \text{ in})^2}{4}$$

$$A_{st} = 4.2951 \text{ in}^2$$

$Ratio$ - Capacity

$$Ratio = \frac{A_{min}}{A_{st}}$$

$$Ratio = \frac{\left(4.1472 \text{ in}^2\right)}{\left(4.2951 \text{ in}^2\right)}$$

$$Ratio = 0.96556$$

Status: **PASS**
Ratio: **0.970**

25.2.3 s_{rebar} - Minimum spacing of reinforcement,

$$s_{rebar} = Max\left[1.5, (1.5\, d_{bar})\right]$$

$$s_{rebar} = Max\left[1.5, \left(1.5 \times (0.625\ \text{in})\right)\right]$$

$$s_{rebar} = 1.5\ \text{in}$$

Ties:

25.7.2.2 Since longitudinal reinforcement is $\leq$ No. 10ø: Use #3(0.375 in)

25.7.2.1 s_{ties} - Maximum spacing of ties,

$$s_{ties} = Min\left[(16\, d_{bar}), (48\, d_{ties}), Min\,(D, b)\right]$$

$$s_{ties} = Min\left[(16 \times (0.625\ \text{in})), (48 \times (0.375\ \text{in})), Min\,((48\ \text{in}), (48\ \text{in}))\right]$$

$$s_{ties} = 10\ \text{in}$$

Summary:

Main reinforcement: **14 - #5 (0.625 in)**

Ties: **#3(0.375 in) - 10 in**

Axial Compression Strength (ACI 318-19, LRFD)

22.4.2.2 ϕP_N - Allowable axial compressive strength

$$\phi P_N = \phi\, 0.80\ \left[\left(0.85\, f'_{ck}\ [A_g - A_{st}]\right) + (f_{yk}\, A_{st})\right]$$

$$\phi P_N = (0.65) \times 0.80 \times \left[\left(0.85 \times (2.5\ \text{ksi}) \times \left[\left(2304\ \text{in}^2\right) - \left(4.2951\ \text{in}^2\right)\right]\right) + \left((60\ \text{ksi}) \times \left(4.2951\ \text{in}^2\right)\right)\right]$$

$$\phi P_N = 2675.2\ \text{kip}$$

$Ratio$ - Capacity

$$Ratio = \frac{P}{\phi P_N}$$

$$Ratio = \frac{(6.791\ \text{kip})}{(2675.2\ \text{kip})}$$

$$Ratio = 0.0025385$$

Status: **PASS**
Ratio: **0.000**

Shear Strength (ACI 318-19, LRFD)

Parameters:

$b_w = 48\ \text{in}$ - Effective width,

22.5.2.2 d - Effective depth

$$d = 0.80\, D$$

$$d = 0.80 \times (48\ \text{in})$$

$$d = 38.4\ \text{in}$$

22.5.5.1.3 λ_s - size effect modification factor

$$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{d}{10}}}, 1\right]$$

$$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{(38.4\ \text{in})}{10}}}, 1\right]$$

$$\lambda_s = 0.64282$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5\ \text{ksi} \rightarrow 2500\ \text{psi}$,

22.5.5.1.1 $V_{c,max}$ - Max shear strength of concrete

$$V_{c,max} = 5\,\lambda_s\,\sqrt{f'_{ck}}\,b_w\,d$$

$$V_{c,max} = 5 \times (0.64282) \times \sqrt{(2500\ \text{psi})} \times (48\ \text{in}) \times (38.4\ \text{in})$$

$$V_{c,max} = 296.21\ \text{kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5\ \text{ksi} \rightarrow 2500\ \text{psi}$, $P = 6.791\ \text{kip} \rightarrow 6791\ \text{lbf}$,

22.5.5.1.1(a) $V_{c,a}$ - Shear strength of concrete (a)

$$V_{c,a} = \left[2\,\lambda_s\,\sqrt{f'_{ck}} + \frac{P}{6\,A_g}\right]\,b_w\,d$$

$$V_{c,a} = \left[2 \times (0.64282) \times \sqrt{(2500 \text{ psi})} + \frac{(6791 \text{ lbf})}{6 \times (2304 \text{ in}^2)}\right] \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{c,a} = 119.39 \text{ kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$,

22.5.5.1.2 $V_{c,b}$ - Shear strength of concrete (b)

$$V_{c,b} = \left[2\, \lambda_s \sqrt{f'_{ck}} + \left(0.05\, f'_{ck}\right)\right]\, b_w\, d$$

$$V_{c,b} = \left[2 \times (0.64282) \times \sqrt{(2500 \text{ psi})} + (0.05 \times (2500 \text{ psi}))\right] \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{c,b} = 348.89 \text{ kip}$$

V_c - Governing shear strength of concrete

$$V_c = Min\left[V_{c,max}, V_{c,a}, V_{c,b}\right]$$

$$V_c = Min\left[(296.21 \text{ kip}), (119.39 \text{ kip}), (348.89 \text{ kip})\right]$$

$$V_c = 119.39 \text{ kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$,

22.5.1.2 $V_{s,a}$ - Shear strength of steel (a)

$$V_{s,a} = 8 \sqrt{f'_{ck}}\, b_w\, d$$

$$V_{s,a} = 8 \times \sqrt{(2500 \text{ psi})} \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{s,a} = 737.28 \text{ kip}$$

A_v - Ties rebar area,

$$A_v = \frac{\pi\, d_{ties}^2}{4}$$

$$A_v = \frac{\pi \times (0.375 \text{ in})^2}{4}$$

$$A_v = 0.11045 \text{ in}^2$$

22.5.8.5.3 $V_{s,b}$ - Shear strength of steel (b)

$$V_{s,b} = \frac{2\, A_v\, f_{ywk}\, d}{s_{ties}}$$

$$V_{s,b} = \frac{2 \times \left(0.11045 \text{ in}^2\right) \times (60 \text{ ksi}) \times (38.4 \text{ in})}{(10 \text{ in})}$$

$$V_{s,b} = 50.894 \text{ kip}$$

V_s - Governing shear strength of steel

$$V_s = MIN\left[V_{s,a}, V_{s,b}\right]$$

$$V_s = MIN\left[(737.28 \text{ kip}), (50.894 \text{ kip})\right]$$

$$V_s = 50.894 \text{ kip}$$

22.5.1.1 ϕV_n - Allowable shear strength

$$\phi V_n = \phi\ (V_c + V_s)$$

$$\phi V_n = (0.65) \times ((119.39 \text{ kip}) + (50.894 \text{ kip}))$$

$$\phi V_n = 110.69 \text{ kip}$$

Considering x-direction:

$V_{max} = 8.6319 \text{ kip}$ - Maximum shear force in the x-direction,
$Ratio$ - Capacity

$$Ratio = \frac{V_{max}}{\phi V_n}$$

$$Ratio = \frac{(8.6319 \text{ kip})}{(110.69 \text{ kip})}$$

$$Ratio = 0.077986$$

Status: **PASS**
Ratio: **0.080**

	Flexural Strength (ACI 318-19, LRFD)	

Flexural Strength (ACI 318-19, LRFD)

S_m - Section modulus

$$S_m = \frac{b\, D^2}{6}$$

$$S_m = \frac{(48\ \mathrm{in}) \times (48\ \mathrm{in})^2}{6}$$

$$S_m = 18432\ \mathrm{in}^3$$

$\lambda = 1$ - Concrete modification factor (Normal concrete),
Allowable flexural strength:
M_n shall be the lesser of:
$\phi M_{n,1}$

$$\phi M_{n,1} = \phi \times 5 \times \lambda \times \sqrt{(f_c')} \times S_m$$

$$\phi M_{n,1} = 0.65 \times 5 \times 1 \times \sqrt{(2.5ksi)} \times 18432.001in^3$$

$$\phi M_{n,1} = 249.600kipft$$

14.5.2.1b $\phi M_{n,2}$

$$\phi M_{n,2} = \phi\, 0.85\, f_{ck}'\, S_m$$

$$\phi M_{n,2} = (0.65) \times 0.85 \times (2.5\ \mathrm{ksi}) \times \left(18432\ \mathrm{in}^3\right)$$

$$\phi M_{n,2} = 2121.6\ \mathrm{kipft}$$

Therefore,
ϕM_n - Allowable flexural strength,

$$\phi M_n = MIN[\phi M_{n,1}, \phi M_{n,2}]$$

$$\phi M_n = MIN[(249.6\ \mathrm{kipft}), (2121.6\ \mathrm{kipft})]$$

$$\phi M_n = 249.6\ \mathrm{kipft}$$

Considering x-direction:

$M_{max} = 25.746\ \mathrm{kipft}$ - Maximum moment in the x-direction,
$Ratio$ - Capacity

$$Ratio = \frac{M_{max}}{\phi M_n}$$

$$Ratio = \frac{(25.746\ \mathrm{kipft})}{(249.6\ \mathrm{kipft})}$$

$$Ratio = 0.10315$$

Status: **PASS**
Ratio: **0.100**

REFERENCES	CALCULATIONS	RESULTS

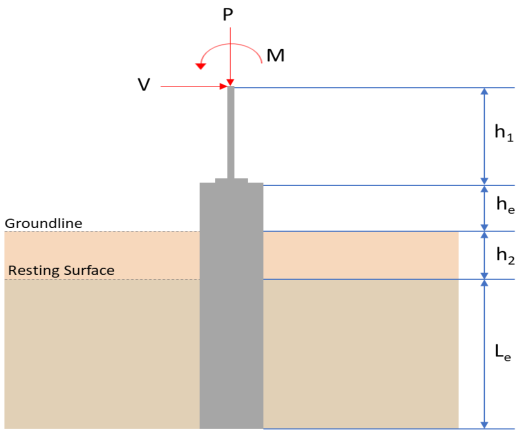
SkyCiv Foundation Design

Pile Foundation

Design Information :

Design code : IBC 2021 (International Building Code)
Unit System : Imperial

Pile Input



Geometry

Pile shape: rectangular
$b = 48\ \mathrm{in}$ - Pile width
$D = 48\ \mathrm{in}$ - Pile depth
$L = 6.25\ \mathrm{ft}$ - Total pile length
$h_1 = 0\ \mathrm{ft}$ - Lateral load height from the top of the pile,
$h_2 = 0\ \mathrm{ft}$ - Depth to resisting surface
$h_e = 0\ \mathrm{ft}$ - Length of pile above the ground

Tabulation of Soil Parameters

Layer	Label	Allowable Bearing Pressure (q_a) (psf)	Allowable Lateral Pressure (R) (psf/ft)
1	Sand, silty sand, clayey sand, silty gravel & clayey gravel	2000.000	150.000

Tabulation of Loads

Load Component	ASD	LRFD
P (kip)	4.713	6.882
V_x (kip)	-2.049	-3.415
V_z (kip)	0.015	0.023
M_x (kipft)	0.048	0.076
M_z (kipft)	23.379	39.705

Material Properties

$f'_{ck} = 2.5\ \mathrm{ksi}$ - Concrete strength,

Required depth to resist lateral loads (ASD)

H - Point of application of the lateral load

$$H = h_1 + h_2 + h_e$$

$$H = (0\ \mathrm{ft}) + (0\ \mathrm{ft}) + (0\ \mathrm{ft})$$

$$H = 0\ \mathrm{ft}$$

Considering x-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\ D}$$

$$H_o = \frac{(-2.049\ \mathrm{kip})}{1.57 \times (48\ \mathrm{in})}$$

$$H_o = -0.32627\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\,H)}{1.57\,D}$$

$$M_o = \frac{(23.379\text{ kipft}) + ((-2.049\text{ kip}) \times (0\text{ ft}))}{1.57 \times (48\text{ in})}$$

$$M_o = 3.7228\text{ kipft/ft}$$

Required depth of embedment in earth:

$$L_x^3 - \left(14.14 \times \frac{H_o \times L_x}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,x} = 5.7089\text{ ft}$ - Required depth in x-direction,

Considering z-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\,b}$$

$$H_o = \frac{(0.015\text{ kip})}{1.57 \times (48\text{ in})}$$

$$H_o = 0.0023885\text{ kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\,H)}{1.57\,b}$$

$$M_o = \frac{(0.048\text{ kipft}) + ((0.015\text{ kip}) \times (0\text{ ft}))}{1.57 \times (48\text{ in})}$$

$$M_o = 0.0076433\text{ kipft/ft}$$

Required depth of embedment in earth:

$$L_z^3 - \left(14.14 \times \frac{H_o \times L_z}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,z} = 0.9049\text{ ft}$ - Required depth in z-direction,

Minimum embedded depth required:

$L_{e,req}$ - Depth of pile required,

$$L_{e,req} = MAX[L_{e,x}, L_{e,z}]$$

$$L_{e,req} = MAX[(5.7089\text{ ft}), (0.9049\text{ ft})]$$

$$L_{e,req} = 5.709\text{ ft}$$

L_e - Actual embedded length of pile,

$$L_e = L - h_e - h_2$$

$$L_e = (6.25\text{ ft}) - (0\text{ ft}) - (0\text{ ft})$$

$$L_e = 6.25\text{ ft}$$

$Ratio$ - Embedded depth

$$Ratio = \frac{L_{e,req}}{L_e}$$

$$Ratio = \frac{(5.709\text{ ft})}{(6.25\text{ ft})}$$

$$Ratio = 0.91344$$

Status: **PASS**
Ratio: **0.910**

End-bearing Capacity (ASD)

A - Pile cross-section area

$$A = b\,D$$

$$A = (48\text{ in}) \times (48\text{ in})$$

$$A = 16\text{ ft}^2$$

q - End-bearing pressure

$$q = \frac{P_v}{A}$$

$$q = \frac{(4.713\ \mathrm{kip})}{(16\ \mathrm{ft}^2)}$$

$$q = 0.29456\ \mathrm{kip/ft}^2$$

Check bearing capacity ratio:

$Ratio$ - Capacity

$$Ratio = \frac{q}{q_a}$$

$$Ratio = \frac{\left(0.29456\ \mathrm{kip/ft}^2\right)}{(2000\ \mathrm{psf})}$$

$$Ratio = 0.14728$$

Status: **PASS**
Ratio: **0.150**

Czerniak

Lateral Soil Pressure (ASD):

L/D - Length to least lateral dimension ratio,

$$L/D = \frac{L}{D}$$

$$L/D = \frac{(6.25\ \mathrm{ft})}{(48\ \mathrm{in})}$$

$$L/D = 1.5625$$

Since L/D $\leq$ 10,

Pile is short.

Considering x-direction:

$H_o = -0.32627\ \mathrm{kip/ft}$ - Lateral force per length of pile,
$M_o = 3.7228\ \mathrm{kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (3.7228\ \mathrm{kipft/ft}) \times (6.25\ \mathrm{ft})) + \left(3 \times (-0.32627\ \mathrm{kip/ft}) \times (6.25\ \mathrm{ft})^2\right)}{(6 \times (3.7228\ \mathrm{kipft/ft})) + (4 \times (-0.32627\ \mathrm{kip/ft}) \times (6.25\ \mathrm{ft}))}$$

$$a = 4.306\ \mathrm{ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\,[(4\,M_o) + (3\,H_o\,L_e)]^2}{L_e^2\,[(3\,M_o) + (2\,H_o\,L_e)]}$$

$$p = \frac{0.75 \times [(4 \times (3.7228\ \mathrm{kipft/ft})) + (3 \times (-0.32627\ \mathrm{kip/ft}) \times (6.25\ \mathrm{ft}))]^{\,2}}{(6.25\ \mathrm{ft})^2 \times [(3 \times (3.7228\ \mathrm{kipft/ft})) + (2 \times (-0.32627\ \mathrm{kip/ft}) \times (6.25\ \mathrm{ft}))]}$$

$$p = 0.20845\ \mathrm{kip/ft}^2$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\,[(2\,M_o) + (H_o\,L_e)]}{L_e^2}$$

$$s = \frac{6 \times [(2 \times (3.7228\ \mathrm{kipft/ft})) + ((-0.32627\ \mathrm{kip/ft}) \times (6.25\ \mathrm{ft}))]}{(6.25\ \mathrm{ft})^2}$$

$$s = 0.83041\ \mathrm{kip/ft}^2$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\,\frac{a}{2}$$

$$p_a = (150\ \mathrm{psf/ft}) \times \frac{(4.306\ \mathrm{ft})}{2}$$

$$p_a = 0.32295\ \mathrm{kip/ft}^2$$

$Ratio$ - Lateral soil capacity

	$$Ratio = \frac{p}{p_a}$$ $$Ratio = \frac{\left(0.20845\ \mathrm{kip/ft^2}\right)}{\left(0.32295\ \mathrm{kip/ft^2}\right)}$$ $$Ratio = 0.64546$$	Status: **PASS** Ratio: **0.650**
	p_s - Allowable lateral soil pressure at depth L_e, $$p_s = R\,L_e$$ $$p_s = (150\ \mathrm{psf/ft}) \times (6.25\ \mathrm{ft})$$ $$p_s = 0.9375\ \mathrm{kip/ft^2}$$ $Ratio$ - Lateral soil capacity $$Ratio = \frac{s}{p_s}$$ $$Ratio = \frac{\left(0.83041\ \mathrm{kip/ft^2}\right)}{\left(0.9375\ \mathrm{kip/ft^2}\right)}$$ $$Ratio = 0.88577$$	Status: **PASS** Ratio: **0.890**
	Considering z-direction: $H_o = 0.0023885\ \mathrm{kip/ft}$ - Lateral force per length of pile, $M_o = 0.0076433\ \mathrm{kipft/ft}$ - Overturning moment per length of pile, a - Distance from resting surface to pivot point, $$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$ $$a = \frac{(4 \times (0.0076433\ \mathrm{kipft/ft}) \times (6.25\ \mathrm{ft})) + \left(3 \times (0.0023885\ \mathrm{kip/ft}) \times (6.25\ \mathrm{ft})^2\right)}{(6 \times (0.0076433\ \mathrm{kipft/ft})) + (4 \times (0.0023885\ \mathrm{kip/ft}) \times (6.25\ \mathrm{ft}))}$$ $$a = 4.4613\ \mathrm{ft}$$ p - Earth pressure against the pile at distance a/2 from resting surface, $$p = \frac{0.75\,[(4\,M_o) + (3\,H_o\,L_e)]^2}{L_e^2\,[(3\,M_o) + (2\,H_o\,L_e)]}$$ $$p = \frac{0.75 \times [(4 \times (0.0076433\ \mathrm{kipft/ft})) + (3 \times (0.0023885\ \mathrm{kip/ft}) \times (6.25\ \mathrm{ft}))]^{\ 2}}{(6.25\ \mathrm{ft})^2 \times [(3 \times (0.0076433\ \mathrm{kipft/ft})) + (2 \times (0.0023885\ \mathrm{kip/ft}) \times (6.25\ \mathrm{ft}))]}$$ $$p = 0.0020656\ \mathrm{kip/ft^2}$$ s - Earth pressure against the pile at distance L_e, $$s = \frac{6\,[(2\,M_o) + (H_o\,L_e)]}{L_e^2}$$ $$s = \frac{6 \times [(2 \times (0.0076433\ \mathrm{kipft/ft})) + ((0.0023885\ \mathrm{kip/ft}) \times (6.25\ \mathrm{ft}))]}{(6.25\ \mathrm{ft})^2}$$ $$s = 0.004641\ \mathrm{kip/ft^2}$$ **Check lateral soil pressure capacity:** p_a - Allowable lateral soil pressure at depth a/2, $$p_a = R\,\frac{a}{2}$$ $$p_a = (150\ \mathrm{psf/ft}) \times \frac{(4.4613\ \mathrm{ft})}{2}$$ $$p_a = 0.33459\ \mathrm{kip/ft^2}$$ $Ratio$ - Lateral soil capacity $$Ratio = \frac{p}{p_a}$$ $$Ratio = \frac{\left(0.0020656\ \mathrm{kip/ft^2}\right)}{\left(0.33459\ \mathrm{kip/ft^2}\right)}$$	

$$Ratio = 0.0061734$$

Status: **PASS**
Ratio: **0.010**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\,L_e$$

$$p_s = (150\ \mathrm{psf/ft}) \times (6.25\ \mathrm{ft})$$

$$p_s = 0.9375\ \mathrm{kip/ft^2}$$

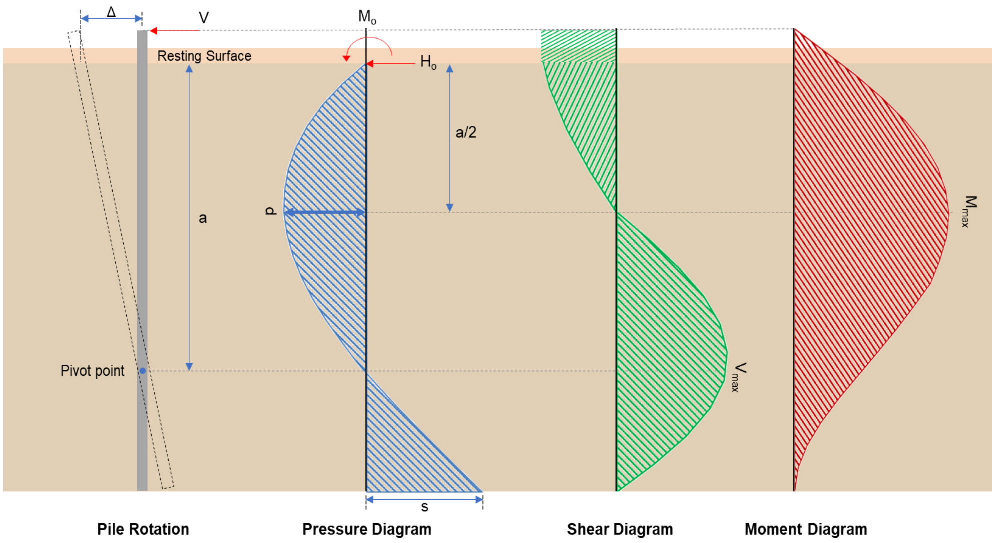
$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{(0.004641\ \mathrm{kip/ft^2})}{(0.9375\ \mathrm{kip/ft^2})}$$

$$Ratio = 0.0049504$$

Status: **PASS**
Ratio: **0.000**



Shear force and Bending moment (x-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\,D}$$

$$H_o = \frac{(-3.415\ \mathrm{kip})}{1.57 \times (48\ \mathrm{in})}$$

$$H_o = -0.54379\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\,H)}{1.57\,D}$$

$$M_o = \frac{(39.705\ \mathrm{kipft}) + ((-3.415\ \mathrm{kip}) \times (0\ \mathrm{ft}))}{1.57 \times (48\ \mathrm{in})}$$

$$M_o = 6.3225\ \mathrm{kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(6.3225\ \mathrm{kipft/ft})}{(-0.54379\ \mathrm{kip/ft})}$$

$$E = 11.627\ \mathrm{ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (6.3225\ \mathrm{kipft/ft}) \times (6.25\ \mathrm{ft})) + \left(3 \times (-0.54379\ \mathrm{kip/ft}) \times (6.25\ \mathrm{ft})^2\right)}{}$$

$$a = \frac{}{(6 \times (6.3225\ \text{kipft/ft})) + (4 \times (-0.54379\ \text{kip/ft}) \times (6.25\ \text{ft}))}$$

$$a = 4.3041\ \text{ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\, D) \left[1 - \left[3 \left(\frac{4\, E}{L_e} + 3\right) \left(\frac{a}{L_e}\right)^2\right] + \left[4 \left(\frac{3\, E}{L_e} + 2\right) \left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((-0.54379\ \text{kip/ft}) \times (48\ \text{in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (11.627\ \text{ft})}{(6.25\ \text{ft})} + 3\right) \times \left(\frac{(4.3041\ \text{ft})}{(6.25\ \text{ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (11.627\ \text{ft})}{(6.25\ \text{ft})} + 2\right) \times \left(\frac{(4.3041\ \text{ft})}{(6.25\ \text{ft})}\right)^3\right]\right]$$

$$V_{max} = 8.5953\ \text{kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, D\, L_e) \left[\left(\frac{E}{L_e} + \frac{a}{2\, L_e}\right) - \left[\left(\frac{4\, E}{L_e} + 3\right) \left(\frac{a}{2\, L_e}\right)^3\right] + \left[\left(\frac{3\, E}{L_e} + 2\right) \left(\frac{a}{2\, L_e}\right)^4\right]\right]$$

$$M_{max} = ((-0.54379\ \text{kip/ft}) \times (48\ \text{in}) \times (6.25\ \text{ft})) \times \left[\left(\frac{(11.627\ \text{ft})}{(6.25\ \text{ft})} + \frac{(4.3041\ \text{ft})}{2 \times (6.25\ \text{ft})}\right) - \left[\left(\frac{4 \times (11.627\ \text{ft})}{(6.25\ \text{ft})} + 3\right) \times \left(\frac{(4.3041\ \text{ft})}{2 \times (6.25\ \text{ft})}\right)^3\right] + \left[\left(\frac{3 \times (11.627\ \text{ft})}{(6.25\ \text{ft})} + 2\right) \times \left(\frac{(4.3041\ \text{ft})}{2 \times (6.25\ \text{ft})}\right)^4\right]\right]$$

$$M_{max} = 25.625\ \text{kipft}$$

Shear force and Bending moment (z-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\, b}$$

$$H_o = \frac{(0.023\ \text{kip})}{1.57 \times (48\ \text{in})}$$

$$H_o = 0.0036624\ \text{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\, H)}{1.57\, b}$$

$$M_o = \frac{(0.076\ \text{kipft}) + ((0.023\ \text{kip}) \times (0\ \text{ft}))}{1.57 \times (48\ \text{in})}$$

$$M_o = 0.012102\ \text{kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(0.012102\ \text{kipft/ft})}{(0.0036624\ \text{kip/ft})}$$

$$E = 3.3043\ \text{ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\, M_o\, L_e) + \left(3\, H_o\, L_e^2\right)}{(6\, M_o) + (4\, H_o\, L_e)}$$

$$a = \frac{(4 \times (0.012102\ \text{kipft/ft}) \times (6.25\ \text{ft})) + \left(3 \times (0.0036624\ \text{kip/ft}) \times (6.25\ \text{ft})^2\right)}{(6 \times (0.012102\ \text{kipft/ft})) + (4 \times (0.0036624\ \text{kip/ft}) \times (6.25\ \text{ft}))}$$

$$a = 4.4571\ \text{ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\, b) \left[1 - \left[3 \left(\frac{4\, E}{L_e} + 3\right) \left(\frac{a}{L_e}\right)^2\right] + \left[4 \left(\frac{3\, E}{L_e} + 2\right) \left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((0.0036624\ \mathrm{kip/ft}) \times (48\ \mathrm{in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (3.3043\ \mathrm{ft})}{(6.25\ \mathrm{ft})} + 3\right) \times \left(\frac{(4.4571\ \mathrm{ft})}{(6.25\ \mathrm{ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (3.3043\ \mathrm{ft})}{(6.25\ \mathrm{ft})} + 2\right) \times \left(\frac{(4.4571\ \mathrm{ft})}{(6.25\ \mathrm{ft})}\right)^3\right]\right]$$

$$V_{max} = 0.023458\ \mathrm{kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, b\, L_e)\left[\left(\frac{E}{L_e} + \frac{a}{2\,L_e}\right) - \left[\left(\frac{4\,E}{L_e} + 3\right)\left(\frac{a}{2\,L_e}\right)^3\right] + \left[\left(\frac{3\,E}{L_e} + 2\right)\left(\frac{a}{2\,L_e}\right)^4\right]\right]$$

$$M_{max} = ((0.0036624\ \mathrm{kip/ft}) \times (48\ \mathrm{in}) \times (6.25\ \mathrm{ft})) \times \left[\left(\frac{(3.3043\ \mathrm{ft})}{(6.25\ \mathrm{ft})} + \frac{(4.4571\ \mathrm{ft})}{2 \times (6.25\ \mathrm{ft})}\right) - \left[\left(\frac{4 \times (3.3043\ \mathrm{ft})}{(6.25\ \mathrm{ft})} + 3\right) \times \left(\frac{(4.4571\ \mathrm{ft})}{2 \times (6.25\ \mathrm{ft})}\right)^3\right] + \left[\left(\frac{3 \times (3.3043\ \mathrm{ft})}{(6.25\ \mathrm{ft})} + 2\right) \times \left(\frac{(4.4571\ \mathrm{ft})}{2 \times (6.25\ \mathrm{ft})}\right)^4\right]\right]$$

$$M_{max} = 0.065132\ \mathrm{kipft}$$

Minimum Reinforcement Check (LRFD)

Parameters:

$f'_{ck} = 2.5\ \mathrm{ksi}$ - Concrete strength,
$f_{yk} = 60\ \mathrm{ksi}$ - Longitudinal reinforcement strength,
$\phi = 0.65$ - Reduction factor for axial strength,
Table 22.4.2.1 $\alpha = 0.8$ - Alpha factor for axial strength,
$A_g = 2304\ \mathrm{in}^2$ - Gross area of concrete,

Longitudinal reinforcement:

Required reinforcement due to axial load, $A_{st,required}$

22.4.2.2, 10.6.1.1 $A_{st,required}$

$$A_{st,required} = Min\left[\frac{\frac{P}{\phi\,\alpha} - (0.85\,f'_{ck}\,A_g)}{f_{yk} - (0.85\,f'_{ck})}, (0.08\,A_g)\right]$$

$$A_{st,required} = Min\left[\frac{\frac{(6.882\ \mathrm{kip})}{(0.65)\times(0.8)} - (0.85 \times (2.5\ \mathrm{ksi}) \times (2304\ \mathrm{in}^2))}{(60\ \mathrm{ksi}) - (0.85 \times (2.5\ \mathrm{ksi}))}, (0.08 \times (2304\ \mathrm{in}^2))\right]$$

$$A_{st,required} = -84.367\ \mathrm{in}^2$$

A_{min} - Governing minimum reinforcement area,

$$A_{min} = Max[A_{st,required}, (0.0018\,A_g)]$$

$$A_{min} = Max\left[(-84.367\ \mathrm{in}^2), (0.0018 \times (2304\ \mathrm{in}^2))\right]$$

$$A_{min} = 4.1472\ \mathrm{in}^2$$

n_{rebar} - Required number of reinforcement,

$$n_{rebar} = \frac{A_{min}}{A_{rebar}}$$

$$n_{rebar} = \frac{(4.1472\ \mathrm{in}^2)}{(0.3068\ \mathrm{in}^2)}$$

$$n_{rebar} = 14$$

A_{st} - Actual total reinforcement area,

$$A_{st} = n_{rebar}\,\frac{\pi\,d_{bar}^2}{4}$$

$$A_{st} = (14) \times \frac{\pi \times (0.625\ \mathrm{in})^2}{4}$$

$$A_{st} = 4.2951\ \mathrm{in}^2$$

$Ratio$ - Capacity

$$Ratio = \frac{A_{min}}{A_{st}}$$

$$Ratio = \frac{(4.1472\ \mathrm{in}^2)}{(4.2951\ \mathrm{in}^2)}$$

	$$Ratio = 0.96556$$	Status: PASS Ratio: 0.970
25.2.3	s_{rebar} - Minimum spacing of reinforcement, $$s_{rebar} = Max\,[1.5, (1.5\; d_{bar})]$$ $$s_{rebar} = Max\,[1.5, (1.5 \times (0.625\text{ in}))]$$ $$s_{rebar} = 1.5\text{ in}$$	
	Ties:	
25.7.2.2 25.7.2.1	Since longitudinal reinforcement is $\leq$ No. 10ø: Use #3(0.375 in) s_{ties} - Maximum spacing of ties, $$s_{ties} = Min\,[(16\; d_{bar}), (48\; d_{ties}), Min\,(D, b)]$$ $$s_{ties} = Min\,[(16 \times (0.625\text{ in})), (48 \times (0.375\text{ in})), Min\,((48\text{ in}), (48\text{ in}))]$$ $$s_{ties} = 10\text{ in}$$	
	Summary: Main reinforcement: **14 - #5 (0.625 in)** Ties: **#3(0.375 in) - 10 in**	
22.4.2.2	**Axial Compression Strength (ACI 318-19, LRFD)** ϕP_N - Allowable axial compressive strength $$\phi P_N = \phi\; 0.80\; \left[\left(0.85\; f'_{ck}\; [A_g - A_{st}]\right) + (f_{yk}\; A_{st})\right]$$ $$\phi P_N = (0.65) \times 0.80 \times \left[\left(0.85 \times (2.5\text{ ksi}) \times \left[\left(2304\text{ in}^2\right) - \left(4.2951\text{ in}^2\right)\right]\right) + \left((60\text{ ksi}) \times \left(4.2951\text{ in}^2\right)\right)\right]$$ $$\phi P_N = 2675.2\text{ kip}$$ $Ratio$ - Capacity $$Ratio = \frac{P}{\phi P_N}$$ $$Ratio = \frac{(6.882\text{ kip})}{(2675.2\text{ kip})}$$ $$Ratio = 0.0025725$$	Status: PASS Ratio: 0.000
22.5.2.2 22.5.5.1.3 22.5.5.1.1	**Shear Strength (ACI 318-19, LRFD)** **Parameters:** $b_w = 48\text{ in}$ - Effective width, d - Effective depth $$d = 0.80\; D$$ $$d = 0.80 \times (48\text{ in})$$ $$d = 38.4\text{ in}$$ λ_s - size effect modification factor $$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{d}{10}}}, 1\right]$$ $$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{(38.4\text{ in})}{10}}}, 1\right]$$ $$\lambda_s = 0.64282$$ The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5\text{ ksi} \rightarrow 2500\text{ psi}$, $V_{c,max}$ - Max shear strength of concrete $$V_{c,max} = 5\; \lambda_s\; \sqrt{f'_{ck}}\; b_w\; d$$ $$V_{c,max} = 5 \times (0.64282) \times \sqrt{(2500\text{ psi})} \times (48\text{ in}) \times (38.4\text{ in})$$	

$$V_{c,max} = 296.21 \text{ kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$, $P = 6.882 \text{ kip} \rightarrow 6882 \text{ lbf}$,

22.5.5.1.1(a) $V_{c,a}$ - Shear strength of concrete (a)

$$V_{c,a} = \left[2\,\lambda_s\,\sqrt{f'_{ck}} + \frac{P}{6\,A_g}\right]\,b_w\,d$$

$$V_{c,a} = \left[2 \times (0.64282) \times \sqrt{(2500 \text{ psi})} + \frac{(6882 \text{ lbf})}{6 \times (2304 \text{ in}^2)}\right] \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{c,a} = 119.4 \text{ kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$,

22.5.5.1.2 $V_{c,b}$ - Shear strength of concrete (b)

$$V_{c,b} = \left[2\,\lambda_s\,\sqrt{f'_{ck}} + \left(0.05\,f'_{ck}\right)\right]\,b_w\,d$$

$$V_{c,b} = \left[2 \times (0.64282) \times \sqrt{(2500 \text{ psi})} + (0.05 \times (2500 \text{ psi}))\right] \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{c,b} = 348.89 \text{ kip}$$

V_c - Governing shear strength of concrete

$$V_c = Min\left[V_{c,max}, V_{c,a}, V_{c,b}\right]$$

$$V_c = Min\left[(296.21 \text{ kip}), (119.4 \text{ kip}), (348.89 \text{ kip})\right]$$

$$V_c = 119.4 \text{ kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$,

22.5.1.2 $V_{s,a}$ - Shear strength of steel (a)

$$V_{s,a} = 8\,\sqrt{f'_{ck}}\,b_w\,d$$

$$V_{s,a} = 8 \times \sqrt{(2500 \text{ psi})} \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{s,a} = 737.28 \text{ kip}$$

A_v - Ties rebar area,

$$A_v = \frac{\pi\,d_{ties}^2}{4}$$

$$A_v = \frac{\pi \times (0.375 \text{ in})^2}{4}$$

$$A_v = 0.11045 \text{ in}^2$$

22.5.8.5.3 $V_{s,b}$ - Shear strength of steel (b)

$$V_{s,b} = \frac{2\,A_v\,f_{ywk}\,d}{s_{ties}}$$

$$V_{s,b} = \frac{2 \times \left(0.11045 \text{ in}^2\right) \times (60 \text{ ksi}) \times (38.4 \text{ in})}{(10 \text{ in})}$$

$$V_{s,b} = 50.894 \text{ kip}$$

V_s - Governing shear strength of steel

$$V_s = MIN\left[V_{s,a}, V_{s,b}\right]$$

$$V_s = MIN\left[(737.28 \text{ kip}), (50.894 \text{ kip})\right]$$

$$V_s = 50.894 \text{ kip}$$

22.5.1.1 ϕV_n - Allowable shear strength

$$\phi V_n = \phi\,\left(V_c + V_s\right)$$

$$\phi V_n = (0.65) \times ((119.4 \text{ kip}) + (50.894 \text{ kip}))$$

$$\phi V_n = 110.69 \text{ kip}$$

Considering x-direction:

$V_{max} = 8.5953 \text{ kip}$ - Maximum shear force in the x-direction,
$Ratio$ - Capacity

$$Ratio = \frac{V_{max}}{\phi V_n}$$

	$$Ratio = \frac{(8.5953\ \text{kip})}{(110.69\ \text{kip})}$$ $$Ratio = 0.07765$$	Status: **PASS** Ratio: **0.080**
	Considering z-direction: $V_{max} = 0.023458\ \text{kip}$ - Maximum shear force in the z-direction, $Ratio$ - Capacity $$Ratio = \frac{V_{max}}{\phi V_n}$$ $$Ratio = \frac{(0.023458\ \text{kip})}{(110.69\ \text{kip})}$$ $$Ratio = 0.00021192$$	Status: **PASS** Ratio: **0.000**
14.5.2.1b	**Flexural Strength (ACI 318-19, LRFD)** S_m - Section modulus $$S_m = \frac{b\,D^2}{6}$$ $$S_m = \frac{(48\ \text{in}) \times (48\ \text{in})^2}{6}$$ $$S_m = 18432\ \text{in}^3$$ $\lambda = 1$ - Concrete modification factor (Normal concrete), Allowable flexural strength: M_n shall be the lesser of: $\phi M_{n,1}$ $$\phi M_{n,1} = \phi \times 5 \times \lambda \times \sqrt{(f'_c)} \times S_m$$ $$\phi M_{n,1} = 0.65 \times 5 \times 1 \times \sqrt{(2.5 ksi)} \times 18432.001 in^3$$ $$\phi M_{n,1} = 249.600 kipft$$ $\phi M_{n,2}$ $$\phi M_{n,2} = \phi\ 0.85\ f'_{ck}\ S_m$$ $$\phi M_{n,2} = (0.65) \times 0.85 \times (2.5\ \text{ksi}) \times \left(18432\ \text{in}^3\right)$$ $$\phi M_{n,2} = 2121.6\ \text{kipft}$$ Therefore, ϕM_n - Allowable flexural strength, $$\phi M_n = MIN[\phi M_{n,1}, \phi M_{n,2}]$$ $$\phi M_n = MIN[(249.6\ \text{kipft}), (2121.6\ \text{kipft})]$$ $$\phi M_n = 249.6\ \text{kipft}$$ **Considering x-direction:** $M_{max} = 25.625\ \text{kipft}$ - Maximum moment in the x-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\phi M_n}$$ $$Ratio = \frac{(25.625\ \text{kipft})}{(249.6\ \text{kipft})}$$ $$Ratio = 0.10266$$	Status: **PASS** Ratio: **0.100**
	Considering z-direction: $M_{max} = 0.065132\ \text{kipft}$ - Maximum moment in the z-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\phi M_n}$$	

	$$Ratio = \frac{(0.065132 \text{ kipft})}{(249.6 \text{ kipft})}$$ $$Ratio = 0.00026095$$	Status: **PASS** Ratio: **0.000**