Your Project Calculations

Project Name: SilverBayYMCA-RevA

S3D Model Link:
https://platform.skyciv.com/structural?preload_name=SilverBayYMCA-RevA&preload_path=Shared%20Enterprise%20Folder/MT_Solar_Projects/4_2023

Public Model Link:
https://platform.skyciv.com/structural-viewer?project_id=1NGiVnuh00eSNRbhtluOsyoQxZQkPJT8ApzgVarLPSicibnbNleArGBoVXkhbzLw

Array Specification

Product:	Beam
Unique ID:	4P-22.5-10TOP-XD-72-L-5Hx12W-6FBA
Duty Classification:	XD
Module Width:	41.10 in
Module Length:	87.20in
Number of Rows:	5
Number of Columns:	12
Total Number of Modules:	60
Desired Tilt Angle:	46
Front Edge Clearance:	12
Total Array Height at Tilt:	24.39 ft
Total Frame Length:	87.00 ft
Frame Weight:	6134 lbs
Array Dimensions N/S:	17.33 ft
Array Dimensions E/W:	88.20 ft
Rail Length:	208.00 in
Rail Spacing:	3.63 ft
Rail Check:	Not Checked

Support Specifications

Pole Size:	10in Pipe Sch 40
Pole Length above Grade:	18.23 ft
Number of Poles:	4
Pole Spacing:	22.5 ft

Foundation Specifications

Foundation Type:	Square
Foundation Dimensions:	48 x 48 in
Foundation Depth (below grade):	Pile 1: 7.75 ft Pile 2: 7.75 ft Pile 3: 7.75 ft Pile 4: 7.75 ft
Foundation Volume:	18.370 y^3
Foundation Result:	PASSED
Mount Twist:	0.274211 kip

Site Info

Risk Category:	I
Exposure:	B
Soil Classification:	sand
Site Location:	87 Silver Bay Rd, Silver Bay, NY 12874, USA
Wind Speed:	100 mph
Snow Load:	50 psf
Design Uplift Pressure:	0.014526 ksf
Design Downforce Pressure:	-0.014526 ksf
Design Snow Pressure:	0.013196 ksf

Design Disclaimer

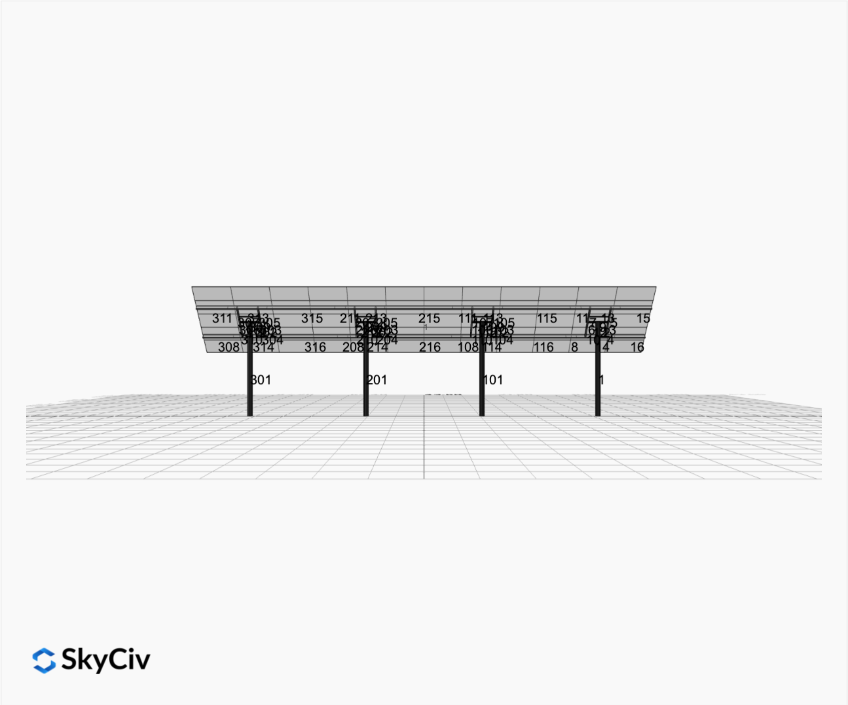
This software should be used for preliminary designs and should not be used as a final design unless reviewed, verified and designed by a qualified structural engineer.

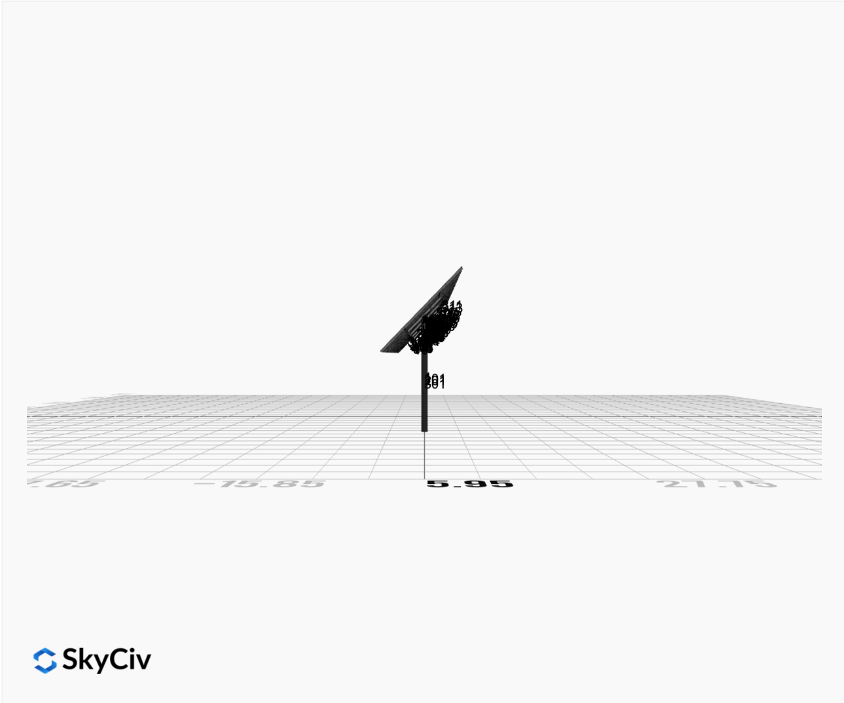
AutoDesigner Input

{"wind_speed_override":null,"snow_load_override":null,"direct_snow_load":false,"product_type":"Beam","project_id":"SilverBayYMCA-RevA","site_address":"87 Silver Bay Rd, Silver Bay, NY 12874, USA","module_width":41.1,"module_length":87.2,"number_rows":"5","number_columns":12,"pole_mount_section":"4_40","core_pipe_width":65,"core_pipe_section":"2_40","adjuster_section":"2_40","core_beam_height":65,"core_beam_section":"HSS3x2x1/8","main_pipe_section":"2_12GA","pole_spacing":"15","tilt_angle":46,"ground_clearance":12,"risk_category":"I","exposure_category":"B","frame_duty_override":"XD","pole_override":"10_40","soil_type":"sand","foundation_type":"Square","foundation_size":48,"check_rails":false}

Design Notes:

- AISC Deflection checks are set to L/1 due to structure design intent
- Foundation Design and Sizing is approximate only

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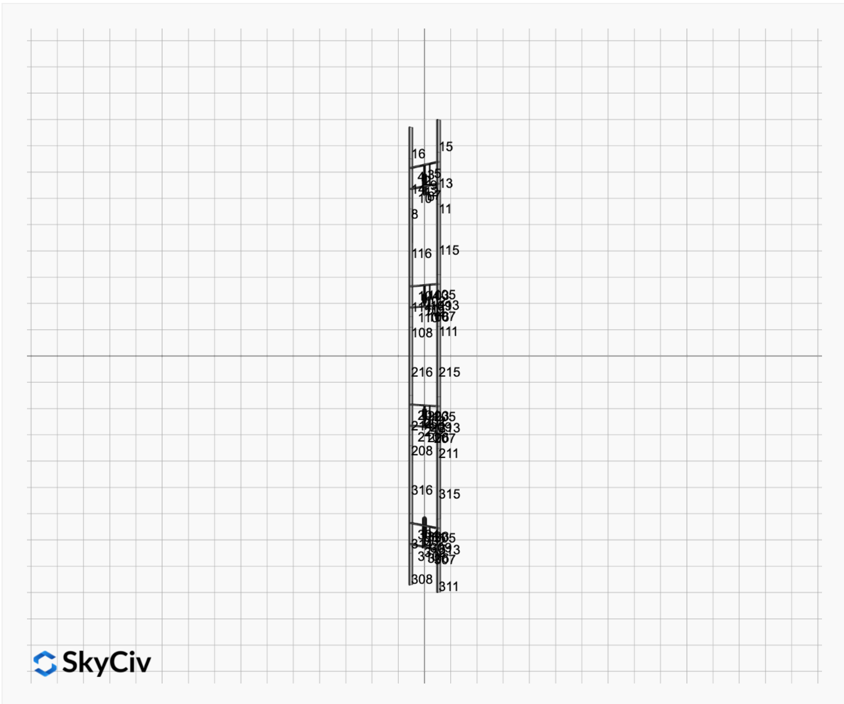
SkyCiv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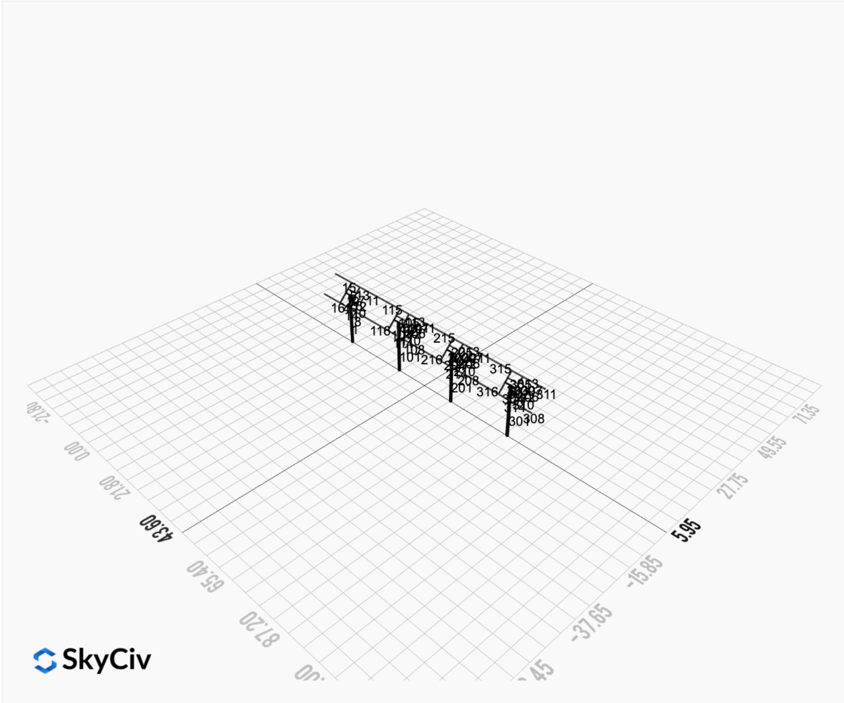
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5.95
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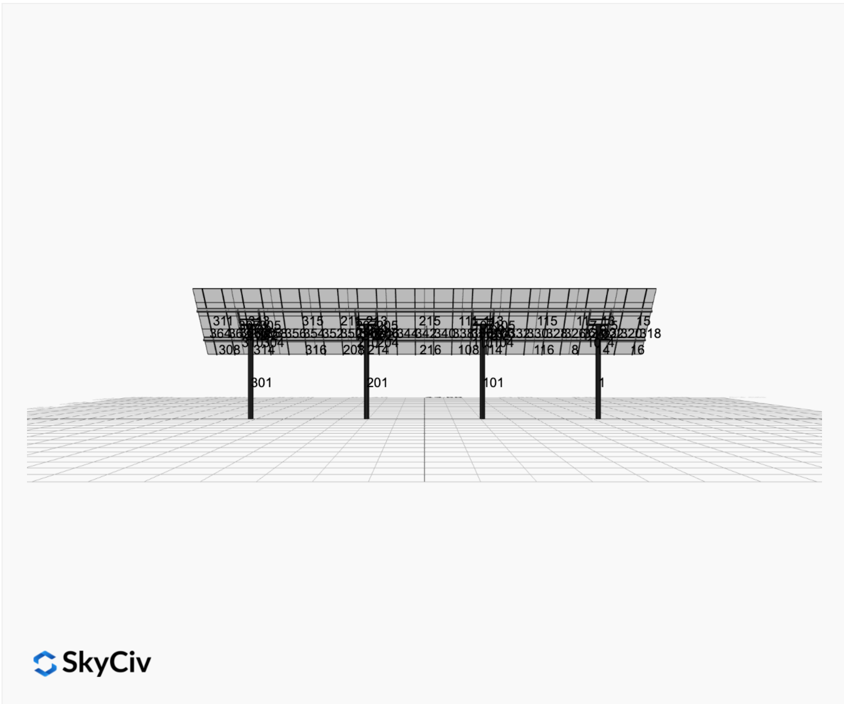
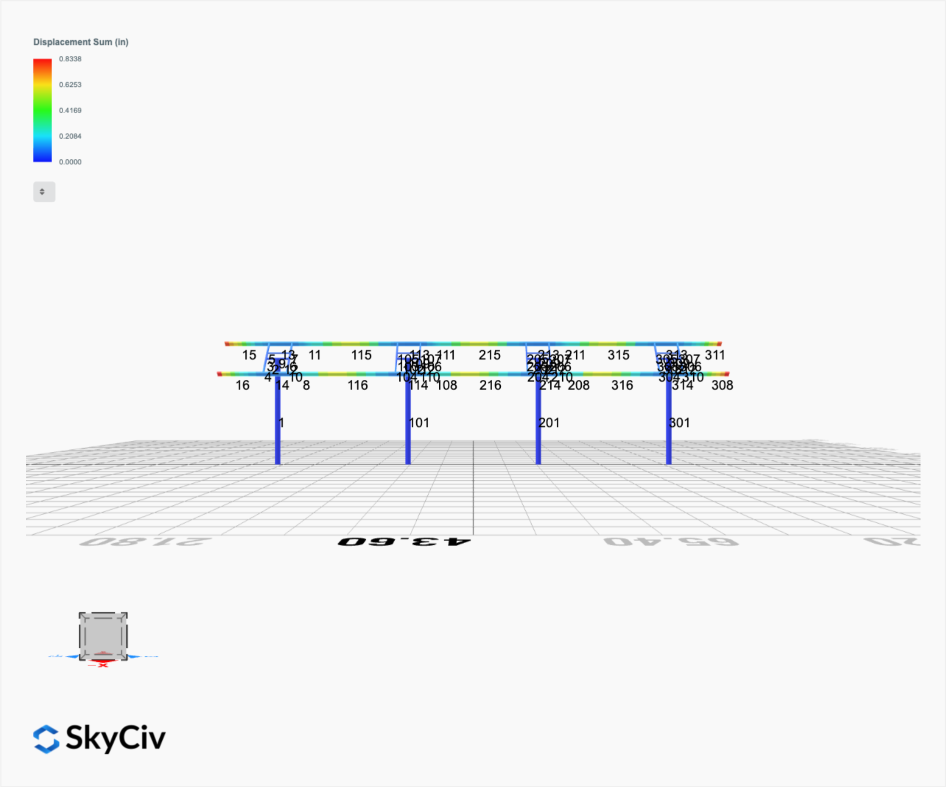
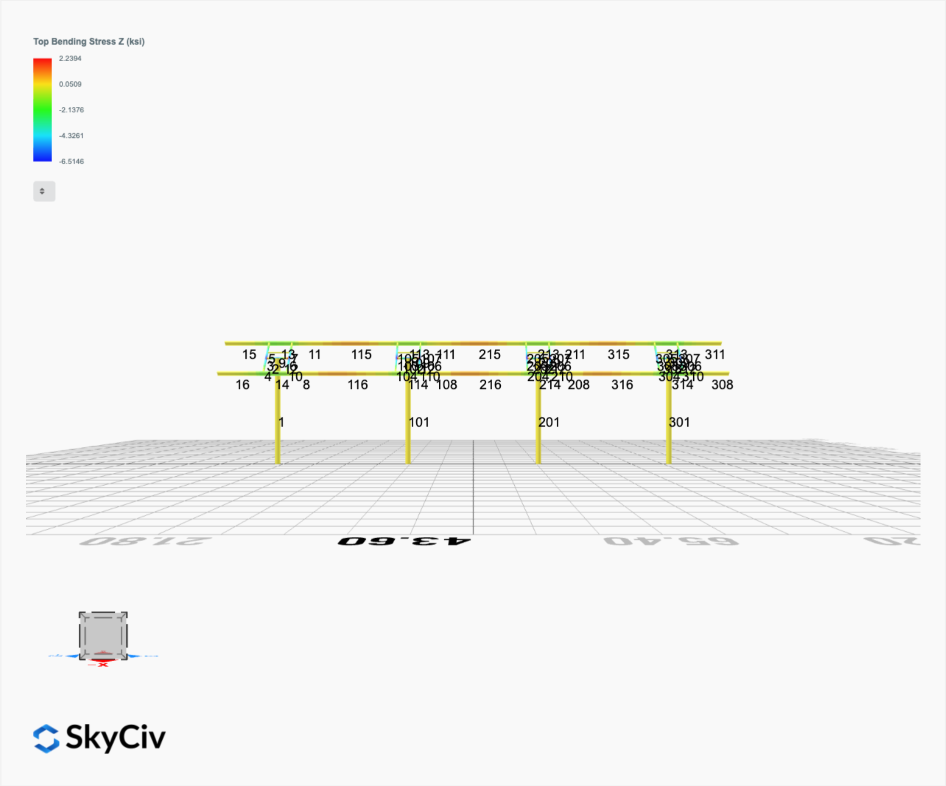
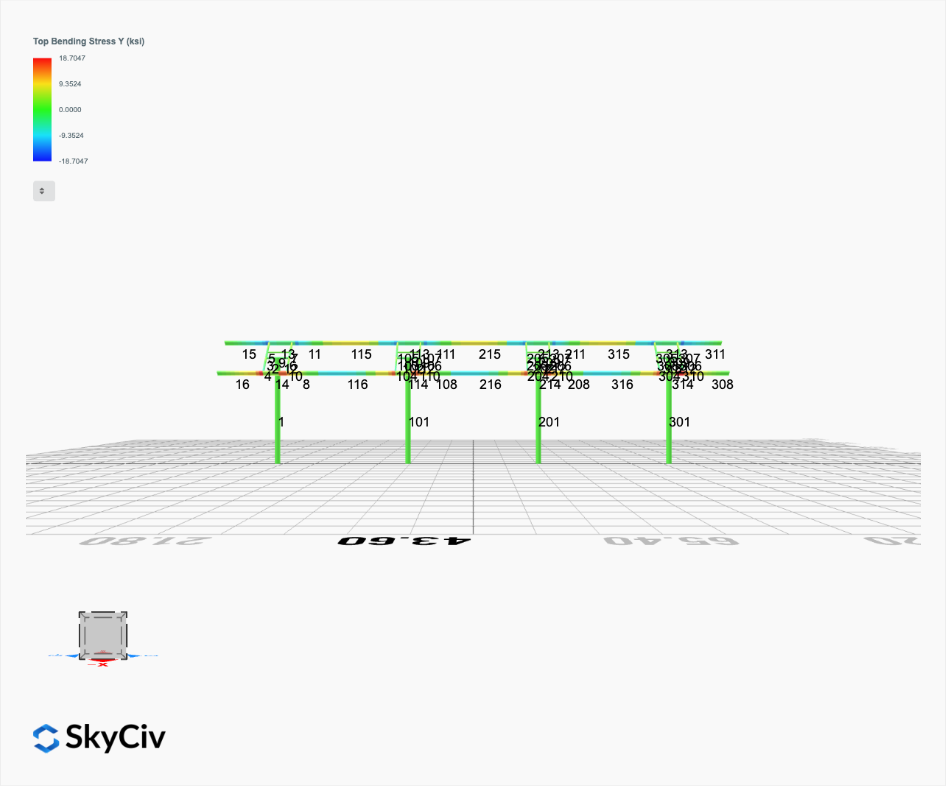
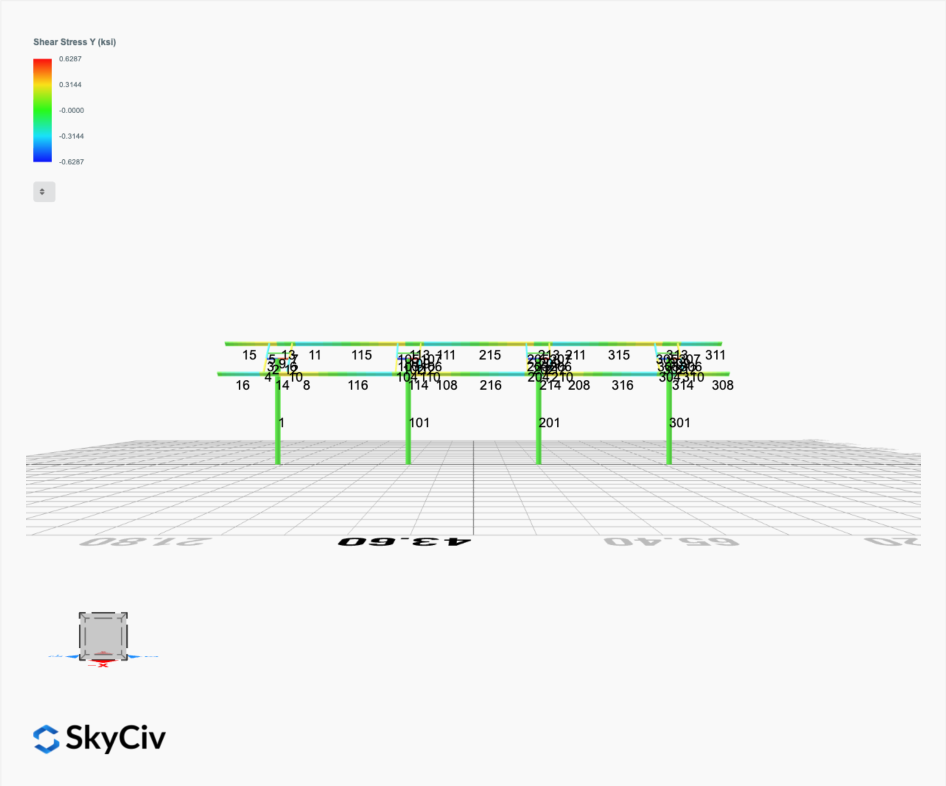

201
101
1

FEM Results (Envelope Worst Case for each member)






Top Bending Stress Y (ksi)
18.7047
9.3524
0.0000
-9.3524
-18.7047
SkyCiv


Shear Stress Y (ksi)
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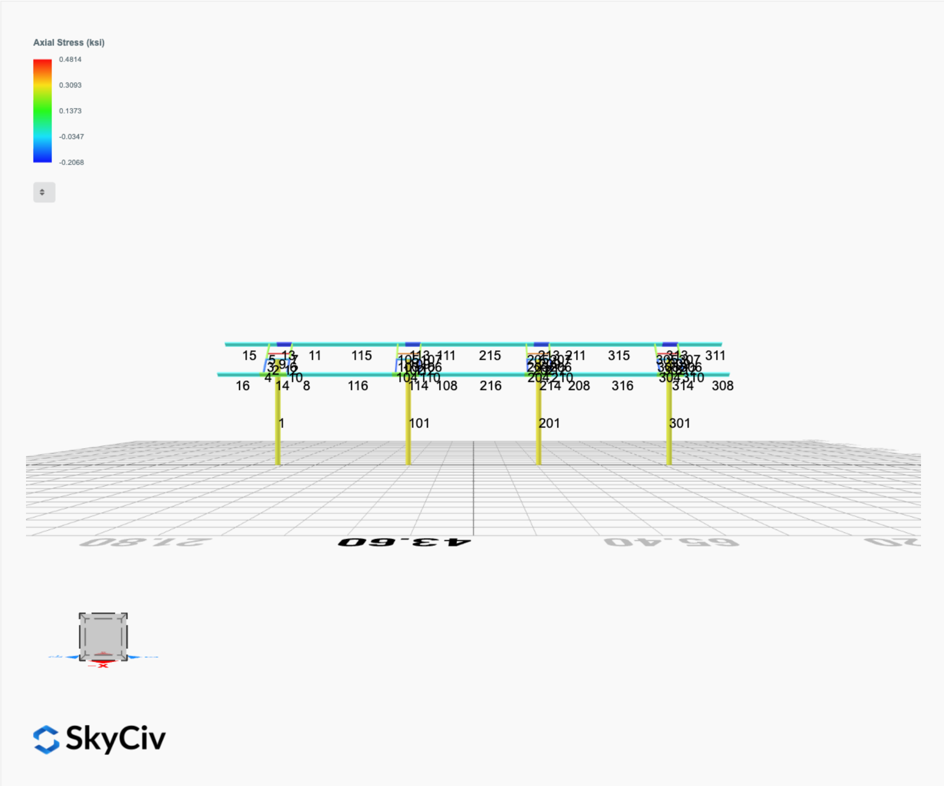
-0.6287
SkyCiv


Axial Stress (ksi)
0.4814
0.3093
0.1373
-0.0347
-0.2068
SkyCiv

Reaction Forces for Foundation 1 (Node ID#1), (kip, kip-ft)

ASD Load Combination Results

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 1. D	-0.0065	3.1726	-0.0033	-0.0162	0.0675	0.1396
ULS: 2. D + L	-0.0065	3.1726	-0.0033	-0.0162	0.0675	0.1396
ULS: 3. D + (S or Lr or R)	-0.0165	6.5204	-0.0084	-0.0405	0.1713	0.3267
ULS: 3. D + (S or Lr or R)	-0.0065	3.1726	-0.0033	-0.0162	0.0675	0.1396
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	-0.0140	5.6834	-0.0071	-0.0344	0.1454	0.2799
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	-0.0065	3.1726	-0.0033	-0.0162	0.0675	0.1396
ULS: 5b. D + 0.7E	-0.0065	3.1726	-0.0033	-0.0162	0.0675	0.1396
ULS: 6b. D + 0.75L + 0.75(0.7)E + 0.75S	-0.0140	5.6834	-0.0071	-0.0344	0.1454	0.2799
ULS: 8. 0.6D + 0.7E	-0.0039	1.9036	-0.0020	-0.0097	0.0405	0.0838
ULS: 5a. D + 0.6W_Wind downforce Case A only	-2.3583	5.4360	-0.0002	0.0013	0.0380	43.6203
ULS: 5a. D + 0.6W_Wind downforce Case B only	-0.0065	3.1726	-0.0033	-0.0162	0.0675	0.1396
ULS: 5a. D + 0.6W_Wind uplift Case A only	2.3443	0.9097	-0.0059	-0.0302	0.0926	-42.2141
ULS: 5a. D + 0.6W_Wind uplift Case B only	-0.0065	3.1726	-0.0033	-0.0162	0.0675	0.1396
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-1.7778	7.3810	-0.0047	-0.0213	0.1232	32.8904
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-0.0140	5.6834	-0.0071	-0.0344	0.1454	0.2799
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	1.7492	3.9863	-0.0090	-0.0449	0.1642	-31.4854
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	-0.0140	5.6834	-0.0071	-0.0344	0.1454	0.2799
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-1.7703	4.8702	-0.0009	-0.0031	0.0454	32.7501
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-0.0065	3.1726	-0.0033	-0.0162	0.0675	0.1396
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	1.7566	1.4755	-0.0053	-0.0267	0.0863	-31.6257
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	-0.0065	3.1726	-0.0033	-0.0162	0.0675	0.1396
ULS: 7. 0.6D + 0.6W_Wind downforce Case A only	-2.3557	4.1670	0.0012	0.0078	0.0110	43.5645
ULS: 7. 0.6D + 0.6W_Wind downforce Case B only	-0.0039	1.9036	-0.0020	-0.0097	0.0405	0.0838
ULS: 7. 0.6D + 0.6W_Wind uplift Case A only	2.3470	-0.3593	-0.0046	-0.0237	0.0656	-42.2699
ULS: 7. 0.6D + 0.6W_Wind uplift Case B only	-0.0039	1.9036	-0.0020	-0.0097	0.0405	0.0838

Worst Case Reactions LRFD

These calculations are taken directly from the FEA via SkyCiv and are used in the Concrete Checks of the Foundatino Module. Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	11.0502
Shear X	-3.9336
Shear Z	-0.0151
Moment X	-0.0746
Moment Y (Twist)	0.2744
Moment Z	74.2267

Worst Case Reactions ASD

These results are taken from the worst case values in the above table and are used in the Soil Checks in the Foundation Module. Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	7.3810
Shear X	-2.3583
Shear Z	-0.0090
Moment X	-0.0449
Moment Y (Twist)	0.1713
Moment Z	43.6203

Reaction Forces for Foundation 2 (Node ID#101), (kip, kip-ft)

ASD Load Combination Results

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 1. D	0.0065	3.3120	0.0007	0.0045	-0.0061	-0.0878
ULS: 2. D + L	0.0065	3.3120	0.0007	0.0045	-0.0061	-0.0878
ULS: 3. D + (S or Lr or R)	0.0165	6.8758	0.0019	0.0114	-0.0155	-0.2502
ULS: 3. D + (S or Lr or R)	0.0065	3.3120	0.0007	0.0045	-0.0061	-0.0878
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	0.0140	5.9848	0.0016	0.0097	-0.0131	-0.2096
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	0.0065	3.3120	0.0007	0.0045	-0.0061	-0.0878
ULS: 5b. D + 0.7E	0.0065	3.3120	0.0007	0.0045	-0.0061	-0.0878

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 6b. D + 0.75L + 0.75(0.7)E + 0.75S	0.0140	5.9848	0.0016	0.0097	-0.0131	-0.2096
ULS: 8. 0.6D + 0.7E	0.0039	1.9872	0.0004	0.0027	-0.0037	-0.0527
ULS: 5a. D + 0.6W_Wind downforce Case A only	-2.4343	5.6767	0.0128	0.0726	-0.0930	44.9928
ULS: 5a. D + 0.6W_Wind downforce Case B only	0.0065	3.3120	0.0007	0.0045	-0.0061	-0.0878
ULS: 5a. D + 0.6W_Wind uplift Case A only	2.4482	0.9468	-0.0110	-0.0612	0.0780	-43.9861
ULS: 5a. D + 0.6W_Wind uplift Case B only	0.0065	3.3120	0.0007	0.0045	-0.0061	-0.0878
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-1.8166	7.7584	0.0107	0.0608	-0.0783	33.6008
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	0.0140	5.9848	0.0016	0.0097	-0.0131	-0.2096
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	1.8452	4.2109	-0.0072	-0.0396	0.0500	-33.1333
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	0.0140	5.9848	0.0016	0.0097	-0.0131	-0.2096
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-1.8241	5.0855	0.0098	0.0555	-0.0713	33.7226
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	0.0065	3.3120	0.0007	0.0045	-0.0061	-0.0878
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	1.8378	1.5381	-0.0080	-0.0448	0.0570	-33.0115
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	0.0065	3.3120	0.0007	0.0045	-0.0061	-0.0878
ULS: 7. 0.6D + 0.6W_Wind downforce Case A only	-2.4369	4.3519	0.0126	0.0708	-0.0906	45.0279
ULS: 7. 0.6D + 0.6W_Wind downforce Case B only	0.0039	1.9872	0.0004	0.0027	-0.0037	-0.0527
ULS: 7. 0.6D + 0.6W_Wind uplift Case A only	2.4456	-0.3780	-0.0113	-0.0630	0.0805	-43.9510
ULS: 7. 0.6D + 0.6W_Wind uplift Case B only	0.0039	1.9872	0.0004	0.0027	-0.0037	-0.0527

Worst Case Reactions LRFD

These calculations are taken directly from the FEA via SkyCiv and are used in the Concrete Checks of the Foundatino Module. Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	11.6465
Shear X	-4.0811
Shear Z	0.0221
Moment X	0.1252
Moment Y (Twist)	0.1606
Moment Z	76.5175

Worst Case Reactions ASD

These results are taken from the worst case values in the above table and are used in the Soil Checks in the Foundation Module. Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	7.7584
Shear X	-2.4482
Shear Z	0.0128
Moment X	0.0726
Moment Y (Twist)	0.0930
Moment Z	45.0279

Reaction Forces for Foundation 3 (Node ID#201), (kip, kip-ft)

ASD Load Combination Results

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 1. D	0.0065	3.3120	-0.0007	-0.0045	0.0061	-0.0878
ULS: 2. D + L	0.0065	3.3120	-0.0007	-0.0045	0.0061	-0.0878
ULS: 3. D + (S or Lr or R)	0.0165	6.8758	-0.0019	-0.0114	0.0155	-0.2502
ULS: 3. D + (S or Lr or R)	0.0065	3.3120	-0.0007	-0.0045	0.0061	-0.0878
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	0.0140	5.9848	-0.0016	-0.0097	0.0131	-0.2096
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	0.0065	3.3120	-0.0007	-0.0045	0.0061	-0.0878
ULS: 5b. D + 0.7E	0.0065	3.3120	-0.0007	-0.0045	0.0061	-0.0878
ULS: 6b. D + 0.75L + 0.75(0.7)E + 0.75S	0.0140	5.9848	-0.0016	-0.0097	0.0131	-0.2096
ULS: 8. 0.6D + 0.7E	0.0039	1.9872	-0.0004	-0.0027	0.0037	-0.0527
ULS: 5a. D + 0.6W_Wind downforce Case A only	-2.4343	5.6767	-0.0129	-0.0725	0.0930	44.9928
ULS: 5a. D + 0.6W_Wind downforce Case B only	0.0065	3.3120	-0.0007	-0.0045	0.0061	-0.0878
ULS: 5a. D + 0.6W_Wind uplift Case A only	2.4482	0.9468	0.0110	0.0612	-0.0780	-43.9861
ULS: 5a. D + 0.6W_Wind uplift Case B only	0.0065	3.3120	-0.0007	-0.0045	0.0061	-0.0878
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-1.8166	7.7584	-0.0107	-0.0607	0.0783	33.6008
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	0.0140	5.9848	-0.0016	-0.0097	0.0131	-0.2096
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	1.8452	4.2109	0.0072	0.0396	-0.0500	-33.1333
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	0.0140	5.9848	-0.0016	-0.0097	0.0131	-0.2096

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-1.8241	5.0855	-0.0098	-0.0555	0.0713	33.7226
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	0.0065	3.3120	-0.0007	-0.0045	0.0061	-0.0878
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	1.8378	1.5381	0.0080	0.0448	-0.0570	-33.0115
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	0.0065	3.3120	-0.0007	-0.0045	0.0061	-0.0878
ULS: 7. 0.6D + 0.6W_Wind downforce Case A only	-2.4369	4.3519	-0.0126	-0.0708	0.0906	45.0279
ULS: 7. 0.6D + 0.6W_Wind downforce Case B only	0.0039	1.9872	-0.0004	-0.0027	0.0037	-0.0527
ULS: 7. 0.6D + 0.6W_Wind uplift Case A only	2.4456	-0.3780	0.0113	0.0630	-0.0805	-43.9510
ULS: 7. 0.6D + 0.6W_Wind uplift Case B only	0.0039	1.9872	-0.0004	-0.0027	0.0037	-0.0527

Worst Case Reactions LRFD

These calculations are taken directly from the FEA via SkyCiv and are used in the Concrete Checks of the Foundatino Module. Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	11.6465
Shear X	-4.0811
Shear Z	-0.0222
Moment X	-0.1253
Moment Y (Twist)	0.1606
Moment Z	76.5178

Worst Case Reactions ASD

These results are taken from the worst case values in the above table and are used in the Soil Checks in the Foundation Module. Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	7.7584
Shear X	-2.4482
Shear Z	-0.0129
Moment X	-0.0725
Moment Y (Twist)	0.0930
Moment Z	45.0279

Reaction Forces for Foundation 4 (Node ID#301), (kip, kip-ft)

ASD Load Combination Results

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 1. D	-0.0065	3.1726	0.0033	0.0162	-0.0675	0.1396
ULS: 2. D + L	-0.0065	3.1726	0.0033	0.0162	-0.0675	0.1396
ULS: 3. D + (S or Lr or R)	-0.0165	6.5204	0.0084	0.0405	-0.1713	0.3267
ULS: 3. D + (S or Lr or R)	-0.0065	3.1726	0.0033	0.0162	-0.0675	0.1396
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	-0.0140	5.6834	0.0071	0.0344	-0.1454	0.2799
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	-0.0065	3.1726	0.0033	0.0162	-0.0675	0.1396
ULS: 5b. D + 0.7E	-0.0065	3.1726	0.0033	0.0162	-0.0675	0.1396
ULS: 6b. D + 0.75L + 0.75(0.7)E + 0.75S	-0.0140	5.6834	0.0071	0.0344	-0.1454	0.2799
ULS: 8. 0.6D + 0.7E	-0.0039	1.9036	0.0020	0.0097	-0.0405	0.0838
ULS: 5a. D + 0.6W_Wind downforce Case A only	-2.3583	5.4360	0.0002	-0.0013	-0.0380	43.6203
ULS: 5a. D + 0.6W_Wind downforce Case B only	-0.0065	3.1726	0.0033	0.0162	-0.0675	0.1396
ULS: 5a. D + 0.6W_Wind uplift Case A only	2.3443	0.9097	0.0059	0.0302	-0.0926	-42.2141
ULS: 5a. D + 0.6W_Wind uplift Case B only	-0.0065	3.1726	0.0033	0.0162	-0.0675	0.1396
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-1.7778	7.3810	0.0047	0.0213	-0.1232	32.8904
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-0.0140	5.6834	0.0071	0.0344	-0.1454	0.2799
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	1.7492	3.9863	0.0090	0.0449	-0.1642	-31.4854
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	-0.0140	5.6834	0.0071	0.0344	-0.1454	0.2799
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-1.7703	4.8702	0.0009	0.0031	-0.0454	32.7501
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-0.0065	3.1726	0.0033	0.0162	-0.0675	0.1396
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	1.7566	1.4755	0.0053	0.0267	-0.0863	-31.6257
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	-0.0065	3.1726	0.0033	0.0162	-0.0675	0.1396
ULS: 7. 0.6D + 0.6W_Wind downforce Case A only	-2.3557	4.1670	-0.0012	-0.0078	-0.0110	43.5645
ULS: 7. 0.6D + 0.6W_Wind downforce Case B only	-0.0039	1.9036	0.0020	0.0097	-0.0405	0.0838
ULS: 7. 0.6D + 0.6W_Wind uplift Case A only	2.3470	-0.3593	0.0046	0.0237	-0.0656	-42.2699
ULS: 7. 0.6D + 0.6W_Wind uplift Case B only	-0.0039	1.9036	0.0020	0.0097	-0.0405	0.0838

Worst Case Reactions LRFD

Worst Case Reactions ASD

These calculations are taken directly from the FEA via SkyCiv and are used in the Concrete Checks of the Foundatino Module.
Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	11.0502
Shear X	-3.9336
Shear Z	0.0151
Moment X	0.0749
Moment Y (Twist)	0.2742
Moment Z	74.2274

These results are taken from the worst case values in the above table and are used in the Soil Checks in the Foundation Module.
Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	7.3810
Shear X	-2.3583
Shear Z	0.0090
Moment X	0.0449
Moment Y (Twist)	0.1713
Moment Z	43.6203

Project Details

Design Code: AISC 360-16 LRFD
Provision: LRFD
Country: United States

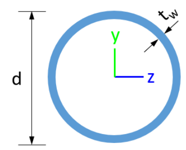
User Name: sales@mtsolar.us
Unit System: imperial

Design Input Information

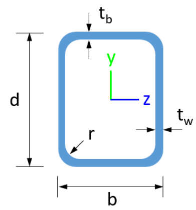
Design Factors			
Φ_t	Φ_c	Φ_b	Φ_v
0.9	0.9	0.9	0.9

Design Materials			
ID	E (ksi)	F_y (ksi)	F_u (ksi)
1	29000	50	65

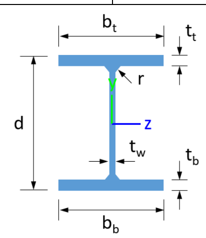
Section Dimensions



ID	Name	d (in)	t_w (in)
3	2in Pipe Sch 120	2.38	0.25
6	4in Pipe Sch 120	4.50	0.44
11	10in Pipe Sch 40	10.75	0.36



ID	Name	d (in)	b (in)	t_w (in)	t_b (in)	r (in)
17	HSS5x3x1/4	5.00	3.00	0.23	0.23	0.23



ID	Name	d (in)	t_w (in)	b_t (in)	b_b (in)	t_t (in)	t_b (in)	r (in)
20	W10x12	9.87	0.19	3.96	3.96	0.21	0.21	0.30

Section Properties								
ID	Name	A (in^2)	J (in^4)	I_{yp} (in^4)	I_{zp} (in^4)	I_w (in^6)	S_{yp} (in^3)	S_{zp} (in^3)
3	2in Pipe Sch 120	1.67	1.91	0.96	0.96	0.00	1.13	1.13
6	4in Pipe Sch 120	5.58	23.29	11.64	11.64	0.00	7.24	7.24
11	10in Pipe Sch 40	11.91	321.47	160.73	160.73	0.00	39.38	39.38

17	HSS5x3x1/4	3.37	11.00	4.81	10.70	62.42	3.77	5.38
20	W10x12	3.54	0.05	2.18	53.80	50.90	1.74	12.60

Member Properties								
Member ID	Section ID	K_zL (ft)	K_yL (ft)	L_b(ft)	C_b	LST	LSC	LD
1	11	38.29	38.29	18.23	-	300	200	1
2	6	1.30	1.30	2.00	-	300	200	1
3	17	0.92	0.92	1.42	1.19,1.18,1.19,1.18,1.19,1.19,1.18,1.18,1.17,1.18,1.18,1.19,1.17,1.19,1.18,1.18,1.18,1.18,1.18,1.19,1.15,1.19,1.18,1.19,1.17,1.19	300	200	1
4	17	2.44	2.44	3.75	1.69,1.68,1.68,1.67,1.68,1.68,1.67,1.68,1.65,1.68,1.67,1.68,1.66,1.68,1.67,1.67,1.68,1.67,1.67,1.68,1.63,1.68,1.67,1.68,1.66,1.68	300	200	1
5	17	1.52	1.52	2.33	1.68,1.67,1.68,1.67,1.68,1.68,1.67,1.67,1.66,1.67,1.67,1.68,1.66,1.68,1.67,1.67,1.67,1.67,1.67,1.68,1.64,1.68,1.67,1.68,1.66,1.68	300	200	1
6	17	0.92	0.92	1.42	1.19,1.18,1.19,1.18,1.18,1.19,1.18,1.18,1.17,1.18,1.18,1.19,1.17,1.19,1.18,1.18,1.18,1.18,1.18,1.19,1.16,1.19,1.18,1.19,1.18,1.19	300	200	1
7	17	1.52	1.52	2.33	1.68,1.67,1.68,1.67,1.68,1.68,1.67,1.67,1.66,1.67,1.67,1.68,1.66,1.68,1.67,1.67,1.67,1.67,1.67,1.68,1.65,1.68,1.67,1.68,1.66,1.68	300	200	1
8	20	1.33	1.33	2.05	2.15,2.15,2.15,2.15,2.15,2.15,2.12,2.15,2.06,2.15,2.11,2.15,2.08,2.15,2.13,2.15,2.19,2.15,2.12,2.15,2.03,2.15,2.11,2.15,2.09,2.15	300	200	1
9	3	2.60	2.60	4.00	-	300	200	1
10	17	2.44	2.44	3.75	1.69,1.68,1.69,1.67,1.68,1.69,1.67,1.68,1.65,1.68,1.67,1.69,1.66,1.69,1.67,1.67,1.68,1.67,1.67,1.69,1.63,1.69,1.67,1.69,1.66,1.69	300	200	1
11	20	1.33	1.33	2.05	2.18,2.18,2.18,2.18,2.18,2.18,2.06,2.18,1.88,2.18,2.05,2.18,1.95,2.18,2.11,2.18,2.23,2.18,2.07,2.18,1.81,2.18,2.04,2.18,1.97,2.18	300	200	1
12	6	1.30	1.30	2.00	-	300	200	1
13	20	4.88	4.00	7.50	1.08,1.08,1.08,1.08,1.08,1.08,1.07,1.08,1.05,1.08,1.07,1.08,1.06,1.08,1.07,1.08,1.09,1.08,1.07,1.08,1.04,1.08,1.07,1.08,1.06,1.08	300	200	1
14	20	4.88	4.00	7.50	1.08,1.08,1.08,1.08,1.08,1.08,1.07,1.08,1.06,1.08,1.07,1.08,1.06,1.08,1.07,1.08,1.08,1.08,1.07,1.08,1.05,1.08,1.07,1.08,1.06,1.08	300	200	1
15	20	12.60	12.60	6.00	2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33	300	200	1
16	20	12.60	12.60	6.00	2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33	300	200	1
101	11	38.29	38.29	18.23	-	300	200	1
102	6	1.30	1.30	2.00	-	300	200	1
103	17	0.92	0.92	1.42	1.19,1.18,1.19,1.18,1.18,1.19,1.18,1.18,1.17,1.18,1.18,1.19,1.17,1.19,1.18,1.18,1.18,1.18,1.18,1.19,1.16,1.19,1.18,1.19,1.17,1.19	300	200	1
104	17	2.44	2.44	3.75	1.68,1.68,1.68,1.67,1.68,1.68,1.67,1.68,1.65,1.68,1.67,1.68,1.66,1.68,1.67,1.67,1.68,1.67,1.67,1.68,1.63,1.68,1.67,1.68,1.66,1.68	300	200	1
105	17	1.52	1.52	2.33	1.68,1.67,1.68,1.67,1.68,1.68,1.67,1.67,1.66,1.67,1.67,1.68,1.66,1.68,1.67,1.67,1.67,1.67,1.67,1.68,1.64,1.68,1.67,1.68,1.66,1.68	300	200	1
106	17	0.92	0.92	1.42	1.19,1.18,1.19,1.18,1.18,1.19,1.18,1.18,1.17,1.18,1.18,1.19,1.17,1.19,1.18,1.18,1.18,1.18,1.18,1.19,1.16,1.19,1.18,1.19,1.18,1.19	300	200	1
107	17	1.52	1.52	2.33	1.68,1.67,1.68,1.67,1.68,1.68,1.67,1.67,1.66,1.67,1.67,1.68,1.66,1.68,1.67,1.67,1.67,1.67,1.67,1.68,1.64,1.68,1.67,1.68,1.66,1.68	300	200	1

108	20	1.33	1.33	2.05	2.30,2.30,2.30,2.30,2.30,2.30,2.34,2.30,2.13,2.30,2.35,2.30,2.21,2.30,2.32,2.30,2.26,2.30,2.34,2.30,2.11,2.30,2.35,2.30,2.28,2.30	300	200	1
109	3	2.60	2.60	4.00	-	300	200	1
110	17	2.44	2.44	3.75	1.68,1.68,1.68,1.67,1.68,1.68,1.67,1.68,1.65,1.68,1.67,1.68,1.66,1.68,1.67,1.67,1.68,1.67,1.67,1.68,1.63,1.68,1.67,1.68,1.66,1.68	300	200	1
111	20	1.33	1.33	2.05	2.28,2.28,2.28,2.28,2.28,2.28,2.34,2.28,2.07,2.28,2.25,2.28,2.11,2.28,2.32,2.28,2.13,2.28,2.36,2.28,1.79,2.28,2.20,2.28,2.12,2.28	300	200	1
112	6	1.30	1.30	2.00	-	300	200	1
113	20	4.88	4.00	7.50	1.03,1.03,1.03,1.03,1.03,1.03,1.03,1.03,1.05,1.03,1.03,1.03,1.04,1.03,1.03,1.03,1.03,1.03,1.03,1.03,1.06,1.03,1.03,1.03,1.04,1.03	300	200	1
114	20	4.88	4.00	7.50	1.03,1.03,1.03,1.03,1.03,1.03,1.03,1.03,1.04,1.03,1.03,1.03,1.04,1.03,1.03,1.03,1.03,1.03,1.03,1.03,1.05,1.03,1.03,1.03,1.03,1.03	300	200	1
115	20	8.42	8.42	12.95	1.17,1.17,1.17,1.17,1.17,1.17,1.16,1.17,1.14,1.17,1.16,1.17,1.15,1.17,1.16,1.17,1.20,1.17,1.16,1.17,1.14,1.17,1.16,1.17,1.15,1.17	300	200	1
116	20	8.42	8.42	12.95	1.17,1.17,1.17,1.17,1.17,1.17,1.16,1.17,1.15,1.17,1.16,1.17,1.15,1.17,1.16,1.17,1.18,1.17,1.16,1.17,1.15,1.17,1.16,1.17,1.16,1.17	300	200	1
201	11	38.29	38.29	18.23	-	300	200	1
202	6	1.30	1.30	2.00	-	300	200	1
203	17	0.92	0.92	1.42	1.19,1.18,1.19,1.18,1.18,1.19,1.18,1.18,1.17,1.18,1.18,1.19,1.17,1.19,1.18,1.18,1.18,1.18,1.18,1.19,1.16,1.19,1.18,1.19,1.18,1.19	300	200	1
204	17	2.44	2.44	3.75	1.68,1.68,1.68,1.67,1.68,1.67,1.68,1.65,1.68,1.67,1.68,1.66,1.68,1.67,1.67,1.68,1.67,1.67,1.68,1.63,1.68,1.67,1.68,1.66,1.68	300	200	1
205	17	1.52	1.52	2.33	1.68,1.67,1.68,1.67,1.68,1.68,1.67,1.67,1.66,1.67,1.67,1.68,1.66,1.68,1.67,1.67,1.67,1.67,1.67,1.68,1.64,1.68,1.67,1.68,1.66,1.68	300	200	1
206	17	0.92	0.92	1.42	1.19,1.18,1.19,1.18,1.18,1.19,1.18,1.18,1.17,1.18,1.18,1.19,1.17,1.19,1.18,1.18,1.18,1.18,1.18,1.19,1.16,1.19,1.18,1.19,1.17,1.19	300	200	1
207	17	1.52	1.52	2.33	1.68,1.67,1.68,1.67,1.68,1.68,1.67,1.67,1.66,1.67,1.67,1.68,1.66,1.68,1.67,1.67,1.67,1.67,1.67,1.68,1.64,1.68,1.67,1.68,1.66,1.68	300	200	1
208	20	1.33	1.33	2.05	2.28,2.28,2.28,2.28,2.28,2.28,2.31,2.28,2.14,2.28,2.32,2.28,2.37,2.28,2.29,2.28,2.23,2.28,2.31,2.28,2.11,2.28,2.32,2.28,2.35,2.28	300	200	1
209	3	2.60	2.60	4.00	-	300	200	1
210	17	2.44	2.44	3.75	1.68,1.68,1.68,1.67,1.68,1.68,1.67,1.68,1.65,1.68,1.67,1.68,1.66,1.68,1.67,1.67,1.68,1.67,1.67,1.68,1.63,1.68,1.67,1.68,1.66,1.68	300	200	1
211	20	1.33	1.33	2.05	2.26,2.26,2.26,2.26,2.26,2.26,2.35,2.26,2.00,2.26,2.37,2.26,2.10,2.26,2.30,2.26,2.01,2.26,2.34,2.26,1.68,2.26,2.35,2.26,2.12,2.26	300	200	1
212	6	1.30	1.30	2.00	-	300	200	1
213	20	4.88	4.00	7.50	1.03,1.03,1.03,1.03,1.03,1.03,1.03,1.03,1.05,1.03,1.03,1.03,1.04,1.03,1.03,1.03,1.03,1.03,1.03,1.03,1.06,1.03,1.03,1.03,1.04,1.03	300	200	1
214	20	4.88	4.00	7.50	1.03,1.03,1.03,1.03,1.03,1.03,1.03,1.03,1.04,1.03,1.03,1.03,1.03,1.03,1.03,1.03,1.03,1.03,1.03,1.03,1.05,1.03,1.03,1.03,1.03,1.03	300	200	1
215	20	8.42	8.42	12.95	1.17,1.17,1.17,1.17,1.17,1.17,1.15,1.17,1.13,1.17,1.15,1.17,1.14,1.17,1.16,1.17,1.19,1.17,1.16,1.17,1.12,1.17,1.15,1.17,1.14,1.17	300	200	1
216	20	8.42	8.42	12.95	1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.15,1.16,1.16,1.16,1.15,1.16,1.16,1.16,1.17,1.16,1.16,1.16,1.14,1.16,1.16,1.16,1.15,1.16	300	200	1
301	11	38.29	38.29	18.23	-	300	200	1

302	6	1.30	1.30	2.00	-	300	200	1
303	17	0.92	0.92	1.42	1.19,1.18,1.19,1.18,1.18,1.19,1.18,1.18,1.17,1.18,1.18,1.19,1.17,1.19,1.18,1.18,1.18,1.18,1.18,1.19,1.16,1.19,1.18,1.19,1.18,1.19	300	200	1
304	17	2.44	2.44	3.75	1.69,1.68,1.69,1.67,1.68,1.69,1.67,1.68,1.65,1.68,1.67,1.69,1.66,1.69,1.67,1.67,1.68,1.67,1.67,1.69,1.63,1.69,1.67,1.69,1.66,1.69	300	200	1
305	17	1.52	1.52	2.33	1.68,1.67,1.68,1.67,1.68,1.68,1.67,1.67,1.66,1.67,1.67,1.68,1.66,1.68,1.67,1.67,1.67,1.67,1.67,1.68,1.65,1.68,1.67,1.68,1.66,1.68	300	200	1
306	17	0.92	0.92	1.42	1.19,1.18,1.19,1.18,1.19,1.19,1.18,1.18,1.17,1.18,1.18,1.19,1.17,1.19,1.18,1.18,1.18,1.18,1.18,1.19,1.15,1.19,1.18,1.19,1.17,1.19	300	200	1
307	17	1.52	1.52	2.33	1.68,1.67,1.68,1.67,1.68,1.68,1.67,1.67,1.66,1.67,1.67,1.68,1.66,1.68,1.67,1.67,1.67,1.67,1.67,1.68,1.64,1.68,1.67,1.68,1.66,1.68	300	200	1
308	20	12.60	12.60	6.00	2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33	300	200	1
309	3	2.60	2.60	4.00	-	300	200	1
310	17	2.44	2.44	3.75	1.69,1.68,1.68,1.67,1.68,1.68,1.67,1.68,1.65,1.68,1.67,1.68,1.66,1.68,1.67,1.67,1.68,1.67,1.67,1.68,1.63,1.68,1.67,1.68,1.66,1.68	300	200	1
311	20	12.60	12.60	6.00	2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33	300	200	1
312	6	1.30	1.30	2.00	-	300	200	1
313	20	4.88	4.00	7.50	1.08,1.08,1.08,1.08,1.08,1.08,1.07,1.08,1.05,1.08,1.07,1.08,1.06,1.08,1.07,1.08,1.09,1.08,1.07,1.08,1.04,1.08,1.07,1.08,1.06,1.08	300	200	1
314	20	4.88	4.00	7.50	1.08,1.08,1.08,1.08,1.08,1.08,1.07,1.08,1.06,1.08,1.07,1.08,1.06,1.08,1.07,1.08,1.08,1.08,1.07,1.08,1.05,1.08,1.07,1.08,1.07,1.08	300	200	1
315	20	8.42	8.42	12.95	1.19,1.19,1.19,1.19,1.19,1.19,1.20,1.19,1.22,1.19,1.20,1.19,1.21,1.19,1.20,1.19,1.18,1.19,1.20,1.19,1.23,1.19,1.20,1.19,1.21,1.19	300	200	1
316	20	8.42	8.42	12.95	1.19,1.20,1.19,1.20,1.19,1.19,1.20,1.20,1.20,1.20,1.20,1.19,1.20,1.19,1.20,1.20,1.19,1.20,1.20,1.19,1.21,1.19,1.20,1.19,1.20,1.19	300	200	1

Member Design Capacity

Member ID	$\Phi_t P_n$ (kip)	$\Phi_c P_n$ (kip)	$\Phi_b M_{zn}$ (k-ft)	$\Phi_b M_{yn}$ (k-ft)	$\Phi_v V_{yn}$ (kip)	$\Phi_v V_{zn}$ (kip)
1	535.87	171.98	147.68	147.68	160.76	160.76
2	251.01	248.88	27.16	27.16	75.30	75.30
3	151.65	150.70	20.17	14.14	54.12	28.95
4	151.65	145.15	20.17	14.14	54.12	28.95
5	151.65	149.10	20.17	14.14	54.12	28.95
6	151.65	150.70	20.17	14.14	54.12	28.95
7	151.65	149.10	20.17	14.14	54.12	28.95
8	159.30	142.47	46.90	6.46	56.26	44.91
9	75.10	66.32	4.25	4.25	22.53	22.53
10	151.65	145.15	20.17	14.14	54.12	28.95
11	159.30	142.47	46.90	6.46	56.26	44.91
12	251.01	248.88	27.16	27.16	75.30	75.30
13	159.30	116.35	31.78	6.46	56.26	44.91
14	159.30	116.35	32.09	6.46	56.26	44.91
15	159.30	21.54	46.90	6.46	56.26	44.91
16	159.30	21.54	46.90	6.46	56.26	44.91
101	535.87	171.98	147.68	147.68	160.76	160.76
102	251.01	248.88	27.16	27.16	75.30	75.30
103	151.65	150.70	20.17	14.14	54.12	28.95

103	151.65	150.70	20.17	14.14	54.12	28.95
104	151.65	145.15	20.17	14.14	54.12	28.95
105	151.65	149.10	20.17	14.14	54.12	28.95
106	151.65	150.70	20.17	14.14	54.12	28.95
107	151.65	149.10	20.17	14.14	54.12	28.95
108	159.30	142.47	46.90	6.46	56.26	44.91
109	75.10	66.32	4.25	4.25	22.53	22.53
110	151.65	145.15	20.17	14.14	54.12	28.95
111	159.30	142.47	46.90	6.46	56.26	44.91
112	251.01	248.88	27.16	27.16	75.30	75.30
113	159.30	116.35	31.48	6.46	56.26	44.91
114	159.30	116.35	31.48	6.46	56.26	44.91
115	159.30	48.27	15.14	6.46	56.26	44.91
116	159.30	48.27	15.28	6.46	56.26	44.91
201	535.87	171.98	147.68	147.68	160.76	160.76
202	251.01	248.88	27.16	27.16	75.30	75.30
203	151.65	150.70	20.17	14.14	54.12	28.95
204	151.65	145.15	20.17	14.14	54.12	28.95
205	151.65	149.10	20.17	14.14	54.12	28.95
206	151.65	150.70	20.17	14.14	54.12	28.95
207	151.65	149.10	20.17	14.14	54.12	28.95
208	159.30	142.47	46.90	6.46	56.26	44.91
209	75.10	66.32	4.25	4.25	22.53	22.53
210	151.65	145.15	20.17	14.14	54.12	28.95
211	159.30	142.47	46.90	6.46	56.26	44.91
212	251.01	248.88	27.16	27.16	75.30	75.30
213	159.30	116.35	31.48	6.46	56.26	44.91
214	159.30	116.35	31.48	6.46	56.26	44.91
215	159.30	48.27	14.88	6.46	56.26	44.91
216	159.30	48.27	15.14	6.46	56.26	44.91
301	535.87	171.98	147.68	147.68	160.76	160.76
302	251.01	248.88	27.16	27.16	75.30	75.30
303	151.65	150.70	20.17	14.14	54.12	28.95
304	151.65	145.15	20.17	14.14	54.12	28.95
305	151.65	149.10	20.17	14.14	54.12	28.95
306	151.65	150.70	20.17	14.14	54.12	28.95
307	151.65	149.10	20.17	14.14	54.12	28.95
308	159.30	21.54	46.90	6.46	56.26	44.91
309	75.10	66.32	4.25	4.25	22.53	22.53
310	151.65	145.15	20.17	14.14	54.12	28.95
311	159.30	21.54	46.90	6.46	56.26	44.91
312	251.01	248.88	27.16	27.16	75.30	75.30
313	159.30	116.35	31.78	6.46	56.26	44.91
314	159.30	116.35	32.09	6.46	56.26	44.91
315	159.30	48.27	15.67	6.46	56.26	44.91
316	159.30	48.27	15.81	6.46	56.26	44.91

Design Ratio

Member ID	P	M_z	M_y	V_y	V_z	(P,M_z,M_y)	Worst LC	KL/r	δ	Status
1	0.064	0.503	0.001	0.024	0.000	0.530	#13	0.625	Not Required	Pass
2	0.006	0.285	0.155	0.067	0.026	0.403	#21	0.036	Not Required	Pass
3	0.010	0.399	0.053	0.040	0.005	0.440	#21	0.046	Not Required	Pass
4	0.009	0.308	0.222	0.040	0.046	0.535	#21	0.082	Not Required	Pass

4	0.009	0.398	0.222	0.040	0.046	0.525	#21	0.082	Not Required	Pass
5	0.009	0.247	0.231	0.039	0.060	0.304	#21	0.076	Not Required	Pass
6	0.010	0.400	0.057	0.040	0.004	0.438	#21	0.046	Not Required	Pass
7	0.010	0.248	0.228	0.039	0.061	0.308	#21	0.076	Not Required	Pass
8	0.000	0.047	0.276	0.027	0.023	0.316	#21	0.102	Not Required	Pass
9	0.028	0.030	0.042	0.001	0.000	0.080	#21	0.206	Not Required	Pass
10	0.010	0.395	0.220	0.039	0.047	0.526	#21	0.082	Not Required	Pass
11	0.000	0.048	0.280	0.027	0.023	0.320	#21	0.102	Not Required	Pass
12	0.006	0.281	0.152	0.068	0.027	0.400	#13	0.036	Not Required	Pass
13	0.011	0.191	0.631	0.034	0.028	0.810	#21	0.306	Not Required	Pass
14	0.011	0.192	0.631	0.033	0.028	0.810	#21	0.204	Not Required	Pass
15	0.000	0.076	0.378	0.021	0.018	0.453	#21	Not Required	Not Required	Pass
16	0.000	0.076	0.378	0.021	0.018	0.453	#21	Not Required	Not Required	Pass
101	0.068	0.518	0.002	0.025	0.000	0.547	#13	0.625	Not Required	Pass
102	0.005	0.295	0.154	0.071	0.027	0.408	#21	0.036	Not Required	Pass
103	0.010	0.411	0.052	0.041	0.001	0.467	#21	0.046	Not Required	Pass
104	0.010	0.411	0.212	0.041	0.045	0.544	#21	0.082	Not Required	Pass
105	0.010	0.255	0.219	0.040	0.057	0.313	#21	0.076	Not Required	Pass
106	0.010	0.423	0.053	0.042	0.002	0.473	#21	0.046	Not Required	Pass
107	0.010	0.262	0.219	0.042	0.057	0.317	#21	0.076	Not Required	Pass
108	0.000	0.033	0.272	0.026	0.023	0.305	#21	0.102	Not Required	Pass
109	0.025	0.029	0.041	0.001	0.000	0.077	#21	0.206	Not Required	Pass
110	0.010	0.421	0.211	0.042	0.045	0.547	#21	0.082	Not Required	Pass
111	0.000	0.032	0.275	0.026	0.023	0.308	#21	0.102	Not Required	Pass
112	0.005	0.302	0.160	0.072	0.028	0.424	#13	0.036	Not Required	Pass
113	0.010	0.152	0.584	0.033	0.028	0.723	#21	0.306	Not Required	Pass
114	0.011	0.155	0.581	0.033	0.028	0.721	#21	0.306	Not Required	Pass
115	0.000	0.237	0.313	0.025	0.023	0.544	#21	0.644	Not Required	Pass
116	0.000	0.234	0.316	0.026	0.023	0.547	#21	0.644	Not Required	Pass
201	0.068	0.518	0.002	0.025	0.000	0.547	#13	0.625	Not Required	Pass
202	0.005	0.302	0.160	0.072	0.028	0.424	#13	0.036	Not Required	Pass
203	0.010	0.423	0.053	0.042	0.002	0.473	#21	0.046	Not Required	Pass
204	0.010	0.421	0.211	0.042	0.045	0.547	#21	0.082	Not Required	Pass
205	0.010	0.262	0.219	0.042	0.057	0.317	#21	0.076	Not Required	Pass
206	0.010	0.411	0.052	0.041	0.001	0.467	#21	0.046	Not Required	Pass
207	0.010	0.255	0.219	0.040	0.057	0.313	#21	0.076	Not Required	Pass
208	0.000	0.034	0.273	0.026	0.023	0.307	#21	0.102	Not Required	Pass
209	0.025	0.029	0.041	0.001	0.000	0.076	#21	0.206	Not Required	Pass
210	0.010	0.411	0.212	0.041	0.045	0.544	#21	0.082	Not Required	Pass
211	0.000	0.033	0.276	0.025	0.023	0.309	#21	0.102	Not Required	Pass
212	0.005	0.295	0.154	0.071	0.027	0.408	#21	0.036	Not Required	Pass
213	0.010	0.152	0.584	0.033	0.028	0.723	#21	0.306	Not Required	Pass
214	0.011	0.155	0.581	0.033	0.028	0.721	#21	0.306	Not Required	Pass
215	0.001	0.266	0.313	0.026	0.023	0.567	#21	0.644	Not Required	Pass
216	0.001	0.262	0.316	0.026	0.023	0.570	#21	0.644	Not Required	Pass
301	0.064	0.503	0.001	0.024	0.000	0.530	#13	0.625	Not Required	Pass
302	0.006	0.281	0.152	0.068	0.027	0.400	#13	0.036	Not Required	Pass
303	0.010	0.400	0.057	0.040	0.004	0.438	#21	0.046	Not Required	Pass
304	0.010	0.395	0.220	0.039	0.047	0.526	#21	0.082	Not Required	Pass
305	0.010	0.248	0.228	0.039	0.061	0.308	#21	0.076	Not Required	Pass
306	0.010	0.399	0.053	0.040	0.005	0.440	#21	0.046	Not Required	Pass
307	0.009	0.247	0.231	0.039	0.060	0.304	#21	0.076	Not Required	Pass
308	0.000	0.076	0.378	0.021	0.018	0.453	#21	Not Required	Not Required	Pass
309	0.028	0.030	0.042	0.001	0.000	0.080	#21	0.206	Not Required	Pass

310	0.009	0.398	0.222	0.040	0.046	0.525	#21	0.082	Not Required	Pass
311	0.000	0.076	0.378	0.021	0.018	0.453	#21	Not Required	Not Required	Pass
312	0.006	0.285	0.155	0.067	0.026	0.403	#21	0.036	Not Required	Pass
313	0.011	0.191	0.631	0.034	0.028	0.810	#21	0.204	Not Required	Pass
314	0.011	0.192	0.631	0.033	0.028	0.810	#21	0.306	Not Required	Pass
315	0.000	0.235	0.313	0.027	0.023	0.539	#21	0.644	Not Required	Pass
316	0.000	0.234	0.316	0.027	0.023	0.542	#21	0.644	Not Required	Pass

Definitions

Φ_t	Safety factor for tensile
Φ_c	Safety factor for compression
Φ_b	Safety factor for flexure
Φ_v	Safety factor for shear
E	Modulus of elasticity
F_y	Specified minimum yield stress
F_u	Specified minimum tensile strength
A	Cross-sectional area
J	Torsional constant
I_{yp}	Moment of inertia about the Y axes
I_{zp}	Moment of inertia about the Z axes
I_w	Warping constant
S_{yp}	Plastic section modulus about the Y axis
S_{zp}	Plastic section modulus about the Z axis
KL	Effective length
C_b	Buckling modification factor (from all load combinations)
L_b	Length between braced points
LST	Limited slenderness for tension
LSC	Limited slenderness for compression
LD	Limited deflection
P_n	Nominal axial strength (tension/comression)
M_n	Nominal flexural strength (about Z/Y axis)
V_n	Nominal shear strength (along Z/Y axis)
P	Design ratio in case of axial force
M_z	Design ratio in case of bending about Z axis
M_y	Design ratio in case of bending about Y axis
V_y	Design ratio in case of shear along Y axis
V_z	Design ratio in case of shear along Z axis
(P,M_z,M_y)	Design ratio in case of axial force and bending action
KL/r	Design ratio in case of section slenderness
δ	Design ratio in case of member deflection
OK	Capacity is provided
NG	Capacity is not provided

REFERENCES	CALCULATIONS	RESULTS

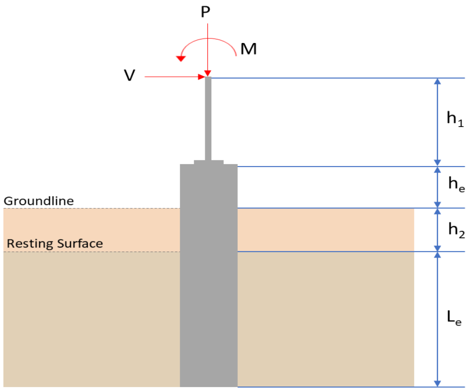
SkyCiv Foundation Design

Pile Foundation

Design Information :

Design code : IBC 2021 (International Building Code)
Unit System : Imperial

Pile Input



Geometry

Pile shape: rectangular
$b = 48\ \text{in}$ - Pile width
$D = 48\ \text{in}$ - Pile depth
$L = 7.75\ \text{ft}$ - Total pile length
$h_1 = 0\ \text{ft}$ - Lateral load height from the top of the pile,
$h_2 = 0\ \text{ft}$ - Depth to resisting surface
$h_e = 0\ \text{ft}$ - Length of pile above the ground

Tabulation of Soil Parameters

Layer	Label	Allowable Bearing Pressure (q_a) (psf)	Allowable Lateral Pressure (R) (psf/ft)
1	Sand, silty sand, clayey sand, silty gravel & clayey gravel	2000.000	150.000

Tabulation of Loads

Load Component	ASD	LRFD
P (kip)	7.381	11.050
V_x (kip)	-2.358	-3.934
V_z (kip)	-0.009	-0.015
M_x (kipft)	-0.045	-0.075
M_z (kipft)	43.620	74.227

Material Properties

$f'_{ck} = 3\ \text{ksi}$ - Concrete strength,

Required depth to resist lateral loads (ASD)

H - Point of application of the lateral load

$$H = h_1 + h_2 + h_e$$

$$H = (0\ \text{ft}) + (0\ \text{ft}) + (0\ \text{ft})$$

$$H = 0\ \text{ft}$$

Considering x-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\ D}$$

$$H_o = \frac{(-2.358\ \text{kip})}{1.57 \times (48\ \text{in})}$$

$$H_o = -0.37548\ \text{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\ H)}{1.57\ D}$$

$$M_o = \frac{(43.62 \text{ kipft}) + ((-2.358 \text{ kip}) \times (0 \text{ ft}))}{1.57 \times (48 \text{ in})}$$

$$M_o = 6.9459 \text{ kipft/ft}$$

Required depth of embedment in earth:

$$L_x^3 - \left(14.14 \times \frac{H_o \times L_x}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,x} = 7.312 \text{ ft}$ - Required depth in x-direction,

Considering z-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\, b}$$

$$H_o = \frac{(-0.009 \text{ kip})}{1.57 \times (48 \text{ in})}$$

$$H_o = -0.0014331 \text{ kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\, H)}{1.57\, b}$$

$$M_o = \frac{(0.045 \text{ kipft}) + ((-0.009 \text{ kip}) \times (0 \text{ ft}))}{1.57 \times (48 \text{ in})}$$

$$M_o = 0.0071656 \text{ kipft/ft}$$

Required depth of embedment in earth:

$$L_z^3 - \left(14.14 \times \frac{H_o \times L_z}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,z} = 0.79648 \text{ ft}$ - Required depth in z-direction,

Minimum embedded depth required:

$L_{e,req}$ - Depth of pile required,

$$L_{e,req} = MAX[L_{e,x}, L_{e,z}]$$

$$L_{e,req} = MAX[(7.312 \text{ ft}), (0.79648 \text{ ft})]$$

$$L_{e,req} = 7.312 \text{ ft}$$

L_e - Actual embedded length of pile,

$$L_e = L - h_e - h_2$$

$$L_e = (7.75 \text{ ft}) - (0 \text{ ft}) - (0 \text{ ft})$$

$$L_e = 7.75 \text{ ft}$$

$Ratio$ - Embedded depth

$$Ratio = \frac{L_{e,req}}{L_e}$$

$$Ratio = \frac{(7.312 \text{ ft})}{(7.75 \text{ ft})}$$

$$Ratio = 0.94348$$

Status: **PASS**
Ratio: **0.940**

End-bearing Capacity (ASD)

A - Pile cross-section area

$$A = b\, D$$

$$A = (48 \text{ in}) \times (48 \text{ in})$$

$$A = 16 \text{ ft}^2$$

q - End-bearing pressure

$$q = \frac{P_v}{A}$$

$$q = \frac{(7.381 \text{ kip})}{(16 \text{ ft}^2)}$$

$$q = 0.46131 \text{ kip/ft}^2$$

$q = 0.46131\ \text{kip/ft}^2$

Check bearing capacity ratio:

$Ratio$ - Capacity

$$Ratio = \frac{q}{q_a}$$

$$Ratio = \frac{(0.46131\ \text{kip/ft}^2)}{(2000\ \text{psf})}$$

$$Ratio = 0.23066$$

Status: **PASS**
Ratio: **0.230**

Czerniak

Lateral Soil Pressure (ASD):

L/D - Length to least lateral dimension ratio,

$$L/D = \frac{L}{D}$$

$$L/D = \frac{(7.75\ \text{ft})}{(48\ \text{in})}$$

$$L/D = 1.9375$$

Since L/D $\leq$ 10,

Pile is short.

Considering x-direction:

$H_o = -0.37548\ \text{kip/ft}$ - Lateral force per length of pile,
$M_o = 6.9459\ \text{kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (6.9459\ \text{kipft/ft}) \times (7.75\ \text{ft})) + \left(3 \times (-0.37548\ \text{kip/ft}) \times (7.75\ \text{ft})^2\right)}{(6 \times (6.9459\ \text{kipft/ft})) + (4 \times (-0.37548\ \text{kip/ft}) \times (7.75\ \text{ft}))}$$

$$a = 5.3077\ \text{ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\,[(4\,M_o) + (3\,H_o\,L_e)]^2}{L_e^2\,[(3\,M_o) + (2\,H_o\,L_e)]}$$

$$p = \frac{0.75 \times [(4 \times (6.9459\ \text{kipft/ft})) + (3 \times (-0.37548\ \text{kip/ft}) \times (7.75\ \text{ft}))]^2}{(7.75\ \text{ft})^2 \times [(3 \times (6.9459\ \text{kipft/ft})) + (2 \times (-0.37548\ \text{kip/ft}) \times (7.75\ \text{ft}))]}$$

$$p = 0.30186\ \text{kip/ft}^2$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\,[(2\,M_o) + (H_o\,L_e)]}{L_e^2}$$

$$s = \frac{6 \times [(2 \times (6.9459\ \text{kipft/ft})) + ((-0.37548\ \text{kip/ft}) \times (7.75\ \text{ft}))]}{(7.75\ \text{ft})^2}$$

$$s = 1.097\ \text{kip/ft}^2$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\,\frac{a}{2}$$

$$p_a = (150\ \text{psf/ft}) \times \frac{(5.3077\ \text{ft})}{2}$$

$$p_a = 0.39807\ \text{kip/ft}^2$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{(0.30186\ \text{kip/ft}^2)}{(0.39807\ \text{kip/ft}^2)}$$

$$Ratio = 0.7583$$

Status: **PASS**
Ratio: **0.760**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\, L_e$$

$$p_s = (150\ \mathrm{psf/ft}) \times (7.75\ \mathrm{ft})$$

$$p_s = 1.1625\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{\left(1.097\ \mathrm{kip/ft^2}\right)}{\left(1.1625\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = 0.94369$$

Status: PASS
Ratio: 0.940

Considering z-direction:

$H_o = -0.0014331\ \mathrm{kip/ft}$ - Lateral force per length of pile,
$M_o = 0.0071656\ \mathrm{kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\, M_o\, L_e) + \left(3\, H_o\, L_e^2\right)}{(6\, M_o) + (4\, H_o\, L_e)}$$

$$a = \frac{(4 \times (0.0071656\ \mathrm{kipft/ft}) \times (7.75\ \mathrm{ft})) + \left(3 \times (-0.0014331\ \mathrm{kip/ft}) \times (7.75\ \mathrm{ft})^2\right)}{(6 \times (0.0071656\ \mathrm{kipft/ft})) + (4 \times (-0.0014331\ \mathrm{kip/ft}) \times (7.75\ \mathrm{ft}))}$$

$$a = 5.4949\ \mathrm{ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\, [(4\, M_o) + (3\, H_o\, L_e)]^2}{L_e^2\, [(3\, M_o) + (2\, H_o\, L_e)]}$$

$$p = \frac{0.75 \times [(4 \times (0.0071656\ \mathrm{kipft/ft})) + (3 \times (-0.0014331\ \mathrm{kip/ft}) \times (7.75\ \mathrm{ft}))]^2}{(7.75\ \mathrm{ft})^2 \times [(3 \times (0.0071656\ \mathrm{kipft/ft})) + (2 \times (-0.0014331\ \mathrm{kip/ft}) \times (7.75\ \mathrm{ft}))]}$$

$$p = -0.00037804\ \mathrm{kip/ft^2}$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\, [(2\, M_o) + (H_o\, L_e)]}{L_e^2}$$

$$s = \frac{6 \times [(2 \times (0.0071656\ \mathrm{kipft/ft})) + ((-0.0014331\ \mathrm{kip/ft}) \times (7.75\ \mathrm{ft}))]}{(7.75\ \mathrm{ft})^2}$$

$$s = 0.00032212\ \mathrm{kip/ft^2}$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\, \frac{a}{2}$$

$$p_a = (150\ \mathrm{psf/ft}) \times \frac{(5.4949\ \mathrm{ft})}{2}$$

$$p_a = 0.41212\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{\left(-0.00037804\ \mathrm{kip/ft^2}\right)}{\left(0.41212\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = -0.00091731$$

Status: PASS
Ratio: 0.000

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\, L_e$$

$$p_s = (150\ \mathrm{psf/ft}) \times (7.75\ \mathrm{ft})$$

$$p_s = 1.1625\ \mathrm{kip/ft^2}$$

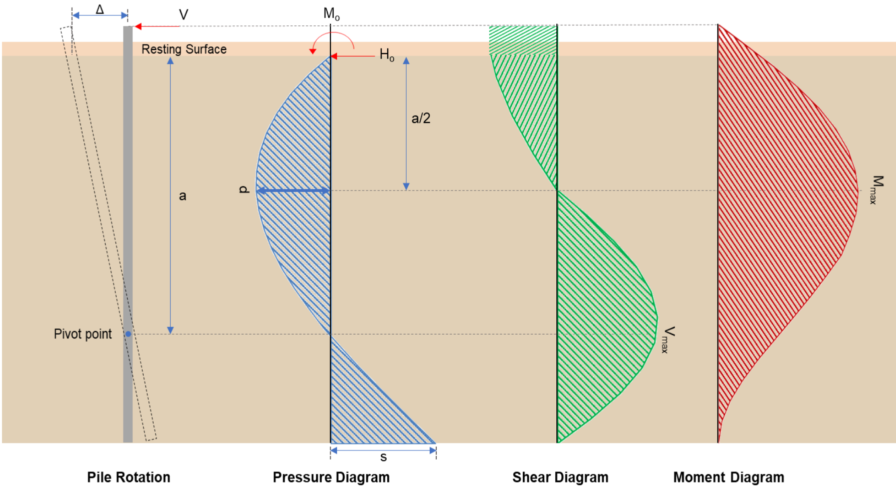
$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{(0.00032212 \text{ kip/ft}^2)}{(1.1625 \text{ kip/ft}^2)}$$

$$Ratio = 0.00027709$$

Status: **PASS**
Ratio: **0.000**



Shear force and Bending moment (x-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\,D}$$

$$H_o = \frac{(-3.934 \text{ kip})}{1.57 \times (48 \text{ in})}$$

$$H_o = -0.62643 \text{ kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\,H)}{1.57\,D}$$

$$M_o = \frac{(74.227 \text{ kipft}) + ((-3.934 \text{ kip}) \times (0 \text{ ft}))}{1.57 \times (48 \text{ in})}$$

$$M_o = 11.82 \text{ kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(11.82 \text{ kipft/ft})}{(-0.62643 \text{ kip/ft})}$$

$$E = 18.868 \text{ ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (11.82 \text{ kipft/ft}) \times (7.75 \text{ ft})) + \left(3 \times (-0.62643 \text{ kip/ft}) \times (7.75 \text{ ft})^2\right)}{(6 \times (11.82 \text{ kipft/ft})) + (4 \times (-0.62643 \text{ kip/ft}) \times (7.75 \text{ ft}))}$$

$$a = 5.3055 \text{ ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\,D)\left[1 - \left[3\left(\frac{4\,E}{L_e} + 3\right)\left(\frac{a}{L_e}\right)^2\right] + \left[4\left(\frac{3\,E}{L_e} + 2\right)\left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((-0.62643 \text{ kip/ft}) \times (48 \text{ in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (18.868 \text{ ft})}{(7.75 \text{ ft})} + 3\right) \times \left(\frac{(5.3055 \text{ ft})}{(7.75 \text{ ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (18.868 \text{ ft})}{(7.75 \text{ ft})} + 2\right) \times \left(\frac{(5.3055 \text{ ft})}{(7.75 \text{ ft})}\right)^3\right]\right]$$

$$V_{max} = 12.453 \text{ kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, D\, L_e) \left[\left(\frac{E}{L_e} + \frac{a}{2\, L_e}\right) - \left[\left(\frac{4\, E}{L_e} + 3\right) \left(\frac{a}{2\, L_e}\right)^3\right] + \left[\left(\frac{3\, E}{L_e} + 2\right) \left(\frac{a}{2\, L_e}\right)^4\right]\right]$$

$$M_{max} = ((-0.62643\ \mathrm{kip/ft}) \times (48\ \mathrm{in}) \times (7.75\ \mathrm{ft})) \times \left[\left(\frac{(18.868\ \mathrm{ft})}{(7.75\ \mathrm{ft})} + \frac{(5.3055\ \mathrm{ft})}{2 \times (7.75\ \mathrm{ft})}\right) - \left[\left(\frac{4 \times (18.868\ \mathrm{ft})}{(7.75\ \mathrm{ft})} + 3\right) \times \left(\frac{(5.3055\ \mathrm{ft})}{2 \times (7.75\ \mathrm{ft})}\right)^3\right] + \left[\left(\frac{3 \times (18.868\ \mathrm{ft})}{(7.75\ \mathrm{ft})} + 2\right) \times \left(\frac{(5.3055\ \mathrm{ft})}{2 \times (7.75\ \mathrm{ft})}\right)^4\right]\right]$$

$$M_{max} = 46.485\ \mathrm{kipft}$$

Shear force and Bending moment (z-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\, b}$$

$$H_o = \frac{(-0.015\ \mathrm{kip})}{1.57 \times (48\ \mathrm{in})}$$

$$H_o = -0.0023885\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\, H)}{1.57\, b}$$

$$M_o = \frac{(0.075\ \mathrm{kipft}) + ((-0.015\ \mathrm{kip}) \times (0\ \mathrm{ft}))}{1.57 \times (48\ \mathrm{in})}$$

$$M_o = 0.011943\ \mathrm{kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(0.011943\ \mathrm{kipft/ft})}{(-0.0023885\ \mathrm{kip/ft})}$$

$$E = 5\ \mathrm{ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\, M_o\, L_e) + \left(3\, H_o\, L_e^2\right)}{(6\, M_o) + (4\, H_o\, L_e)}$$

$$a = \frac{(4 \times (0.011943\ \mathrm{kipft/ft}) \times (7.75\ \mathrm{ft})) + \left(3 \times (-0.0023885\ \mathrm{kip/ft}) \times (7.75\ \mathrm{ft})^2\right)}{(6 \times (0.011943\ \mathrm{kipft/ft})) + (4 \times (-0.0023885\ \mathrm{kip/ft}) \times (7.75\ \mathrm{ft}))}$$

$$a = 5.4949\ \mathrm{ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\, b) \left[1 - \left[3\left(\frac{4\, E}{L_e} + 3\right) \left(\frac{a}{L_e}\right)^2\right] + \left[4\left(\frac{3\, E}{L_e} + 2\right) \left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((-0.0023885\ \mathrm{kip/ft}) \times (48\ \mathrm{in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (5\ \mathrm{ft})}{(7.75\ \mathrm{ft})} + 3\right) \times \left(\frac{(5.4949\ \mathrm{ft})}{(7.75\ \mathrm{ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (5\ \mathrm{ft})}{(7.75\ \mathrm{ft})} + 2\right) \times \left(\frac{(5.4949\ \mathrm{ft})}{(7.75\ \mathrm{ft})}\right)^3\right]\right]$$

$$V_{max} = 0.017249\ \mathrm{kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, b\, L_e) \left[\left(\frac{E}{L_e} + \frac{a}{2\, L_e}\right) - \left[\left(\frac{4\, E}{L_e} + 3\right) \left(\frac{a}{2\, L_e}\right)^3\right] + \left[\left(\frac{3\, E}{L_e} + 2\right) \left(\frac{a}{2\, L_e}\right)^4\right]\right]$$

$$M_{max} = ((-0.0023885\ \mathrm{kip/ft}) \times (48\ \mathrm{in}) \times (7.75\ \mathrm{ft})) \times \left[\left(\frac{(5\ \mathrm{ft})}{(7.75\ \mathrm{ft})} + \frac{(5.4949\ \mathrm{ft})}{2 \times (7.75\ \mathrm{ft})}\right) - \left[\left(\frac{4 \times (5\ \mathrm{ft})}{(7.75\ \mathrm{ft})} + 3\right) \times \left(\frac{(5.4949\ \mathrm{ft})}{2 \times (7.75\ \mathrm{ft})}\right)^3\right] + \left[\left(\frac{3 \times (5\ \mathrm{ft})}{(7.75\ \mathrm{ft})} + 2\right) \times \left(\frac{(5.4949\ \mathrm{ft})}{2 \times (7.75\ \mathrm{ft})}\right)^4\right]\right]$$

$$M_{max} = 0.060213 \text{ kipft}$$

Minimum Reinforcement Check (LRFD)

Parameters:

$f'_{ck} = 3 \text{ ksi}$ - Concrete strength,
$f_{yk} = 60 \text{ ksi}$ - Longitudinal reinforcement strength,
$\phi = 0.65$ - Reduction factor for axial strength,
Table 22.4.2.1 $\alpha = 0.8$ - Alpha factor for axial strength,
$A_g = 2304 \text{ in}^2$ - Gross area of concrete,

Longitudinal reinforcement:

Required reinforcement due to axial load, $A_{st,required}$
22.4.2.2, 10.6.1.1 $A_{st,required}$

$$A_{st,required} = Min\left[\frac{\frac{P}{\phi\ \alpha} - (0.85\ f'_{ck}\ A_g)}{f_{yk} - (0.85\ f'_{ck})}, (0.08\ A_g)\right]$$

$$A_{st,required} = Min\left[\frac{\frac{(11.05 \text{ kip})}{(0.65)\times(0.8)} - \left(0.85 \times (3 \text{ ksi}) \times \left(2304 \text{ in}^2\right)\right)}{(60 \text{ ksi}) - (0.85 \times (3 \text{ ksi}))}, \left(0.08 \times \left(2304 \text{ in}^2\right)\right)\right]$$

$$A_{st,required} = -101.9 \text{ in}^2$$

A_{min} - Governing minimum reinforcement area,

$$A_{min} = Max\left[A_{st,required}, (0.0018\ A_g)\right]$$

$$A_{min} = Max\left[\left(-101.9 \text{ in}^2\right), \left(0.0018 \times \left(2304 \text{ in}^2\right)\right)\right]$$

$$A_{min} = 4.1472 \text{ in}^2$$

n_{rebar} - Required number of reinforcement,

$$n_{rebar} = \frac{A_{min}}{A_{rebar}}$$

$$n_{rebar} = \frac{\left(4.1472 \text{ in}^2\right)}{\left(0.3068 \text{ in}^2\right)}$$

$$n_{rebar} = 14$$

A_{st} - Actual total reinforcement area,

$$A_{st} = n_{rebar}\ \frac{\pi\ d_{bar}^2}{4}$$

$$A_{st} = (14) \times \frac{\pi \times (0.625 \text{ in})^2}{4}$$

$$A_{st} = 4.2951 \text{ in}^2$$

$Ratio$ - Capacity

$$Ratio = \frac{A_{min}}{A_{st}}$$

$$Ratio = \frac{\left(4.1472 \text{ in}^2\right)}{\left(4.2951 \text{ in}^2\right)}$$

$$Ratio = 0.96556$$

Status: **PASS**
Ratio: **0.970**

25.2.3 s_{rebar} - Minimum spacing of reinforcement,

$$s_{rebar} = Max\left[1.5, (1.5\ d_{bar})\right]$$

$$s_{rebar} = Max\left[1.5, (1.5 \times (0.625 \text{ in}))\right]$$

$$s_{rebar} = 1.5 \text{ in}$$

Ties:

25.7.2.2 Since longitudinal reinforcement is $\leq$ No. 10ø: Use #3(0.375 in)
25.7.2.1 s_{ties} - Maximum spacing of ties,

$$s_{ties} = Min\left[(16\ d_{bar}), (48\ d_{ties}), Min(D, b)\right]$$

$$s_{ties} = Min\left[(16 \times (0.625 \text{ in})), (48 \times (0.375 \text{ in})), Min((48 \text{ in}), (48 \text{ in}))\right]$$

$$s_{ties} = 10 \text{ in}$$

Summary:

Main reinforcement: **14 - #5 (0.625 in)**

Ties: **#3(0.375 in) - 10 in**

Axial Compression Strength (ACI 318-19, LRFD)

22.4.2.2 ϕP_N - Allowable axial compressive strength

$$\phi P_N = \phi\, 0.80\, \left[\left(0.85\, f'_{ck}\, \left[A_g - A_{st}\right]\right) + \left(f_{yk}\, A_{st}\right)\right]$$

$$\phi P_N = (0.65) \times 0.80 \times \left[\left(0.85 \times (3\ \mathrm{ksi}) \times \left[\left(2304\ \mathrm{in}^2\right) - \left(4.2951\ \mathrm{in}^2\right)\right]\right) + \left((60\ \mathrm{ksi}) \times \left(4.2951\ \mathrm{in}^2\right)\right)\right]$$

$$\phi P_N = 3183.4\ \mathrm{kip}$$

$Ratio$ - Capacity

$$Ratio = \frac{P}{\phi P_N}$$

$$Ratio = \frac{(11.05\ \mathrm{kip})}{(3183.4\ \mathrm{kip})}$$

$$Ratio = 0.0034711$$

Status: **PASS**
Ratio: **0.000**

Shear Strength (ACI 318-19, LRFD)

Parameters:

$b_w = 48\ \mathrm{in}$ - Effective width,

22.5.2.2 d - Effective depth

$$d = 0.80\, D$$

$$d = 0.80 \times (48\ \mathrm{in})$$

$$d = 38.4\ \mathrm{in}$$

22.5.5.1.3 λ_s - size effect modification factor

$$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{d}{10}}}, 1\right]$$

$$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{(38.4\ \mathrm{in})}{10}}}, 1\right]$$

$$\lambda_s = 0.64282$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 3\ \mathrm{ksi} \rightarrow 3000\ \mathrm{psi}$,

22.5.5.1.1 $V_{c,max}$ - Max shear strength of concrete

$$V_{c,max} = 5\, \lambda_s\, \sqrt{f'_{ck}}\, b_w\, d$$

$$V_{c,max} = 5 \times (0.64282) \times \sqrt{(3000\ \mathrm{psi})} \times (48\ \mathrm{in}) \times (38.4\ \mathrm{in})$$

$$V_{c,max} = 324.49\ \mathrm{kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 3\ \mathrm{ksi} \rightarrow 3000\ \mathrm{psi}$, $P = 11.05\ \mathrm{kip} \rightarrow 11050\ \mathrm{lbf}$,

22.5.5.1.1(a) $V_{c,a}$ - Shear strength of concrete (a)

$$V_{c,a} = \left[2\, \lambda_s\, \sqrt{f'_{ck}} + \frac{P}{6\, A_g}\right]\, b_w\, d$$

$$V_{c,a} = \left[2 \times (0.64282) \times \sqrt{(3000\ \mathrm{psi})} + \frac{(11050\ \mathrm{lbf})}{6 \times \left(2304\ \mathrm{in}^2\right)}\right] \times (48\ \mathrm{in}) \times (38.4\ \mathrm{in})$$

$$V_{c,a} = 131.27\ \mathrm{kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 3\ \mathrm{ksi} \rightarrow 3000\ \mathrm{psi}$,

22.5.5.1.2 $V_{c,b}$ - Shear strength of concrete (b)

$$V_{c,b} = \left[2\, \lambda_s\, \sqrt{f'_{ck}} + \left(0.05\, f'_{ck}\right)\right]\, b_w\, d$$

$$V_{c,b} = \left[2 \times (0.64282) \times \sqrt{(3000\ \mathrm{psi})} + (0.05 \times (3000\ \mathrm{psi}))\right] \times (48\ \mathrm{in}) \times (38.4\ \mathrm{in})$$

$$V_{c,b} = 406.27\ \mathrm{kip}$$

V_c - Governing shear strength of concrete

$$V_c = Min\left[V_{c,max}, V_{c,a}, V_{c,b}\right]$$

$$V_c = Min\left[(324.49\ \mathrm{kip}), (131.27\ \mathrm{kip}), (406.27\ \mathrm{kip})\right]$$

$$V_c = 131.27\ \mathrm{kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 3\ \mathrm{ksi} \to 3000\ \mathrm{psi}$,

22.5.1.2 $V_{s,a}$ - Shear strength of steel (a)

$$V_{s,a} = 8\sqrt{f'_{ck}}\, b_w\, d$$

$$V_{s,a} = 8 \times \sqrt{(3000\ \mathrm{psi})} \times (48\ \mathrm{in}) \times (38.4\ \mathrm{in})$$

$$V_{s,a} = 807.65\ \mathrm{kip}$$

A_v - Ties rebar area,

$$A_v = \frac{\pi\, d_{ties}^2}{4}$$

$$A_v = \frac{\pi \times (0.375\ \mathrm{in})^2}{4}$$

$$A_v = 0.11045\ \mathrm{in}^2$$

22.5.8.5.3 $V_{s,b}$ - Shear strength of steel (b)

$$V_{s,b} = \frac{2\, A_v\, f_{ywk}\, d}{s_{ties}}$$

$$V_{s,b} = \frac{2 \times \left(0.11045\ \mathrm{in}^2\right) \times (60\ \mathrm{ksi}) \times (38.4\ \mathrm{in})}{(10\ \mathrm{in})}$$

$$V_{s,b} = 50.894\ \mathrm{kip}$$

V_s - Governing shear strength of steel

$$V_s = MIN\left[V_{s,a}, V_{s,b}\right]$$

$$V_s = MIN\left[(807.65\ \mathrm{kip}), (50.894\ \mathrm{kip})\right]$$

$$V_s = 50.894\ \mathrm{kip}$$

22.5.1.1 ϕV_n - Allowable shear strength

$$\phi V_n = \phi\ (V_c + V_s)$$

$$\phi V_n = (0.65) \times ((131.27\ \mathrm{kip}) + (50.894\ \mathrm{kip}))$$

$$\phi V_n = 118.4\ \mathrm{kip}$$

Considering x-direction:

$V_{max} = 12.453\ \mathrm{kip}$ - Maximum shear force in the x-direction,
$Ratio$ - Capacity

$$Ratio = \frac{V_{max}}{\phi V_n}$$

$$Ratio = \frac{(12.453\ \mathrm{kip})}{(118.4\ \mathrm{kip})}$$

$$Ratio = 0.10517$$

Status: **PASS**
Ratio: **0.110**

Considering z-direction:

$V_{max} = 0.017249\ \mathrm{kip}$ - Maximum shear force in the z-direction,
$Ratio$ - Capacity

$$Ratio = \frac{V_{max}}{\phi V_n}$$

$$Ratio = \frac{(0.017249\ \mathrm{kip})}{(118.4\ \mathrm{kip})}$$

$$Ratio = 0.00014568$$

Status: **PASS**
Ratio: **0.000**

Flexural Strength (ACI 318-19, LRFD)

S_m - Section modulus

$$S_m = \frac{b\, D^2}{6}$$

$$S_m = \frac{(48\ \mathrm{in}) \times (48\ \mathrm{in})^2}{6}$$

$$S_m = 18432\ \mathrm{in}^3$$

	$\lambda = 1$ - Concrete modification factor (Normal concrete), Allowable flexural strength: M_n shall be the lesser of: $\phi M_{n,1}$ $$\phi M_{n,1} = \phi \times 5 \times \lambda \times \sqrt{(f'_c)} \times S_m$$ $$\phi M_{n,1} = 0.65 \times 5 \times 1 \times \sqrt{(3ksi)} \times 18432.001in^3$$ $$\phi M_{n,1} = 273.423kipft$$	
14.5.2.1b	$\phi M_{n,2}$ $$\phi M_{n,2} = \phi\, 0.85\, f'_{ck}\, S_m$$ $$\phi M_{n,2} = (0.65) \times 0.85 \times (3 \text{ ksi}) \times \left(18432 \text{ in}^3\right)$$ $$\phi M_{n,2} = 2545.9 \text{ kipft}$$ Therefore, ϕM_n - Allowable flexural strength, $$\phi M_n = MIN[\phi M_{n,1}, \phi M_{n,2}]$$ $$\phi M_n = MIN[(273.42 \text{ kipft}), (2545.9 \text{ kipft})]$$ $$\phi M_n = 273.42 \text{ kipft}$$ **Considering x-direction:** $M_{max} = 46.485$ kipft - Maximum moment in the x-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\phi M_n}$$ $$Ratio = \frac{(46.485 \text{ kipft})}{(273.42 \text{ kipft})}$$ $$Ratio = 0.17001$$	Status: **PASS** Ratio: **0.170**
	Considering z-direction: $M_{max} = 0.060213$ kipft - Maximum moment in the z-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\phi M_n}$$ $$Ratio = \frac{(0.060213 \text{ kipft})}{(273.42 \text{ kipft})}$$ $$Ratio = 0.00022022$$	Status: **PASS** Ratio: **0.000**

REFERENCES	CALCULATIONS	RESULTS

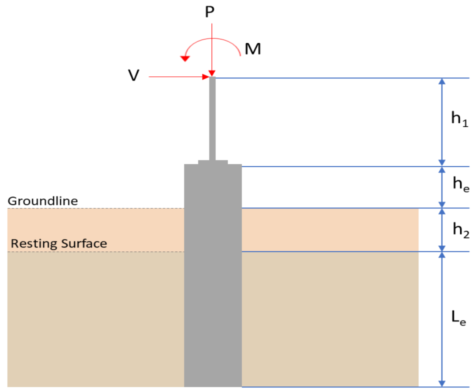
SkyCiv Foundation Design

Pile Foundation

Design Information :

Design code : IBC 2021 (International Building Code)
Unit System : Imperial

Pile Input



Geometry

Pile shape: rectangular
$b = 48\ \mathrm{in}$ - Pile width
$D = 48\ \mathrm{in}$ - Pile depth
$L = 7.75\ \mathrm{ft}$ - Total pile length
$h_1 = 0\ \mathrm{ft}$ - Lateral load height from the top of the pile,
$h_2 = 0\ \mathrm{ft}$ - Depth to resisting surface
$h_e = 0\ \mathrm{ft}$ - Length of pile above the ground

Tabulation of Soil Parameters

Layer	Label	Allowable Bearing Pressure (q_a) (psf)	Allowable Lateral Pressure (R) (psf/ft)
1	Sand, silty sand, clayey sand, silty gravel & clayey gravel	2000.000	150.000

Tabulation of Loads

Load Component	ASD	LRFD
P (kip)	7.758	11.647
V_x (kip)	-2.448	-4.081
V_z (kip)	0.013	0.022
M_x (kipft)	0.073	0.125
M_z (kipft)	45.028	76.518

Material Properties

$f'_{ck} = 3\ \mathrm{ksi}$ - Concrete strength,

Required depth to resist lateral loads (ASD)

H - Point of application of the lateral load

$$H = h_1 + h_2 + h_e$$

$$H = (0\ \mathrm{ft}) + (0\ \mathrm{ft}) + (0\ \mathrm{ft})$$

$$H = 0\ \mathrm{ft}$$

Considering x-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\,D}$$

$$H_o = \frac{(-2.448\ \mathrm{kip})}{1.57 \times (48\ \mathrm{in})}$$

$$H_o = -0.38981\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\,H)}{1.57\,D}$$

$$M_o = \frac{(45.028\ \text{kipft}) + ((-2.448\ \text{kip}) \times (0\ \text{ft}))}{1.57 \times (48\ \text{in})}$$

$$M_o = 7.1701\ \text{kipft/ft}$$

Required depth of embedment in earth:

$$L_x^3 - \left(14.14 \times \frac{H_o \times L_x}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,x} = 7.3749\ \text{ft}$ - Required depth in x-direction,

Considering z-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\,b}$$

$$H_o = \frac{(0.013\ \text{kip})}{1.57 \times (48\ \text{in})}$$

$$H_o = 0.0020701\ \text{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\,H)}{1.57\,b}$$

$$M_o = \frac{(0.073\ \text{kipft}) + ((0.013\ \text{kip}) \times (0\ \text{ft}))}{1.57 \times (48\ \text{in})}$$

$$M_o = 0.011624\ \text{kipft/ft}$$

Required depth of embedment in earth:

$$L_z^3 - \left(14.14 \times \frac{H_o \times L_z}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,z} = 1.0184\ \text{ft}$ - Required depth in z-direction,

Minimum embedded depth required:

$L_{e,req}$ - Depth of pile required,

$$L_{e,req} = MAX[L_{e,x}, L_{e,z}]$$

$$L_{e,req} = MAX[(7.3749\ \text{ft}), (1.0184\ \text{ft})]$$

$$L_{e,req} = 7.375\ \text{ft}$$

L_e - Actual embedded length of pile,

$$L_e = L - h_e - h_2$$

$$L_e = (7.75\ \text{ft}) - (0\ \text{ft}) - (0\ \text{ft})$$

$$L_e = 7.75\ \text{ft}$$

$Ratio$ - Embedded depth

$$Ratio = \frac{L_{e,req}}{L_e}$$

$$Ratio = \frac{(7.375\ \text{ft})}{(7.75\ \text{ft})}$$

$$Ratio = 0.95161$$

Status: **PASS**
Ratio: **0.950**

End-bearing Capacity (ASD)

A - Pile cross-section area

$$A = b\,D$$

$$A = (48\ \text{in}) \times (48\ \text{in})$$

$$A = 16\ \text{ft}^2$$

q - End-bearing pressure

$$q = \frac{P_v}{A}$$

$$q = \frac{(7.758\ \text{kip})}{(16\ \text{ft}^2)}$$

$$q = 0.48488\ \text{kip/ft}^2$$

$q = 0.48488\ \text{kip/ft}$

Check bearing capacity ratio:

$Ratio$ - Capacity

$$Ratio = \frac{q}{q_a}$$

$$Ratio = \frac{\left(0.48488\ \text{kip/ft}^2\right)}{(2000\ \text{psf})}$$

$$Ratio = 0.24244$$

Status: **PASS**
Ratio: **0.240**

Czerniak

Lateral Soil Pressure (ASD):

L/D - Length to least lateral dimension ratio,

$$L/D = \frac{L}{D}$$

$$L/D = \frac{(7.75\ \text{ft})}{(48\ \text{in})}$$

$$L/D = 1.9375$$

Since L/D $\leq$ 10,

Pile is short.

Considering x-direction:

$H_o = -0.38981\ \text{kip/ft}$ - Lateral force per length of pile,
$M_o = 7.1701\ \text{kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (7.1701\ \text{kipft/ft}) \times (7.75\ \text{ft})) + \left(3 \times (-0.38981\ \text{kip/ft}) \times (7.75\ \text{ft})^2\right)}{(6 \times (7.1701\ \text{kipft/ft})) + (4 \times (-0.38981\ \text{kip/ft}) \times (7.75\ \text{ft}))}$$

$$a = 5.3083\ \text{ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\,[(4\,M_o) + (3\,H_o\,L_e)]^2}{L_e^2\,[(3\,M_o) + (2\,H_o\,L_e)]}$$

$$p = \frac{0.75 \times [(4 \times (7.1701\ \text{kipft/ft})) + (3 \times (-0.38981\ \text{kip/ft}) \times (7.75\ \text{ft}))]^2}{(7.75\ \text{ft})^2 \times [(3 \times (7.1701\ \text{kipft/ft})) + (2 \times (-0.38981\ \text{kip/ft}) \times (7.75\ \text{ft}))]}$$

$$p = 0.31067\ \text{kip/ft}^2$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\,[(2\,M_o) + (H_o\,L_e)]}{L_e^2}$$

$$s = \frac{6 \times [(2 \times (7.1701\ \text{kipft/ft})) + ((-0.38981\ \text{kip/ft}) \times (7.75\ \text{ft}))]}{(7.75\ \text{ft})^2}$$

$$s = 1.1307\ \text{kip/ft}^2$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\,\frac{a}{2}$$

$$p_a = (150\ \text{psf/ft}) \times \frac{(5.3083\ \text{ft})}{2}$$

$$p_a = 0.39812\ \text{kip/ft}^2$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{\left(0.31067\ \text{kip/ft}^2\right)}{\left(0.39812\ \text{kip/ft}^2\right)}$$

$$Ratio = 0.78033$$

Status: **PASS**
Ratio: **0.780**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\,L_e$$

$$p_s = (150\ \text{psf/ft}) \times (7.75\ \text{ft})$$

$$p_s = 1.1625\ \text{kip/ft}^2$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{\left(1.1307\ \text{kip/ft}^2\right)}{\left(1.1625\ \text{kip/ft}^2\right)}$$

$$Ratio = 0.97267$$

Status: **PASS**
Ratio: **0.970**

Considering z-direction:

$H_o = 0.0020701\ \text{kip/ft}$ - Lateral force per length of pile,
$M_o = 0.011624\ \text{kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (0.011624\ \text{kipft/ft}) \times (7.75\ \text{ft})) + \left(3 \times (0.0020701\ \text{kip/ft}) \times (7.75\ \text{ft})^2\right)}{(6 \times (0.011624\ \text{kipft/ft})) + (4 \times (0.0020701\ \text{kip/ft}) \times (7.75\ \text{ft}))}$$

$$a = 5.4761\ \text{ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\,[(4\,M_o) + (3\,H_o\,L_e)]^2}{L_e^2\,[(3\,M_o) + (2\,H_o\,L_e)]}$$

$$p = \frac{0.75 \times [(4 \times (0.011624\ \text{kipft/ft})) + (3 \times (0.0020701\ \text{kip/ft}) \times (7.75\ \text{ft}))]^2}{(7.75\ \text{ft})^2 \times [(3 \times (0.011624\ \text{kipft/ft})) + (2 \times (0.0020701\ \text{kip/ft}) \times (7.75\ \text{ft}))]}$$

$$p = 0.0016698\ \text{kip/ft}^2$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\,[(2\,M_o) + (H_o\,L_e)]}{L_e^2}$$

$$s = \frac{6 \times [(2 \times (0.011624\ \text{kipft/ft})) + ((0.0020701\ \text{kip/ft}) \times (7.75\ \text{ft}))]}{(7.75\ \text{ft})^2}$$

$$s = 0.0039251\ \text{kip/ft}^2$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\,\frac{a}{2}$$

$$p_a = (150\ \text{psf/ft}) \times \frac{(5.4761\ \text{ft})}{2}$$

$$p_a = 0.41071\ \text{kip/ft}^2$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{\left(0.0016698\ \text{kip/ft}^2\right)}{\left(0.41071\ \text{kip/ft}^2\right)}$$

$$Ratio = 0.0040657$$

Status: **PASS**
Ratio: **0.000**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\,L_e$$

$$p_s = (150\ \text{psf/ft}) \times (7.75\ \text{ft})$$

$$p_s = 1.1625\ \text{kip/ft}^2$$

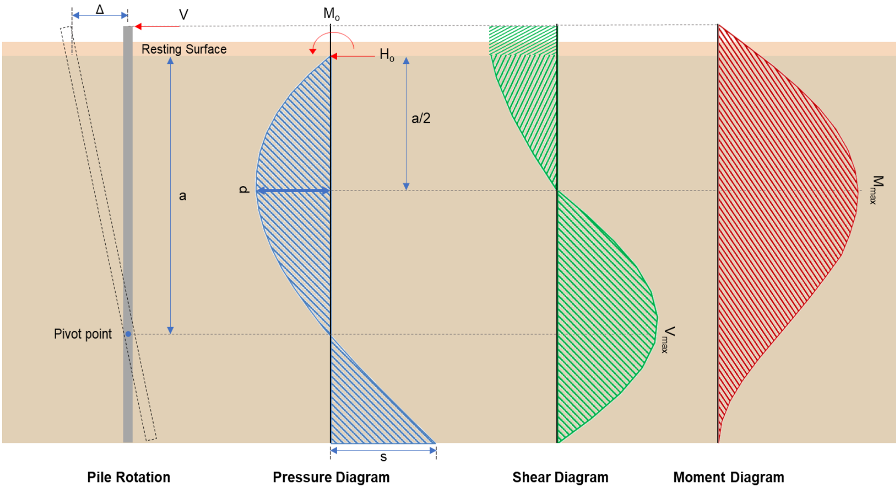
$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{(0.0039251\ \mathrm{kip/ft^2})}{(1.1625\ \mathrm{kip/ft^2})}$$

$$Ratio = 0.0033764$$

Status: **PASS**
Ratio: **0.000**



Shear force and Bending moment (x-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\ D}$$

$$H_o = \frac{(-4.081\ \mathrm{kip})}{1.57 \times (48\ \mathrm{in})}$$

$$H_o = -0.64984\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\ H)}{1.57\ D}$$

$$M_o = \frac{(76.518\ \mathrm{kipft}) + ((-4.081\ \mathrm{kip}) \times (0\ \mathrm{ft}))}{1.57 \times (48\ \mathrm{in})}$$

$$M_o = 12.184\ \mathrm{kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(12.184\ \mathrm{kipft/ft})}{(-0.64984\ \mathrm{kip/ft})}$$

$$E = 18.75\ \mathrm{ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\ M_o\ L_e) + \left(3\ H_o\ L_e^2\right)}{(6\ M_o) + (4\ H_o\ L_e)}$$

$$a = \frac{(4 \times (12.184\ \mathrm{kipft/ft}) \times (7.75\ \mathrm{ft})) + \left(3 \times (-0.64984\ \mathrm{kip/ft}) \times (7.75\ \mathrm{ft})^2\right)}{(6 \times (12.184\ \mathrm{kipft/ft})) + (4 \times (-0.64984\ \mathrm{kip/ft}) \times (7.75\ \mathrm{ft}))}$$

$$a = 5.3062\ \mathrm{ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\ D) \left[1 - \left[3 \left(\frac{4\ E}{L_e} + 3\right) \left(\frac{a}{L_e}\right)^2\right] + \left[4 \left(\frac{3\ E}{L_e} + 2\right) \left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((-0.64984\ \mathrm{kip/ft}) \times (48\ \mathrm{in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (18.75\ \mathrm{ft})}{(7.75\ \mathrm{ft})} + 3\right) \times \left(\frac{(5.3062\ \mathrm{ft})}{(7.75\ \mathrm{ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (18.75\ \mathrm{ft})}{(7.75\ \mathrm{ft})} + 2\right) \times \left(\frac{(5.3062\ \mathrm{ft})}{(7.75\ \mathrm{ft})}\right)^3\right]\right]$$

$$V_{max} = 12.848\ \mathrm{kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, D\, L_e) \left[\left(\frac{E}{L_e} + \frac{a}{2\, L_e} \right) - \left[\left(\frac{4\, E}{L_e} + 3 \right) \left(\frac{a}{2\, L_e} \right)^3 \right] + \left[\left(\frac{3\, E}{L_e} + 2 \right) \left(\frac{a}{2\, L_e} \right)^4 \right] \right]$$

$$M_{max} = ((-0.64984 \text{ kip/ft}) \times (48 \text{ in}) \times (7.75 \text{ ft})) \times \left[\left(\frac{(18.75 \text{ ft})}{(7.75 \text{ ft})} + \frac{(5.3062 \text{ ft})}{2 \times (7.75 \text{ ft})} \right) - \left[\left(\frac{4 \times (18.75 \text{ ft})}{(7.75 \text{ ft})} + 3 \right) \times \left(\frac{(5.3062 \text{ ft})}{2 \times (7.75 \text{ ft})} \right)^3 \right] + \left[\left(\frac{3 \times (18.75 \text{ ft})}{(7.75 \text{ ft})} + 2 \right) \times \left(\frac{(5.3062 \text{ ft})}{2 \times (7.75 \text{ ft})} \right)^4 \right] \right]$$

$$M_{max} = 47.95 \text{ kipft}$$

Shear force and Bending moment (z-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\, b}$$

$$H_o = \frac{(0.022 \text{ kip})}{1.57 \times (48 \text{ in})}$$

$$H_o = 0.0035032 \text{ kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\, H)}{1.57\, b}$$

$$M_o = \frac{(0.125 \text{ kipft}) + ((0.022 \text{ kip}) \times (0 \text{ ft}))}{1.57 \times (48 \text{ in})}$$

$$M_o = 0.019904 \text{ kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(0.019904 \text{ kipft/ft})}{(0.0035032 \text{ kip/ft})}$$

$$E = 5.6818 \text{ ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\, M_o\, L_e) + \left(3\, H_o\, L_e^2\right)}{(6\, M_o) + (4\, H_o\, L_e)}$$

$$a = \frac{(4 \times (0.019904 \text{ kipft/ft}) \times (7.75 \text{ ft})) + \left(3 \times (0.0035032 \text{ kip/ft}) \times (7.75 \text{ ft})^2\right)}{(6 \times (0.019904 \text{ kipft/ft})) + (4 \times (0.0035032 \text{ kip/ft}) \times (7.75 \text{ ft}))}$$

$$a = 5.4742 \text{ ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\, b) \left[1 - \left[3 \left(\frac{4\, E}{L_e} + 3 \right) \left(\frac{a}{L_e} \right)^2 \right] + \left[4 \left(\frac{3\, E}{L_e} + 2 \right) \left(\frac{a}{L_e} \right)^3 \right] \right]$$

$$V_{max} = ((0.0035032 \text{ kip/ft}) \times (48 \text{ in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (5.6818 \text{ ft})}{(7.75 \text{ ft})} + 3 \right) \times \left(\frac{(5.4742 \text{ ft})}{(7.75 \text{ ft})} \right)^2 \right] + \left[4 \times \left(\frac{3 \times (5.6818 \text{ ft})}{(7.75 \text{ ft})} + 2 \right) \times \left(\frac{(5.4742 \text{ ft})}{(7.75 \text{ ft})} \right)^3 \right] \right]$$

$$V_{max} = 0.027465 \text{ kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, b\, L_e) \left[\left(\frac{E}{L_e} + \frac{a}{2\, L_e} \right) - \left[\left(\frac{4\, E}{L_e} + 3 \right) \left(\frac{a}{2\, L_e} \right)^3 \right] + \left[\left(\frac{3\, E}{L_e} + 2 \right) \left(\frac{a}{2\, L_e} \right)^4 \right] \right]$$

$$M_{max} = ((0.0035032 \text{ kip/ft}) \times (48 \text{ in}) \times (7.75 \text{ ft})) \times \left[\left(\frac{(5.6818 \text{ ft})}{(7.75 \text{ ft})} + \frac{(5.4742 \text{ ft})}{2 \times (7.75 \text{ ft})} \right) - \left[\left(\frac{4 \times (5.6818 \text{ ft})}{(7.75 \text{ ft})} + 3 \right) \times \left(\frac{(5.4742 \text{ ft})}{2 \times (7.75 \text{ ft})} \right)^3 \right] + \left[\left(\frac{3 \times (5.6818 \text{ ft})}{(7.75 \text{ ft})} + 2 \right) \times \left(\frac{(5.4742 \text{ ft})}{2 \times (7.75 \text{ ft})} \right)^4 \right] \right]$$

$$M_{max} = 0.096686 \text{ kipft}$$

Minimum Reinforcement Check (LRFD)

Parameters:

$f'_{ck} = 3 \text{ ksi}$ - Concrete strength,
$f_{yk} = 60 \text{ ksi}$ - Longitudinal reinforcement strength,
$\phi = 0.65$ - Reduction factor for axial strength,
Table 22.4.2.1 $\alpha = 0.8$ - Alpha factor for axial strength,
$A_g = 2304 \text{ in}^2$ - Gross area of concrete,

Longitudinal reinforcement:

Required reinforcement due to axial load, $A_{st,required}$
22.4.2.2, 10.6.1.1 $A_{st,required}$

$$A_{st,required} = Min\left[\frac{\frac{P}{\phi\ \alpha} - (0.85\ f'_{ck}\ A_g)}{f_{yk} - (0.85\ f'_{ck})}, (0.08\ A_g)\right]$$

$$A_{st,required} = Min\left[\frac{\frac{(11.647 \text{ kip})}{(0.65)\times(0.8)} - (0.85 \times (3 \text{ ksi}) \times (2304 \text{ in}^2))}{(60 \text{ ksi}) - (0.85 \times (3 \text{ ksi}))}, (0.08 \times (2304 \text{ in}^2))\right]$$

$$A_{st,required} = -101.88 \text{ in}^2$$

A_{min} - Governing minimum reinforcement area,

$$A_{min} = Max\left[A_{st,required}, (0.0018\ A_g)\right]$$

$$A_{min} = Max\left[(-101.88 \text{ in}^2), (0.0018 \times (2304 \text{ in}^2))\right]$$

$$A_{min} = 4.1472 \text{ in}^2$$

n_{rebar} - Required number of reinforcement,

$$n_{rebar} = \frac{A_{min}}{A_{rebar}}$$

$$n_{rebar} = \frac{(4.1472 \text{ in}^2)}{(0.3068 \text{ in}^2)}$$

$$n_{rebar} = 14$$

A_{st} - Actual total reinforcement area,

$$A_{st} = n_{rebar}\ \frac{\pi\ d_{bar}^2}{4}$$

$$A_{st} = (14) \times \frac{\pi \times (0.625 \text{ in})^2}{4}$$

$$A_{st} = 4.2951 \text{ in}^2$$

$Ratio$ - Capacity

$$Ratio = \frac{A_{min}}{A_{st}}$$

$$Ratio = \frac{(4.1472 \text{ in}^2)}{(4.2951 \text{ in}^2)}$$

$$Ratio = 0.96556$$

Status: **PASS**
Ratio: **0.970**

25.2.3 s_{rebar} - Minimum spacing of reinforcement,

$$s_{rebar} = Max\left[1.5, (1.5\ d_{bar})\right]$$

$$s_{rebar} = Max\left[1.5, (1.5 \times (0.625 \text{ in}))\right]$$

$$s_{rebar} = 1.5 \text{ in}$$

Ties:

25.7.2.2 Since longitudinal reinforcement is $\leq$ No. 10ø: Use #3(0.375 in)
25.7.2.1 s_{ties} - Maximum spacing of ties,

$$s_{ties} = Min\left[(16\ d_{bar}), (48\ d_{ties}), Min(D, b)\right]$$

$$s_{ties} = Min\left[(16 \times (0.625 \text{ in})), (48 \times (0.375 \text{ in})), Min((48 \text{ in}), (48 \text{ in}))\right]$$

$$s_{ties} = 10 \text{ in}$$

Summary:

Main reinforcement: **14 - #5 (0.625 in)**

	Ties: **#3(0.375 in) - 10 in**	

Axial Compression Strength (ACI 318-19, LRFD)

22.4.2.2 ϕP_N - Allowable axial compressive strength

$$\phi P_N = \phi\, 0.80\, \left[\left(0.85\, f'_{ck}\, [A_g - A_{st}]\right) + (f_{yk}\, A_{st})\right]$$

$$\phi P_N = (0.65) \times 0.80 \times \left[\left(0.85 \times (3\text{ ksi}) \times \left[\left(2304\text{ in}^2\right) - \left(4.2951\text{ in}^2\right)\right]\right) + \left((60\text{ ksi}) \times \left(4.2951\text{ in}^2\right)\right)\right]$$

$$\phi P_N = 3183.4\text{ kip}$$

$Ratio$ - Capacity

$$Ratio = \frac{P}{\phi P_N}$$

$$Ratio = \frac{(11.647\text{ kip})}{(3183.4\text{ kip})}$$

$$Ratio = 0.0036586$$

Status: **PASS**
Ratio: **0.000**

Shear Strength (ACI 318-19, LRFD)

Parameters:

$b_w = 48\text{ in}$ - Effective width,

22.5.2.2 d - Effective depth

$$d = 0.80\, D$$

$$d = 0.80 \times (48\text{ in})$$

$$d = 38.4\text{ in}$$

22.5.5.1.3 λ_s - size effect modification factor

$$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{d}{10}}}, 1\right]$$

$$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{(38.4\text{ in})}{10}}}, 1\right]$$

$$\lambda_s = 0.64282$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 3\text{ ksi} \rightarrow 3000\text{ psi}$,

22.5.5.1.1 $V_{c,max}$ - Max shear strength of concrete

$$V_{c,max} = 5\, \lambda_s\, \sqrt{f'_{ck}}\, b_w\, d$$

$$V_{c,max} = 5 \times (0.64282) \times \sqrt{(3000\text{ psi})} \times (48\text{ in}) \times (38.4\text{ in})$$

$$V_{c,max} = 324.49\text{ kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 3\text{ ksi} \rightarrow 3000\text{ psi}$, $P = 11.647\text{ kip} \rightarrow 11647\text{ lbf}$,

22.5.5.1.1(a) $V_{c,a}$ - Shear strength of concrete (a)

$$V_{c,a} = \left[2\, \lambda_s\, \sqrt{f'_{ck}} + \frac{P}{6\, A_g}\right]\, b_w\, d$$

$$V_{c,a} = \left[2 \times (0.64282) \times \sqrt{(3000\text{ psi})} + \frac{(11647\text{ lbf})}{6 \times (2304\text{ in}^2)}\right] \times (48\text{ in}) \times (38.4\text{ in})$$

$$V_{c,a} = 131.35\text{ kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 3\text{ ksi} \rightarrow 3000\text{ psi}$,

22.5.5.1.2 $V_{c,b}$ - Shear strength of concrete (b)

$$V_{c,b} = \left[2\, \lambda_s\, \sqrt{f'_{ck}} + \left(0.05\, f'_{ck}\right)\right]\, b_w\, d$$

$$V_{c,b} = \left[2 \times (0.64282) \times \sqrt{(3000\text{ psi})} + (0.05 \times (3000\text{ psi}))\right] \times (48\text{ in}) \times (38.4\text{ in})$$

$$V_{c,b} = 406.27\text{ kip}$$

V_c - Governing shear strength of concrete

$$V_c = Min\left[V_{c,max}, V_{c,a}, V_{c,b}\right]$$

$$V_c = Min\left[(324.49\text{ kip}), (131.35\text{ kip}), (406.27\text{ kip})\right]$$

$$V_c = 131.35\text{ kip}$$

	The following variables were converted to be consistent with empirical formula $f'_{ck} = 3\ \text{ksi} \rightarrow 3000\ \text{psi}$,	
22.5.1.2	$V_{s,a}$ - Shear strength of steel (a) $$V_{s,a} = 8\sqrt{f'_{ck}}\, b_w\, d$$ $$V_{s,a} = 8 \times \sqrt{(3000\ \text{psi})} \times (48\ \text{in}) \times (38.4\ \text{in})$$ $$V_{s,a} = 807.65\ \text{kip}$$ A_v - Ties rebar area, $$A_v = \frac{\pi\, d_{ties}^2}{4}$$ $$A_v = \frac{\pi \times (0.375\ \text{in})^2}{4}$$ $$A_v = 0.11045\ \text{in}^2$$	
22.5.8.5.3	$V_{s,b}$ - Shear strength of steel (b) $$V_{s,b} = \frac{2\, A_v\, f_{ywk}\, d}{s_{ties}}$$ $$V_{s,b} = \frac{2 \times \left(0.11045\ \text{in}^2\right) \times (60\ \text{ksi}) \times (38.4\ \text{in})}{(10\ \text{in})}$$ $$V_{s,b} = 50.894\ \text{kip}$$ V_s - Governing shear strength of steel $$V_s = MIN\left[V_{s,a}, V_{s,b}\right]$$ $$V_s = MIN\left[(807.65\ \text{kip}), (50.894\ \text{kip})\right]$$ $$V_s = 50.894\ \text{kip}$$	
22.5.1.1	ϕV_n - Allowable shear strength $$\phi V_n = \phi\ (V_c + V_s)$$ $$\phi V_n = (0.65) \times ((131.35\ \text{kip}) + (50.894\ \text{kip}))$$ $$\phi V_n = 118.46\ \text{kip}$$ **Considering x-direction:** $V_{max} = 12.848\ \text{kip}$ - Maximum shear force in the x-direction, $Ratio$ - Capacity $$Ratio = \frac{V_{max}}{\phi V_n}$$ $$Ratio = \frac{(12.848\ \text{kip})}{(118.46\ \text{kip})}$$ $$Ratio = 0.10846$$	Status: **PASS** Ratio: **0.110**
	Considering z-direction: $V_{max} = 0.027465\ \text{kip}$ - Maximum shear force in the z-direction, $Ratio$ - Capacity $$Ratio = \frac{V_{max}}{\phi V_n}$$ $$Ratio = \frac{(0.027465\ \text{kip})}{(118.46\ \text{kip})}$$ $$Ratio = 0.00023185$$	Status: **PASS** Ratio: **0.000**
	Flexural Strength (ACI 318-19, LRFD) S_m - Section modulus $$S_m = \frac{b\, D^2}{6}$$ $$S_m = \frac{(48\ \text{in}) \times (48\ \text{in})^2}{6}$$ $$S_m = 18432\ \text{in}^3$$	

	$\lambda = 1$ - Concrete modification factor (Normal concrete), Allowable flexural strength: M_n shall be the lesser of: $\phi M_{n,1}$ $$\phi M_{n,1} = \phi \times 5 \times \lambda \times \sqrt{(f_c')} \times S_m$$ $$\phi M_{n,1} = 0.65 \times 5 \times 1 \times \sqrt{(3ksi)} \times 18432.001in^3$$ $$\phi M_{n,1} = 273.423kipft$$	
14.5.2.1b	$\phi M_{n,2}$ $$\phi M_{n,2} = \phi\, 0.85\, f_{ck}'\, S_m$$ $$\phi M_{n,2} = (0.65) \times 0.85 \times (3 \text{ ksi}) \times \left(18432 \text{ in}^3\right)$$ $$\phi M_{n,2} = 2545.9 \text{ kipft}$$ Therefore, ϕM_n - Allowable flexural strength, $$\phi M_n = MIN[\phi M_{n,1}, \phi M_{n,2}]$$ $$\phi M_n = MIN[(273.42 \text{ kipft}), (2545.9 \text{ kipft})]$$ $$\phi M_n = 273.42 \text{ kipft}$$ **Considering x-direction:** $M_{max} = 47.95 \text{ kipft}$ - Maximum moment in the x-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\phi M_n}$$ $$Ratio = \frac{(47.95 \text{ kipft})}{(273.42 \text{ kipft})}$$ $$Ratio = 0.17537$$	Status: **PASS** Ratio: **0.180**
	Considering z-direction: $M_{max} = 0.096686 \text{ kipft}$ - Maximum moment in the z-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\phi M_n}$$ $$Ratio = \frac{(0.096686 \text{ kipft})}{(273.42 \text{ kipft})}$$ $$Ratio = 0.00035361$$	Status: **PASS** Ratio: **0.000**

REFERENCES	CALCULATIONS	RESULTS

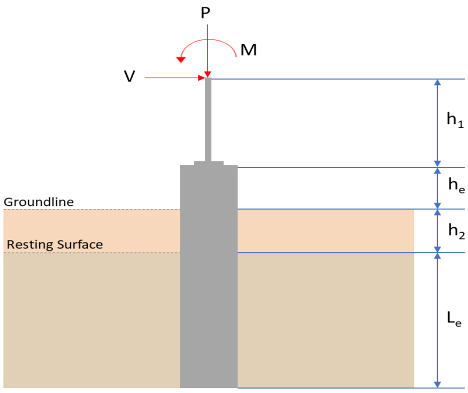
SkyCiv Foundation Design

Pile Foundation

Design Information :

Design code : IBC 2021 (International Building Code)
Unit System : Imperial

Pile Input



Geometry

Pile shape: rectangular
$b = 48\ \mathrm{in}$ - Pile width
$D = 48\ \mathrm{in}$ - Pile depth
$L = 7.75\ \mathrm{ft}$ - Total pile length
$h_1 = 0\ \mathrm{ft}$ - Lateral load height from the top of the pile,
$h_2 = 0\ \mathrm{ft}$ - Depth to resisting surface
$h_e = 0\ \mathrm{ft}$ - Length of pile above the ground

Tabulation of Soil Parameters

Layer	Label	Allowable Bearing Pressure (q_a) (psf)	Allowable Lateral Pressure (R) (psf/ft)
1	Sand, silty sand, clayey sand, silty gravel & clayey gravel	2000.000	150.000

Tabulation of Loads

Load Component	ASD	LRFD
P (kip)	7.758	11.647
V_x (kip)	-2.448	-4.081
V_z (kip)	-0.013	-0.022
M_x (kipft)	-0.073	-0.125
M_z (kipft)	45.028	76.518

Material Properties

$f'_{ck} = 3\ \mathrm{ksi}$ - Concrete strength,

Required depth to resist lateral loads (ASD)

H - Point of application of the lateral load

$$H = h_1 + h_2 + h_e$$

$$H = (0\ \mathrm{ft}) + (0\ \mathrm{ft}) + (0\ \mathrm{ft})$$

$$H = 0\ \mathrm{ft}$$

Considering x-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\ D}$$

$$H_o = \frac{(-2.448\ \mathrm{kip})}{1.57 \times (48\ \mathrm{in})}$$

$$H_o = -0.38981\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\ H)}{1.57\ D}$$

$$M_o = \frac{(45.028\ \text{kipft}) + ((-2.448\ \text{kip}) \times (0\ \text{ft}))}{1.57 \times (48\ \text{in})}$$

$$M_o = 7.1701\ \text{kipft/ft}$$

Required depth of embedment in earth:

$$L_x^3 - \left(14.14 \times \frac{H_o \times L_x}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,x} = 7.3749\ \text{ft}$ - Required depth in x-direction,

Considering z-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\, b}$$

$$H_o = \frac{(-0.013\ \text{kip})}{1.57 \times (48\ \text{in})}$$

$$H_o = -0.0020701\ \text{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\, H)}{1.57\, b}$$

$$M_o = \frac{(0.073\ \text{kipft}) + ((-0.013\ \text{kip}) \times (0\ \text{ft}))}{1.57 \times (48\ \text{in})}$$

$$M_o = 0.011624\ \text{kipft/ft}$$

Required depth of embedment in earth:

$$L_z^3 - \left(14.14 \times \frac{H_o \times L_z}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,z} = 0.93357\ \text{ft}$ - Required depth in z-direction,

Minimum embedded depth required:

$L_{e,req}$ - Depth of pile required,

$$L_{e,req} = MAX[L_{e,x}, L_{e,z}]$$

$$L_{e,req} = MAX[(7.3749\ \text{ft}), (0.93357\ \text{ft})]$$

$$L_{e,req} = 7.375\ \text{ft}$$

L_e - Actual embedded length of pile,

$$L_e = L - h_e - h_2$$

$$L_e = (7.75\ \text{ft}) - (0\ \text{ft}) - (0\ \text{ft})$$

$$L_e = 7.75\ \text{ft}$$

$Ratio$ - Embedded depth

$$Ratio = \frac{L_{e,req}}{L_e}$$

$$Ratio = \frac{(7.375\ \text{ft})}{(7.75\ \text{ft})}$$

$$Ratio = 0.95161$$

Status: **PASS**
Ratio: **0.950**

End-bearing Capacity (ASD)

A - Pile cross-section area

$$A = b\, D$$

$$A = (48\ \text{in}) \times (48\ \text{in})$$

$$A = 16\ \text{ft}^2$$

q - End-bearing pressure

$$q = \frac{P_v}{A}$$

$$q = \frac{(7.758\ \text{kip})}{(16\ \text{ft}^2)}$$

$$q = 0.48488\ \text{kip/ft}^2$$

$q = 0.48488\ \text{kip/ft}$

Check bearing capacity ratio:

$Ratio$ - Capacity

$$Ratio = \frac{q}{q_a}$$

$$Ratio = \frac{\left(0.48488\ \text{kip/ft}^2\right)}{(2000\ \text{psf})}$$

$$Ratio = 0.24244$$

Status: **PASS**
Ratio: **0.240**

Czerniak

Lateral Soil Pressure (ASD):

L/D - Length to least lateral dimension ratio,

$$L/D = \frac{L}{D}$$

$$L/D = \frac{(7.75\ \text{ft})}{(48\ \text{in})}$$

$$L/D = 1.9375$$

Since L/D $\leq$ 10,

Pile is short.

Considering x-direction:

$H_o = -0.38981\ \text{kip/ft}$ - Lateral force per length of pile,
$M_o = 7.1701\ \text{kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\ M_o\ L_e) + \left(3\ H_o\ L_e^2\right)}{(6\ M_o) + (4\ H_o\ L_e)}$$

$$a = \frac{(4 \times (7.1701\ \text{kipft/ft}) \times (7.75\ \text{ft})) + \left(3 \times (-0.38981\ \text{kip/ft}) \times (7.75\ \text{ft})^2\right)}{(6 \times (7.1701\ \text{kipft/ft})) + (4 \times (-0.38981\ \text{kip/ft}) \times (7.75\ \text{ft}))}$$

$$a = 5.3083\ \text{ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\ [(4\ M_o) + (3\ H_o\ L_e)]^2}{L_e^2\ [(3\ M_o) + (2\ H_o\ L_e)]}$$

$$p = \frac{0.75 \times [(4 \times (7.1701\ \text{kipft/ft})) + (3 \times (-0.38981\ \text{kip/ft}) \times (7.75\ \text{ft}))]^{\ 2}}{(7.75\ \text{ft})^2 \times [(3 \times (7.1701\ \text{kipft/ft})) + (2 \times (-0.38981\ \text{kip/ft}) \times (7.75\ \text{ft}))]}$$

$$p = 0.31067\ \text{kip/ft}^2$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\ [(2\ M_o) + (H_o\ L_e)]}{L_e^2}$$

$$s = \frac{6 \times [(2 \times (7.1701\ \text{kipft/ft})) + ((-0.38981\ \text{kip/ft}) \times (7.75\ \text{ft}))]}{(7.75\ \text{ft})^2}$$

$$s = 1.1307\ \text{kip/ft}^2$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\ \frac{a}{2}$$

$$p_a = (150\ \text{psf/ft}) \times \frac{(5.3083\ \text{ft})}{2}$$

$$p_a = 0.39812\ \text{kip/ft}^2$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{\left(0.31067\ \text{kip/ft}^2\right)}{\left(0.39812\ \text{kip/ft}^2\right)}$$

$$Ratio = 0.78033$$

Status: **PASS**
Ratio: **0.780**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\,L_e$$

$$p_s = (150\ \mathrm{psf/ft}) \times (7.75\ \mathrm{ft})$$

$$p_s = 1.1625\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{\left(1.1307\ \mathrm{kip/ft^2}\right)}{\left(1.1625\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = 0.97267$$

Status: **PASS**
Ratio: **0.970**

Considering z-direction:

$H_o = -0.0020701\ \mathrm{kip/ft}$ - Lateral force per length of pile,
$M_o = 0.011624\ \mathrm{kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (0.011624\ \mathrm{kipft/ft}) \times (7.75\ \mathrm{ft})) + \left(3 \times (-0.0020701\ \mathrm{kip/ft}) \times (7.75\ \mathrm{ft})^2\right)}{(6 \times (0.011624\ \mathrm{kipft/ft})) + (4 \times (-0.0020701\ \mathrm{kip/ft}) \times (7.75\ \mathrm{ft}))}$$

$$a = 5.4761\ \mathrm{ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\,[(4\,M_o) + (3\,H_o\,L_e)]^2}{L_e^2\,[(3\,M_o) + (2\,H_o\,L_e)]}$$

$$p = \frac{0.75 \times [(4 \times (0.011624\ \mathrm{kipft/ft})) + (3 \times (-0.0020701\ \mathrm{kip/ft}) \times (7.75\ \mathrm{ft}))]^{\ 2}}{(7.75\ \mathrm{ft})^2 \times [(3 \times (0.011624\ \mathrm{kipft/ft})) + (2 \times (-0.0020701\ \mathrm{kip/ft}) \times (7.75\ \mathrm{ft}))]}$$

$$p = 0.000011937\ \mathrm{kip/ft^2}$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\,[(2\,M_o) + (H_o\,L_e)]}{L_e^2}$$

$$s = \frac{6 \times [(2 \times (0.011624\ \mathrm{kipft/ft})) + ((-0.0020701\ \mathrm{kip/ft}) \times (7.75\ \mathrm{ft}))]}{(7.75\ \mathrm{ft})^2}$$

$$s = 0.00071979\ \mathrm{kip/ft^2}$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\,\frac{a}{2}$$

$$p_a = (150\ \mathrm{psf/ft}) \times \frac{(5.4761\ \mathrm{ft})}{2}$$

$$p_a = 0.41071\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{\left(0.000011937\ \mathrm{kip/ft^2}\right)}{\left(0.41071\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = 0.000029065$$

Status: **PASS**
Ratio: **0.000**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\,L_e$$

$$p_s = (150\ \mathrm{psf/ft}) \times (7.75\ \mathrm{ft})$$

$$p_s = 1.1625\ \mathrm{kip/ft^2}$$

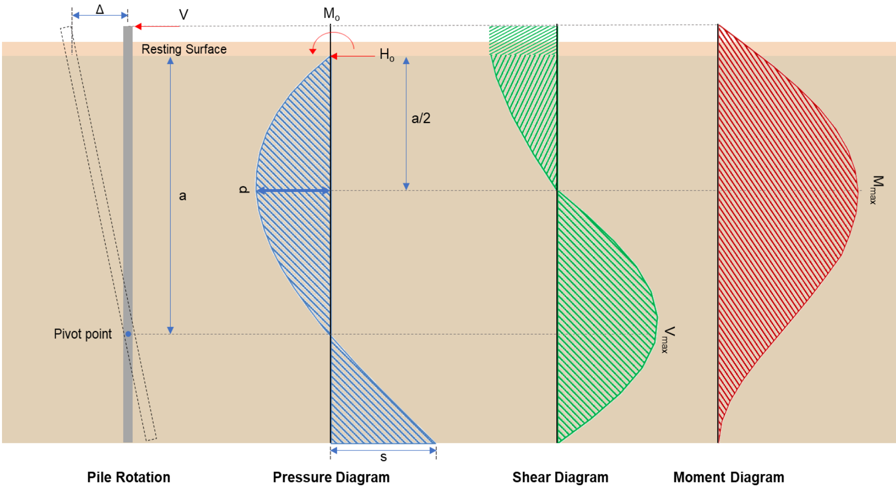
$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{(0.00071979\ \mathrm{kip/ft^2})}{(1.1625\ \mathrm{kip/ft^2})}$$

$$Ratio = 0.00061918$$

Status: **PASS**
Ratio: **0.000**



Shear force and Bending moment (x-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\ D}$$

$$H_o = \frac{(-4.081\ \mathrm{kip})}{1.57 \times (48\ \mathrm{in})}$$

$$H_o = -0.64984\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\ H)}{1.57\ D}$$

$$M_o = \frac{(76.518\ \mathrm{kipft}) + ((-4.081\ \mathrm{kip}) \times (0\ \mathrm{ft}))}{1.57 \times (48\ \mathrm{in})}$$

$$M_o = 12.184\ \mathrm{kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(12.184\ \mathrm{kipft/ft})}{(-0.64984\ \mathrm{kip/ft})}$$

$$E = 18.75\ \mathrm{ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\ M_o\ L_e) + \left(3\ H_o\ L_e^2\right)}{(6\ M_o) + (4\ H_o\ L_e)}$$

$$a = \frac{(4 \times (12.184\ \mathrm{kipft/ft}) \times (7.75\ \mathrm{ft})) + \left(3 \times (-0.64984\ \mathrm{kip/ft}) \times (7.75\ \mathrm{ft})^2\right)}{(6 \times (12.184\ \mathrm{kipft/ft})) + (4 \times (-0.64984\ \mathrm{kip/ft}) \times (7.75\ \mathrm{ft}))}$$

$$a = 5.3062\ \mathrm{ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\ D) \left[1 - \left[3 \left(\frac{4\ E}{L_e} + 3\right) \left(\frac{a}{L_e}\right)^2\right] + \left[4 \left(\frac{3\ E}{L_e} + 2\right) \left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((-0.64984\ \mathrm{kip/ft}) \times (48\ \mathrm{in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (18.75\ \mathrm{ft})}{(7.75\ \mathrm{ft})} + 3\right) \times \left(\frac{(5.3062\ \mathrm{ft})}{(7.75\ \mathrm{ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (18.75\ \mathrm{ft})}{(7.75\ \mathrm{ft})} + 2\right) \times \left(\frac{(5.3062\ \mathrm{ft})}{(7.75\ \mathrm{ft})}\right)^3\right]\right]$$

$$V_{max} = 12.848\ \mathrm{kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\,D\,L_e)\left[\left(\frac{E}{L_e} + \frac{a}{2\,L_e}\right) - \left[\left(\frac{4\,E}{L_e} + 3\right)\left(\frac{a}{2\,L_e}\right)^3\right] + \left[\left(\frac{3\,E}{L_e} + 2\right)\left(\frac{a}{2\,L_e}\right)^4\right]\right]$$

$$M_{max} = ((-0.64984\ \mathrm{kip/ft}) \times (48\ \mathrm{in}) \times (7.75\ \mathrm{ft})) \times \left[\left(\frac{(18.75\ \mathrm{ft})}{(7.75\ \mathrm{ft})} + \frac{(5.3062\ \mathrm{ft})}{2 \times (7.75\ \mathrm{ft})}\right) - \left[\left(\frac{4 \times (18.75\ \mathrm{ft})}{(7.75\ \mathrm{ft})} + 3\right) \times \left(\frac{(5.3062\ \mathrm{ft})}{2 \times (7.75\ \mathrm{ft})}\right)^3\right] + \left[\left(\frac{3 \times (18.75\ \mathrm{ft})}{(7.75\ \mathrm{ft})} + 2\right) \times \left(\frac{(5.3062\ \mathrm{ft})}{2 \times (7.75\ \mathrm{ft})}\right)^4\right]\right]$$

$$M_{max} = 47.95\ \mathrm{kipft}$$

Shear force and Bending moment (z-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\,b}$$

$$H_o = \frac{(-0.022\ \mathrm{kip})}{1.57 \times (48\ \mathrm{in})}$$

$$H_o = -0.0035032\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\,H)}{1.57\,b}$$

$$M_o = \frac{(0.125\ \mathrm{kipft}) + ((-0.022\ \mathrm{kip}) \times (0\ \mathrm{ft}))}{1.57 \times (48\ \mathrm{in})}$$

$$M_o = 0.019904\ \mathrm{kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(0.019904\ \mathrm{kipft/ft})}{(-0.0035032\ \mathrm{kip/ft})}$$

$$E = 5.6818\ \mathrm{ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (0.019904\ \mathrm{kipft/ft}) \times (7.75\ \mathrm{ft})) + \left(3 \times (-0.0035032\ \mathrm{kip/ft}) \times (7.75\ \mathrm{ft})^2\right)}{(6 \times (0.019904\ \mathrm{kipft/ft})) + (4 \times (-0.0035032\ \mathrm{kip/ft}) \times (7.75\ \mathrm{ft}))}$$

$$a = 5.4742\ \mathrm{ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\,b)\left[1 - \left[3\left(\frac{4\,E}{L_e} + 3\right)\left(\frac{a}{L_e}\right)^2\right] + \left[4\left(\frac{3\,E}{L_e} + 2\right)\left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((-0.0035032\ \mathrm{kip/ft}) \times (48\ \mathrm{in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (5.6818\ \mathrm{ft})}{(7.75\ \mathrm{ft})} + 3\right) \times \left(\frac{(5.4742\ \mathrm{ft})}{(7.75\ \mathrm{ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (5.6818\ \mathrm{ft})}{(7.75\ \mathrm{ft})} + 2\right) \times \left(\frac{(5.4742\ \mathrm{ft})}{(7.75\ \mathrm{ft})}\right)^3\right]\right]$$

$$V_{max} = 0.027465\ \mathrm{kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\,b\,L_e)\left[\left(\frac{E}{L_e} + \frac{a}{2\,L_e}\right) - \left[\left(\frac{4\,E}{L_e} + 3\right)\left(\frac{a}{2\,L_e}\right)^3\right] + \left[\left(\frac{3\,E}{L_e} + 2\right)\left(\frac{a}{2\,L_e}\right)^4\right]\right]$$

$$M_{max} = ((-0.0035032\ \mathrm{kip/ft}) \times (48\ \mathrm{in}) \times (7.75\ \mathrm{ft})) \times \left[\left(\frac{(5.6818\ \mathrm{ft})}{(7.75\ \mathrm{ft})} + \frac{(5.4742\ \mathrm{ft})}{2 \times (7.75\ \mathrm{ft})}\right) - \left[\left(\frac{4 \times (5.6818\ \mathrm{ft})}{(7.75\ \mathrm{ft})} + 3\right) \times \left(\frac{(5.4742\ \mathrm{ft})}{2 \times (7.75\ \mathrm{ft})}\right)^3\right] + \left[\left(\frac{3 \times (5.6818\ \mathrm{ft})}{(7.75\ \mathrm{ft})} + 2\right) \times \left(\frac{(5.4742\ \mathrm{ft})}{2 \times (7.75\ \mathrm{ft})}\right)^4\right]\right]$$

$$M_{max} = 0.096686 \text{ kipft}$$

Minimum Reinforcement Check (LRFD)

Parameters:

$f'_{ck} = 3 \text{ ksi}$ - Concrete strength,
$f_{yk} = 60 \text{ ksi}$ - Longitudinal reinforcement strength,
$\phi = 0.65$ - Reduction factor for axial strength,
Table 22.4.2.1 $\alpha = 0.8$ - Alpha factor for axial strength,
$A_g = 2304 \text{ in}^2$ - Gross area of concrete,

Longitudinal reinforcement:

Required reinforcement due to axial load, $A_{st,required}$
22.4.2.2, 10.6.1.1 $A_{st,required}$

$$A_{st,required} = Min\left[\frac{\frac{P}{\phi\ \alpha} - (0.85\ f'_{ck}\ A_g)}{f_{yk} - (0.85\ f'_{ck})}, (0.08\ A_g)\right]$$

$$A_{st,required} = Min\left[\frac{\frac{(11.647 \text{ kip})}{(0.65)\times(0.8)} - (0.85 \times (3 \text{ ksi}) \times (2304 \text{ in}^2))}{(60 \text{ ksi}) - (0.85 \times (3 \text{ ksi}))}, (0.08 \times (2304 \text{ in}^2))\right]$$

$$A_{st,required} = -101.88 \text{ in}^2$$

A_{min} - Governing minimum reinforcement area,

$$A_{min} = Max\left[A_{st,required}, (0.0018\ A_g)\right]$$

$$A_{min} = Max\left[(-101.88 \text{ in}^2), (0.0018 \times (2304 \text{ in}^2))\right]$$

$$A_{min} = 4.1472 \text{ in}^2$$

n_{rebar} - Required number of reinforcement,

$$n_{rebar} = \frac{A_{min}}{A_{rebar}}$$

$$n_{rebar} = \frac{(4.1472 \text{ in}^2)}{(0.3068 \text{ in}^2)}$$

$$n_{rebar} = 14$$

A_{st} - Actual total reinforcement area,

$$A_{st} = n_{rebar}\ \frac{\pi\ d_{bar}^2}{4}$$

$$A_{st} = (14) \times \frac{\pi \times (0.625 \text{ in})^2}{4}$$

$$A_{st} = 4.2951 \text{ in}^2$$

$Ratio$ - Capacity

$$Ratio = \frac{A_{min}}{A_{st}}$$

$$Ratio = \frac{(4.1472 \text{ in}^2)}{(4.2951 \text{ in}^2)}$$

$$Ratio = 0.96556$$

Status: **PASS**
Ratio: **0.970**

25.2.3 s_{rebar} - Minimum spacing of reinforcement,

$$s_{rebar} = Max\left[1.5, (1.5\ d_{bar})\right]$$

$$s_{rebar} = Max\left[1.5, (1.5 \times (0.625 \text{ in}))\right]$$

$$s_{rebar} = 1.5 \text{ in}$$

Ties:

25.7.2.2 Since longitudinal reinforcement is $\leq$ No. 10ø: Use #3(0.375 in)
25.7.2.1 s_{ties} - Maximum spacing of ties,

$$s_{ties} = Min\left[(16\ d_{bar}), (48\ d_{ties}), Min(D, b)\right]$$

$$s_{ties} = Min\left[(16 \times (0.625 \text{ in})), (48 \times (0.375 \text{ in})), Min((48 \text{ in}), (48 \text{ in}))\right]$$

$$s_{ties} = 10 \text{ in}$$

Summary:

Main reinforcement: **14 - #5 (0.625 in)**

	Ties: **#3(0.375 in) - 10 in**	

Axial Compression Strength (ACI 318-19, LRFD)

22.4.2.2 ϕP_N - Allowable axial compressive strength

$$\phi P_N = \phi\, 0.80\, \left[\left(0.85\, f'_{ck}\, [A_g - A_{st}]\right) + \left(f_{yk}\, A_{st}\right)\right]$$

$$\phi P_N = (0.65) \times 0.80 \times \left[\left(0.85 \times (3\ \text{ksi}) \times \left[\left(2304\ \text{in}^2\right) - \left(4.2951\ \text{in}^2\right)\right]\right) + \left((60\ \text{ksi}) \times \left(4.2951\ \text{in}^2\right)\right)\right]$$

$$\phi P_N = 3183.4\ \text{kip}$$

$Ratio$ - Capacity

$$Ratio = \frac{P}{\phi P_N}$$

$$Ratio = \frac{(11.647\ \text{kip})}{(3183.4\ \text{kip})}$$

$$Ratio = 0.0036586$$

Status: **PASS**
Ratio: **0.000**

Shear Strength (ACI 318-19, LRFD)

Parameters:

$b_w = 48\ \text{in}$ - Effective width,

22.5.2.2 d - Effective depth

$$d = 0.80\, D$$

$$d = 0.80 \times (48\ \text{in})$$

$$d = 38.4\ \text{in}$$

22.5.5.1.3 λ_s - size effect modification factor

$$\lambda_s = MIN\left[\sqrt{\frac{2}{1+\frac{d}{10}}}, 1\right]$$

$$\lambda_s = MIN\left[\sqrt{\frac{2}{1+\frac{(38.4\ \text{in})}{10}}}, 1\right]$$

$$\lambda_s = 0.64282$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 3\ \text{ksi} \rightarrow 3000\ \text{psi}$,

22.5.5.1.1 $V_{c,max}$ - Max shear strength of concrete

$$V_{c,max} = 5\, \lambda_s\, \sqrt{f'_{ck}}\, b_w\, d$$

$$V_{c,max} = 5 \times (0.64282) \times \sqrt{(3000\ \text{psi})} \times (48\ \text{in}) \times (38.4\ \text{in})$$

$$V_{c,max} = 324.49\ \text{kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 3\ \text{ksi} \rightarrow 3000\ \text{psi}$, $P = 11.647\ \text{kip} \rightarrow 11647\ \text{lbf}$,

22.5.5.1.1(a) $V_{c,a}$ - Shear strength of concrete (a)

$$V_{c,a} = \left[2\, \lambda_s\, \sqrt{f'_{ck}} + \frac{P}{6\, A_g}\right] b_w\, d$$

$$V_{c,a} = \left[2 \times (0.64282) \times \sqrt{(3000\ \text{psi})} + \frac{(11647\ \text{lbf})}{6 \times \left(2304\ \text{in}^2\right)}\right] \times (48\ \text{in}) \times (38.4\ \text{in})$$

$$V_{c,a} = 131.35\ \text{kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 3\ \text{ksi} \rightarrow 3000\ \text{psi}$,

22.5.5.1.2 $V_{c,b}$ - Shear strength of concrete (b)

$$V_{c,b} = \left[2\, \lambda_s\, \sqrt{f'_{ck}} + \left(0.05\, f'_{ck}\right)\right] b_w\, d$$

$$V_{c,b} = \left[2 \times (0.64282) \times \sqrt{(3000\ \text{psi})} + (0.05 \times (3000\ \text{psi}))\right] \times (48\ \text{in}) \times (38.4\ \text{in})$$

$$V_{c,b} = 406.27\ \text{kip}$$

V_c - Governing shear strength of concrete

$$V_c = Min\left[V_{c,max}, V_{c,a}, V_{c,b}\right]$$

$$V_c = Min\left[(324.49\ \text{kip}), (131.35\ \text{kip}), (406.27\ \text{kip})\right]$$

$$V_c = 131.35\ \text{kip}$$

	The following variables were converted to be consistent with empirical formula $f'_{ck} = 3\ \mathrm{ksi} \rightarrow 3000\ \mathrm{psi}$,	
22.5.1.2	$V_{s,a}$ - Shear strength of steel (a) $$V_{s,a} = 8\sqrt{f'_{ck}}\, b_w\, d$$ $$V_{s,a} = 8 \times \sqrt{(3000\ \mathrm{psi})} \times (48\ \mathrm{in}) \times (38.4\ \mathrm{in})$$ $$V_{s,a} = 807.65\ \mathrm{kip}$$ A_v - Ties rebar area, $$A_v = \frac{\pi\, d_{ties}^2}{4}$$ $$A_v = \frac{\pi \times (0.375\ \mathrm{in})^2}{4}$$ $$A_v = 0.11045\ \mathrm{in}^2$$	
22.5.8.5.3	$V_{s,b}$ - Shear strength of steel (b) $$V_{s,b} = \frac{2\, A_v\, f_{ywk}\, d}{s_{ties}}$$ $$V_{s,b} = \frac{2 \times \left(0.11045\ \mathrm{in}^2\right) \times (60\ \mathrm{ksi}) \times (38.4\ \mathrm{in})}{(10\ \mathrm{in})}$$ $$V_{s,b} = 50.894\ \mathrm{kip}$$ V_s - Governing shear strength of steel $$V_s = MIN\left[V_{s,a}, V_{s,b}\right]$$ $$V_s = MIN\left[(807.65\ \mathrm{kip}), (50.894\ \mathrm{kip})\right]$$ $$V_s = 50.894\ \mathrm{kip}$$	
22.5.1.1	ϕV_n - Allowable shear strength $$\phi V_n = \phi\ (V_c + V_s)$$ $$\phi V_n = (0.65) \times ((131.35\ \mathrm{kip}) + (50.894\ \mathrm{kip}))$$ $$\phi V_n = 118.46\ \mathrm{kip}$$ **Considering x-direction:** $V_{max} = 12.848\ \mathrm{kip}$ - Maximum shear force in the x-direction, $Ratio$ - Capacity $$Ratio = \frac{V_{max}}{\phi V_n}$$ $$Ratio = \frac{(12.848\ \mathrm{kip})}{(118.46\ \mathrm{kip})}$$ $$Ratio = 0.10846$$	Status: **PASS** Ratio: **0.110**
	Considering z-direction: $V_{max} = 0.027465\ \mathrm{kip}$ - Maximum shear force in the z-direction, $Ratio$ - Capacity $$Ratio = \frac{V_{max}}{\phi V_n}$$ $$Ratio = \frac{(0.027465\ \mathrm{kip})}{(118.46\ \mathrm{kip})}$$ $$Ratio = 0.00023185$$	Status: **PASS** Ratio: **0.000**
	Flexural Strength (ACI 318-19, LRFD) S_m - Section modulus $$S_m = \frac{b\, D^2}{6}$$ $$S_m = \frac{(48\ \mathrm{in}) \times (48\ \mathrm{in})^2}{6}$$ $$S_m = 18432\ \mathrm{in}^3$$	

	$\lambda = 1$ - Concrete modification factor (Normal concrete), Allowable flexural strength: M_n shall be the lesser of: $\phi M_{n,1}$ $$\phi M_{n,1} = \phi \times 5 \times \lambda \times \sqrt{(f_c')} \times S_m$$ $$\phi M_{n,1} = 0.65 \times 5 \times 1 \times \sqrt{(3ksi)} \times 18432.001in^3$$ $$\phi M_{n,1} = 273.423kipft$$	
14.5.2.1b	$\phi M_{n,2}$ $$\phi M_{n,2} = \phi\, 0.85\, f_{ck}'\, S_m$$ $$\phi M_{n,2} = (0.65) \times 0.85 \times (3\ \mathrm{ksi}) \times \left(18432\ \mathrm{in}^3\right)$$ $$\phi M_{n,2} = 2545.9\ \mathrm{kipft}$$ Therefore, ϕM_n - Allowable flexural strength, $$\phi M_n = MIN[\phi M_{n,1}, \phi M_{n,2}]$$ $$\phi M_n = MIN[(273.42\ \mathrm{kipft}), (2545.9\ \mathrm{kipft})]$$ $$\phi M_n = 273.42\ \mathrm{kipft}$$ **Considering x-direction:** $M_{max} = 47.95\ \mathrm{kipft}$ - Maximum moment in the x-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\phi M_n}$$ $$Ratio = \frac{(47.95\ \mathrm{kipft})}{(273.42\ \mathrm{kipft})}$$ $$Ratio = 0.17537$$	Status: **PASS** Ratio: **0.180**
	Considering z-direction: $M_{max} = 0.096686\ \mathrm{kipft}$ - Maximum moment in the z-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\phi M_n}$$ $$Ratio = \frac{(0.096686\ \mathrm{kipft})}{(273.42\ \mathrm{kipft})}$$ $$Ratio = 0.00035361$$	Status: **PASS** Ratio: **0.000**

REFERENCES	CALCULATIONS	RESULTS

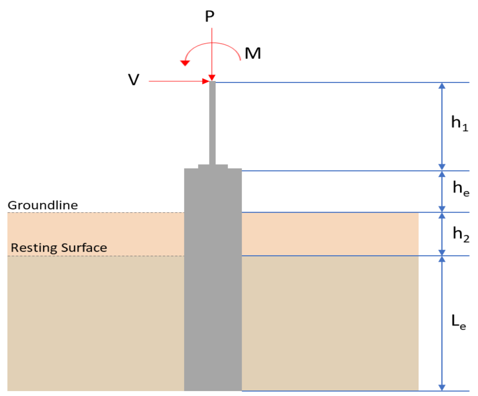
SkyCiv Foundation Design

Pile Foundation

Design Information :

Design code : IBC 2021 (International Building Code)
Unit System : Imperial

Pile Input



Geometry

Pile shape: rectangular
$b = 48\ \text{in}$ - Pile width
$D = 48\ \text{in}$ - Pile depth
$L = 7.75\ \text{ft}$ - Total pile length
$h_1 = 0\ \text{ft}$ - Lateral load height from the top of the pile,
$h_2 = 0\ \text{ft}$ - Depth to resisting surface
$h_e = 0\ \text{ft}$ - Length of pile above the ground

Tabulation of Soil Parameters

Layer	Label	Allowable Bearing Pressure (q_a) (psf)	Allowable Lateral Pressure (R) (psf/ft)
1	Sand, silty sand, clayey sand, silty gravel & clayey gravel	2000.000	150.000

Tabulation of Loads

Load Component	ASD	LRFD
P (kip)	7.381	11.050
V_x (kip)	-2.358	-3.934
V_z (kip)	0.009	0.015
M_x (kipft)	0.045	0.075
M_z (kipft)	43.620	74.227

Material Properties

$f'_{ck} = 3\ \text{ksi}$ - Concrete strength,

Required depth to resist lateral loads (ASD)

H - Point of application of the lateral load

$$H = h_1 + h_2 + h_e$$

$$H = (0\ \text{ft}) + (0\ \text{ft}) + (0\ \text{ft})$$

$$H = 0\ \text{ft}$$

Considering x-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\ D}$$

$$H_o = \frac{(-2.358\ \text{kip})}{1.57 \times (48\ \text{in})}$$

$$H_o = -0.37548\ \text{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\ H)}{1.57\ D}$$

$$M_o = \frac{(43.62\ \text{kipft}) + ((-2.358\ \text{kip}) \times (0\ \text{ft}))}{1.57 \times (48\ \text{in})}$$

$$M_o = 6.9459\ \text{kipft/ft}$$

Required depth of embedment in earth:

$$L_x^3 - \left(14.14 \times \frac{H_o \times L_x}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,x} = 7.312\ \text{ft}$ - Required depth in x-direction,

Considering z-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\, b}$$

$$H_o = \frac{(0.009\ \text{kip})}{1.57 \times (48\ \text{in})}$$

$$H_o = 0.0014331\ \text{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\, H)}{1.57\, b}$$

$$M_o = \frac{(0.045\ \text{kipft}) + ((0.009\ \text{kip}) \times (0\ \text{ft}))}{1.57 \times (48\ \text{in})}$$

$$M_o = 0.0071656\ \text{kipft/ft}$$

Required depth of embedment in earth:

$$L_z^3 - \left(14.14 \times \frac{H_o \times L_z}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,z} = 0.86528\ \text{ft}$ - Required depth in z-direction,

Minimum embedded depth required:

$L_{e,req}$ - Depth of pile required,

$$L_{e,req} = MAX[L_{e,x}, L_{e,z}]$$

$$L_{e,req} = MAX[(7.312\ \text{ft}), (0.86528\ \text{ft})]$$

$$L_{e,req} = 7.312\ \text{ft}$$

L_e - Actual embedded length of pile,

$$L_e = L - h_e - h_2$$

$$L_e = (7.75\ \text{ft}) - (0\ \text{ft}) - (0\ \text{ft})$$

$$L_e = 7.75\ \text{ft}$$

$Ratio$ - Embedded depth

$$Ratio = \frac{L_{e,req}}{L_e}$$

$$Ratio = \frac{(7.312\ \text{ft})}{(7.75\ \text{ft})}$$

$$Ratio = 0.94348$$

Status: **PASS**
Ratio: **0.940**

End-bearing Capacity (ASD)

A - Pile cross-section area

$$A = b\, D$$

$$A = (48\ \text{in}) \times (48\ \text{in})$$

$$A = 16\ \text{ft}^2$$

q - End-bearing pressure

$$q = \frac{P_v}{A}$$

$$q = \frac{(7.381\ \text{kip})}{(16\ \text{ft}^2)}$$

$$q = 0.46131\ \text{kip/ft}^2$$

$q = 0.46131\ \text{kip/ft}^2$

Check bearing capacity ratio:

$Ratio$ - Capacity

$$Ratio = \frac{q}{q_a}$$

$$Ratio = \frac{(0.46131\ \text{kip/ft}^2)}{(2000\ \text{psf})}$$

$$Ratio = 0.23066$$

Status: **PASS**
Ratio: **0.230**

Czerniak

Lateral Soil Pressure (ASD):

L/D - Length to least lateral dimension ratio,

$$L/D = \frac{L}{D}$$

$$L/D = \frac{(7.75\ \text{ft})}{(48\ \text{in})}$$

$$L/D = 1.9375$$

Since L/D $\leq$ 10,

Pile is short.

Considering x-direction:

$H_o = -0.37548\ \text{kip/ft}$ - Lateral force per length of pile,
$M_o = 6.9459\ \text{kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (6.9459\ \text{kipft/ft}) \times (7.75\ \text{ft})) + \left(3 \times (-0.37548\ \text{kip/ft}) \times (7.75\ \text{ft})^2\right)}{(6 \times (6.9459\ \text{kipft/ft})) + (4 \times (-0.37548\ \text{kip/ft}) \times (7.75\ \text{ft}))}$$

$$a = 5.3077\ \text{ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\,[(4\,M_o) + (3\,H_o\,L_e)]^2}{L_e^2\,[(3\,M_o) + (2\,H_o\,L_e)]}$$

$$p = \frac{0.75 \times [(4 \times (6.9459\ \text{kipft/ft})) + (3 \times (-0.37548\ \text{kip/ft}) \times (7.75\ \text{ft}))]^2}{(7.75\ \text{ft})^2 \times [(3 \times (6.9459\ \text{kipft/ft})) + (2 \times (-0.37548\ \text{kip/ft}) \times (7.75\ \text{ft}))]}$$

$$p = 0.30186\ \text{kip/ft}^2$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\,[(2\,M_o) + (H_o\,L_e)]}{L_e^2}$$

$$s = \frac{6 \times [(2 \times (6.9459\ \text{kipft/ft})) + ((-0.37548\ \text{kip/ft}) \times (7.75\ \text{ft}))]}{(7.75\ \text{ft})^2}$$

$$s = 1.097\ \text{kip/ft}^2$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\,\frac{a}{2}$$

$$p_a = (150\ \text{psf/ft}) \times \frac{(5.3077\ \text{ft})}{2}$$

$$p_a = 0.39807\ \text{kip/ft}^2$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{(0.30186\ \text{kip/ft}^2)}{(0.39807\ \text{kip/ft}^2)}$$

$$Ratio = 0.7583$$

Status: **PASS**
Ratio: **0.760**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\,L_e$$

$$p_s = (150\ \mathrm{psf/ft}) \times (7.75\ \mathrm{ft})$$

$$p_s = 1.1625\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{(1.097\ \mathrm{kip/ft^2})}{(1.1625\ \mathrm{kip/ft^2})}$$

$$Ratio = 0.94369$$

Status: **PASS**
Ratio: **0.940**

Considering z-direction:

$H_o = 0.0014331\ \mathrm{kip/ft}$ - Lateral force per length of pile,
$M_o = 0.0071656\ \mathrm{kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (0.0071656\ \mathrm{kipft/ft}) \times (7.75\ \mathrm{ft})) + \left(3 \times (0.0014331\ \mathrm{kip/ft}) \times (7.75\ \mathrm{ft})^2\right)}{(6 \times (0.0071656\ \mathrm{kipft/ft})) + (4 \times (0.0014331\ \mathrm{kip/ft}) \times (7.75\ \mathrm{ft}))}$$

$$a = 5.4949\ \mathrm{ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\,[(4\,M_o) + (3\,H_o\,L_e)]^2}{L_e^2\,[(3\,M_o) + (2\,H_o\,L_e)]}$$

$$p = \frac{0.75 \times [(4 \times (0.0071656\ \mathrm{kipft/ft})) + (3 \times (0.0014331\ \mathrm{kip/ft}) \times (7.75\ \mathrm{ft}))]^{\ 2}}{(7.75\ \mathrm{ft})^2 \times [(3 \times (0.0071656\ \mathrm{kipft/ft})) + (2 \times (0.0014331\ \mathrm{kip/ft}) \times (7.75\ \mathrm{ft}))]}$$

$$p = 0.0010975\ \mathrm{kip/ft^2}$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\,[(2\,M_o) + (H_o\,L_e)]}{L_e^2}$$

$$s = \frac{6 \times [(2 \times (0.0071656\ \mathrm{kipft/ft})) + ((0.0014331\ \mathrm{kip/ft}) \times (7.75\ \mathrm{ft}))]}{(7.75\ \mathrm{ft})^2}$$

$$s = 0.0025411\ \mathrm{kip/ft^2}$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\,\frac{a}{2}$$

$$p_a = (150\ \mathrm{psf/ft}) \times \frac{(5.4949\ \mathrm{ft})}{2}$$

$$p_a = 0.41212\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{(0.0010975\ \mathrm{kip/ft^2})}{(0.41212\ \mathrm{kip/ft^2})}$$

$$Ratio = 0.0026631$$

Status: **PASS**
Ratio: **0.000**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\,L_e$$

$$p_s = (150\ \mathrm{psf/ft}) \times (7.75\ \mathrm{ft})$$

$$p_s = 1.1625\ \mathrm{kip/ft^2}$$

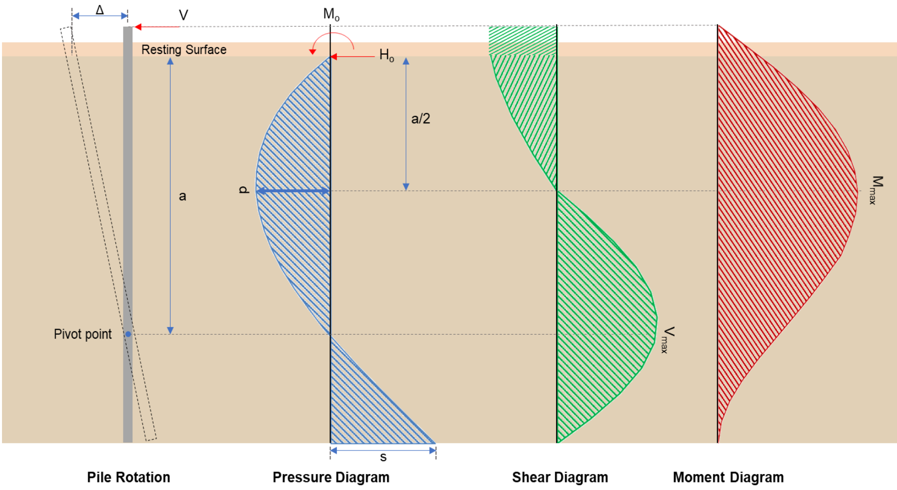
$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{(0.0025411 \text{ kip/ft}^2)}{(1.1625 \text{ kip/ft}^2)}$$

$$Ratio = 0.0021859$$

Status: **PASS**
Ratio: **0.000**



Shear force and Bending moment (x-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\,D}$$

$$H_o = \frac{(-3.934 \text{ kip})}{1.57 \times (48 \text{ in})}$$

$$H_o = -0.62643 \text{ kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\,H)}{1.57\,D}$$

$$M_o = \frac{(74.227 \text{ kipft}) + ((-3.934 \text{ kip}) \times (0 \text{ ft}))}{1.57 \times (48 \text{ in})}$$

$$M_o = 11.82 \text{ kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(11.82 \text{ kipft/ft})}{(-0.62643 \text{ kip/ft})}$$

$$E = 18.868 \text{ ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (11.82 \text{ kipft/ft}) \times (7.75 \text{ ft})) + \left(3 \times (-0.62643 \text{ kip/ft}) \times (7.75 \text{ ft})^2\right)}{(6 \times (11.82 \text{ kipft/ft})) + (4 \times (-0.62643 \text{ kip/ft}) \times (7.75 \text{ ft}))}$$

$$a = 5.3055 \text{ ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\,D)\left[1 - \left[3\left(\frac{4\,E}{L_e} + 3\right)\left(\frac{a}{L_e}\right)^2\right] + \left[4\left(\frac{3\,E}{L_e} + 2\right)\left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((-0.62643 \text{ kip/ft}) \times (48 \text{ in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (18.868 \text{ ft})}{(7.75 \text{ ft})} + 3\right) \times \left(\frac{(5.3055 \text{ ft})}{(7.75 \text{ ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (18.868 \text{ ft})}{(7.75 \text{ ft})} + 2\right) \times \left(\frac{(5.3055 \text{ ft})}{(7.75 \text{ ft})}\right)^3\right]\right]$$

$$V_{max} = 12.453 \text{ kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\,D\,L_e)\left[\left(\frac{E}{L_e} + \frac{a}{2\,L_e}\right) - \left[\left(\frac{4\,E}{L_e} + 3\right)\left(\frac{a}{2\,L_e}\right)^3\right] + \left[\left(\frac{3\,E}{L_e} + 2\right)\left(\frac{a}{2\,L_e}\right)^4\right]\right]$$

$$M_{max} = ((-0.62643\text{ kip/ft}) \times (48\text{ in}) \times (7.75\text{ ft})) \times \left[\left(\frac{(18.868\text{ ft})}{(7.75\text{ ft})} + \frac{(5.3055\text{ ft})}{2 \times (7.75\text{ ft})}\right) - \left[\left(\frac{4 \times (18.868\text{ ft})}{(7.75\text{ ft})} + 3\right) \times \left(\frac{(5.3055\text{ ft})}{2 \times (7.75\text{ ft})}\right)^3\right] + \left[\left(\frac{3 \times (18.868\text{ ft})}{(7.75\text{ ft})} + 2\right) \times \left(\frac{(5.3055\text{ ft})}{2 \times (7.75\text{ ft})}\right)^4\right]\right]$$

$$M_{max} = 46.485\text{ kipft}$$

Shear force and Bending moment (z-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\,b}$$

$$H_o = \frac{(0.015\text{ kip})}{1.57 \times (48\text{ in})}$$

$$H_o = 0.0023885\text{ kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\,H)}{1.57\,b}$$

$$M_o = \frac{(0.075\text{ kipft}) + ((0.015\text{ kip}) \times (0\text{ ft}))}{1.57 \times (48\text{ in})}$$

$$M_o = 0.011943\text{ kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(0.011943\text{ kipft/ft})}{(0.0023885\text{ kip/ft})}$$

$$E = 5\text{ ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (0.011943\text{ kipft/ft}) \times (7.75\text{ ft})) + \left(3 \times (0.0023885\text{ kip/ft}) \times (7.75\text{ ft})^2\right)}{(6 \times (0.011943\text{ kipft/ft})) + (4 \times (0.0023885\text{ kip/ft}) \times (7.75\text{ ft}))}$$

$$a = 5.4949\text{ ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\,b)\left[1 - \left[3\left(\frac{4\,E}{L_e} + 3\right)\left(\frac{a}{L_e}\right)^2\right] + \left[4\left(\frac{3\,E}{L_e} + 2\right)\left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((0.0023885\text{ kip/ft}) \times (48\text{ in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (5\text{ ft})}{(7.75\text{ ft})} + 3\right) \times \left(\frac{(5.4949\text{ ft})}{(7.75\text{ ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (5\text{ ft})}{(7.75\text{ ft})} + 2\right) \times \left(\frac{(5.4949\text{ ft})}{(7.75\text{ ft})}\right)^3\right]\right]$$

$$V_{max} = 0.017249\text{ kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\,b\,L_e)\left[\left(\frac{E}{L_e} + \frac{a}{2\,L_e}\right) - \left[\left(\frac{4\,E}{L_e} + 3\right)\left(\frac{a}{2\,L_e}\right)^3\right] + \left[\left(\frac{3\,E}{L_e} + 2\right)\left(\frac{a}{2\,L_e}\right)^4\right]\right]$$

$$M_{max} = ((0.0023885\text{ kip/ft}) \times (48\text{ in}) \times (7.75\text{ ft})) \times \left[\left(\frac{(5\text{ ft})}{(7.75\text{ ft})} + \frac{(5.4949\text{ ft})}{2 \times (7.75\text{ ft})}\right) - \left[\left(\frac{4 \times (5\text{ ft})}{(7.75\text{ ft})} + 3\right) \times \left(\frac{(5.4949\text{ ft})}{2 \times (7.75\text{ ft})}\right)^3\right] + \left[\left(\frac{3 \times (5\text{ ft})}{(7.75\text{ ft})} + 2\right) \times \left(\frac{(5.4949\text{ ft})}{2 \times (7.75\text{ ft})}\right)^4\right]\right]$$

$$M_{max} = 0.060213 \text{ kipft}$$

Minimum Reinforcement Check (LRFD)

Parameters:

$f'_{ck} = 3 \text{ ksi}$ - Concrete strength,
$f_{yk} = 60 \text{ ksi}$ - Longitudinal reinforcement strength,
$\phi = 0.65$ - Reduction factor for axial strength,
Table 22.4.2.1 $\alpha = 0.8$ - Alpha factor for axial strength,
$A_g = 2304 \text{ in}^2$ - Gross area of concrete,

Longitudinal reinforcement:

Required reinforcement due to axial load, $A_{st,required}$
22.4.2.2, 10.6.1.1 $A_{st,required}$

$$A_{st,required} = Min\left[\frac{\frac{P}{\phi\ \alpha} - (0.85\ f'_{ck}\ A_g)}{f_{yk} - (0.85\ f'_{ck})}, (0.08\ A_g)\right]$$

$$A_{st,required} = Min\left[\frac{\frac{(11.05 \text{ kip})}{(0.65)\times(0.8)} - \left(0.85 \times (3 \text{ ksi}) \times \left(2304 \text{ in}^2\right)\right)}{(60 \text{ ksi}) - (0.85 \times (3 \text{ ksi}))}, \left(0.08 \times \left(2304 \text{ in}^2\right)\right)\right]$$

$$A_{st,required} = -101.9 \text{ in}^2$$

A_{min} - Governing minimum reinforcement area,

$$A_{min} = Max\left[A_{st,required}, (0.0018\ A_g)\right]$$

$$A_{min} = Max\left[\left(-101.9 \text{ in}^2\right), \left(0.0018 \times \left(2304 \text{ in}^2\right)\right)\right]$$

$$A_{min} = 4.1472 \text{ in}^2$$

n_{rebar} - Required number of reinforcement,

$$n_{rebar} = \frac{A_{min}}{A_{rebar}}$$

$$n_{rebar} = \frac{\left(4.1472 \text{ in}^2\right)}{\left(0.3068 \text{ in}^2\right)}$$

$$n_{rebar} = 14$$

A_{st} - Actual total reinforcement area,

$$A_{st} = n_{rebar}\ \frac{\pi\ d_{bar}^2}{4}$$

$$A_{st} = (14) \times \frac{\pi \times (0.625 \text{ in})^2}{4}$$

$$A_{st} = 4.2951 \text{ in}^2$$

$Ratio$ - Capacity

$$Ratio = \frac{A_{min}}{A_{st}}$$

$$Ratio = \frac{\left(4.1472 \text{ in}^2\right)}{\left(4.2951 \text{ in}^2\right)}$$

$$Ratio = 0.96556$$

Status: **PASS**
Ratio: **0.970**

25.2.3 s_{rebar} - Minimum spacing of reinforcement,

$$s_{rebar} = Max\left[1.5, (1.5\ d_{bar})\right]$$

$$s_{rebar} = Max\left[1.5, (1.5 \times (0.625 \text{ in}))\right]$$

$$s_{rebar} = 1.5 \text{ in}$$

Ties:

25.7.2.2 Since longitudinal reinforcement is $\leq$ No. 10ø: Use #3(0.375 in)
25.7.2.1 s_{ties} - Maximum spacing of ties,

$$s_{ties} = Min\left[(16\ d_{bar}), (48\ d_{ties}), Min\,(D, b)\right]$$

$$s_{ties} = Min\left[(16 \times (0.625 \text{ in})), (48 \times (0.375 \text{ in})), Min\,((48 \text{ in}), (48 \text{ in}))\right]$$

$$s_{ties} = 10 \text{ in}$$

Summary:

Main reinforcement: **14 - #5 (0.625 in)**

	Ties: **#3(0.375 in) - 10 in**	
	Axial Compression Strength (ACI 318-19, LRFD)	
22.4.2.2	ϕP_N - Allowable axial compressive strength $$\phi P_N = \phi\ 0.80\ \left[\left(0.85\ f'_{ck}\ [A_g - A_{st}]\right) + (f_{yk}\ A_{st})\right]$$ $$\phi P_N = (0.65) \times 0.80 \times \left[\left(0.85 \times (3\ \text{ksi}) \times \left[\left(2304\ \text{in}^2\right) - \left(4.2951\ \text{in}^2\right)\right]\right) + \left((60\ \text{ksi}) \times \left(4.2951\ \text{in}^2\right)\right)\right]$$ $$\phi P_N = 3183.4\ \text{kip}$$ $Ratio$ - Capacity $$Ratio = \frac{P}{\phi P_N}$$ $$Ratio = \frac{(11.05\ \text{kip})}{(3183.4\ \text{kip})}$$ $$Ratio = 0.0034711$$	Status: **PASS** Ratio: **0.000**
	Shear Strength (ACI 318-19, LRFD) **Parameters:** $b_w = 48\ \text{in}$ - Effective width,	
22.5.2.2	d - Effective depth $$d = 0.80\ D$$ $$d = 0.80 \times (48\ \text{in})$$ $$d = 38.4\ \text{in}$$	
22.5.5.1.3	λ_s - size effect modification factor $$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{d}{10}}}, 1\right]$$ $$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{(38.4\ \text{in})}{10}}}, 1\right]$$ $$\lambda_s = 0.64282$$ The following variables were converted to be consistent with empirical formula $f'_{ck} = 3\ \text{ksi} \rightarrow 3000\ \text{psi}$,	
22.5.5.1.1	$V_{c,max}$ - Max shear strength of concrete $$V_{c,max} = 5\ \lambda_s\ \sqrt{f'_{ck}}\ b_w\ d$$ $$V_{c,max} = 5 \times (0.64282) \times \sqrt{(3000\ \text{psi})} \times (48\ \text{in}) \times (38.4\ \text{in})$$ $$V_{c,max} = 324.49\ \text{kip}$$ The following variables were converted to be consistent with empirical formula $f'_{ck} = 3\ \text{ksi} \rightarrow 3000\ \text{psi}$,$P = 11.05\ \text{kip} \rightarrow 11050\ \text{lbf}$,	
22.5.5.1.1(a)	$V_{c,a}$ - Shear strength of concrete (a) $$V_{c,a} = \left[2\ \lambda_s\ \sqrt{f'_{ck}} + \frac{P}{6\ A_g}\right]\ b_w\ d$$ $$V_{c,a} = \left[2 \times (0.64282) \times \sqrt{(3000\ \text{psi})} + \frac{(11050\ \text{lbf})}{6 \times (2304\ \text{in}^2)}\right] \times (48\ \text{in}) \times (38.4\ \text{in})$$ $$V_{c,a} = 131.27\ \text{kip}$$ The following variables were converted to be consistent with empirical formula $f'_{ck} = 3\ \text{ksi} \rightarrow 3000\ \text{psi}$,	
22.5.5.1.2	$V_{c,b}$ - Shear strength of concrete (b) $$V_{c,b} = \left[2\ \lambda_s\ \sqrt{f'_{ck}} + \left(0.05\ f'_{ck}\right)\right]\ b_w\ d$$ $$V_{c,b} = \left[2 \times (0.64282) \times \sqrt{(3000\ \text{psi})} + (0.05 \times (3000\ \text{psi}))\right] \times (48\ \text{in}) \times (38.4\ \text{in})$$ $$V_{c,b} = 406.27\ \text{kip}$$ V_c - Governing shear strength of concrete $$V_c = Min\left[V_{c,max}, V_{c,a}, V_{c,b}\right]$$ $$V_c = Min\left[(324.49\ \text{kip}), (131.27\ \text{kip}), (406.27\ \text{kip})\right]$$ $$V_c = 131.27\ \text{kip}$$	

	The following variables were converted to be consistent with empirical formula $f'_{ck} = 3\ \mathrm{ksi} \rightarrow 3000\ \mathrm{psi}$,	
22.5.1.2	$V_{s,a}$ - Shear strength of steel (a) $$V_{s,a} = 8\sqrt{f'_{ck}}\, b_w\, d$$ $$V_{s,a} = 8 \times \sqrt{(3000\ \mathrm{psi})} \times (48\ \mathrm{in}) \times (38.4\ \mathrm{in})$$ $$V_{s,a} = 807.65\ \mathrm{kip}$$	
	A_v - Ties rebar area, $$A_v = \frac{\pi\, d_{ties}^2}{4}$$ $$A_v = \frac{\pi \times (0.375\ \mathrm{in})^2}{4}$$ $$A_v = 0.11045\ \mathrm{in}^2$$	
22.5.8.5.3	$V_{s,b}$ - Shear strength of steel (b) $$V_{s,b} = \frac{2\, A_v\, f_{ywk}\, d}{s_{ties}}$$ $$V_{s,b} = \frac{2 \times \left(0.11045\ \mathrm{in}^2\right) \times (60\ \mathrm{ksi}) \times (38.4\ \mathrm{in})}{(10\ \mathrm{in})}$$ $$V_{s,b} = 50.894\ \mathrm{kip}$$	
	V_s - Governing shear strength of steel $$V_s = MIN\left[V_{s,a}, V_{s,b}\right]$$ $$V_s = MIN\left[(807.65\ \mathrm{kip}), (50.894\ \mathrm{kip})\right]$$ $$V_s = 50.894\ \mathrm{kip}$$	
22.5.1.1	ϕV_n - Allowable shear strength $$\phi V_n = \phi\, (V_c + V_s)$$ $$\phi V_n = (0.65) \times ((131.27\ \mathrm{kip}) + (50.894\ \mathrm{kip}))$$ $$\phi V_n = 118.4\ \mathrm{kip}$$	
	Considering x-direction: $V_{max} = 12.453\ \mathrm{kip}$ - Maximum shear force in the x-direction, $Ratio$ - Capacity $$Ratio = \frac{V_{max}}{\phi V_n}$$ $$Ratio = \frac{(12.453\ \mathrm{kip})}{(118.4\ \mathrm{kip})}$$ $$Ratio = 0.10517$$	Status: **PASS** Ratio: **0.110**
	Considering z-direction: $V_{max} = 0.017249\ \mathrm{kip}$ - Maximum shear force in the z-direction, $Ratio$ - Capacity $$Ratio = \frac{V_{max}}{\phi V_n}$$ $$Ratio = \frac{(0.017249\ \mathrm{kip})}{(118.4\ \mathrm{kip})}$$ $$Ratio = 0.00014568$$	Status: **PASS** Ratio: **0.000**
	Flexural Strength (ACI 318-19, LRFD) S_m - Section modulus $$S_m = \frac{b\, D^2}{6}$$ $$S_m = \frac{(48\ \mathrm{in}) \times (48\ \mathrm{in})^2}{6}$$ $$S_m = 18432\ \mathrm{in}^3$$	

	$\lambda = 1$ - Concrete modification factor (Normal concrete), Allowable flexural strength: M_n shall be the lesser of: $\phi M_{n,1}$ $$\phi M_{n,1} = \phi \times 5 \times \lambda \times \sqrt{(f_c')} \times S_m$$ $$\phi M_{n,1} = 0.65 \times 5 \times 1 \times \sqrt{(3ksi)} \times 18432.001in^3$$ $$\phi M_{n,1} = 273.423kipft$$	
14.5.2.1b	$\phi M_{n,2}$ $$\phi M_{n,2} = \phi \, 0.85 \, f_{ck}' \, S_m$$ $$\phi M_{n,2} = (0.65) \times 0.85 \times (3 \text{ ksi}) \times \left(18432 \text{ in}^3\right)$$ $$\phi M_{n,2} = 2545.9 \text{ kipft}$$ Therefore, ϕM_n - Allowable flexural strength, $$\phi M_n = MIN[\phi M_{n,1}, \phi M_{n,2}]$$ $$\phi M_n = MIN[(273.42 \text{ kipft}), (2545.9 \text{ kipft})]$$ $$\phi M_n = 273.42 \text{ kipft}$$ **Considering x-direction:** $M_{max} = 46.485 \text{ kipft}$ - Maximum moment in the x-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\phi M_n}$$ $$Ratio = \frac{(46.485 \text{ kipft})}{(273.42 \text{ kipft})}$$ $$Ratio = 0.17001$$	Status: **PASS** Ratio: **0.170**
	Considering z-direction: $M_{max} = 0.060213 \text{ kipft}$ - Maximum moment in the z-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\phi M_n}$$ $$Ratio = \frac{(0.060213 \text{ kipft})}{(273.42 \text{ kipft})}$$ $$Ratio = 0.00022022$$	Status: **PASS** Ratio: **0.000**