

Project Details



Project Name: MTSOLAR_DJ2JEC4GEF88-Alam-30-

Panel

Location: Nice, CA 95464, USA

Unique ID: 2P-19.75-6TOP-SD-57-L-5Hx6W-LCG3

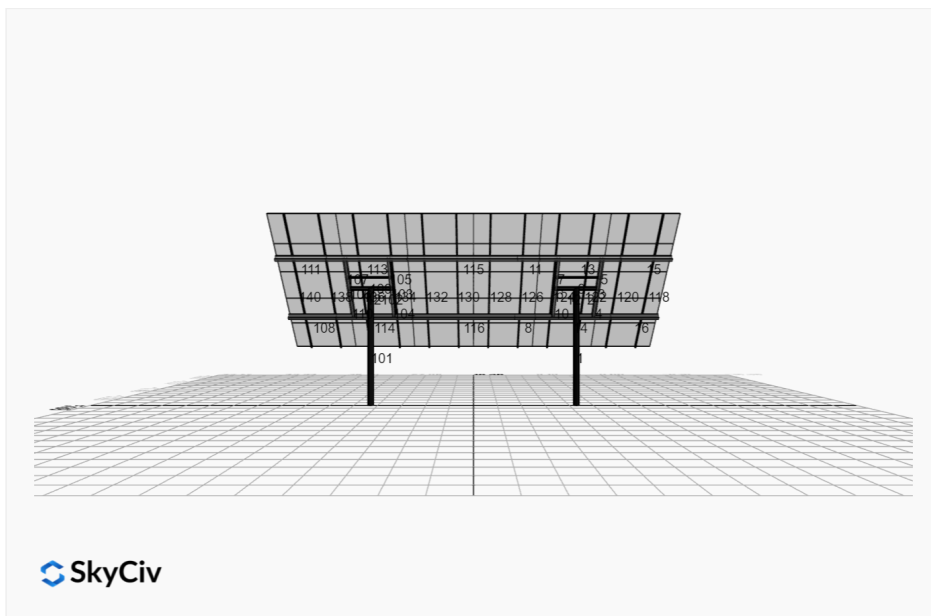
Dealer: _____

Date: Mon Aug 05 2024

Number of Modules: 30

Number of Poles: 2

Date Sold:



Array Dimensions N/S	17.21 ft
Array Dimensions E/W	37.22 ft
Winter Tilt Angle	46.1
Front Edge Clearance	5 ft

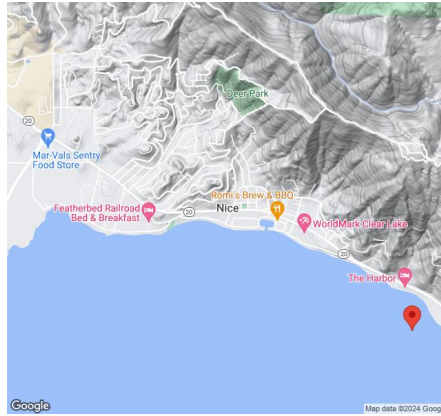
MT Solar Bill of Materials (2P-19.75-6TOP-SD-57-L-5Hx6W-LCG3)

Part	Short Description	BOM Qty
MTS-PC-6	6IN Pole Cap Assembly	2
MTS-HF-SD	H-Frame Assembly-SD	2
MTS-SD-Wing-57	57IN SD Wing	4
MTS-SD-Splice-90	90IN SD Splice	2
MTS-SD-Splice-57	57IN SD Splice	2
MTS-CLAMP-ANGLE-4PK	Angle Clamp	6

Rail Bill of Materials

Part	Qty
Rails (204in)	12
Rail Attachment	48
Module Mid Clamp	48
Module End Clamp	24
Ground Lug	6

Site Details:



Site Address: Nice, CA 95464, USA

Array Specification

Duty Classification:	SD
Module Width:	40.80 in
Module Length:	73.44in
Number of Rows:	5
Number of Columns:	6
Total Number of Modules:	30
Winter Tilt Angle:	46.1
Front Edge Clearance:	5
Total Array Height at Tilt:	17.32 ft
Total Frame Length:	36.75 ft
Frame Weight:	2182 lbs
Array Dimensions N/S:	17.21 ft
Array Dimensions E/W:	37.22 ft
Rail Length:	206.50 in
Rail Spacing:	3.06 ft

Support Specifications

Pole Size:	6in Pipe Sch 80
Pole Length above Grade:	11.20 ft
Number of Poles:	2
Pole Spacing:	19.75 ft

Foundation Specifications

Foundation Type:	Square
Foundation Dimensions:	48 x 48 in
Foundation Depth (below grade):	Pile 1: 6.25 ft Pile 2: 6.25 ft
Foundation Volume:	7.407 y ³

Site Info

Risk Category:	I
Exposure:	C
Soil Classification:	sand
Site Location:	Nice, CA 95464, USA
Wind Speed:	87 mph
Snow Load:	20 psf

Design Disclaimer

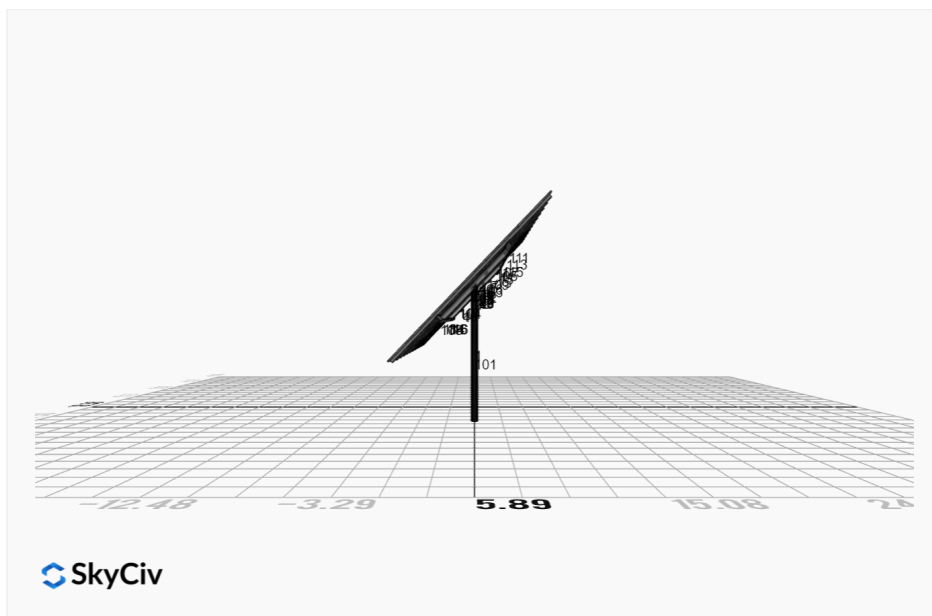
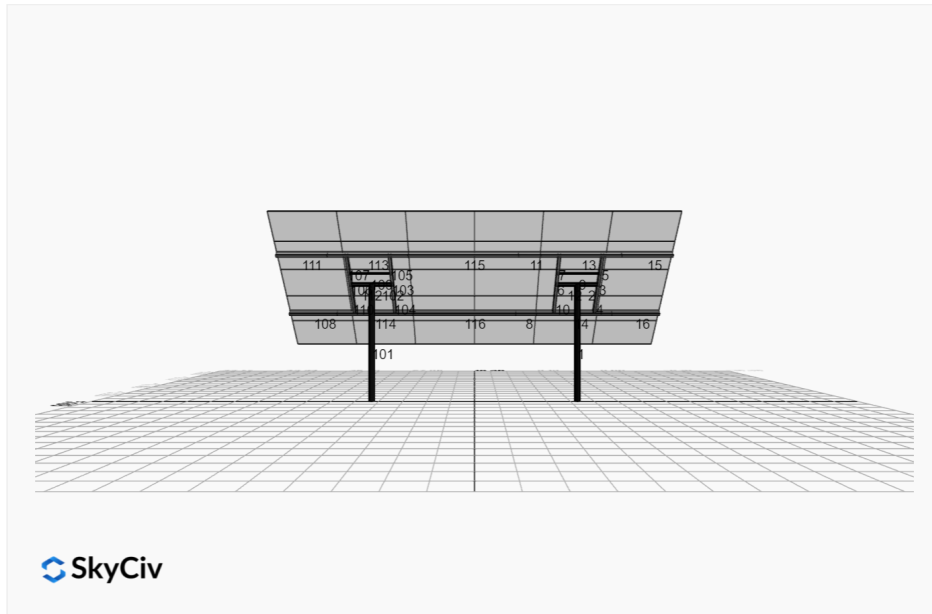
This software should be used for preliminary designs and should not be used as a final design unless reviewed, verified and designed by a qualified structural engineer.

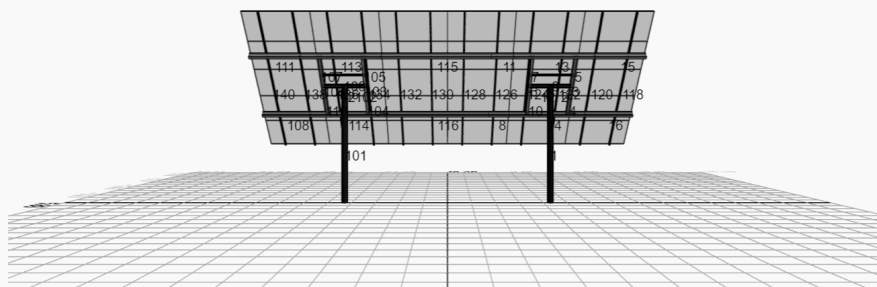
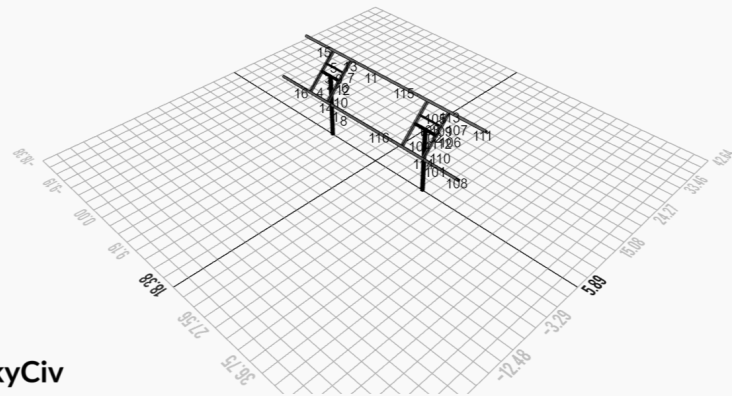
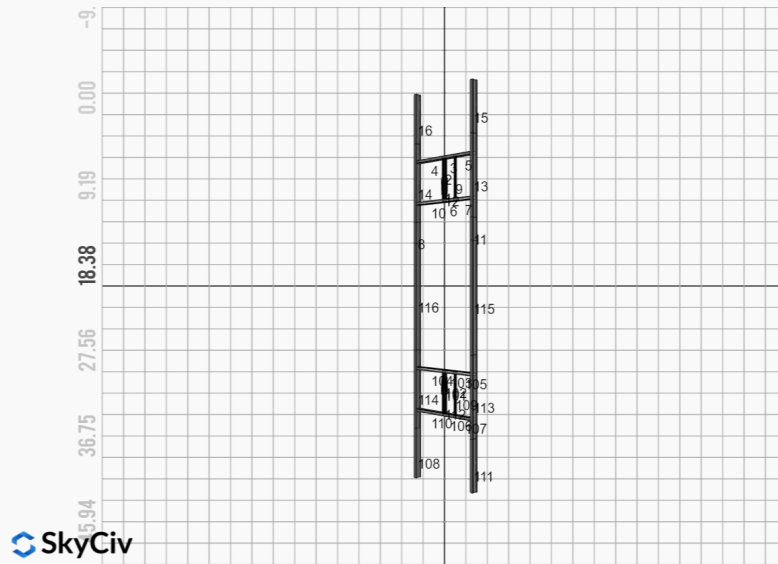
AutoDesigner Input

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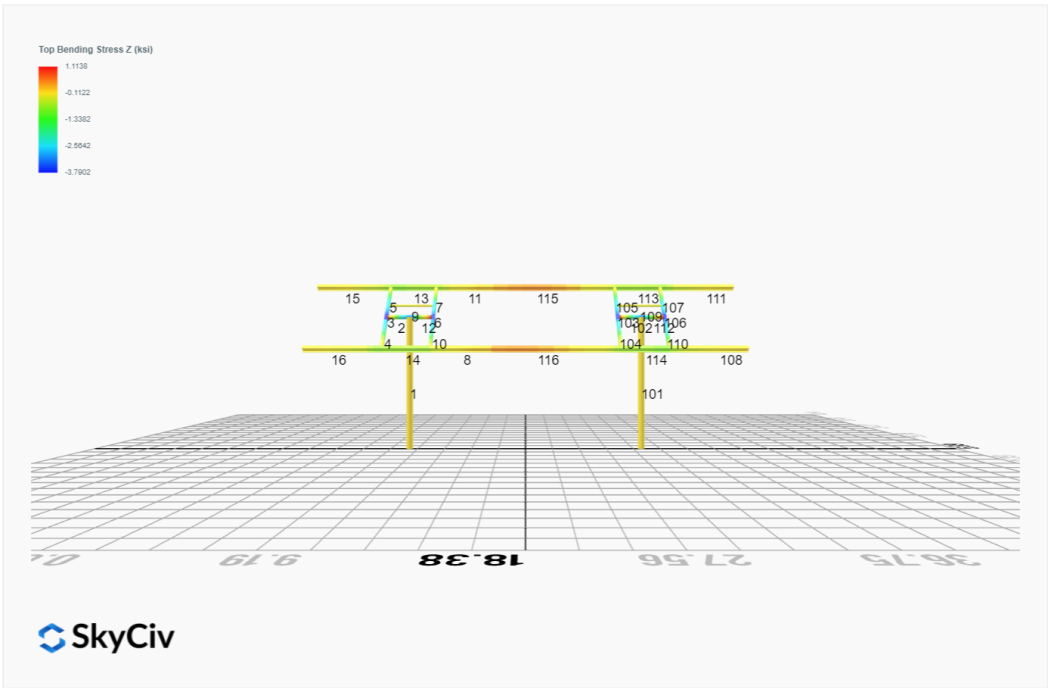
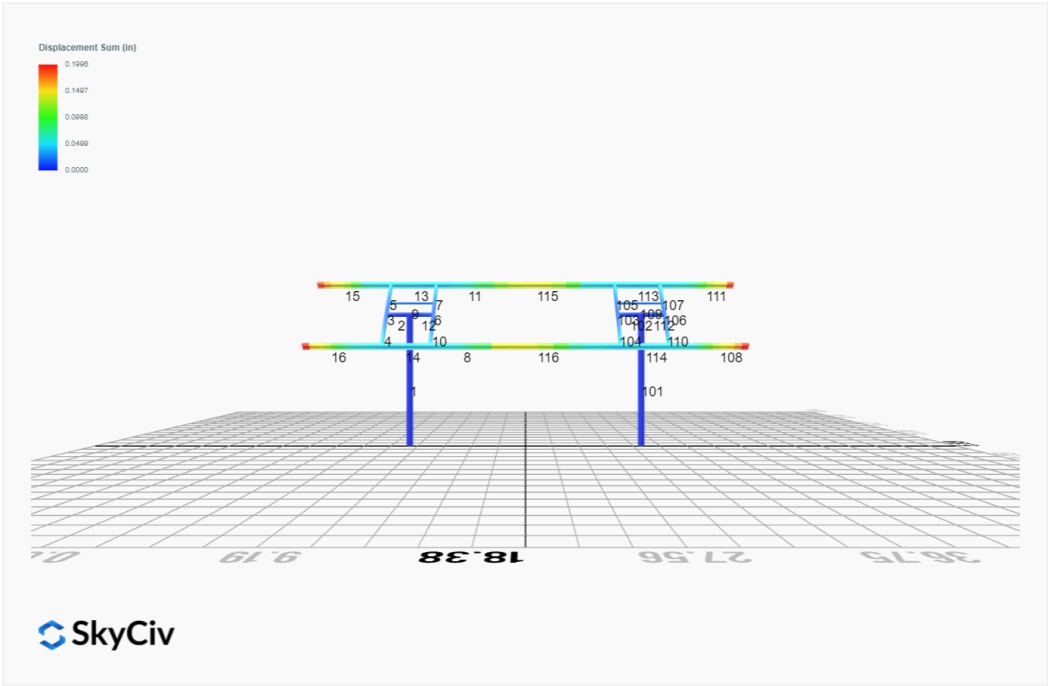
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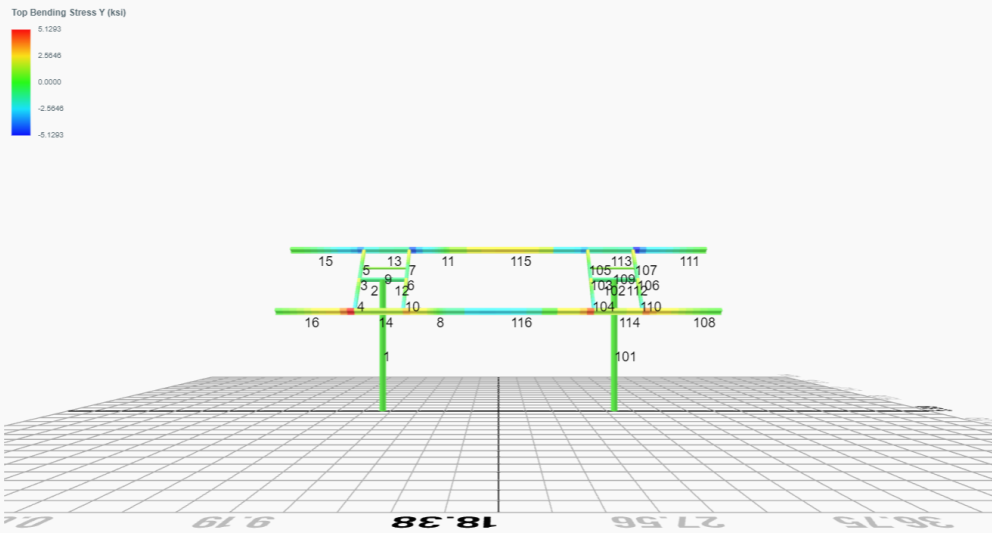
- AISC Deflection checks are set to L/1 due to structure design intent
- Foundation Soil Parameters used in this Autodesign are all estimates, proper geotechnical reports are required to confirm soil profiles
- Wind speeds, snow loads and other site specific results are based on ASCE 7 2016
- Steel frame design checks are based on AISC 360 2016 (LRFD)
- Foundation Design and Sizing is approximate only



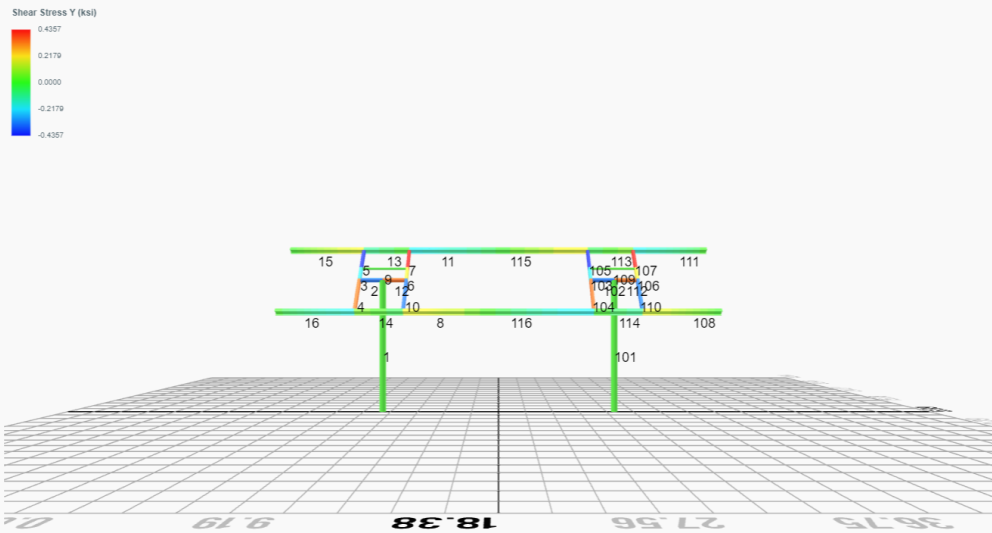


FEM Results (Envelope Worst Case for each member)

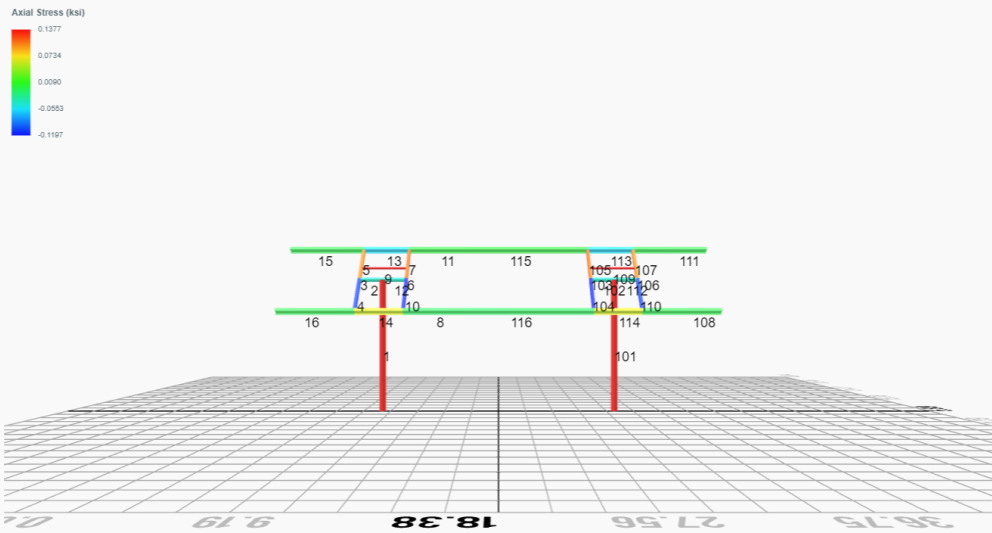




 SkyCiv



 SkyCiv



Reaction Forces for Foundation 1 (Node ID#1), (kip, kip-ft)

ASD Load Combination Results

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 1. D	0.0000	2.2646	-0.0049	-0.0144	0.0460	0.0186
ULS: 2. D + L	0.0000	2.2646	-0.0049	-0.0144	0.0460	0.0186
ULS: 3. D + (S or Lr or R)	0.0000	3.4171	-0.0080	-0.0235	0.0753	0.0210
ULS: 3. D + (S or Lr or R)	0.0000	2.2646	-0.0049	-0.0144	0.0460	0.0186
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	0.0000	3.1290	-0.0072	-0.0212	0.0680	0.0204
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	0.0000	2.2646	-0.0049	-0.0144	0.0460	0.0186
ULS: 5b. D + 0.7E	0.0000	2.2646	-0.0049	-0.0144	0.0460	0.0186
ULS: 6b. D + 0.75L + 0.75(0.7)E + 0.75S	0.0000	3.1290	-0.0072	-0.0212	0.0680	0.0204
ULS: 8. 0.6D + 0.7E	0.0000	1.3588	-0.0029	-0.0087	0.0276	0.0112
ULS: 5a. D + 0.6W_Wind downforce Case A only	-2.0862	4.2722	-0.0242	-0.0787	0.1143	23.7923
ULS: 5a. D + 0.6W_Wind downforce Case B only	0.0000	2.2646	-0.0049	-0.0144	0.0460	0.0186
ULS: 5a. D + 0.6W_Wind uplift Case A only	2.0862	0.2571	0.0144	0.0495	-0.0223	-22.9531
ULS: 5a. D + 0.6W_Wind uplift Case B only	0.0000	2.2646	-0.0049	-0.0144	0.0460	0.0186
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-1.5646	4.6346	-0.0217	-0.0694	0.1192	17.8507
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	0.0000	3.1290	-0.0072	-0.0212	0.0680	0.0204
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	1.5646	1.6233	0.0073	0.0267	0.0167	-17.2083
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	0.0000	3.1290	-0.0072	-0.0212	0.0680	0.0204
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-1.5646	3.7703	-0.0194	-0.0626	0.0972	17.8489
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	0.0000	2.2646	-0.0049	-0.0144	0.0460	0.0186
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	1.5646	0.7589	0.0096	0.0335	-0.0052	-17.2102
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	0.0000	2.2646	-0.0049	-0.0144	0.0460	0.0186
ULS: 7. 0.6D + 0.6W_Wind downforce Case A only	-2.0862	3.3663	-0.0223	-0.0729	0.0959	23.7849
ULS: 7. 0.6D + 0.6W_Wind downforce Case B only	0.0000	1.3588	-0.0029	-0.0087	0.0276	0.0112
ULS: 7. 0.6D + 0.6W_Wind uplift Case A only	2.0862	-0.6488	0.0164	0.0553	-0.0407	-22.9605
ULS: 7. 0.6D + 0.6W_Wind uplift Case B only	0.0000	1.3588	-0.0029	-0.0087	0.0276	0.0112

Worst Case Reactions LRFD

These calculations are taken directly from the FEA via SkyCiv and are used in the Concrete Checks of the Foundation Module.
Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	6.6397
Shear X	-3.4769
Shear Z	-0.0398
Moment X	-0.1296
Moment Y (Twist)	0.1840
Moment Z	40.3934

Worst Case Reactions ASD

These results are taken from the worst case values in the above table and are used in the Soil Checks in the Foundation Module.
Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	4.6346
Shear X	-2.0862
Shear Z	-0.0242
Moment X	-0.0787
Moment Y (Twist)	0.1192
Moment Z	23.7923

Reaction Forces for Foundation 2 (Node ID#101), (kip, kip-ft)

ASD Load Combination Results

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 1. D	-0.0000	2.2646	0.0049	0.0145	-0.0460	0.0186
ULS: 2. D + L	-0.0000	2.2646	0.0049	0.0145	-0.0460	0.0186
ULS: 3. D + (S or Lr or R)	-0.0000	3.4171	0.0080	0.0235	-0.0753	0.0210
ULS: 3. D + (S or Lr or R)	-0.0000	2.2646	0.0049	0.0145	-0.0460	0.0186
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	-0.0000	3.1290	0.0072	0.0212	-0.0680	0.0204

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	-0.0000	2.2646	0.0049	0.0145	-0.0460	0.0186
ULS: 5b. D + 0.7E	-0.0000	2.2646	0.0049	0.0145	-0.0460	0.0186
ULS: 6b. D + 0.75L + 0.75(0.7)E + 0.75S	-0.0000	3.1290	0.0072	0.0212	-0.0680	0.0204
ULS: 8. 0.6D + 0.7E	-0.0000	1.3588	0.0029	0.0087	-0.0276	0.0112
ULS: 5a. D + 0.6W_Wind downforce Case A only	-2.0862	4.2722	0.0242	0.0787	-0.1143	23.7923
ULS: 5a. D + 0.6W_Wind downforce Case B only	-0.0000	2.2646	0.0049	0.0145	-0.0460	0.0186
ULS: 5a. D + 0.6W_Wind uplift Case A only	2.0862	0.2571	-0.0144	-0.0495	0.0223	-22.9531
ULS: 5a. D + 0.6W_Wind uplift Case B only	-0.0000	2.2646	0.0049	0.0145	-0.0460	0.0186
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-1.5646	4.6346	0.0217	0.0694	-0.1192	17.8507
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-0.0000	3.1290	0.0072	0.0212	-0.0680	0.0204
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	1.5646	1.6233	-0.0073	-0.0267	-0.0167	-17.2083
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	-0.0000	3.1290	0.0072	0.0212	-0.0680	0.0204
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-1.5646	3.7703	0.0194	0.0626	-0.0972	17.8489
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-0.0000	2.2646	0.0049	0.0145	-0.0460	0.0186
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	1.5646	0.7589	-0.0096	-0.0335	0.0053	-17.2102
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	-0.0000	2.2646	0.0049	0.0145	-0.0460	0.0186
ULS: 7. 0.6D + 0.6W_Wind downforce Case A only	-2.0862	3.3663	0.0223	0.0729	-0.0959	23.7849
ULS: 7. 0.6D + 0.6W_Wind downforce Case B only	-0.0000	1.3588	0.0029	0.0087	-0.0276	0.0112
ULS: 7. 0.6D + 0.6W_Wind uplift Case A only	2.0862	-0.6488	-0.0164	-0.0553	0.0407	-22.9605
ULS: 7. 0.6D + 0.6W_Wind uplift Case B only	-0.0000	1.3588	0.0029	0.0087	-0.0276	0.0112

Worst Case Reactions LRFD

These calculations are taken directly from the FEA via SkyCiv and are used in the Concrete Checks of the Foundation Module.
Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	6.6397
Shear X	-3.4769
Shear Z	0.0398
Moment X	0.1296
Moment Y (Twist)	0.1841
Moment Z	40.3940

Worst Case Reactions ASD

These results are taken from the worst case values in the above table and are used in the Soil Checks in the Foundation Module.
Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	4.6346
Shear X	-2.0862
Shear Z	0.0242
Moment X	0.0787
Moment Y (Twist)	0.1192
Moment Z	23.7923

Project Details

Design Code: AISC 360-16 LRFD
Provision: LRFD
Country: United States

User Name: sales@mtsolar.us
Project Name: MTSOLAR_DJ2JEC4GEF88-Alam-30-Panel
Unit System: imperial



Design Input Information

Design Factors			
Φ_t	Φ_c	Φ_b	Φ_v
0.9	0.9	0.9	0.9

Design Materials			
ID	E (ksi)	F_y (ksi)	F_u (ksi)
1	29000	50	65

Section Dimensions								
ID	Name	d (in)	t_w (in)					
1	2in Pipe Sch 40	2.38	0.15					
4	4in Pipe Sch 40	4.50	0.24					
8	6in Pipe Sch 80	6.63	0.43					
ID	Name	d (in)	b (in)	t_w (in)	t_b (in)	r (in)		
15	HSS5x3x1/8	5.00	3.00	0.12	0.12	0.12		
ID	Name	d (in)	t_w (in)	b_t (in)	b_b (in)	t_t (in)	t_b (in)	r (in)
18	W6x9	5.90	0.17	3.94	3.94	0.21	0.21	0.25

Section Properties								
ID	Name	A (in ²)	J (in ⁴)	I_{y0} (in ⁴)	I_{z0} (in ⁴)	I_w (in ⁶)	S_{y0} (in ³)	S_{z0} (in ³)

				-		-	-	
113	18	4.88	4.00	7.5 0	1.10,1.10,1.10,1.10,1.10,1.10,1.10,1.10,1.10,1.10,1.10,1.10,1.10,1.10,1.10,1.10,1.10,1.10,1.09,1.10,1.10,1.10,1.1 0,1.10,1.10,1.10,1.10,1.10	30 0	20 0	1
114	18	4.88	4.00	7.5 0	1.10,1.10,1.10,1.10,1.10,1.10,1.10,1.10,1.10,1.10,1.10,1.10,1.10,1.10,1.10,1.10,1.10,1.10,1.08,1.10,1.10,1.10,1.1 0,1.10,1.10,1.10,1.10,1.10	30 0	20 0	1
115	18	6.63	6.63	10. 20	1.17,1.17,1.17,1.17,1.17,1.17,1.17,1.17,1.17,1.17,1.17,1.17,1.17,1.17,1.17,1.17,1.17,1.17,1.18,1.17,1.17,1.17,1.1 7,1.17,1.17,1.17,1.17,1.17	30 0	20 0	1
116	18	6.63	6.63	10. 20	1.17,1.17,1.17,1.17,1.17,1.17,1.17,1.17,1.17,1.17,1.17,1.17,1.17,1.17,1.17,1.17,1.17,1.17,1.18,1.17,1.17,1.17,1.1 7,1.17,1.17,1.17,1.17,1.17	30 0	20 0	1

Member ID	$\Phi_t P_n$ (kip)	$\Phi_c P_n$ (kip)	$\Phi_b M_{zn}$ (k-ft)	$\Phi_b M_{yn}$ (k-ft)	$\Phi_v V_{yn}$ (kip)	$\Phi_v V_{zn}$ (kip)
1	378.22	114.84	62.23	62.23	113.47	113.47
2	142.83	141.72	16.17	16.17	42.85	42.85
3	79.65	74.02	10.99	6.26	29.14	16.61
4	79.65	72.01	10.99	6.26	29.14	16.61
5	79.65	73.44	10.99	6.26	29.14	16.61
6	79.65	74.02	10.99	6.26	29.14	16.61
7	79.65	73.44	10.99	6.26	29.14	16.61
8	120.60	115.40	23.36	6.45	30.09	45.74
9	48.35	43.11	2.85	2.85	14.51	14.51
10	79.65	72.01	10.99	6.26	29.14	16.61
11	120.60	115.40	23.36	6.45	30.09	45.74
12	142.83	141.72	16.17	16.17	42.85	42.85
13	120.60	84.03	19.14	6.45	30.09	45.74
14	120.60	84.03	19.12	6.45	30.09	45.74
15	120.60	34.69	23.36	6.45	30.09	45.74
16	120.60	34.69	23.36	6.45	30.09	45.74
101	378.22	114.84	62.23	62.23	113.47	113.47
102	142.83	141.72	16.17	16.17	42.85	42.85
103	79.65	74.02	10.99	6.26	29.14	16.61
104	79.65	72.01	10.99	6.26	29.14	16.61
105	79.65	73.44	10.99	6.26	29.14	16.61
106	79.65	74.02	10.99	6.26	29.14	16.61
107	79.65	73.44	10.99	6.26	29.14	16.61
108	120.60	34.69	23.36	6.45	30.09	45.74
109	48.35	43.11	2.85	2.85	14.51	14.51
110	79.65	72.01	10.99	6.26	29.14	16.61
111	120.60	34.69	23.36	6.45	30.09	45.74
112	142.83	141.72	16.17	16.17	42.85	42.85
113	120.60	84.03	19.16	6.45	30.09	45.74
114	120.60	84.03	19.10	6.45	30.09	45.74
115	120.60	68.63	15.93	6.45	30.09	45.74
116	120.60	68.63	15.93	6.45	30.09	45.74

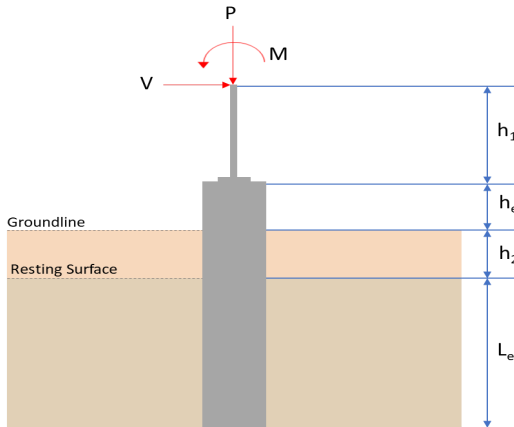
Member ID	P	M _z	M _y	V _y	V _z	(P,M _z ,M _y)	Worst LC	KL/r	δ	Status
1	0.058	0.649	0.005	0.031	0.000	0.680	#13	0.643	Not Required	Pass
2	0.003	0.334	0.230	0.074	0.042	0.565	#13	0.034	Not Required	Pass
3	0.010	0.593	0.065	0.060	0.007	0.643	#13	0.044	Not Required	Pass
4	0.000	0.500	0.100	0.000	0.000	0.600	#13	0.000	Not Required	Pass

4	0.009	0.590	0.178	0.059	0.030	0.007	#13	0.078	Not Required	Pass
5	0.010	0.368	0.186	0.059	0.037	0.399	#13	0.073	Not Required	Pass
6	0.010	0.573	0.071	0.057	0.008	0.625	#13	0.044	Not Required	Pass
7	0.010	0.356	0.187	0.057	0.038	0.390	#13	0.073	Not Required	Pass
8	0.000	0.041	0.088	0.037	0.010	0.120	#21	0.088	Not Required	Pass
9	0.011	0.049	0.054	0.001	0.000	0.107	#13	0.198	Not Required	Pass
10	0.010	0.570	0.183	0.057	0.031	0.665	#13	0.078	Not Required	Pass
11	0.000	0.041	0.090	0.037	0.010	0.121	#21	0.088	Not Required	Pass
12	0.003	0.315	0.218	0.072	0.041	0.534	#13	0.034	Not Required	Pass
13	0.005	0.206	0.257	0.048	0.013	0.411	#21	0.265	Not Required	Pass
14	0.006	0.210	0.257	0.048	0.013	0.411	#21	0.177	Not Required	Pass
15	0.000	0.088	0.137	0.029	0.008	0.205	#21	Not Required	Not Required	Pass
16	0.000	0.088	0.137	0.029	0.008	0.205	#21	Not Required	Not Required	Pass
101	0.058	0.649	0.005	0.031	0.000	0.680	#13	0.643	Not Required	Pass
102	0.003	0.315	0.218	0.072	0.041	0.534	#13	0.034	Not Required	Pass
103	0.010	0.573	0.071	0.057	0.008	0.625	#13	0.044	Not Required	Pass
104	0.010	0.570	0.183	0.057	0.031	0.665	#13	0.078	Not Required	Pass
105	0.010	0.356	0.187	0.057	0.038	0.390	#13	0.073	Not Required	Pass
106	0.010	0.593	0.065	0.060	0.007	0.643	#13	0.044	Not Required	Pass
107	0.010	0.368	0.186	0.059	0.037	0.399	#13	0.073	Not Required	Pass
108	0.000	0.088	0.137	0.029	0.008	0.205	#21	Not Required	Not Required	Pass
109	0.011	0.049	0.054	0.001	0.000	0.107	#13	0.198	Not Required	Pass
110	0.009	0.590	0.178	0.059	0.030	0.667	#13	0.078	Not Required	Pass
111	0.000	0.088	0.137	0.029	0.008	0.205	#21	Not Required	Not Required	Pass
112	0.003	0.334	0.230	0.074	0.042	0.565	#13	0.034	Not Required	Pass
113	0.005	0.206	0.257	0.048	0.013	0.411	#21	0.177	Not Required	Pass
114	0.006	0.210	0.257	0.048	0.013	0.411	#21	0.265	Not Required	Pass
115	0.000	0.155	0.138	0.037	0.010	0.258	#21	0.439	Not Required	Pass
116	0.000	0.155	0.140	0.037	0.010	0.259	#21	0.439	Not Required	Pass

Definitions

Φ_t	Safety factor for tensile
Φ_c	Safety factor for compression
Φ_b	Safety factor for flexure
Φ_v	Safety factor for shear
E	Modulus of elasticity
F_y	Specified minimum yield stress
F_u	Specified minimum tensile strength
A	Cross-sectional area
J	Torsional constant
I_{yp}	Moment of inertia about the Y axes
I_{zp}	Moment of inertia about the Z axes
I_w	Warping constant
S_{yp}	Plastic section modulus about the Y axis
S_{zp}	Plastic section modulus about the Z axis
KL	Effective length
C_b	Buckling modification factor (from all load combinations)
L_b	Length between braced points
LST	Limited slenderness for tension
LSC	Limited slenderness for compression
LD	Limited deflection
P_n	Nominal axial strength (tension/compression)
M_n	Nominal flexural strength (about Z/Y axis)
V_n	Nominal shear strength (along Z/Y axis)
P	Design ratio in case of axial force
M_z	Design ratio in case of bending about Z axis
M_y	Design ratio in case of bending about Y axis
V_y	Design ratio in case of shear along Y axis

V_z	Design ratio in case of shear along Z axis
(P,M_z,M_y)	Design ratio in case of axial force and bending action
KL/r	Design ratio in case of section slenderness
δ	Design ratio in case of member deflection
OK	Capacity is provided
NG	Capacity is not provided

REFERENCES	CALCULATIONS	RESULTS																											
	<div>SkyCiv Foundation Design</div> <div>Pile Foundation</div> <div>Design Information :</div> <div>Design code : IBC 2021 (International Building Code)</div> <div>Unit System : Imperial</div>																												
	<div>Pile Input</div> <div></div> <div>Geometry</div> <div>Pile shape: rectangular</div> <div>b = 48 in - Pile width</div> <div>D = 48 in - Pile depth</div> <div>L = 6.25 ft - Total pile length</div> <div>h1 = 0 ft - Lateral load height from the top of the pile,</div> <div>h2 = 0 ft - Depth to resisting surface</div> <div>he = 0 ft - Length of pile above the ground</div> <div>Tabulation of Soil Parameters</div> <table><tr><th>Layer</th><th>Label</th><th>Allowable Bearing Pressure (qa) (psf)</th><th>Allowable Lateral Pressure (R) (psf/ft)</th></tr><tr><td>1</td><td>Sand, silty sand, clayey sand, silty gravel & clayey gravel</td><td>2000.000</td><td>150.000</td></tr></table> <div>Tabulation of Loads</div> <table><tr><th>Load Component</th><th>ASD</th><th>LRFD</th></tr><tr><td>P (kip)</td><td>4.635</td><td>6.640</td></tr><tr><td>Vx (kip)</td><td>-2.086</td><td>-3.477</td></tr><tr><td>Vz (kip)</td><td>-0.024</td><td>-0.040</td></tr><tr><td>Mx (kipft)</td><td>-0.079</td><td>-0.130</td></tr><tr><td>Mz (kipft)</td><td>23.792</td><td>40.393</td></tr></table> <div>Material Properties</div> <div>f'ck = 2.5 ksi - Concrete strength,</div>	Layer	Label	Allowable Bearing Pressure (qa) (psf)	Allowable Lateral Pressure (R) (psf/ft)	1	Sand, silty sand, clayey sand, silty gravel & clayey gravel	2000.000	150.000	Load Component	ASD	LRFD	P (kip)	4.635	6.640	Vx (kip)	-2.086	-3.477	Vz (kip)	-0.024	-0.040	Mx (kipft)	-0.079	-0.130	Mz (kipft)	23.792	40.393	<div>Required depth to resist lateral loads (ASD)</div> <div>H - Point of application of the lateral load</div> <div>$H = h_1 + h_2 + h_e$</div> <div>$H = (0 \text{ ft}) + (0 \text{ ft}) + (0 \text{ ft})$</div> <div>$H = 0 \text{ ft}$</div> <div>Considering x-direction:</div> <div>Ho - Lateral force per length of pile,</div> <div>$H_o = \frac{V_x}{1.57 \text{ } D}$</div> <div>$H_o = \frac{(-2.086 \text{ kip})}{1.57 \times (48 \text{ in})}$</div> <div>$H_o = -0.33217 \text{ kip/ft}$</div>	
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Mz (kipft)	23.792	40.393																											

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x H)}{1.57 D}$$

$$M_o = \frac{(23.792 \text{ kipft}) + ((-2.086 \text{ kip}) \times (0 \text{ ft}))}{1.57 \times (48 \text{ in})}$$

$$M_o = 3.7885 \text{ kipft/ft}$$

Required depth of embedment in earth:

$$L_x^3 - \left(14.14 \times \frac{H_o \times L_x}{R} \right) - \left(18.85 \times \frac{M_o}{R} \right) = 0$$

Solving the cubic equation:

$L_{e,x} = 5.7363 \text{ ft}$ - Required depth in x-direction,

Considering z-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57 b}$$

$$H_o = \frac{(-0.024 \text{ kip})}{1.57 \times (48 \text{ in})}$$

$$H_o = -0.0038217 \text{ kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z H)}{1.57 b}$$

$$M_o = \frac{(0.079 \text{ kipft}) + ((-0.024 \text{ kip}) \times (0 \text{ ft}))}{1.57 \times (48 \text{ in})}$$

$$M_o = 0.01258 \text{ kipft/ft}$$

Required depth of embedment in earth:

$$L_z^3 - \left(14.14 \times \frac{H_o \times L_z}{R} \right) - \left(18.85 \times \frac{M_o}{R} \right) = 0$$

Solving the cubic equation:

$L_{e,z} = 0.92605 \text{ ft}$ - Required depth in z-direction,

Minimum embedded depth required:

$L_{e,req}$ - Depth of pile required,

$$L_{e,req} = MAX[L_{e,x}, L_{e,z}]$$

$$L_{e,req} = MAX[(5.7363 \text{ ft}), (0.92605 \text{ ft})]$$

$$L_{e,req} = 5.736 \text{ ft}$$

L_e - Actual embedded length of pile,

$$L_e = L - h_e - h_2$$

$$L_e = (6.25 \text{ ft}) - (0 \text{ ft}) - (0 \text{ ft})$$

$$L_e = 6.25 \text{ ft}$$

Ratio - Embedded depth

$$Ratio = \frac{L_{e,req}}{L_e}$$

$$Ratio = \frac{(5.736 \text{ ft})}{(6.25 \text{ ft})}$$

$$Ratio = 0.91776$$

Status: **PASS**
Ratio: **0.920**

End-bearing Capacity (ASD)

A - Pile cross-section area

$$A = b D$$

$$A = (48 \text{ in}) \times (48 \text{ in})$$

$$A = 16 \text{ ft}^2$$

q - End-bearing pressure

	$q = \frac{P_v}{A}$ $q = \frac{(4.635 \text{ kip})}{(16 \text{ ft}^2)}$ $q = 0.28969 \text{ kip/ft}^2$ <p>Check bearing capacity ratio:</p> <p>Ratio - Capacity</p> $\text{Ratio} = \frac{q}{q_a}$ $\text{Ratio} = \frac{(0.28969 \text{ kip/ft}^2)}{(2000 \text{ psf})}$ $\text{Ratio} = 0.14484$	<p>Status: PASS Ratio: 0.140</p>
Czerniak	<p>Lateral Soil Pressure (ASD):</p> <p>L/D - Length to least lateral dimension ratio,</p> $L/D = \frac{L}{D}$ $L/D = \frac{(6.25 \text{ ft})}{(48 \text{ in})}$ $L/D = 1.5625$ <p>Since $L/D \leq 10$,</p> <p>Pile is short.</p> <p>Considering x-direction:</p> <p>$H_o = -0.33217 \text{ kip/ft}$ - Lateral force per length of pile, $M_o = 3.7885 \text{ kipft/ft}$ - Overturning moment per length of pile, a - Distance from resting surface to pivot point,</p> $a = \frac{(4 M_o L_e) + (3 H_o L_e^2)}{(6 M_o) + (4 H_o L_e)}$ $a = \frac{(4 \times (3.7885 \text{ kipft/ft}) \times (6.25 \text{ ft})) + (3 \times (-0.33217 \text{ kip/ft}) \times (6.25 \text{ ft})^2)}{(6 \times (3.7885 \text{ kipft/ft})) + (4 \times (-0.33217 \text{ kip/ft}) \times (6.25 \text{ ft}))}$ $a = 4.306 \text{ ft}$ <p>p - Earth pressure against the pile at distance $a/2$ from resting surface,</p> $p = \frac{0.75 [(4 M_o) + (3 H_o L_e)]^2}{L_e^2 [(3 M_o) + (2 H_o L_e)]}$ $p = \frac{0.75 \times [(4 \times (3.7885 \text{ kipft/ft})) + (3 \times (-0.33217 \text{ kip/ft}) \times (6.25 \text{ ft}))]^2}{(6.25 \text{ ft})^2 \times [(3 \times (3.7885 \text{ kipft/ft})) + (2 \times (-0.33217 \text{ kip/ft}) \times (6.25 \text{ ft}))]}$ $p = 0.21207 \text{ kip/ft}^2$ <p>s - Earth pressure against the pile at distance L_e,</p> $s = \frac{6 [(2 M_o) + (H_o L_e)]}{L_e^2}$ $s = \frac{6 \times [(2 \times (3.7885 \text{ kipft/ft})) + ((-0.33217 \text{ kip/ft}) \times (6.25 \text{ ft}))]}{(6.25 \text{ ft})^2}$ $s = 0.84496 \text{ kip/ft}^2$ <p>Check lateral soil pressure capacity:</p> <p>p_a - Allowable lateral soil pressure at depth $a/2$,</p> $p_a = R \frac{a}{2}$ $p_a = (150 \text{ psf/ft}) \times \frac{(4.306 \text{ ft})}{2}$ $p_a = 0.32295 \text{ kip/ft}^2$ <p>Ratio - Lateral soil capacity</p>	

	$Ratio = \frac{p}{p_a}$ $Ratio = \frac{(0.21207 \text{ kip/ft}^2)}{(0.32295 \text{ kip/ft}^2)}$ $Ratio = 0.65665$ <p>p_s - Allowable lateral soil pressure at depth L_e,</p> $p_s = R L_e$ $p_s = (150 \text{ psf/ft}) \times (6.25 \text{ ft})$ $p_s = 0.9375 \text{ kip/ft}^2$ <p>Ratio - Lateral soil capacity</p> $Ratio = \frac{s}{p_s}$ $Ratio = \frac{(0.84496 \text{ kip/ft}^2)}{(0.9375 \text{ kip/ft}^2)}$ $Ratio = 0.90129$	<p>Status: PASS Ratio: 0.660</p> <p>Status: PASS Ratio: 0.900</p>
	<p>Considering z-direction:</p> <p>$H_o = -0.0038217 \text{ kip/ft}$ - Lateral force per length of pile, $M_o = 0.01258 \text{ kipft/ft}$ - Overturning moment per length of pile, a - Distance from resting surface to pivot point,</p> $a = \frac{(4 M_o L_e) + (3 H_o L_e^2)}{(6 M_o) + (4 H_o L_e)}$ $a = \frac{(4 \times (0.01258 \text{ kipft/ft}) \times (6.25 \text{ ft})) + (3 \times (-0.0038217 \text{ kip/ft}) \times (6.25 \text{ ft})^2)}{(6 \times (0.01258 \text{ kipft/ft})) + (4 \times (-0.0038217 \text{ kip/ft}) \times (6.25 \text{ ft}))}$ $a = 4.4576 \text{ ft}$ <p>p - Earth pressure against the pile at distance $a/2$ from resting surface,</p> $p = \frac{0.75 [(4 M_o) + (3 H_o L_e)]^2}{L_e^2 [(3 M_o) + (2 H_o L_e)]}$ $p = \frac{0.75 \times [(4 \times (0.01258 \text{ kipft/ft})) + (3 \times (-0.0038217 \text{ kip/ft}) \times (6.25 \text{ ft}))]^2}{(6.25 \text{ ft})^2 \times [(3 \times (0.01258 \text{ kipft/ft})) + (2 \times (-0.0038217 \text{ kip/ft}) \times (6.25 \text{ ft}))]}$ $p = -0.00087139 \text{ kip/ft}^2$ <p>s - Earth pressure against the pile at distance L_e,</p> $s = \frac{6 [(2 M_o) + (H_o L_e)]}{L_e^2}$ $s = \frac{6 \times [(2 \times (0.01258 \text{ kipft/ft})) + ((-0.0038217 \text{ kip/ft}) \times (6.25 \text{ ft}))]}{(6.25 \text{ ft})^2}$ $s = 0.00019567 \text{ kip/ft}^2$ <p>Check lateral soil pressure capacity:</p> <p>p_a - Allowable lateral soil pressure at depth $a/2$,</p> $p_a = R \frac{a}{2}$ $p_a = (150 \text{ psf/ft}) \times \frac{(4.4576 \text{ ft})}{2}$ $p_a = 0.33432 \text{ kip/ft}^2$ <p>Ratio - Lateral soil capacity</p> $Ratio = \frac{p}{p_a}$ $Ratio = \frac{(-0.00087139 \text{ kip/ft}^2)}{(0.33432 \text{ kip/ft}^2)}$	

$$Ratio = -0.0026064$$

Status: **PASS**
Ratio: **0.000**

p_s - Allowable lateral soil pressure at depth L_e ,

$$p_s = R L_e$$

$$p_s = (150 \text{ psf/ft}) \times (6.25 \text{ ft})$$

$$p_s = 0.9375 \text{ kip/ft}^2$$

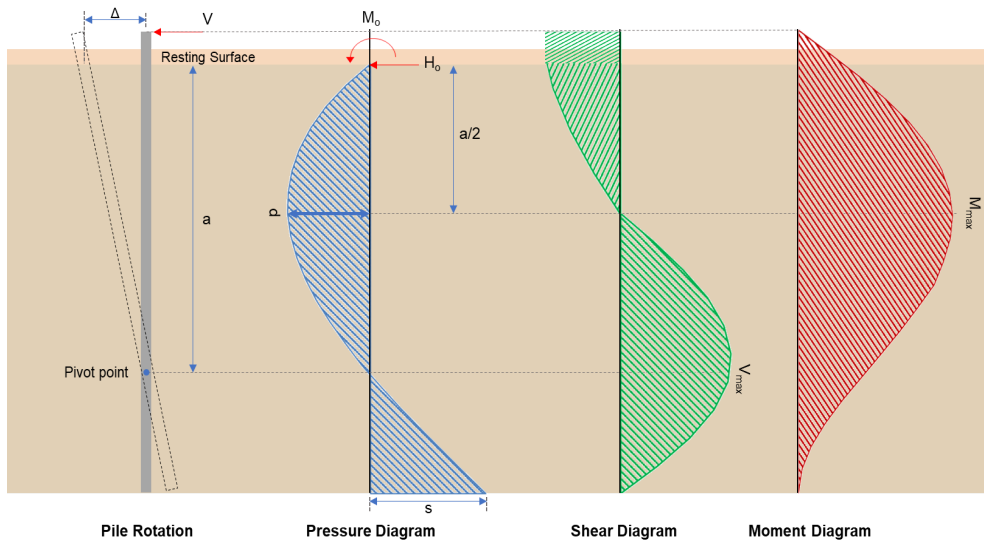
Ratio - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{(0.00019567 \text{ kip/ft}^2)}{(0.9375 \text{ kip/ft}^2)}$$

$$Ratio = 0.00020871$$

Status: **PASS**
Ratio: **0.000**



Shear force and Bending moment (x-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57 D}$$

$$H_o = \frac{(-3.477 \text{ kip})}{1.57 \times (48 \text{ in})}$$

$$H_o = -0.55366 \text{ kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x H)}{1.57 D}$$

$$M_o = \frac{(40.393 \text{ kipft}) + ((-3.477 \text{ kip}) \times (0 \text{ ft}))}{1.57 \times (48 \text{ in})}$$

$$M_o = 6.432 \text{ kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(6.432 \text{ kipft/ft})}{(-0.55366 \text{ kip/ft})}$$

$$E = 11.617 \text{ ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4 M_o L_e) + (3 H_o L_e^2)}{(6 M_o) + (4 H_o L_e)}$$

$$a = \frac{(4 \times (6.432 \text{ kipft/ft}) \times (6.25 \text{ ft})) + (3 \times (-0.55366 \text{ kip/ft}) \times (6.25 \text{ ft})^2)}{(6 \times 6.432 \text{ kipft/ft}) + (4 \times (-0.55366 \text{ kip/ft}) \times 6.25 \text{ ft})}$$

$$a = \frac{(-0.55366 \text{ kip/ft}) \times (48 \text{ in}) \times (6.25 \text{ ft})}{(6 \times (6.432 \text{ kip/ft})) + (4 \times (-0.55366 \text{ kip/ft}) \times (6.25 \text{ ft}))}$$

$$a = 4.3042 \text{ ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o D) \left[1 - \left[3 \left(\frac{4 E}{L_e} + 3 \right) \left(\frac{a}{L_e} \right)^2 \right] + \left[4 \left(\frac{3 E}{L_e} + 2 \right) \left(\frac{a}{L_e} \right)^3 \right] \right]$$

$$V_{max} = ((-0.55366 \text{ kip/ft}) \times (48 \text{ in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (11.617 \text{ ft})}{(6.25 \text{ ft})} + 3 \right) \times \left(\frac{(4.3042 \text{ ft})}{(6.25 \text{ ft})} \right)^2 \right] + \left[4 \times \left(\frac{3 \times (11.617 \text{ ft})}{(6.25 \text{ ft})} + 2 \right) \times \left(\frac{(4.3042 \text{ ft})}{(6.25 \text{ ft})} \right)^3 \right] \right]$$

$$V_{max} = 8.7454 \text{ kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o D L_e) \left[\left(\frac{E}{L_e} + \frac{a}{2 L_e} \right) - \left[\left(\frac{4 E}{L_e} + 3 \right) \left(\frac{a}{2 L_e} \right)^3 \right] + \left[\left(\frac{3 E}{L_e} + 2 \right) \left(\frac{a}{2 L_e} \right)^4 \right] \right]$$

$$M_{max} = ((-0.55366 \text{ kip/ft}) \times (48 \text{ in}) \times (6.25 \text{ ft})) \times \left[\left(\frac{(11.617 \text{ ft})}{(6.25 \text{ ft})} + \frac{(4.3042 \text{ ft})}{2 \times (6.25 \text{ ft})} \right) - \left[\left(\frac{4 \times (11.617 \text{ ft})}{(6.25 \text{ ft})} + 3 \right) \times \left(\frac{(4.3042 \text{ ft})}{2 \times (6.25 \text{ ft})} \right)^3 \right] + \left[\left(\frac{3 \times (11.617 \text{ ft})}{(6.25 \text{ ft})} + 2 \right) \times \left(\frac{(4.3042 \text{ ft})}{2 \times (6.25 \text{ ft})} \right)^4 \right] \right]$$

$$M_{max} = 26.072 \text{ kipft}$$

Shear force and Bending moment (z-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57 b}$$

$$H_o = \frac{(-0.04 \text{ kip})}{1.57 \times (48 \text{ in})}$$

$$H_o = -0.0063694 \text{ kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z H)}{1.57 b}$$

$$M_o = \frac{(0.13 \text{ kipft}) + ((-0.04 \text{ kip}) \times (0 \text{ ft}))}{1.57 \times (48 \text{ in})}$$

$$M_o = 0.020701 \text{ kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(0.020701 \text{ kipft/ft})}{(-0.0063694 \text{ kip/ft})}$$

$$E = 3.25 \text{ ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4 M_o L_e) + (3 H_o L_e^2)}{(6 M_o) + (4 H_o L_e)}$$

$$a = \frac{(4 \times (0.020701 \text{ kipft/ft}) \times (6.25 \text{ ft})) + (3 \times (-0.0063694 \text{ kip/ft}) \times (6.25 \text{ ft})^2)}{(6 \times (0.020701 \text{ kipft/ft})) + (4 \times (-0.0063694 \text{ kip/ft}) \times (6.25 \text{ ft}))}$$

$$a = 4.4593 \text{ ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o b) \left[1 - \left[3 \left(\frac{4 E}{L_e} + 3 \right) \left(\frac{a}{L_e} \right)^2 \right] + \left[4 \left(\frac{3 E}{L_e} + 2 \right) \left(\frac{a}{L_e} \right)^3 \right] \right]$$

$$V_{max} = ((-0.0063694 \text{ kip/ft}) \times (48 \text{ in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (3.25 \text{ ft})}{(6.25 \text{ ft})} + 3 \right) \times \left(\frac{(4.4593 \text{ ft})}{(6.25 \text{ ft})} \right)^2 \right] + \left[4 \times \left(\frac{3 \times (3.25 \text{ ft})}{(6.25 \text{ ft})} + 2 \right) \times \left(\frac{(4.4593 \text{ ft})}{(6.25 \text{ ft})} \right)^3 \right] \right]$$

$$V_{max} = 0.040408 \text{ kip}$$

M_{max} - Max bending moment located at depth $a/2$,

$$M_{max} = (H_o \ b \ L_e) \left[\left(\frac{E}{L_e} + \frac{a}{2 L_e} \right) - \left[\left(\frac{4 E}{L_e} + 3 \right) \left(\frac{a}{2 L_e} \right)^3 \right] + \left[\left(\frac{3 E}{L_e} + 2 \right) \left(\frac{a}{2 L_e} \right)^4 \right] \right]$$

$$M_{max} = ((-0.0063694 \text{ kip/ft}) \times (48 \text{ in}) \times (6.25 \text{ ft})) \times \left[\left(\frac{(3.25 \text{ ft})}{(6.25 \text{ ft})} + \frac{(4.4593 \text{ ft})}{2 \times (6.25 \text{ ft})} \right) - \left[\left(\frac{4 \times (3.25 \text{ ft})}{(6.25 \text{ ft})} + 3 \right) \times \left(\frac{(4.4593 \text{ ft})}{2 \times (6.25 \text{ ft})} \right)^3 \right] + \left[\left(\frac{3 \times (3.25 \text{ ft})}{(6.25 \text{ ft})} + 2 \right) \times \left(\frac{(4.4593 \text{ ft})}{2 \times (6.25 \text{ ft})} \right)^4 \right] \right]$$

$$M_{max} = 0.11206 \text{ kipft}$$

Minimum Reinforcement Check (LRFD)

Parameters:

$f'_{ck} = 2.5 \text{ ksi}$ - Concrete strength,

$f_{yk} = 60 \text{ ksi}$ - Longitudinal reinforcement strength,

$\phi = 0.65$ - Reduction factor for axial strength,

$\alpha = 0.8$ - Alpha factor for axial strength,

$A_g = 2304 \text{ in}^2$ - Gross area of concrete,

Longitudinal reinforcement:

Required reinforcement due to axial load, $A_{st,required}$

$A_{st,required}$

$$A_{st,required} = \text{Min} \left[\frac{\frac{P}{\phi \alpha} - (0.85 f'_{ck} A_g)}{f_{yk} - (0.85 f'_{ck})}, (0.08 A_g) \right]$$

$$A_{st,required} = \text{Min} \left[\frac{\frac{(6.64 \text{ kip})}{(0.65) \times (0.8)} - (0.85 \times (2.5 \text{ ksi}) \times (2304 \text{ in}^2))}{(60 \text{ ksi}) - (0.85 \times (2.5 \text{ ksi}))}, (0.08 \times (2304 \text{ in}^2)) \right]$$

$$A_{st,required} = -84.375 \text{ in}^2$$

A_{min} - Governing minimum reinforcement area,

$$A_{min} = \text{Max} [A_{st,required}, (0.0018 A_g)]$$

$$A_{min} = \text{Max} [(-84.375 \text{ in}^2), (0.0018 \times (2304 \text{ in}^2))]$$

$$A_{min} = 4.1472 \text{ in}^2$$

n_{rebar} - Required number of reinforcement,

$$n_{rebar} = \frac{A_{min}}{A_{rebar}}$$

$$n_{rebar} = \frac{(4.1472 \text{ in}^2)}{(0.3068 \text{ in}^2)}$$

$$n_{rebar} = 14$$

A_{st} - Actual total reinforcement area,

$$A_{st} = n_{rebar} \frac{\pi d_{bar}^2}{4}$$

$$A_{st} = (14) \times \frac{\pi \times (0.625 \text{ in})^2}{4}$$

$$A_{st} = 4.2951 \text{ in}^2$$

Ratio - Capacity

$$\text{Ratio} = \frac{A_{min}}{A_{st}}$$

$$\text{Ratio} = \frac{(4.1472 \text{ in}^2)}{(4.2951 \text{ in}^2)}$$

Table 22.4.2.1

22.4.2.2, 10.6.1.1

<p>25.2.3</p>	<p style="text-align: center;">$Ratio = 0.96556$</p> <p>s_{rebar} - Minimum spacing of reinforcement,</p> $s_{rebar} = Max[1.5, (1.5 d_{bar})]$ $s_{rebar} = Max[1.5, (1.5 \times (0.625 \text{ in}))]$ $s_{rebar} = 1.5 \text{ in}$ <p>Ties:</p> <p>25.7.2.2 Since longitudinal reinforcement is \leq No. 10ø: Use #3(0.375 in)</p> <p>25.7.2.1 s_{ties} - Maximum spacing of ties,</p> $s_{ties} = Min[(16 d_{bar}), (48 d_{ties}), Min(D, b)]$ $s_{ties} = Min[(16 \times (0.625 \text{ in})), (48 \times (0.375 \text{ in})), Min((48 \text{ in}), (48 \text{ in}))]$ $s_{ties} = 10 \text{ in}$ <p>Summary:</p> <p style="text-align: center;">Main reinforcement: 14 - #5 (0.625 in) Ties: #3(0.375 in) - 10 in</p>	<p>Status: PASS Ratio: 0.970</p>
<p>22.4.2.2</p>	<p>Axial Compression Strength (ACI 318-19, LRFD)</p> <p>ϕP_N - Allowable axial compressive strength</p> $\phi P_N = \phi 0.80 [(0.85 f'_{ck} [A_g - A_{st}]) + (f_{yk} A_{st})]$ $\phi P_N = (0.65) \times 0.80 \times [(0.85 \times (2.5 \text{ ksi}) \times [(2304 \text{ in}^2) - (4.2951 \text{ in}^2)]) + ((60 \text{ ksi}) \times (4.2951 \text{ in}^2))]$ $\phi P_N = 2675.2 \text{ kip}$ <p>$Ratio$ - Capacity</p> $Ratio = \frac{P}{\phi P_N}$ $Ratio = \frac{(6.64 \text{ kip})}{(2675.2 \text{ kip})}$ $Ratio = 0.0024821$	<p>Status: PASS Ratio: 0.000</p>
<p>22.5.2.2</p> <p>22.5.1.3</p> <p>22.5.1.1</p>	<p>Shear Strength (ACI 318-19, LRFD)</p> <p>Parameters:</p> <p>$b_w = 48 \text{ in}$ - Effective width, d - Effective depth</p> $d = 0.80 D$ $d = 0.80 \times (48 \text{ in})$ $d = 38.4 \text{ in}$ <p>λ_s - size effect modification factor</p> $\lambda_s = MIN \left[\sqrt{\frac{2}{1 + \frac{d}{10}}}, 1 \right]$ $\lambda_s = MIN \left[\sqrt{\frac{2}{1 + \frac{(38.4 \text{ in})}{10}}}, 1 \right]$ $\lambda_s = 0.64282$ <p>The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$,</p> <p>$V_{c,max}$ - Max shear strength of concrete</p> $V_{c,max} = 5 \lambda_s \sqrt{f'_{ck}} b_w d$ $V_{c,max} = 5 \times (0.64282) \times \sqrt{(2500 \text{ psi})} \times (48 \text{ in}) \times (38.4 \text{ in})$	

$$V_{c,max} = 296.21 \text{ kip}$$

22.5.5.1.1(a) The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$, $P = 6.64 \text{ kip} \rightarrow 6640 \text{ lbf}$,
 $V_{c,a}$ - Shear strength of concrete (a)

$$V_{c,a} = \left[2 \lambda_s \sqrt{f'_{ck}} + \frac{P}{6 A_g} \right] b_w d$$

$$V_{c,a} = \left[2 \times (0.64282) \times \sqrt{(2500 \text{ psi})} + \frac{(6640 \text{ lbf})}{6 \times (2304 \text{ in}^2)} \right] \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{c,a} = 119.37 \text{ kip}$$

22.5.5.1.2 The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$,
 $V_{c,b}$ - Shear strength of concrete (b)

$$V_{c,b} = \left[2 \lambda_s \sqrt{f'_{ck}} + (0.05 f'_{ck}) \right] b_w d$$

$$V_{c,b} = \left[2 \times (0.64282) \times \sqrt{(2500 \text{ psi})} + (0.05 \times (2500 \text{ psi})) \right] \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{c,b} = 348.89 \text{ kip}$$

V_c - Governing shear strength of concrete

$$V_c = \text{Min}[V_{c,max}, V_{c,a}, V_{c,b}]$$

$$V_c = \text{Min}[(296.21 \text{ kip}), (119.37 \text{ kip}), (348.89 \text{ kip})]$$

$$V_c = 119.37 \text{ kip}$$

22.5.1.2 The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$,
 $V_{s,a}$ - Shear strength of steel (a)

$$V_{s,a} = 8 \sqrt{f'_{ck}} b_w d$$

$$V_{s,a} = 8 \times \sqrt{(2500 \text{ psi})} \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{s,a} = 737.28 \text{ kip}$$

A_v - Ties rebar area,

$$A_v = \frac{\pi d_{ties}^2}{4}$$

$$A_v = \frac{\pi \times (0.375 \text{ in})^2}{4}$$

$$A_v = 0.11045 \text{ in}^2$$

22.5.8.5.3 $V_{s,b}$ - Shear strength of steel (b)

$$V_{s,b} = \frac{2 A_v f_{yw} d}{s_{ties}}$$

$$V_{s,b} = \frac{2 \times (0.11045 \text{ in}^2) \times (60 \text{ ksi}) \times (38.4 \text{ in})}{(10 \text{ in})}$$

$$V_{s,b} = 50.894 \text{ kip}$$

V_s - Governing shear strength of steel

$$V_s = \text{MIN}[V_{s,a}, V_{s,b}]$$

$$V_s = \text{MIN}[(737.28 \text{ kip}), (50.894 \text{ kip})]$$

$$V_s = 50.894 \text{ kip}$$

22.5.1.1 ϕV_n - Allowable shear strength

$$\phi V_n = \phi (V_c + V_s)$$

$$\phi V_n = (0.65) \times ((119.37 \text{ kip}) + (50.894 \text{ kip}))$$

$$\phi V_n = 110.67 \text{ kip}$$

Considering x-direction:

$V_{max} = 8.7454 \text{ kip}$ - Maximum shear force in the x-direction,
 $Ratio$ - Capacity

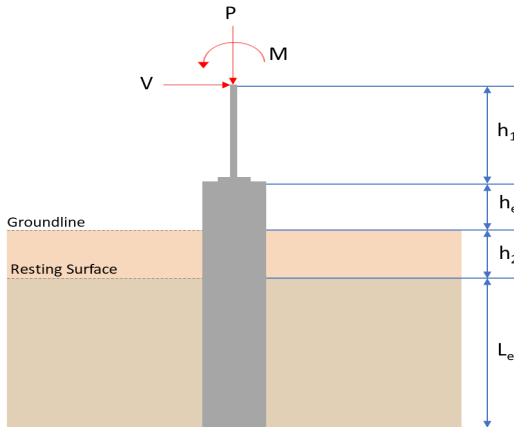
$$Ratio = \frac{V_{max}}{\phi V_n}$$

	$Ratio = \frac{(8.7454 \text{ kip})}{(110.67 \text{ kip})}$ $Ratio = 0.079021$ <p>Considering z-direction:</p> <p>$V_{max} = 0.040408 \text{ kip}$ - Maximum shear force in the z-direction, <i>Ratio</i> - Capacity</p> $Ratio = \frac{V_{max}}{\phi V_n}$ $Ratio = \frac{(0.040408 \text{ kip})}{(110.67 \text{ kip})}$ $Ratio = 0.00036512$ <p>Status: PASS Ratio: 0.080</p>	
14.5.2.1b	<p>Flexural Strength (ACI 318-19, LRFD)</p> <p>S_m - Section modulus</p> $S_m = \frac{b D^2}{6}$ $S_m = \frac{(48 \text{ in}) \times (48 \text{ in})^2}{6}$ $S_m = 18432 \text{ in}^3$ <p>$\lambda = 1$ - Concrete modification factor (Normal concrete), Allowable flexural strength: M_n shall be the lesser of:</p> <p>$\phi M_{n,1}$</p> $\phi M_{n,1} = \phi \times 5 \times \lambda \times \sqrt{f'_c} \times S_m$ $\phi M_{n,1} = 0.65 \times 5 \times 1 \times \sqrt{(2.5 \text{ ksi})} \times 18432.001 \text{ in}^3$ $\phi M_{n,1} = 249.600 \text{ kipft}$ <p>$\phi M_{n,2}$</p> $\phi M_{n,2} = \phi \times 0.85 \times f'_c \times S_m$ $\phi M_{n,2} = (0.65) \times 0.85 \times (2.5 \text{ ksi}) \times (18432 \text{ in}^3)$ $\phi M_{n,2} = 2121.6 \text{ kipft}$ <p>Therefore, ϕM_n - Allowable flexural strength,</p> $\phi M_n = MIN[\phi M_{n,1}, \phi M_{n,2}]$ $\phi M_n = MIN[(249.6 \text{ kipft}), (2121.6 \text{ kipft})]$ $\phi M_n = 249.6 \text{ kipft}$ <p>Considering x-direction:</p> <p>$M_{max} = 26.072 \text{ kipft}$ - Maximum moment in the x-direction, <i>Ratio</i> - Capacity</p> $Ratio = \frac{M_{max}}{\phi M_n}$ $Ratio = \frac{(26.072 \text{ kipft})}{(249.6 \text{ kipft})}$ $Ratio = 0.10445$ <p>Status: PASS Ratio: 0.100</p>	
	<p>Considering z-direction:</p> <p>$M_{max} = 0.11206 \text{ kipft}$ - Maximum moment in the z-direction, <i>Ratio</i> - Capacity</p> $Ratio = \frac{M_{max}}{\phi M_n}$	

$$Ratio = \frac{(0.11206 \text{ kipft})}{(249.6 \text{ kipft})}$$

$$Ratio = 0.00044898$$

Status: **PASS**
Ratio: **0.000**

REFERENCES	CALCULATIONS	RESULTS																											
	<div>SkyCiv Foundation Design</div> <div>Pile Foundation</div> <div>Design Information :</div> <div>Design code : IBC 2021 (International Building Code)</div> <div>Unit System : Imperial</div>																												
	<div>Pile Input</div> <div></div> <div>Geometry</div> <div>Pile shape: rectangular</div> <div>b = 48 in - Pile width</div> <div>D = 48 in - Pile depth</div> <div>L = 6.25 ft - Total pile length</div> <div>h1 = 0 ft - Lateral load height from the top of the pile,</div> <div>h2 = 0 ft - Depth to resisting surface</div> <div>he = 0 ft - Length of pile above the ground</div> <div>Tabulation of Soil Parameters</div> <table><tr><th>Layer</th><th>Label</th><th>Allowable Bearing Pressure (qa) (psf)</th><th>Allowable Lateral Pressure (R) (psf/ft)</th></tr><tr><td>1</td><td>Sand, silty sand, clayey sand, silty gravel & clayey gravel</td><td>2000.000</td><td>150.000</td></tr></table> <div>Tabulation of Loads</div> <table><tr><th>Load Component</th><th>ASD</th><th>LRFD</th></tr><tr><td>P (kip)</td><td>4.635</td><td>6.640</td></tr><tr><td>Vx (kip)</td><td>-2.086</td><td>-3.477</td></tr><tr><td>Vz (kip)</td><td>0.024</td><td>0.040</td></tr><tr><td>Mx (kipft)</td><td>0.079</td><td>0.130</td></tr><tr><td>Mz (kipft)</td><td>23.792</td><td>40.394</td></tr></table> <div>Material Properties</div> <div>f'ck = 2.5 ksi - Concrete strength,</div>	Layer	Label	Allowable Bearing Pressure (qa) (psf)	Allowable Lateral Pressure (R) (psf/ft)	1	Sand, silty sand, clayey sand, silty gravel & clayey gravel	2000.000	150.000	Load Component	ASD	LRFD	P (kip)	4.635	6.640	Vx (kip)	-2.086	-3.477	Vz (kip)	0.024	0.040	Mx (kipft)	0.079	0.130	Mz (kipft)	23.792	40.394	<div>Required depth to resist lateral loads (ASD)</div> <div>H - Point of application of the lateral load</div> <div>$H = h_1 + h_2 + h_e$</div> <div>$H = (0 \text{ ft}) + (0 \text{ ft}) + (0 \text{ ft})$</div> <div>$H = 0 \text{ ft}$</div> <div>Considering x-direction:</div> <div>Ho - Lateral force per length of pile,</div> <div>$H_o = \frac{V_x}{1.57 \, D}$</div> <div>$H_o = \frac{(-2.086 \text{ kip})}{1.57 \times (48 \text{ in})}$</div> <div>$H_o = -0.33217 \text{ kip/ft}$</div>	
Layer	Label	Allowable Bearing Pressure (qa) (psf)	Allowable Lateral Pressure (R) (psf/ft)																										
1	Sand, silty sand, clayey sand, silty gravel & clayey gravel	2000.000	150.000																										
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Vz (kip)	0.024	0.040																											
Mx (kipft)	0.079	0.130																											
Mz (kipft)	23.792	40.394																											

	<p>M_o - Moment per length of pile,</p> $M_o = \frac{M_z + (V_x H)}{1.57 D}$ $M_o = \frac{(23.792 \text{ kipft}) + ((-2.086 \text{ kip}) \times (0 \text{ ft}))}{1.57 \times (48 \text{ in})}$ $M_o = 3.7885 \text{ kipft/ft}$ <p>Required depth of embedment in earth:</p> $L_x^3 - \left(14.14 \times \frac{H_o \times L_x}{R} \right) - \left(18.85 \times \frac{M_o}{R} \right) = 0$ <p>Solving the cubic equation: $L_{e,x} = 5.7363 \text{ ft}$ - Required depth in x-direction,</p> <p>Considering z-direction:</p> <p>H_o - Lateral force per length of pile,</p> $H_o = \frac{V_z}{1.57 b}$ $H_o = \frac{(0.024 \text{ kip})}{1.57 \times (48 \text{ in})}$ $H_o = 0.0038217 \text{ kip/ft}$ <p>M_o - Moment per length of pile,</p> $M_o = \frac{M_x + (V_z H)}{1.57 b}$ $M_o = \frac{(0.079 \text{ kipft}) + ((0.024 \text{ kip}) \times (0 \text{ ft}))}{1.57 \times (48 \text{ in})}$ $M_o = 0.01258 \text{ kipft/ft}$ <p>Required depth of embedment in earth:</p> $L_z^3 - \left(14.14 \times \frac{H_o \times L_z}{R} \right) - \left(18.85 \times \frac{M_o}{R} \right) = 0$ <p>Solving the cubic equation: $L_{e,z} = 1.0782 \text{ ft}$ - Required depth in z-direction,</p> <p>Minimum embedded depth required:</p> <p>$L_{e,req}$ - Depth of pile required,</p> $L_{e,req} = \text{MAX}[L_{e,x}, L_{e,z}]$ $L_{e,req} = \text{MAX}[(5.7363 \text{ ft}), (1.0782 \text{ ft})]$ $L_{e,req} = 5.736 \text{ ft}$ <p>L_e - Actual embedded length of pile,</p> $L_e = L - h_e - h_2$ $L_e = (6.25 \text{ ft}) - (0 \text{ ft}) - (0 \text{ ft})$ $L_e = 6.25 \text{ ft}$ <p>Ratio - Embedded depth</p> $\text{Ratio} = \frac{L_{e,req}}{L_e}$ $\text{Ratio} = \frac{(5.736 \text{ ft})}{(6.25 \text{ ft})}$ $\text{Ratio} = 0.91776$	<p>Status: PASS Ratio: 0.920</p>
	<p>End-bearing Capacity (ASD)</p> <p>A - Pile cross-section area</p> $A = b D$ $A = (48 \text{ in}) \times (48 \text{ in})$ $A = 16 \text{ ft}^2$ <p>q - End-bearing pressure</p>	

	$q = \frac{P_v}{A}$ $q = \frac{(4.635 \text{ kip})}{(16 \text{ ft}^2)}$ $q = 0.28969 \text{ kip/ft}^2$ <p>Check bearing capacity ratio:</p> <p>Ratio - Capacity</p> $\text{Ratio} = \frac{q}{q_a}$ $\text{Ratio} = \frac{(0.28969 \text{ kip/ft}^2)}{(2000 \text{ psf})}$ $\text{Ratio} = 0.14484$	<p>Status: PASS Ratio: 0.140</p>
Czerniak	<p>Lateral Soil Pressure (ASD):</p> <p>L/D - Length to least lateral dimension ratio,</p> $L/D = \frac{L}{D}$ $L/D = \frac{(6.25 \text{ ft})}{(48 \text{ in})}$ $L/D = 1.5625$ <p>Since $L/D \leq 10$,</p> <p>Pile is short.</p> <p>Considering x-direction:</p> <p>$H_o = -0.33217 \text{ kip/ft}$ - Lateral force per length of pile, $M_o = 3.7885 \text{ kipft/ft}$ - Overturning moment per length of pile, a - Distance from resting surface to pivot point,</p> $a = \frac{(4 M_o L_e) + (3 H_o L_e^2)}{(6 M_o) + (4 H_o L_e)}$ $a = \frac{(4 \times (3.7885 \text{ kipft/ft}) \times (6.25 \text{ ft})) + (3 \times (-0.33217 \text{ kip/ft}) \times (6.25 \text{ ft})^2)}{(6 \times (3.7885 \text{ kipft/ft})) + (4 \times (-0.33217 \text{ kip/ft}) \times (6.25 \text{ ft}))}$ $a = 4.306 \text{ ft}$ <p>p - Earth pressure against the pile at distance $a/2$ from resting surface,</p> $p = \frac{0.75 [(4 M_o) + (3 H_o L_e)]^2}{L_e^2 [(3 M_o) + (2 H_o L_e)]}$ $p = \frac{0.75 \times [(4 \times (3.7885 \text{ kipft/ft})) + (3 \times (-0.33217 \text{ kip/ft}) \times (6.25 \text{ ft}))]^2}{(6.25 \text{ ft})^2 \times [(3 \times (3.7885 \text{ kipft/ft})) + (2 \times (-0.33217 \text{ kip/ft}) \times (6.25 \text{ ft}))]}$ $p = 0.21207 \text{ kip/ft}^2$ <p>s - Earth pressure against the pile at distance L_e,</p> $s = \frac{6 [(2 M_o) + (H_o L_e)]}{L_e^2}$ $s = \frac{6 \times [(2 \times (3.7885 \text{ kipft/ft})) + ((-0.33217 \text{ kip/ft}) \times (6.25 \text{ ft}))]}{(6.25 \text{ ft})^2}$ $s = 0.84496 \text{ kip/ft}^2$ <p>Check lateral soil pressure capacity:</p> <p>p_a - Allowable lateral soil pressure at depth $a/2$,</p> $p_a = R \frac{a}{2}$ $p_a = (150 \text{ psf/ft}) \times \frac{(4.306 \text{ ft})}{2}$ $p_a = 0.32295 \text{ kip/ft}^2$ <p>Ratio - Lateral soil capacity</p>	

	$Ratio = \frac{p}{p_a}$ $Ratio = \frac{(0.21207 \text{ kip/ft}^2)}{(0.32295 \text{ kip/ft}^2)}$ $Ratio = 0.65665$ <p>p_s - Allowable lateral soil pressure at depth L_e,</p> $p_s = R L_e$ $p_s = (150 \text{ psf/ft}) \times (6.25 \text{ ft})$ $p_s = 0.9375 \text{ kip/ft}^2$ <p>Ratio - Lateral soil capacity</p> $Ratio = \frac{s}{p_s}$ $Ratio = \frac{(0.84496 \text{ kip/ft}^2)}{(0.9375 \text{ kip/ft}^2)}$ $Ratio = 0.90129$	<p>Status: PASS Ratio: 0.660</p> <p>Status: PASS Ratio: 0.900</p>
	<p>Considering z-direction:</p> <p>$H_o = 0.0038217 \text{ kip/ft}$ - Lateral force per length of pile, $M_o = 0.01258 \text{ kipft/ft}$ - Overturning moment per length of pile, a - Distance from resting surface to pivot point,</p> $a = \frac{(4 M_o L_e) + (3 H_o L_e^2)}{(6 M_o) + (4 H_o L_e)}$ $a = \frac{(4 \times (0.01258 \text{ kipft/ft}) \times (6.25 \text{ ft})) + (3 \times (0.0038217 \text{ kip/ft}) \times (6.25 \text{ ft})^2)}{(6 \times (0.01258 \text{ kipft/ft})) + (4 \times (0.0038217 \text{ kip/ft}) \times (6.25 \text{ ft}))}$ $a = 4.4576 \text{ ft}$ <p>p - Earth pressure against the pile at distance $a/2$ from resting surface,</p> $p = \frac{0.75 [(4 M_o) + (3 H_o L_e)]^2}{L_e^2 [(3 M_o) + (2 H_o L_e)]}$ $p = \frac{0.75 \times [(4 \times (0.01258 \text{ kipft/ft})) + (3 \times (0.0038217 \text{ kip/ft}) \times (6.25 \text{ ft}))]^2}{(6.25 \text{ ft})^2 \times [(3 \times (0.01258 \text{ kipft/ft})) + (2 \times (0.0038217 \text{ kip/ft}) \times (6.25 \text{ ft}))]}$ $p = 0.0033406 \text{ kip/ft}^2$ <p>s - Earth pressure against the pile at distance L_e,</p> $s = \frac{6 [(2 M_o) + (H_o L_e)]}{L_e^2}$ $s = \frac{6 \times [(2 \times (0.01258 \text{ kipft/ft})) + ((0.0038217 \text{ kip/ft}) \times (6.25 \text{ ft}))]}{(6.25 \text{ ft})^2}$ $s = 0.0075332 \text{ kip/ft}^2$ <p>Check lateral soil pressure capacity:</p> <p>p_a - Allowable lateral soil pressure at depth $a/2$,</p> $p_a = R \frac{a}{2}$ $p_a = (150 \text{ psf/ft}) \times \frac{(4.4576 \text{ ft})}{2}$ $p_a = 0.33432 \text{ kip/ft}^2$ <p>Ratio - Lateral soil capacity</p> $Ratio = \frac{p}{p_a}$ $Ratio = \frac{(0.0033406 \text{ kip/ft}^2)}{(0.33432 \text{ kip/ft}^2)}$	

$$Ratio = 0.0099922$$

Status: **PASS**
Ratio: **0.010**

p_s - Allowable lateral soil pressure at depth L_e ,

$$p_s = R L_e$$

$$p_s = (150 \text{ psf/ft}) \times (6.25 \text{ ft})$$

$$p_s = 0.9375 \text{ kip/ft}^2$$

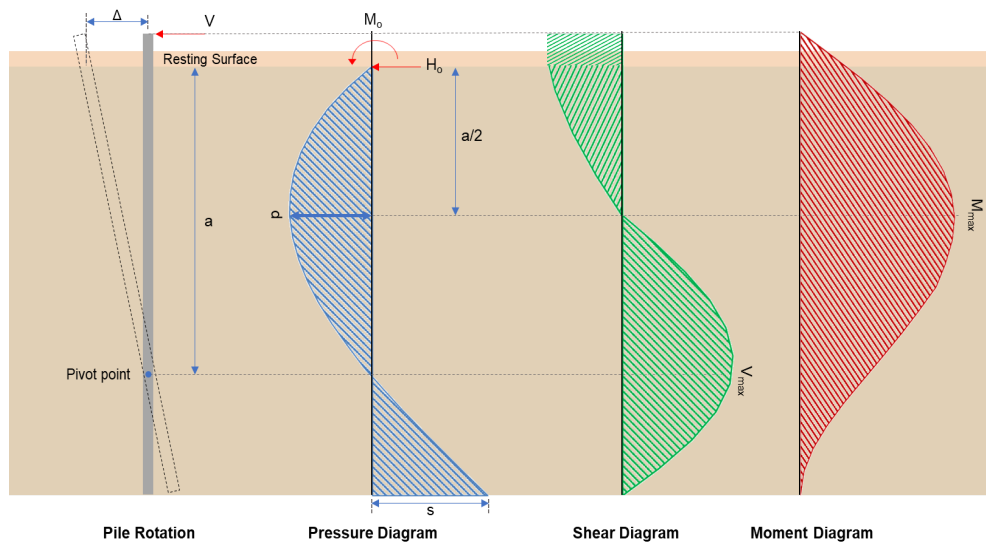
Ratio - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{(0.0075332 \text{ kip/ft}^2)}{(0.9375 \text{ kip/ft}^2)}$$

$$Ratio = 0.0080355$$

Status: **PASS**
Ratio: **0.010**



Shear force and Bending moment (x-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57 D}$$

$$H_o = \frac{(-3.477 \text{ kip})}{1.57 \times (48 \text{ in})}$$

$$H_o = -0.55366 \text{ kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x H)}{1.57 D}$$

$$M_o = \frac{(40.394 \text{ kipft}) + ((-3.477 \text{ kip}) \times (0 \text{ ft}))}{1.57 \times (48 \text{ in})}$$

$$M_o = 6.4322 \text{ kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(6.4322 \text{ kipft/ft})}{(-0.55366 \text{ kip/ft})}$$

$$E = 11.617 \text{ ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4 M_o L_e) + (3 H_o L_e^2)}{(6 M_o) + (4 H_o L_e)}$$

$$a = \frac{(4 \times (6.4322 \text{ kipft/ft}) \times (6.25 \text{ ft})) + (3 \times (-0.55366 \text{ kip/ft}) \times (6.25 \text{ ft})^2)}{(6 \times (6.4322 \text{ kipft/ft})) + (4 \times (-0.55366 \text{ kip/ft}) \times (6.25 \text{ ft}))}$$

$$a = \frac{(-0.55366 \text{ kip/ft}) \times (48 \text{ in}) \times (6.25 \text{ ft})}{(6 \times (6.4322 \text{ kip/ft}) + (4 \times (-0.55366 \text{ kip/ft}) \times (6.25 \text{ ft}))}$$

$$a = 4.3042 \text{ ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o D) \left[1 - \left[3 \left(\frac{4 E}{L_e} + 3 \right) \left(\frac{a}{L_e} \right)^2 \right] + \left[4 \left(\frac{3 E}{L_e} + 2 \right) \left(\frac{a}{L_e} \right)^3 \right] \right]$$

$$V_{max} = ((-0.55366 \text{ kip/ft}) \times (48 \text{ in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (11.617 \text{ ft})}{(6.25 \text{ ft})} + 3 \right) \times \left(\frac{(4.3042 \text{ ft})}{(6.25 \text{ ft})} \right)^2 \right] + \left[4 \times \left(\frac{3 \times (11.617 \text{ ft})}{(6.25 \text{ ft})} + 2 \right) \times \left(\frac{(4.3042 \text{ ft})}{(6.25 \text{ ft})} \right)^3 \right] \right]$$

$$V_{max} = 8.7456 \text{ kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o D L_e) \left[\left(\frac{E}{L_e} + \frac{a}{2 L_e} \right) - \left[\left(\frac{4 E}{L_e} + 3 \right) \left(\frac{a}{2 L_e} \right)^3 \right] + \left[\left(\frac{3 E}{L_e} + 2 \right) \left(\frac{a}{2 L_e} \right)^4 \right] \right]$$

$$M_{max} = ((-0.55366 \text{ kip/ft}) \times (48 \text{ in}) \times (6.25 \text{ ft})) \times \left[\left(\frac{(11.617 \text{ ft})}{(6.25 \text{ ft})} + \frac{(4.3042 \text{ ft})}{2 \times (6.25 \text{ ft})} \right) - \left[\left(\frac{4 \times (11.617 \text{ ft})}{(6.25 \text{ ft})} + 3 \right) \times \left(\frac{(4.3042 \text{ ft})}{2 \times (6.25 \text{ ft})} \right)^3 \right] + \left[\left(\frac{3 \times (11.617 \text{ ft})}{(6.25 \text{ ft})} + 2 \right) \times \left(\frac{(4.3042 \text{ ft})}{2 \times (6.25 \text{ ft})} \right)^4 \right] \right]$$

$$M_{max} = 26.072 \text{ kipft}$$

Shear force and Bending moment (z-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57 b}$$

$$H_o = \frac{(0.04 \text{ kip})}{1.57 \times (48 \text{ in})}$$

$$H_o = 0.0063694 \text{ kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z H)}{1.57 b}$$

$$M_o = \frac{(0.13 \text{ kipft}) + ((0.04 \text{ kip}) \times (0 \text{ ft}))}{1.57 \times (48 \text{ in})}$$

$$M_o = 0.020701 \text{ kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(0.020701 \text{ kipft/ft})}{(0.0063694 \text{ kip/ft})}$$

$$E = 3.25 \text{ ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4 M_o L_e) + (3 H_o L_e^2)}{(6 M_o) + (4 H_o L_e)}$$

$$a = \frac{(4 \times (0.020701 \text{ kipft/ft}) \times (6.25 \text{ ft})) + (3 \times (0.0063694 \text{ kip/ft}) \times (6.25 \text{ ft})^2)}{(6 \times (0.020701 \text{ kipft/ft})) + (4 \times (0.0063694 \text{ kip/ft}) \times (6.25 \text{ ft}))}$$

$$a = 4.4593 \text{ ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o b) \left[1 - \left[3 \left(\frac{4 E}{L_e} + 3 \right) \left(\frac{a}{L_e} \right)^2 \right] + \left[4 \left(\frac{3 E}{L_e} + 2 \right) \left(\frac{a}{L_e} \right)^3 \right] \right]$$

$$V_{max} = ((0.0063694 \text{ kip/ft}) \times (48 \text{ in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (3.25 \text{ ft})}{(6.25 \text{ ft})} + 3 \right) \times \left(\frac{(4.4593 \text{ ft})}{(6.25 \text{ ft})} \right)^2 \right] + \left[4 \times \left(\frac{3 \times (3.25 \text{ ft})}{(6.25 \text{ ft})} + 2 \right) \times \left(\frac{(4.4593 \text{ ft})}{(6.25 \text{ ft})} \right)^3 \right] \right]$$

$$V_{max} = 0.040408 \text{ kip}$$

M_{max} - Max bending moment located at depth $a/2$,

$$M_{max} = (H_o \ b \ L_e) \left[\left(\frac{E}{L_e} + \frac{a}{2 \ L_e} \right) - \left[\left(\frac{4 \ E}{L_e} + 3 \right) \left(\frac{a}{2 \ L_e} \right)^3 \right] + \left[\left(\frac{3 \ E}{L_e} + 2 \right) \left(\frac{a}{2 \ L_e} \right)^4 \right] \right]$$

$$M_{max} = ((0.0063694 \text{ kip/ft}) \times (48 \text{ in}) \times (6.25 \text{ ft})) \times \left[\left(\frac{(3.25 \text{ ft})}{(6.25 \text{ ft})} + \frac{(4.4593 \text{ ft})}{2 \times (6.25 \text{ ft})} \right) - \left[\left(\frac{4 \times (3.25 \text{ ft})}{(6.25 \text{ ft})} + 3 \right) \times \left(\frac{(4.4593 \text{ ft})}{2 \times (6.25 \text{ ft})} \right)^3 \right] + \left[\left(\frac{3 \times (3.25 \text{ ft})}{(6.25 \text{ ft})} + 2 \right) \times \left(\frac{(4.4593 \text{ ft})}{2 \times (6.25 \text{ ft})} \right)^4 \right] \right]$$

$$M_{max} = 0.11206 \text{ kipft}$$

Minimum Reinforcement Check (LRFD)

Parameters:

$f'_{ck} = 2.5 \text{ ksi}$ - Concrete strength,
 $f_{yk} = 60 \text{ ksi}$ - Longitudinal reinforcement strength,
 $\phi = 0.65$ - Reduction factor for axial strength,
 $\alpha = 0.8$ - Alpha factor for axial strength,
 $A_g = 2304 \text{ in}^2$ - Gross area of concrete,

Longitudinal reinforcement:

Required reinforcement due to axial load, $A_{st,required}$

$A_{st,required}$

$$A_{st,required} = Min \left[\frac{\frac{P}{\phi \ \alpha} - (0.85 \ f'_{ck} \ A_g)}{f_{yk} - (0.85 \ f'_{ck})}, (0.08 \ A_g) \right]$$

$$A_{st,required} = Min \left[\frac{\frac{(6.64 \text{ kip})}{(0.65) \times (0.8)} - (0.85 \times (2.5 \text{ ksi}) \times (2304 \text{ in}^2))}{(60 \text{ ksi}) - (0.85 \times (2.5 \text{ ksi}))}, (0.08 \times (2304 \text{ in}^2)) \right]$$

$$A_{st,required} = -84.375 \text{ in}^2$$

A_{min} - Governing minimum reinforcement area,

$$A_{min} = Max [A_{st,required}, (0.0018 \ A_g)]$$

$$A_{min} = Max [(-84.375 \text{ in}^2), (0.0018 \times (2304 \text{ in}^2))]$$

$$A_{min} = 4.1472 \text{ in}^2$$

n_{rebar} - Required number of reinforcement,

$$n_{rebar} = \frac{A_{min}}{A_{rebar}}$$

$$n_{rebar} = \frac{(4.1472 \text{ in}^2)}{(0.3068 \text{ in}^2)}$$

$$n_{rebar} = 14$$

A_{st} - Actual total reinforcement area,

$$A_{st} = n_{rebar} \frac{\pi \ d_{bar}^2}{4}$$

$$A_{st} = (14) \times \frac{\pi \times (0.625 \text{ in})^2}{4}$$

$$A_{st} = 4.2951 \text{ in}^2$$

Ratio - Capacity

$$Ratio = \frac{A_{min}}{A_{st}}$$

$$Ratio = \frac{(4.1472 \text{ in}^2)}{(4.2951 \text{ in}^2)}$$

Table 22.4.2.1

22.4.2.2, 10.6.1.1

<p>25.2.3</p>	<div> <div> $Ratio = 0.96556$ </div> </div> <p>s_{rebar} - Minimum spacing of reinforcement,</p> <div> $s_{rebar} = Max[1.5, (1.5 d_{bar})]$ $s_{rebar} = Max[1.5, (1.5 \times (0.625 \text{ in}))]$ $s_{rebar} = 1.5 \text{ in}$ </div> <p>Ties:</p> <p>25.7.2.2 Since longitudinal reinforcement is \leq No. 10ø: Use #3(0.375 in)</p> <p>25.7.2.1 s_{ties} - Maximum spacing of ties,</p> <div> $s_{ties} = Min[(16 d_{bar}), (48 d_{ties}), Min(D, b)]$ $s_{ties} = Min[(16 \times (0.625 \text{ in})), (48 \times (0.375 \text{ in})), Min((48 \text{ in}), (48 \text{ in}))]$ $s_{ties} = 10 \text{ in}$ </div> <p>Summary:</p> <p>Main reinforcement: 14 - #5 (0.625 in) Ties: #3(0.375 in) - 10 in</p>
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Status: **PASS**
Ratio: **0.970**

$$V_{c,max} = 296.21 \text{ kip}$$

22.5.5.1.1(a) The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$, $P = 6.64 \text{ kip} \rightarrow 6640 \text{ lbf}$,
 $V_{c,a}$ - Shear strength of concrete (a)

$$V_{c,a} = \left[2 \lambda_s \sqrt{f'_{ck}} + \frac{P}{6 A_g} \right] b_w d$$

$$V_{c,a} = \left[2 \times (0.64282) \times \sqrt{(2500 \text{ psi})} + \frac{(6640 \text{ lbf})}{6 \times (2304 \text{ in}^2)} \right] \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{c,a} = 119.37 \text{ kip}$$

22.5.5.1.2 The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$,
 $V_{c,b}$ - Shear strength of concrete (b)

$$V_{c,b} = \left[2 \lambda_s \sqrt{f'_{ck}} + (0.05 f'_{ck}) \right] b_w d$$

$$V_{c,b} = \left[2 \times (0.64282) \times \sqrt{(2500 \text{ psi})} + (0.05 \times (2500 \text{ psi})) \right] \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{c,b} = 348.89 \text{ kip}$$

V_c - Governing shear strength of concrete

$$V_c = \text{Min}[V_{c,max}, V_{c,a}, V_{c,b}]$$

$$V_c = \text{Min}[(296.21 \text{ kip}), (119.37 \text{ kip}), (348.89 \text{ kip})]$$

$$V_c = 119.37 \text{ kip}$$

22.5.1.2 The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$,
 $V_{s,a}$ - Shear strength of steel (a)

$$V_{s,a} = 8 \sqrt{f'_{ck}} b_w d$$

$$V_{s,a} = 8 \times \sqrt{(2500 \text{ psi})} \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{s,a} = 737.28 \text{ kip}$$

A_v - Ties rebar area,

$$A_v = \frac{\pi d_{ties}^2}{4}$$

$$A_v = \frac{\pi \times (0.375 \text{ in})^2}{4}$$

$$A_v = 0.11045 \text{ in}^2$$

22.5.8.5.3 $V_{s,b}$ - Shear strength of steel (b)

$$V_{s,b} = \frac{2 A_v f_{yw} d}{s_{ties}}$$

$$V_{s,b} = \frac{2 \times (0.11045 \text{ in}^2) \times (60 \text{ ksi}) \times (38.4 \text{ in})}{(10 \text{ in})}$$

$$V_{s,b} = 50.894 \text{ kip}$$

V_s - Governing shear strength of steel

$$V_s = \text{MIN}[V_{s,a}, V_{s,b}]$$

$$V_s = \text{MIN}[(737.28 \text{ kip}), (50.894 \text{ kip})]$$

$$V_s = 50.894 \text{ kip}$$

22.5.1.1 ϕV_n - Allowable shear strength

$$\phi V_n = \phi (V_c + V_s)$$

$$\phi V_n = (0.65) \times ((119.37 \text{ kip}) + (50.894 \text{ kip}))$$

$$\phi V_n = 110.67 \text{ kip}$$

Considering x-direction:

$V_{max} = 8.7456 \text{ kip}$ - Maximum shear force in the x-direction,
Ratio - Capacity

$$\text{Ratio} = \frac{V_{max}}{\phi V_n}$$

	$Ratio = \frac{(8.7456 \text{ kip})}{(110.67 \text{ kip})}$ $Ratio = 0.079023$ <p>Considering z-direction:</p> <p>$V_{max} = 0.040408 \text{ kip}$ - Maximum shear force in the z-direction, <i>Ratio</i> - Capacity</p> $Ratio = \frac{V_{max}}{\phi V_n}$ $Ratio = \frac{(0.040408 \text{ kip})}{(110.67 \text{ kip})}$ $Ratio = 0.00036512$ <p>Status: PASS Ratio: 0.080</p>	
14.5.2.1b	<p>Flexural Strength (ACI 318-19, LRFD)</p> <p>S_m - Section modulus</p> $S_m = \frac{b D^2}{6}$ $S_m = \frac{(48 \text{ in}) \times (48 \text{ in})^2}{6}$ $S_m = 18432 \text{ in}^3$ <p>$\lambda = 1$ - Concrete modification factor (Normal concrete), Allowable flexural strength: M_n shall be the lesser of: $\phi M_{n,1}$</p> $\phi M_{n,1} = \phi \times 5 \times \lambda \times \sqrt{f'_c} \times S_m$ $\phi M_{n,1} = 0.65 \times 5 \times 1 \times \sqrt{(2.5 \text{ ksi})} \times 18432.001 \text{ in}^3$ $\phi M_{n,1} = 249.600 \text{ kipft}$ <p>$\phi M_{n,2}$</p> $\phi M_{n,2} = \phi \times 0.85 \times f'_c \times S_m$ $\phi M_{n,2} = (0.65) \times 0.85 \times (2.5 \text{ ksi}) \times (18432 \text{ in}^3)$ $\phi M_{n,2} = 2121.6 \text{ kipft}$ <p>Therefore, ϕM_n - Allowable flexural strength,</p> $\phi M_n = MIN[\phi M_{n,1}, \phi M_{n,2}]$ $\phi M_n = MIN[(249.6 \text{ kipft}), (2121.6 \text{ kipft})]$ $\phi M_n = 249.6 \text{ kipft}$ <p>Considering x-direction:</p> <p>$M_{max} = 26.072 \text{ kipft}$ - Maximum moment in the x-direction, <i>Ratio</i> - Capacity</p> $Ratio = \frac{M_{max}}{\phi M_n}$ $Ratio = \frac{(26.072 \text{ kipft})}{(249.6 \text{ kipft})}$ $Ratio = 0.10446$ <p>Status: PASS Ratio: 0.100</p>	
	<p>Considering z-direction:</p> <p>$M_{max} = 0.11206 \text{ kipft}$ - Maximum moment in the z-direction, <i>Ratio</i> - Capacity</p> $Ratio = \frac{M_{max}}{\phi M_n}$	

$$Ratio = \frac{(0.11206 \text{ kipft})}{(249.6 \text{ kipft})}$$

$$Ratio = 0.00044898$$

Status: **PASS**
Ratio: **0.000**