Project Details

Project Name: Simpson Denver - Jefferson Square 5x14 - V1Jb

Location: 8600 E Jefferson Ave, Denver, CO 80237, USA

Unique ID: 4P-19.75-6TOP-HD-72-L-5Hx14W-1607

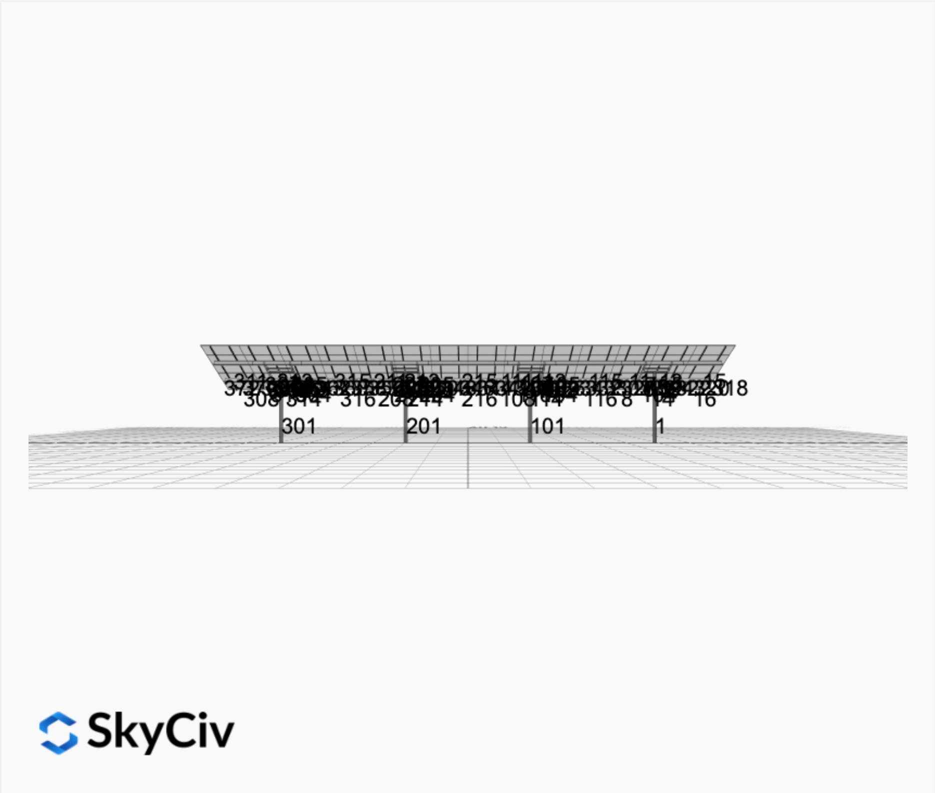
Dealer: ______________________________

Date: Fri Jul 11 2025

Number of Modules: 70

Number of Poles: 4

Date Sold: ______________________________



Array Dimensions N/S	18.83 ft
Array Dimensions E/W	80.27 ft
Winter Tilt Angle	20
Front Edge Clearance	8 ft

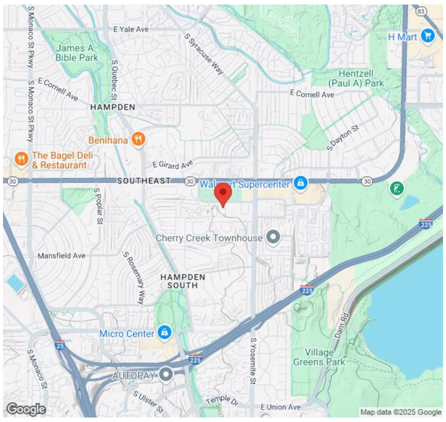
MT Solar Bill of Materials (4P-19.75-6TOP-HD-72-L-5Hx14W-1607)

Part	Short Description	BOM Qty
MTS-PC-6	6IN Pole Cap Assembly	4
MTS-HF-HD	H-Frame Assembly-HD	4
MTS-HD-Wing-72	72IN HD Wing	4
MTS-HD-Splice-90	90IN HD Splice	6
MTS-HD-Splice-57	57IN HD Splice	6
MTS-CLAMP-ANGLE-4PK	Angle Clamp	14

Rail Bill of Materials

Part	Qty
Rails (226in)	28
Rail Attachment	112
Module Mid Clamp	112
Module End Clamp	56
Ground Lug	14

Site Details:



Site Address: 8600 E Jefferson Ave, Denver, CO 80237, USA

Array Specification

Duty Classification:	HD
Module Width:	44.70 in
Module Length:	67.80in
Number of Rows:	5
Number of Columns:	14
Total Number of Modules:	70
Winter Tilt Angle:	20
Front Edge Clearance:	8
Total Array Height at Tilt:	14.44 ft
Total Frame Length:	78.75 ft
Module Info/Notes:	410w
Array Dimensions N/S:	18.83 ft
Array Dimensions E/W:	80.27 ft
Rail Length:	226.00 in
Rail Spacing:	2.87 ft

Support Specifications

Pole Size:	6in Pipe Sch 40
Pole Length above Grade:	11.22 ft
Number of Poles:	4
Pole Spacing:	19.75 ft

Foundation Specifications

Foundation Type:	Square
Foundation Dimensions:	48 x 48 in
Foundation Depth (below grade):	Pile 1: 5.75 ft Pile 2: 5.75 ft Pile 3: 5.75 ft Pile 4: 5.75 ft
Foundation Volume:	13.630 y^3

Site Info

Risk Category:	I
Exposure:	B
Soil Classification:	sand
Site Location:	8600 E Jefferson Ave, Denver, CO 80237, USA
Wind Speed:	101 mph

Snow Load:	20 psf

Design Disclaimer

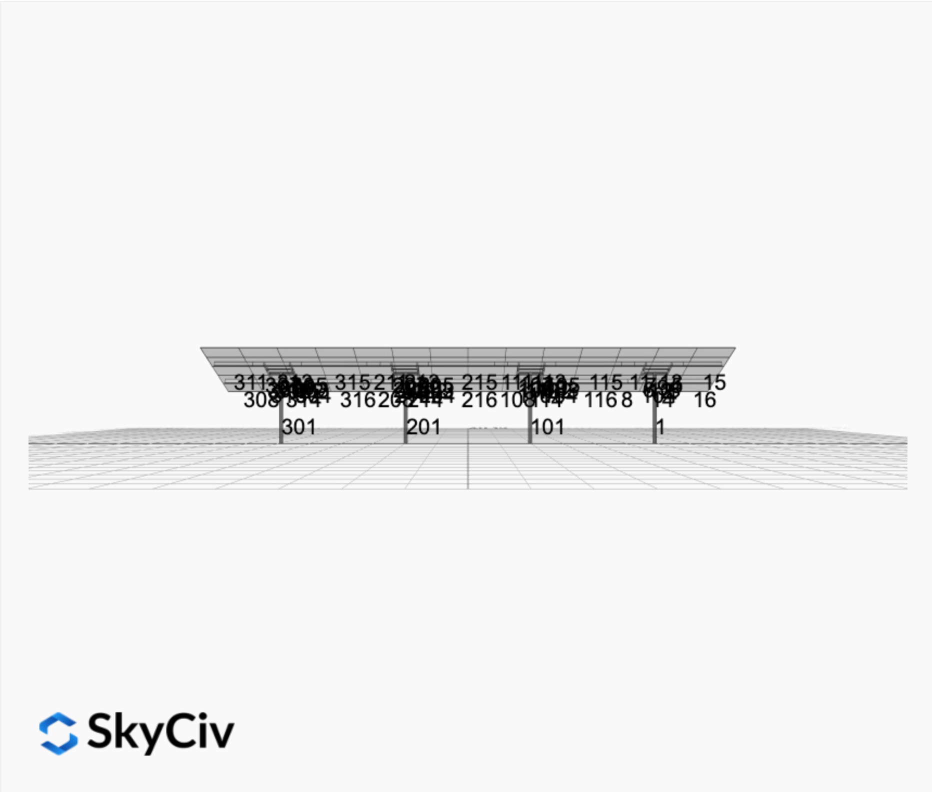
This software should be used for preliminary designs and should not be used as a final design unless reviewed, verified and designed by a qualified structural engineer.

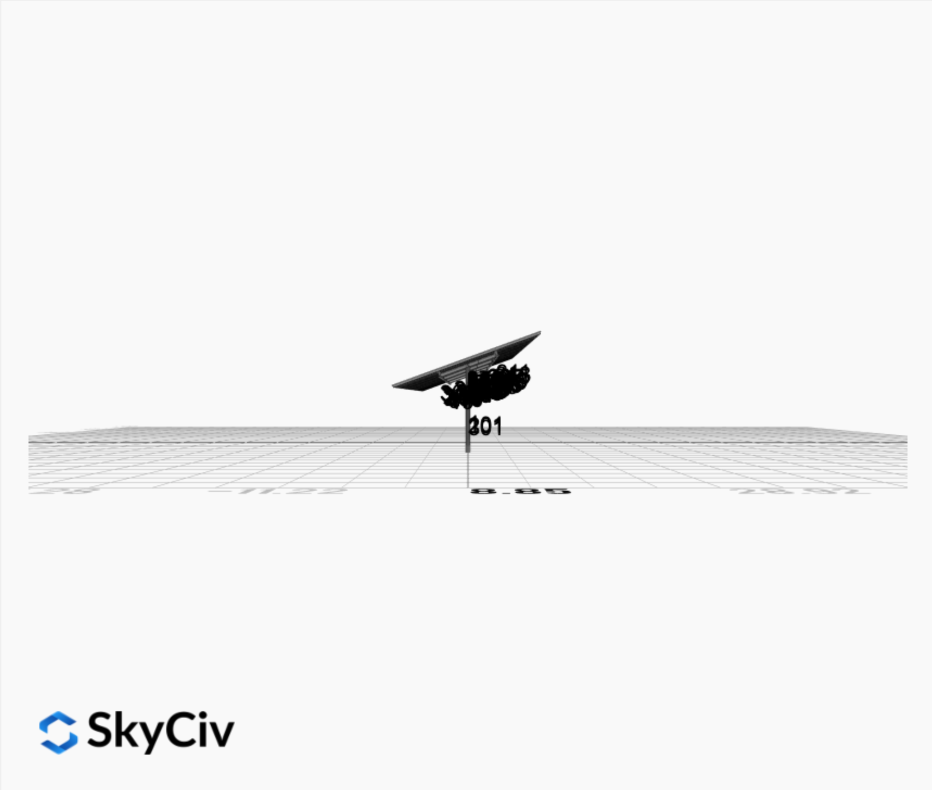
AutoDesigner Input

{"wind_speed_override":null,"snow_load_override":null,"direct_snow_load":false,"add_angle_brace":false,"product_type":"Beam","designer_name":"","designer_email":"","designer_phone":"","project_id":"Simpson Denver - Jefferson Square 5x14 - V1Jb","site_address":"8600 E Jefferson Ave, Denver, CO 80237, USA","module_info":"410w","module_width":44.7,"module_length":67.8,"number_rows":"5","number_columns":14,"pole_mount_section":"4_40","core_pipe_width":65,"core_pipe_section":"2_40","adjuster_section":"2_40","core_beam_height":65,"core_beam_section":"HSS3x2x1/8","main_pipe_section":"2_12GA","pole_spacing":"15","tilt_angle":20,"ground_clearance":8,"risk_category":"I","exposure_category":"B","frame_duty_override":"auto","pole_override":"auto","soil_type":"sand","customer_foundation_override":"48_Square","foundation_type":"Square","foundation_size":48,"check_rails":true}

Design Notes:

- AISC Deflection checks are set to L/1 due to structure design intent
- Foundation Soil Parameters used in this Autodesign are all estimates, proper geotechnical reports are required to confirm soil profiles
- Wind speeds, snow loads and other site specific results are based on ASCE 7 2016
- Steel frame design checks are based on AISC 360 2016 (LRFD)

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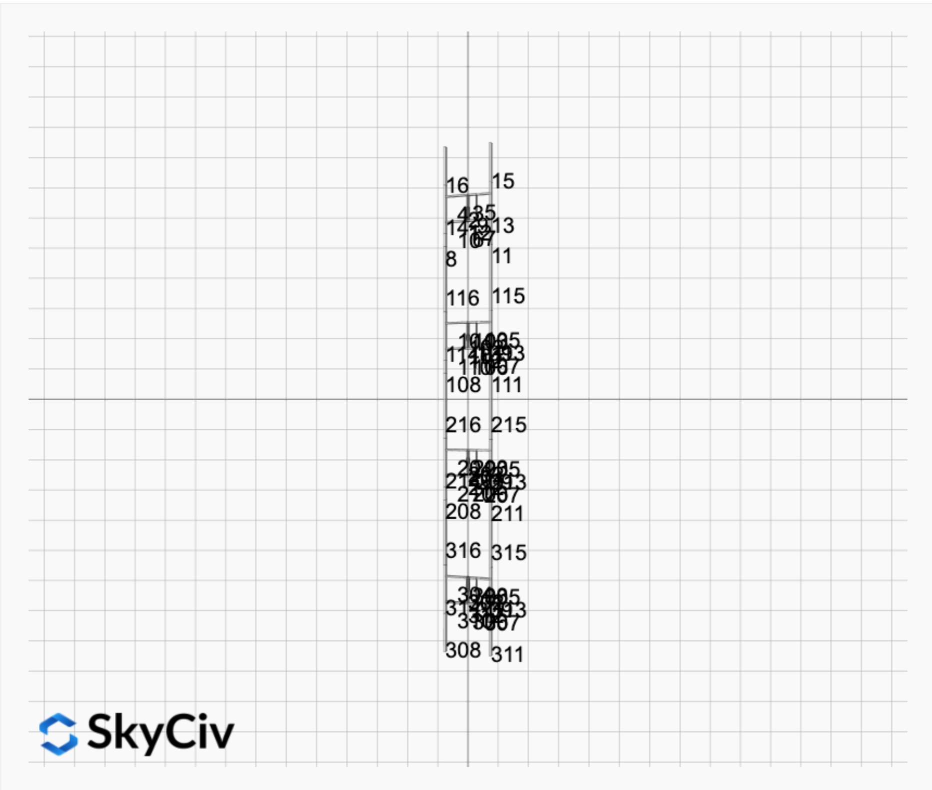
SkyCiv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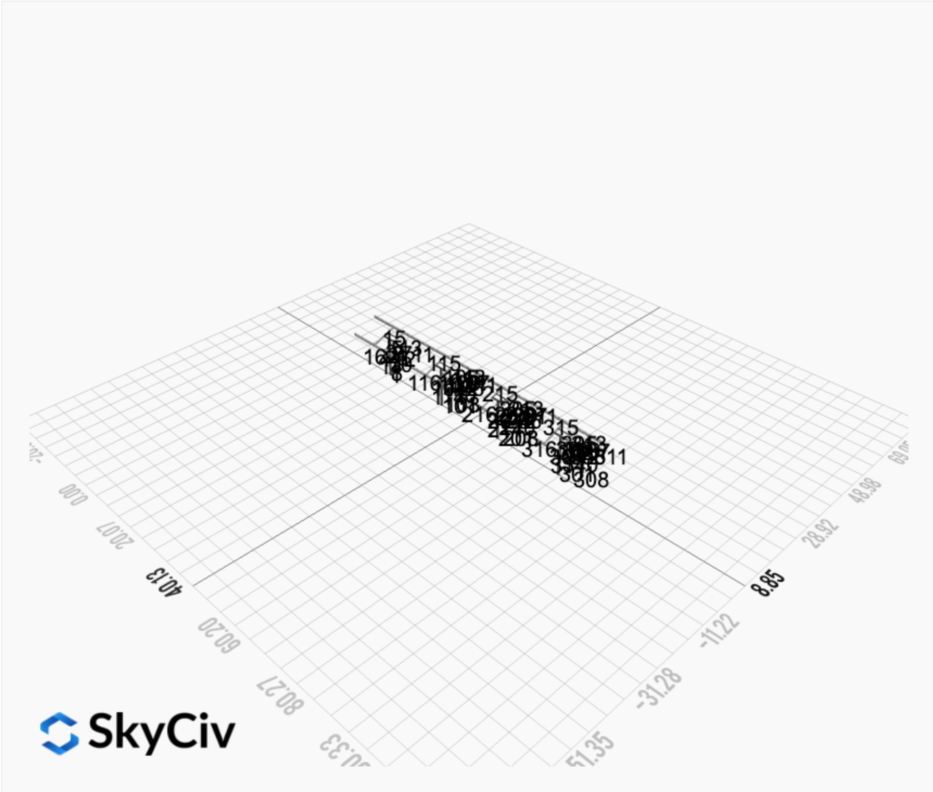
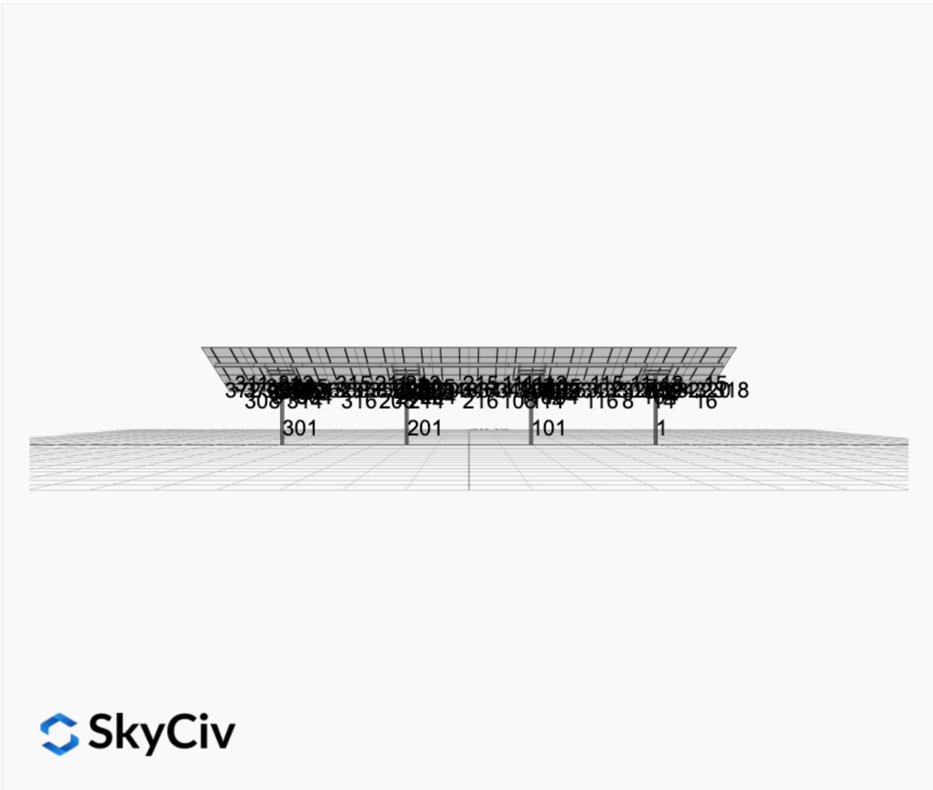
SkyCiv



0.00
20.07
40.13
60.20
80.27
8.85
28.92
48.98
-11.22
-31.28
308
SkyCiv



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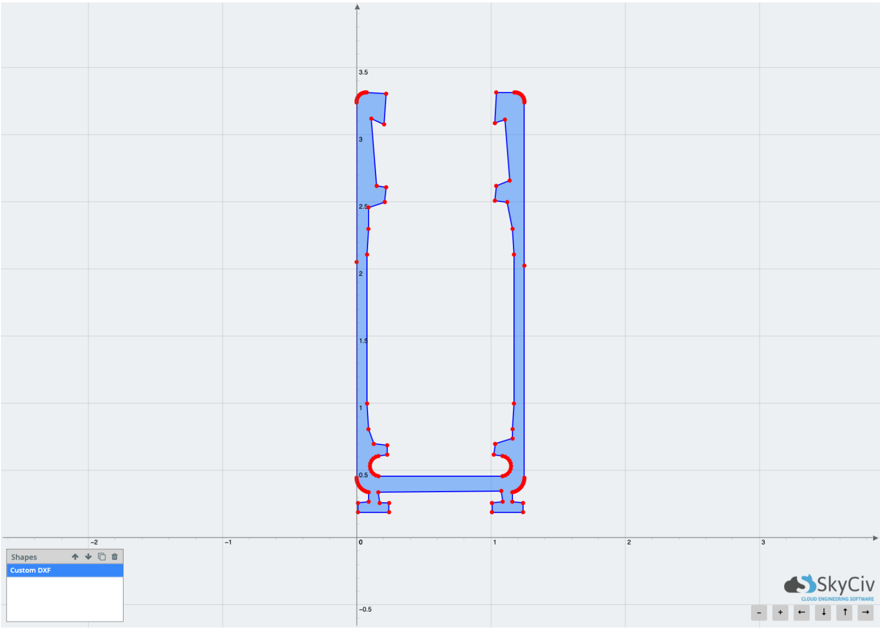
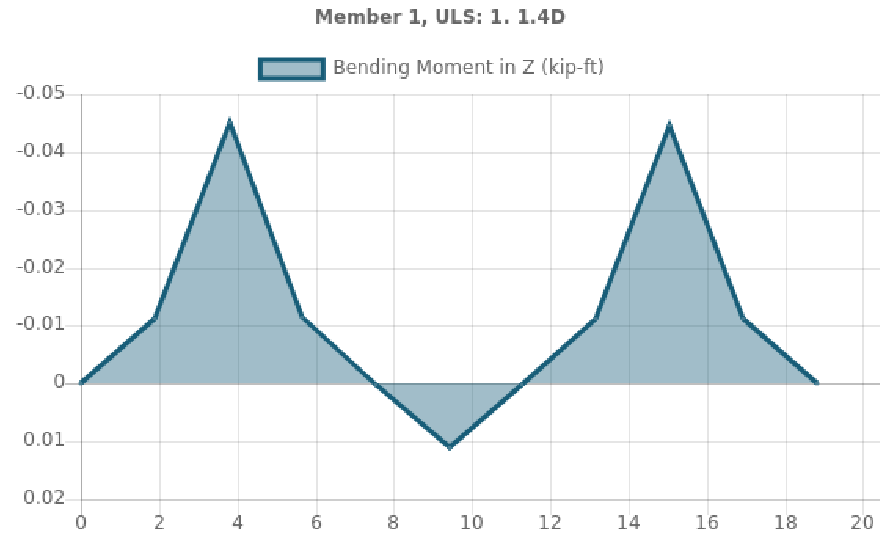
SkyCiv

Rail Design Check

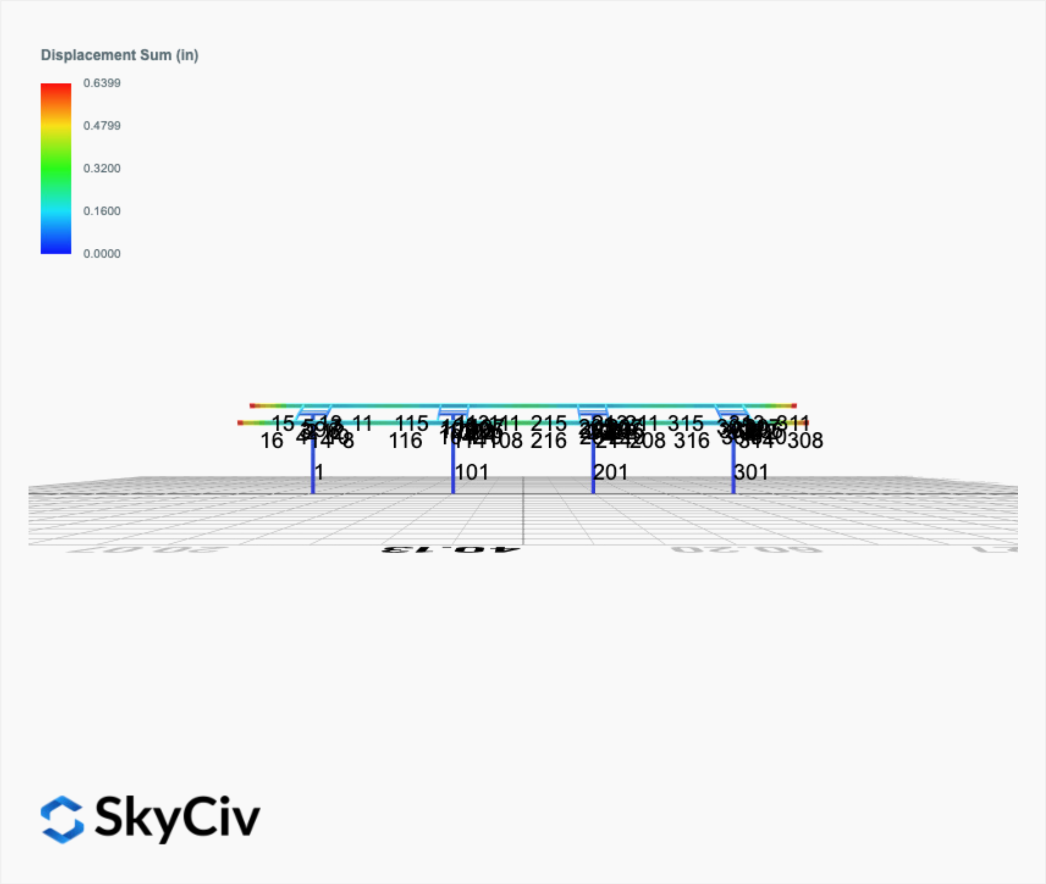
Rail Length: 18.833333333333332 ft
Additional Restraints Required: 4ft Spread Clamps
Tributary Width: 2.8666666666666667 ft
Material: Aluminium
Density: 169 lb/ft^3
Elasticity Modulus: 10000 ksi
Fy: 34.5 ksi
Fu: 37 ksi
Snow (X): 0.0296 kip/ft
Snow (Y): -0.0108 kip/ft
Wind uplift Case A: 0.0381 kip/ft
Wind uplift Case A: 0.0381 kip/ft
Wind uplift Case B (X): 0.0000 kip/ft
Wind uplift Case B (Y): 0.0567 kip/ft

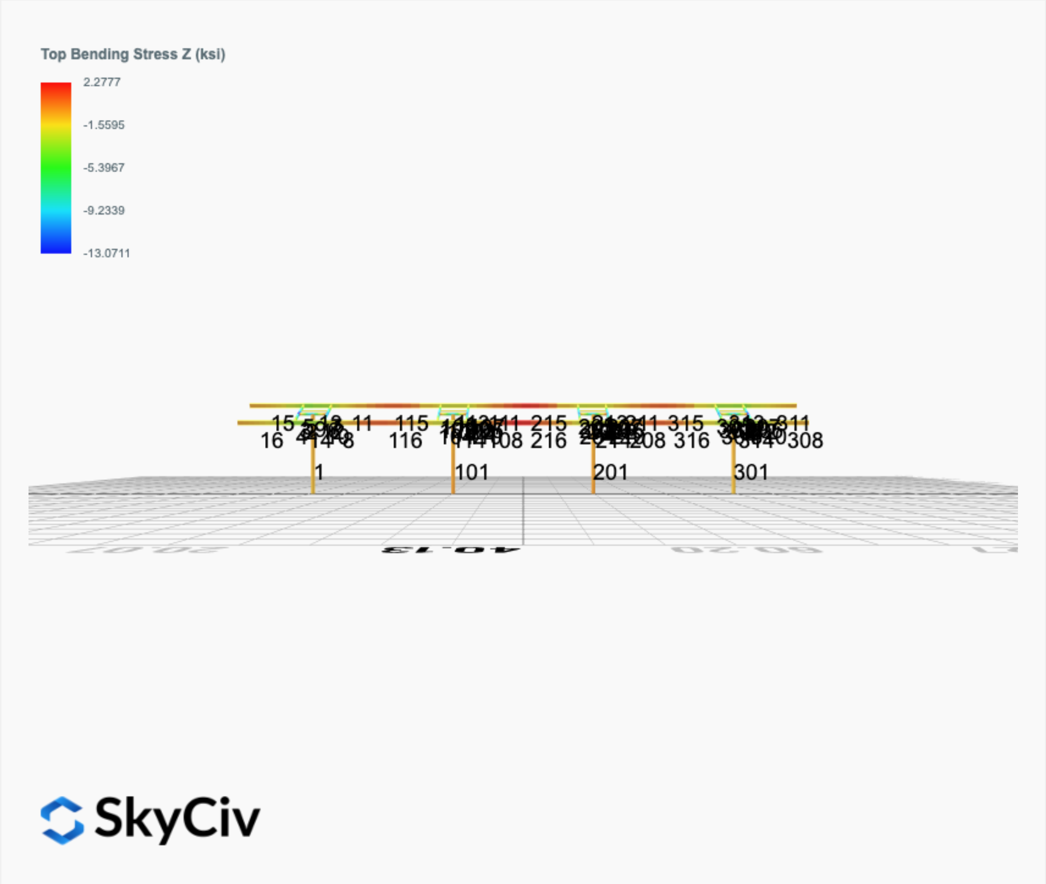


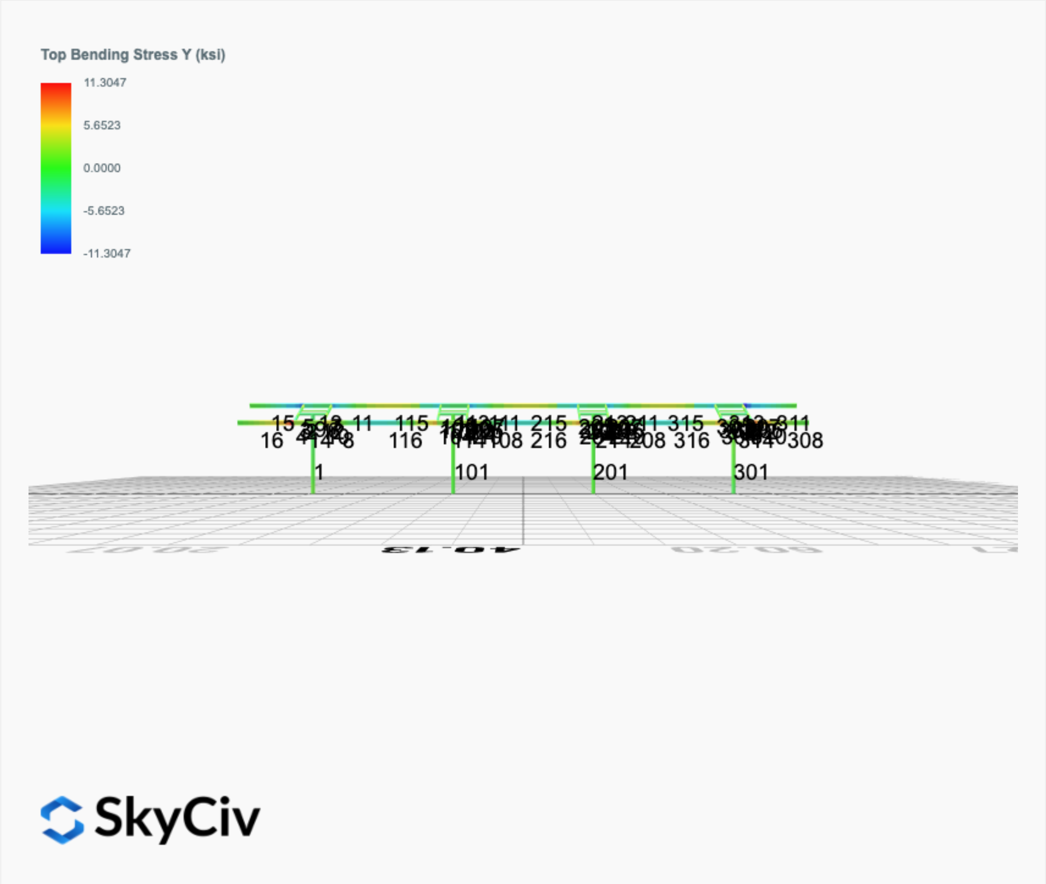
Result Check	Max Limit	Max Value	Utility	Status
Custom Stress Limit	34.5	8.55313082	0.248	PASS
Material Yield	34.5	8.55313082	0.248	PASS
Material Strength	37	8.55313082	0.231	PASS

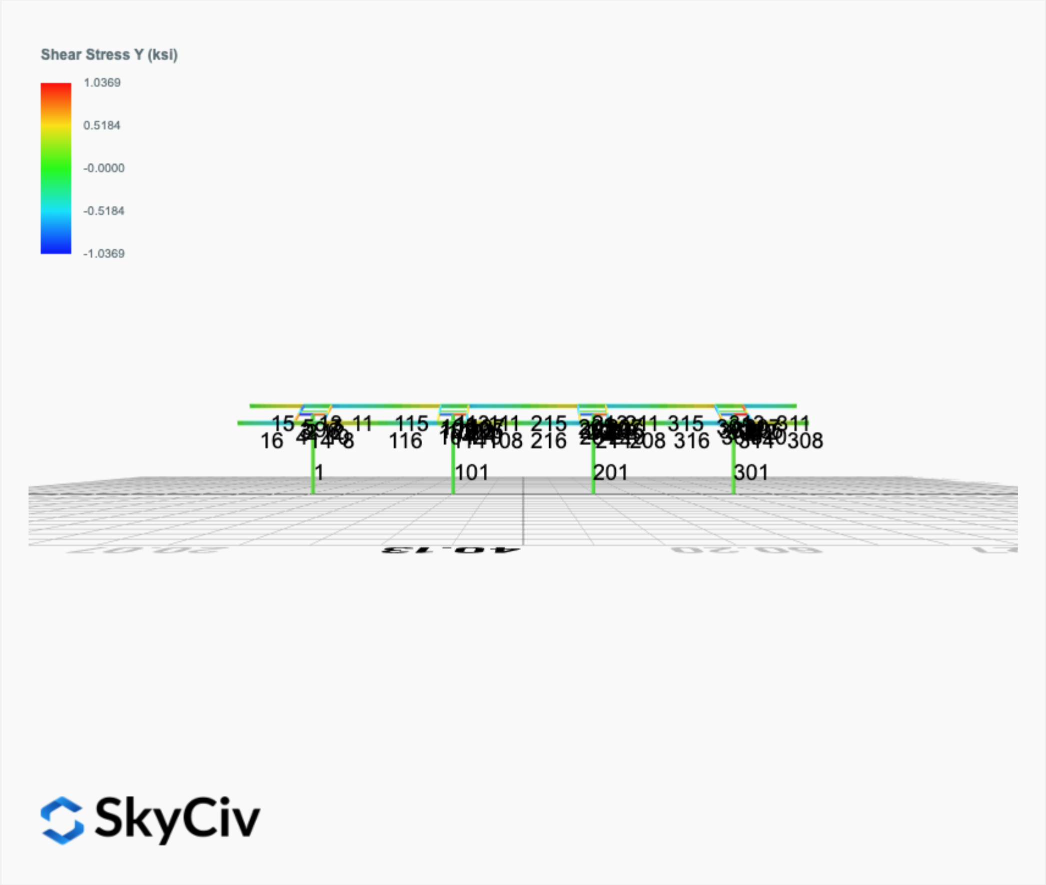


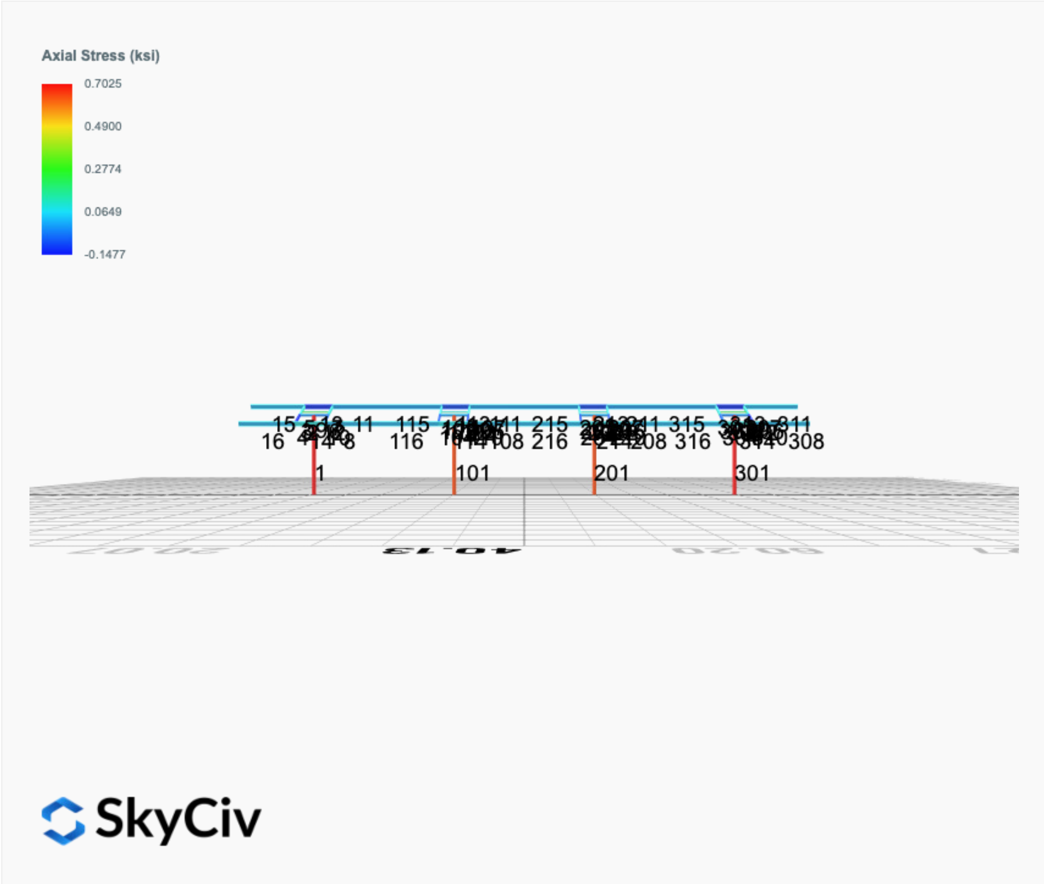
FEM Results (Envelope Worst Case for each member)






Top Bending Stress Y (ksi)
11.3047
5.6523
0.0000
-5.6523
-11.3047
SkyCiv


Shear Stress Y (ksi)
1.0369
0.5184
-0.0000
-0.5184
-1.0369
SkyCiv


Axial Stress (ksi)
0.7025
0.4900
0.2774
0.0649
-0.1477
1
101
201
301
SkyCiv

Reaction Forces for Foundation 1 (Node ID#1), (kip, kip-ft)

ASD Load Combination Results

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 1. D	-0.0158	2.5930	-0.0544	-0.1920	0.0434	0.1873
ULS: 2. D + L	-0.0158	2.5930	-0.0544	-0.1920	0.0434	0.1873
ULS: 3. D + (S or Lr or R)	-0.0443	6.5141	-0.1528	-0.5395	0.1219	0.4870
ULS: 3. D + (S or Lr or R)	-0.0158	2.5930	-0.0544	-0.1920	0.0434	0.1873
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	-0.0372	5.5338	-0.1282	-0.4526	0.1023	0.4121
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	-0.0158	2.5930	-0.0544	-0.1920	0.0434	0.1873
ULS: 5b. D + 0.7E	-0.0158	2.5930	-0.0544	-0.1920	0.0434	0.1873
ULS: 6b. D + 0.75L + 0.75(0.7)E + 0.75S	-0.0372	5.5338	-0.1282	-0.4526	0.1023	0.4121
ULS: 8. 0.6D + 0.7E	-0.0095	1.5558	-0.0327	-0.1152	0.0261	0.1124
ULS: 5a. D + 0.6W_Wind downforce Case A only	-1.1718	5.7905	-0.1429	-0.5035	0.1336	14.3351
ULS: 5a. D + 0.6W_Wind downforce Case B only	-1.1718	5.7905	-0.1429	-0.5035	0.1336	14.3351
ULS: 5a. D + 0.6W_Wind uplift Case A only	0.9668	-0.1206	0.0191	0.0653	-0.0310	-9.7292
ULS: 5a. D + 0.6W_Wind uplift Case B only	0.8325	0.2369	0.0137	0.0463	-0.0275	-16.5072
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-0.9041	7.9320	-0.1946	-0.6863	0.1699	11.0230
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-0.9041	7.9320	-0.1946	-0.6863	0.1699	11.0230
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	0.6998	3.4986	-0.0731	-0.2596	0.0465	-7.0253
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	0.5991	3.7668	-0.0771	-0.2739	0.0491	-12.1088
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-0.8828	4.9911	-0.1208	-0.4257	0.1111	10.7982
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-0.8828	4.9911	-0.1208	-0.4257	0.1111	10.7982
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	0.7211	0.5578	0.0007	0.0010	-0.0124	-7.2501
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	0.6204	0.8259	-0.0034	-0.0133	-0.0098	-12.3336
ULS: 7. 0.6D + 0.6W_Wind downforce Case A only	-1.1654	4.7533	-0.1211	-0.4268	0.1162	14.2602
ULS: 7. 0.6D + 0.6W_Wind downforce Case B only	-1.1654	4.7533	-0.1211	-0.4268	0.1162	14.2602
ULS: 7. 0.6D + 0.6W_Wind uplift Case A only	0.9731	-1.1578	0.0409	0.1421	-0.0484	-9.8041
ULS: 7. 0.6D + 0.6W_Wind uplift Case B only	0.8388	-0.8003	0.0354	0.1231	-0.0449	-16.5821

Worst Case Reactions LRFD

These calculations are taken directly from the FEA via SkyCiv and are used in the Concrete Checks of the Foundatino Module.
Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	12.0497
Shear X	-1.9582
Shear Z	-0.2984
Moment X	-1.0584
Moment Y (Twist)	0.2547
Moment Z	28.7345

Worst Case Reactions ASD

These results are taken from the worst case values in the above table and are used in the Soil Checks in the Foundation Module.
Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	7.9320
Shear X	-1.1718
Shear Z	-0.1946
Moment X	-0.6863
Moment Y (Twist)	0.1699
Moment Z	16.5821

Reaction Forces for Foundation 2 (Node ID#101), (kip, kip-ft)

ASD Load Combination Results

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 1. D	0.0158	2.4934	0.0107	0.0380	-0.0090	-0.1278
ULS: 2. D + L	0.0158	2.4934	0.0107	0.0380	-0.0090	-0.1278
ULS: 3. D + (S or Lr or R)	0.0443	6.2349	0.0299	0.1067	-0.0253	-0.4019
ULS: 3. D + (S or Lr or R)	0.0158	2.4934	0.0107	0.0380	-0.0090	-0.1278
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	0.0372	5.2996	0.0251	0.0895	-0.0212	-0.3334

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	0.0158	2.4934	0.0107	0.0380	-0.0090	-0.1278
ULS: 5b. D + 0.7E	0.0158	2.4934	0.0107	0.0380	-0.0090	-0.1278
ULS: 6b. D + 0.75L + 0.75(0.7)E + 0.75S	0.0372	5.2996	0.0251	0.0895	-0.0212	-0.3334
ULS: 8. 0.6D + 0.7E	0.0095	1.4960	0.0064	0.0228	-0.0054	-0.0767
ULS: 5a. D + 0.6W_Wind downforce Case A only	-1.0944	5.5221	0.0285	0.1016	-0.0229	13.5079
ULS: 5a. D + 0.6W_Wind downforce Case B only	-1.0944	5.5221	0.0285	0.1016	-0.0229	13.5079
ULS: 5a. D + 0.6W_Wind uplift Case A only	0.9560	-0.0759	-0.0051	-0.0178	0.0040	-9.6743
ULS: 5a. D + 0.6W_Wind uplift Case B only	0.8385	0.2584	-0.0011	-0.0038	-0.0016	-16.3505
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-0.7955	7.5711	0.0384	0.1372	-0.0316	9.8934
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-0.7955	7.5711	0.0384	0.1372	-0.0316	9.8934
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	0.7423	3.3726	0.0132	0.0477	-0.0114	-7.4932
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	0.6542	3.6233	0.0163	0.0581	-0.0156	-12.5004
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-0.8168	4.7649	0.0240	0.0857	-0.0194	10.0990
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-0.8168	4.7649	0.0240	0.0857	-0.0194	10.0990
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	0.7210	0.5664	-0.0012	-0.0038	0.0008	-7.2877
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	0.6329	0.8171	0.0018	0.0066	-0.0034	-12.2948
ULS: 7. 0.6D + 0.6W_Wind downforce Case A only	-1.1007	4.5247	0.0242	0.0864	-0.0193	13.5590
ULS: 7. 0.6D + 0.6W_Wind downforce Case B only	-1.1007	4.5247	0.0242	0.0864	-0.0193	13.5590
ULS: 7. 0.6D + 0.6W_Wind uplift Case A only	0.9497	-1.0733	-0.0094	-0.0330	0.0076	-9.6232
ULS: 7. 0.6D + 0.6W_Wind uplift Case B only	0.8322	-0.7389	-0.0054	-0.0190	0.0020	-16.2994

Worst Case Reactions LRFD

These calculations are taken directly from the FEA via SkyCiv and are used in the Concrete Checks of the Foundatino Module.
Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	11.5028
Shear X	-1.8504
Shear Z	0.0579
Moment X	0.2079
Moment Y (Twist)	0.0471
Moment Z	28.5602

Worst Case Reactions ASD

These results are taken from the worst case values in the above table and are used in the Soil Checks in the Foundation Module.
Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	7.5711
Shear X	-1.1007
Shear Z	0.0384
Moment X	0.1372
Moment Y (Twist)	0.0316
Moment Z	16.3505

Reaction Forces for Foundation 3 (Node ID#201), (kip, kip-ft)

ASD Load Combination Results

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 1. D	0.0158	2.4934	-0.0107	-0.0380	0.0090	-0.1278
ULS: 2. D + L	0.0158	2.4934	-0.0107	-0.0380	0.0090	-0.1278
ULS: 3. D + (S or Lr or R)	0.0443	6.2349	-0.0299	-0.1067	0.0252	-0.4019
ULS: 3. D + (S or Lr or R)	0.0158	2.4934	-0.0107	-0.0380	0.0090	-0.1278
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	0.0372	5.2996	-0.0251	-0.0895	0.0212	-0.3334
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	0.0158	2.4934	-0.0107	-0.0380	0.0090	-0.1278
ULS: 5b. D + 0.7E	0.0158	2.4934	-0.0107	-0.0380	0.0090	-0.1278
ULS: 6b. D + 0.75L + 0.75(0.7)E + 0.75S	0.0372	5.2996	-0.0251	-0.0895	0.0212	-0.3334
ULS: 8. 0.6D + 0.7E	0.0095	1.4960	-0.0064	-0.0228	0.0054	-0.0767
ULS: 5a. D + 0.6W_Wind downforce Case A only	-1.0944	5.5221	-0.0285	-0.1016	0.0229	13.5079
ULS: 5a. D + 0.6W_Wind downforce Case B only	-1.0944	5.5221	-0.0285	-0.1016	0.0229	13.5079
ULS: 5a. D + 0.6W_Wind uplift Case A only	0.9560	-0.0759	0.0051	0.0178	-0.0040	-9.6743
ULS: 5a. D + 0.6W_Wind uplift Case B only	0.8385	0.2584	0.0011	0.0038	0.0016	-16.3505

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-0.7955	7.5711	-0.0384	-0.1372	0.0316	9.8934
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-0.7955	7.5711	-0.0384	-0.1372	0.0316	9.8934
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	0.7423	3.3726	-0.0132	-0.0476	0.0114	-7.4932
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	0.6542	3.6233	-0.0163	-0.0581	0.0156	-12.5004
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-0.8168	4.7649	-0.0240	-0.0857	0.0194	10.0990
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-0.8168	4.7649	-0.0240	-0.0857	0.0194	10.0990
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	0.7210	0.5664	0.0012	0.0038	-0.0008	-7.2877
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	0.6329	0.8171	-0.0018	-0.0066	0.0034	-12.2948
ULS: 7. 0.6D + 0.6W_Wind downforce Case A only	-1.1007	4.5247	-0.0242	-0.0864	0.0193	13.5590
ULS: 7. 0.6D + 0.6W_Wind downforce Case B only	-1.1007	4.5247	-0.0242	-0.0864	0.0193	13.5590
ULS: 7. 0.6D + 0.6W_Wind uplift Case A only	0.9497	-1.0733	0.0094	0.0330	-0.0076	-9.6232
ULS: 7. 0.6D + 0.6W_Wind uplift Case B only	0.8322	-0.7389	0.0054	0.0190	-0.0020	-16.2994

Worst Case Reactions LRFD

These calculations are taken directly from the FEA via SkyCiv and are used in the Concrete Checks of the Foundatino Module.
Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	11.5028
Shear X	-1.8504
Shear Z	-0.0579
Moment X	-0.2078
Moment Y (Twist)	0.0470
Moment Z	28.5604

Worst Case Reactions ASD

These results are taken from the worst case values in the above table and are used in the Soil Checks in the Foundation Module.
Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	7.5711
Shear X	-1.1007
Shear Z	-0.0384
Moment X	-0.1372
Moment Y (Twist)	0.0316
Moment Z	16.3505

Reaction Forces for Foundation 4 (Node ID#301), (kip, kip-ft)

ASD Load Combination Results

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 1. D	-0.0158	2.5930	0.0544	0.1920	-0.0434	0.1873
ULS: 2. D + L	-0.0158	2.5930	0.0544	0.1920	-0.0434	0.1873
ULS: 3. D + (S or Lr or R)	-0.0443	6.5141	0.1528	0.5395	-0.1219	0.4870
ULS: 3. D + (S or Lr or R)	-0.0158	2.5930	0.0544	0.1920	-0.0434	0.1873
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	-0.0372	5.5338	0.1282	0.4526	-0.1023	0.4121
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	-0.0158	2.5930	0.0544	0.1920	-0.0434	0.1873
ULS: 5b. D + 0.7E	-0.0158	2.5930	0.0544	0.1920	-0.0434	0.1873
ULS: 6b. D + 0.75L + 0.75(0.7)E + 0.75S	-0.0372	5.5338	0.1282	0.4526	-0.1023	0.4121
ULS: 8. 0.6D + 0.7E	-0.0095	1.5558	0.0327	0.1152	-0.0261	0.1124
ULS: 5a. D + 0.6W_Wind downforce Case A only	-1.1718	5.7905	0.1429	0.5036	-0.1336	14.3351
ULS: 5a. D + 0.6W_Wind downforce Case B only	-1.1718	5.7905	0.1429	0.5036	-0.1336	14.3351
ULS: 5a. D + 0.6W_Wind uplift Case A only	0.9668	-0.1206	-0.0191	-0.0653	0.0310	-9.7292
ULS: 5a. D + 0.6W_Wind uplift Case B only	0.8325	0.2369	-0.0137	-0.0463	0.0275	-16.5072
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-0.9041	7.9320	0.1946	0.6863	-0.1699	11.0230
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-0.9041	7.9320	0.1946	0.6863	-0.1699	11.0230
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	0.6998	3.4986	0.0731	0.2596	-0.0465	-7.0253
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	0.5991	3.7668	0.0771	0.2740	-0.0491	-12.1088
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-0.8828	4.9911	0.1208	0.4257	-0.1111	10.7981
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-0.8828	4.9911	0.1208	0.4257	-0.1111	10.7981
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	0.7211	0.5578	-0.0007	-0.0010	0.0124	-7.2501
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	0.6204	0.8259	0.0034	0.0133	0.0098	-12.3336

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 7. 0.6D + 0.6W_Wind downforce Case A only	-1.1654	4.7533	0.1211	0.4268	-0.1162	14.2602
ULS: 7. 0.6D + 0.6W_Wind downforce Case B only	-1.1654	4.7533	0.1211	0.4268	-0.1162	14.2602
ULS: 7. 0.6D + 0.6W_Wind uplift Case A only	0.9731	-1.1578	-0.0409	-0.1421	0.0484	-9.8041
ULS: 7. 0.6D + 0.6W_Wind uplift Case B only	0.8388	-0.8003	-0.0354	-0.1231	0.0449	-16.5821

Worst Case Reactions LRFD

These calculations are taken directly from the FEA via SkyCiv and are used in the Concrete Checks of the Foundatino Module.
Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	12.0497
Shear X	-1.9582
Shear Z	0.2984
Moment X	1.0586
Moment Y (Twist)	0.2549
Moment Z	28.7353

Worst Case Reactions ASD

These results are taken from the worst case values in the above table and are used in the Soil Checks in the Foundation Module.
Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	7.9320
Shear X	-1.1718
Shear Z	0.1946
Moment X	0.6863
Moment Y (Twist)	0.1699
Moment Z	16.5821

Project Details

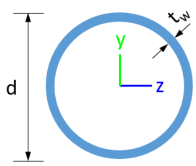
Design Code:	AISC 360-16 LRFD
Provision:	LRFD
Country:	United States
User Name:	sales@mtsolar.us
Project Name:	Simpson Denver - Jefferson Square 5x14 - V1Jb
Unit System:	imperial

Design Input Information

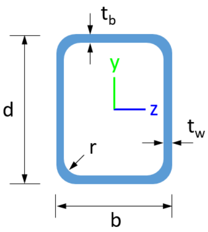
Design Factors			
Φ_t	Φ_c	Φ_b	Φ_v
0.9	0.9	0.9	0.9

Design Materials			
ID	E (ksi)	F_y (ksi)	F_u (ksi)
1	29000	50	65

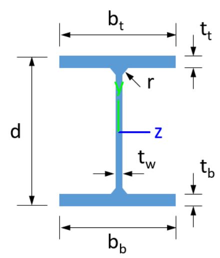
Section Dimensions



ID	Name	d (in)	t_w (in)					
2	2in Pipe Sch 80	2.38	0.22					
5	4in Pipe Sch 80	4.50	0.34					
7	6in Pipe Sch 40	6.63	0.28					



ID	Name	d (in)	b (in)	t_w (in)	t_b (in)	r (in)		
16	HSS5x3x3/16	5.00	3.00	0.17	0.17	0.17		



ID	Name	d (in)	t_w (in)	b_t (in)	b_b (in)	t_t (in)	t_b (in)	r (in)
19	W8x10	7.89	0.17	3.94	3.94	0.20	0.20	0.30

Section Properties								
ID	Name	A (in^2)	J (in^4)	I_{yp} (in^4)	I_{zp} (in^4)	I_w (in^6)	S_{yp} (in^3)	S_{zp} (in^3)

2	2in Pipe Sch 80	1.48	1.74	0.87	0.87	0.00	1.02	1.02
5	4in Pipe Sch 80	4.41	19.22	9.61	9.61	0.00	5.85	5.85
7	6in Pipe Sch 40	5.58	56.28	28.14	28.14	0.00	11.28	11.28
16	HSS5x3x3/16	2.58	8.64	3.85	8.53	0.73	2.96	4.21
19	W8x10	2.96	0.04	2.09	30.80	30.90	1.66	8.87

Member Properties								
Member ID	Section ID	K_zL (ft)	K_yL (ft)	L_b(ft)	C_b	LST	LSC	LD
1	7	23.56	23.56	11.22	-	300	200	1
2	5	1.30	1.30	2.00	-	300	200	1
3	16	0.92	0.92	1.42	1.19,1.18,1.19,1.18,1.19,1.19,1.18,1.18,1.04,1.18,1.18,1.18,1.17,1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.24,1.17,1.18,1.18,1.18,1.18	300	200	1
4	16	2.44	2.44	3.75	1.68,1.67,1.68,1.67,1.68,1.68,1.67,1.67,1.63,1.68,1.67,1.67,1.65,1.69,1.67,1.67,1.67,1.67,1.67,1.67,1.23,1.68,1.67,1.67,1.66,1.69	300	200	1
5	16	1.52	1.52	2.33	1.68,1.67,1.68,1.67,1.67,1.68,1.67,1.67,1.54,1.66,1.67,1.67,1.66,1.66,1.67,1.67,1.67,1.67,1.67,1.67,1.75,1.65,1.67,1.67,1.66,1.66	300	200	1
6	16	0.92	0.92	1.42	1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.01,1.17,1.18,1.18,1.17,1.17,1.18,1.18,1.18,1.18,1.18,1.18,1.24,1.16,1.18,1.18,1.17,1.17	300	200	1
7	16	1.52	1.52	2.33	1.68,1.67,1.68,1.67,1.67,1.68,1.67,1.67,1.52,1.66,1.67,1.67,1.65,1.66,1.67,1.67,1.67,1.67,1.67,1.67,1.76,1.65,1.67,1.67,1.66,1.66	300	200	1
8	19	1.33	1.33	2.05	1.78,1.77,1.78,1.77,1.77,1.78,1.77,1.77,1.78,1.88,1.77,1.77,1.78,2.02,1.77,1.77,1.77,1.80,1.77,1.77,1.78,1.86,1.77,1.77,1.78,2.23	300	200	1
9	2	2.60	2.60	4.00	-	300	200	1
10	16	2.44	2.44	3.75	1.68,1.68,1.68,1.67,1.68,1.68,1.67,1.67,1.62,1.68,1.67,1.67,1.65,1.69,1.67,1.67,1.67,1.67,1.67,1.67,1.22,1.68,1.67,1.67,1.66,1.69	300	200	1
11	19	1.33	1.33	2.05	1.82,1.81,1.82,1.82,1.82,1.82,1.82,1.82,1.56,1.89,1.81,1.81,1.78,1.86,1.82,1.82,1.82,1.79,1.81,1.81,2.03,1.91,1.81,1.81,1.79,1.86	300	200	1
12	5	1.30	1.30	2.00	-	300	200	1
13	19	4.88	4.00	7.50	1.08,1.08,1.08,1.08,1.08,1.08,1.08,1.08,1.08,1.19,1.08,1.08,1.08,1.14,1.08,1.08,1.08,1.09,1.08,1.08,1.12,1.22,1.08,1.08,1.08,1.14	300	200	1
14	19	4.88	4.00	7.50	1.08,1.08,1.08,1.08,1.08,1.08,1.08,1.11,1.16,1.08,1.08,1.08,1.30,1.08,1.08,1.08,1.08,1.08,1.37,1.14,1.08,1.08,1.08,1.49	300	200	1
15	19	12.60	12.60	6.00	2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33	300	200	1
16	19	12.60	12.60	6.00	2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33	300	200	1
101	7	23.56	23.56	11.22	-	300	200	1
102	5	1.30	1.30	2.00	-	300	200	1
103	16	0.92	0.92	1.42	1.19,1.18,1.19,1.18,1.18,1.19,1.18,1.18,1.00,1.17,1.18,1.18,1.17,1.17,1.18,1.18,1.18,1.18,1.18,1.18,1.24,1.16,1.18,1.18,1.17,1.18	300	200	1
104	16	2.44	2.44	3.75	1.68,1.68,1.68,1.67,1.68,1.68,1.67,1.67,1.62,1.68,1.67,1.67,1.65,1.69,1.67,1.67,1.67,1.67,1.67,1.67,1.21,1.68,1.67,1.67,1.66,1.69	300	200	1
105	16	1.52	1.52	2.33	1.68,1.67,1.68,1.67,1.67,1.68,1.67,1.67,1.50,1.66,1.67,1.67,1.65,1.66,1.67,1.67,1.67,1.67,1.67,1.67,1.76,1.65,1.67,1.67,1.66,1.66	300	200	1
106	16	0.92	0.92	1.42	1.19,1.18,1.19,1.18,1.18,1.19,1.18,1.18,1.02,1.17,1.18,1.18,1.17,1.17,1.18,1.18,1.18,1.18,1.18,1.18,1.24,1.16,1.18,1.18,1.17,1.18	300	200	1
107	16	1.52	1.52	2.33	1.68,1.67,1.68,1.67,1.67,1.68,1.67,1.67,1.52,1.66,1.67,1.67,1.66,1.66,1.67,1.67,1.67,1.67,1.67,1.67,1.76,1.65,1.67,1.67,1.66,1.66	300	200	1
108	19	1.33	1.33	2.05	1.92,1.92,1.92,1.93,1.92,1.92,1.92,1.92,2.06,2.07,1.91,1.91,1.98,2.12,1.92,1.92,1.92,2.00,1.92,1.92,2.13,2.07,1.91,1.91,1.97,2.32	300	200	1
109	2	2.60	2.60	4.00	-	300	200	1
110	16	2.44	2.44	3.75	1.68,1.68,1.68,1.67,1.68,1.68,1.67,1.67,1.62,1.68,1.67,1.67,1.65,1.69,1.67,1.67,1.67,1.67,1.67,1.67,1.21,1.68,1.67,1.67,1.66,1.69	300	200	1
111	19	1.33	1.33	2.05	2.06,2.06,2.06,2.06,2.06,2.06,2.06,2.06,1.52,2.09,2.06,2.06,2.03,2.08,2.06,2.06,2.06,2.00,2.06,2.06,2.09,2.10,2.06,2.06,2.05,2.08	300	200	1
112	5	1.30	1.30	2.00	-	300	200	1

113	19	4.88	4.00	7.50	1.05,1.05,1.05,1.05,1.05,1.05,1.05,1.05,1.05,1.11,1.05,1.05,1.05,1.06,1.05,1.05,1.05,1.05,1.05,1.05,1.05,1.15,1.05,1.05,1.05,1.06	300	200	1
114	19	4.88	4.00	7.50	1.05,1.05,1.05,1.05,1.05,1.05,1.05,1.05,1.06,1.08,1.05,1.05,1.06,1.24,1.05,1.05,1.05,1.06,1.05,1.05,1.44,1.06,1.05,1.05,1.06,1.42	300	200	1
115	19	6.63	6.63	10.20	1.17,1.17,1.17,1.17,1.17,1.17,1.17,1.17,1.16,1.17,1.17,1.17,1.17,1.17,1.17,1.17,1.17,1.17,1.17,1.17,1.18,1.18,1.17,1.17,1.17,1.17	300	200	1
116	19	6.63	6.63	10.20	1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.17,1.17,1.16,1.16,1.16,1.18,1.16,1.16,1.16,1.16,1.16,1.16,1.19,1.17,1.16,1.16,1.16,1.21	300	200	1
201	7	23.56	23.56	11.22	-	300	200	1
202	5	1.30	1.30	2.00	-	300	200	1
203	16	0.92	0.92	1.42	1.19,1.18,1.19,1.18,1.18,1.19,1.18,1.18,1.02,1.17,1.18,1.18,1.17,1.17,1.18,1.18,1.18,1.18,1.18,1.18,1.24,1.16,1.18,1.18,1.17,1.18	300	200	1
204	16	2.44	2.44	3.75	1.68,1.68,1.68,1.67,1.68,1.68,1.67,1.67,1.62,1.68,1.67,1.67,1.65,1.69,1.67,1.67,1.67,1.67,1.67,1.67,1.22,1.68,1.67,1.67,1.66,1.69	300	200	1
205	16	1.52	1.52	2.33	1.68,1.67,1.68,1.67,1.67,1.68,1.67,1.67,1.52,1.66,1.67,1.67,1.66,1.66,1.67,1.67,1.67,1.67,1.67,1.67,1.76,1.65,1.67,1.67,1.66,1.66	300	200	1
206	16	0.92	0.92	1.42	1.19,1.18,1.19,1.18,1.18,1.19,1.18,1.18,1.00,1.17,1.18,1.18,1.17,1.17,1.18,1.18,1.18,1.18,1.18,1.18,1.24,1.16,1.18,1.18,1.17,1.18	300	200	1
207	16	1.52	1.52	2.33	1.68,1.67,1.68,1.67,1.67,1.68,1.67,1.67,1.50,1.66,1.67,1.67,1.65,1.66,1.67,1.67,1.67,1.67,1.67,1.67,1.76,1.65,1.67,1.67,1.66,1.66	300	200	1
208	19	1.33	1.33	2.05	2.07,2.07,2.07,2.07,2.07,2.07,2.07,2.07,2.09,2.21,2.07,2.07,2.08,2.34,2.07,2.07,2.07,2.08,2.07,2.07,2.30,2.12,2.07,2.07,2.08,2.27	300	200	1
209	2	2.60	2.60	4.00	-	300	200	1
210	16	2.44	2.44	3.75	1.68,1.68,1.68,1.67,1.68,1.68,1.67,1.67,1.62,1.68,1.67,1.67,1.65,1.69,1.67,1.67,1.67,1.67,1.67,1.67,1.21,1.68,1.67,1.67,1.66,1.69	300	200	1
211	19	1.33	1.33	2.05	2.10,2.10,2.10,2.12,2.10,2.10,2.13,2.13,1.72,2.38,2.12,2.12,2.09,2.36,2.13,2.13,2.13,2.09,2.11,2.11,2.36,2.36,2.12,2.12,2.09,2.33	300	200	1
212	5	1.30	1.30	2.00	-	300	200	1
213	19	4.88	4.00	7.50	1.05,1.05,1.05,1.05,1.05,1.05,1.05,1.05,1.05,1.11,1.05,1.05,1.05,1.06,1.05,1.05,1.05,1.05,1.05,1.05,1.05,1.15,1.05,1.05,1.05,1.06	300	200	1
214	19	4.88	4.00	7.50	1.05,1.05,1.05,1.05,1.05,1.05,1.05,1.05,1.06,1.08,1.05,1.05,1.06,1.23,1.05,1.05,1.05,1.06,1.05,1.05,1.44,1.06,1.05,1.05,1.06,1.42	300	200	1
215	19	6.63	6.63	10.20	1.14,1.14,1.14,1.14,1.14,1.14,1.15,1.15,1.11,1.16,1.15,1.15,1.14,1.15,1.15,1.15,1.15,1.14,1.14,1.14,1.16,1.16,1.15,1.15,1.14,1.15	300	200	1
216	19	6.63	6.63	10.20	1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.15,1.14,1.14,1.14,1.17,1.14,1.14,1.14,1.14,1.14,1.14,1.17,1.15,1.14,1.14,1.14,1.19	300	200	1
301	7	23.56	23.56	11.22	-	300	200	1
302	5	1.30	1.30	2.00	-	300	200	1
303	16	0.92	0.92	1.42	1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.01,1.17,1.18,1.18,1.17,1.17,1.18,1.18,1.18,1.18,1.18,1.18,1.24,1.16,1.18,1.18,1.17,1.17	300	200	1
304	16	2.44	2.44	3.75	1.68,1.68,1.68,1.67,1.68,1.68,1.67,1.67,1.62,1.68,1.67,1.67,1.65,1.69,1.67,1.67,1.67,1.67,1.67,1.67,1.22,1.68,1.67,1.67,1.66,1.69	300	200	1
305	16	1.52	1.52	2.33	1.68,1.67,1.68,1.67,1.67,1.68,1.67,1.67,1.52,1.66,1.67,1.67,1.65,1.66,1.67,1.67,1.67,1.67,1.67,1.67,1.76,1.65,1.67,1.67,1.66,1.66	300	200	1
306	16	0.92	0.92	1.42	1.19,1.18,1.19,1.18,1.19,1.19,1.18,1.18,1.04,1.18,1.18,1.18,1.17,1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.24,1.17,1.18,1.18,1.18,1.18	300	200	1
307	16	1.52	1.52	2.33	1.67,1.67,1.68,1.67,1.67,1.68,1.67,1.67,1.54,1.66,1.67,1.67,1.66,1.66,1.67,1.67,1.67,1.67,1.67,1.67,1.75,1.65,1.67,1.67,1.66,1.66	300	200	1
308	19	12.60	12.60	6.00	2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33	300	200	1
309	2	2.60	2.60	4.00	-	300	200	1
310	16	2.44	2.44	3.75	1.68,1.67,1.68,1.67,1.68,1.68,1.67,1.67,1.63,1.68,1.67,1.67,1.65,1.69,1.67,1.67,1.67,1.67,1.67,1.67,1.23,1.68,1.67,1.67,1.66,1.69	300	200	1
311	19	12.60	12.60	6.00	2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33	300	200	1
312	5	1.30	1.30	2.00	-	300	200	1
313	19	4.88	4.00	7.50	1.08,1.08,1.08,1.08,1.08,1.08,1.08,1.08,1.08,1.19,1.08,1.08,1.08,1.14,1.08,1.08,1.08,1.09,1.08,1.08,1.12,1.22,1.08,1.08,1.08,1.14	300	200	1
314	19	4.88	4.00	7.5	1.08,1.08,1.08,1.08,1.08,1.08,1.08,1.08,1.11,1.16,1.08,1.08,1.08,1.30,1.08,1.08,1.08,1.08,1.08,1.08,1.3	30	20	1

314	19	4.88	4.00	0	7,1.14,1.08,1.08,1.08,1.49	0	0	1
315	19	6.63	6.63	10.20	1.30,1.30,1.30,1.30,1.30,1.30,1.30,1.30,1.80,1.28,1.30,1.30,1.34,1.28,1.30,1.30,1.29,1.33,1.30,1.30,1.25,1.27,1.30,1.30,1.33,1.28	300	200	1
316	19	6.63	6.63	10.20	1.34,1.34,1.34,1.34,1.34,1.34,1.35,1.35,1.34,1.28,1.35,1.35,1.34,1.25,1.35,1.35,1.34,1.31,1.35,1.35,1.39,1.28,1.35,1.35,1.34,1.23	300	200	1

Member Design Capacity

Member ID	$\Phi_t P_n$ (kip)	$\Phi_c P_n$ (kip)	$\Phi_b M_{zn}$ (k-ft)	$\Phi_b M_{yn}$ (k-ft)	$\Phi_v V_{yn}$ (kip)	$\Phi_v V_{zn}$ (kip)
1	251.16	79.52	42.30	42.30	75.35	75.35
2	198.33	196.72	21.95	21.95	59.50	59.50
3	116.10	115.41	15.79	11.10	42.08	23.28
4	116.10	111.33	15.79	11.10	42.08	23.28
5	116.10	114.23	15.79	11.10	42.08	23.28
6	116.10	115.41	15.79	11.10	42.08	23.28
7	116.10	114.23	15.79	11.10	42.08	23.28
8	133.20	123.95	32.87	6.12	40.24	43.62
9	66.48	58.89	3.82	3.82	19.94	19.94
10	116.10	111.33	15.79	11.10	42.08	23.28
11	133.20	123.95	32.87	6.12	40.24	43.62
12	198.33	196.72	21.95	21.95	59.50	59.50
13	133.20	85.85	24.72	6.12	40.24	43.62
14	133.20	85.85	24.75	6.12	40.24	43.62
15	133.20	20.65	32.87	6.12	40.24	43.62
16	133.20	20.65	32.87	6.12	40.24	43.62
101	251.16	79.52	42.30	42.30	75.35	75.35
102	198.33	196.72	21.95	21.95	59.50	59.50
103	116.10	115.41	15.79	11.10	42.08	23.28
104	116.10	111.33	15.79	11.10	42.08	23.28
105	116.10	114.23	15.79	11.10	42.08	23.28
106	116.10	115.41	15.79	11.10	42.08	23.28
107	116.10	114.23	15.79	11.10	42.08	23.28
108	133.20	123.95	32.87	6.12	40.24	43.62
109	66.48	58.89	3.82	3.82	19.94	19.94
110	116.10	111.33	15.79	11.10	42.08	23.28
111	133.20	123.95	32.87	6.12	40.24	43.62
112	198.33	196.72	21.95	21.95	59.50	59.50
113	133.20	85.85	24.02	6.12	40.24	43.62
114	133.20	85.85	24.16	6.12	40.24	43.62
115	133.20	69.16	18.00	6.12	40.24	43.62
116	133.20	69.16	17.98	6.12	40.24	43.62
201	251.16	79.52	42.30	42.30	75.35	75.35
202	198.33	196.72	21.95	21.95	59.50	59.50
203	116.10	115.41	15.79	11.10	42.08	23.28
204	116.10	111.33	15.79	11.10	42.08	23.28
205	116.10	114.23	15.79	11.10	42.08	23.28
206	116.10	115.41	15.79	11.10	42.08	23.28
207	116.10	114.23	15.79	11.10	42.08	23.28
208	133.20	123.95	32.87	6.12	40.24	43.62
209	66.48	58.89	3.82	3.82	19.94	19.94
210	116.10	111.33	15.79	11.10	42.08	23.28
211	133.20	123.95	32.87	6.12	40.24	43.62

212	198.33	196.72	21.95	21.95	59.50	59.50
213	133.20	85.85	23.96	6.12	40.24	43.62
214	133.20	85.85	24.16	6.12	40.24	43.62
215	133.20	69.16	17.22	6.12	40.24	43.62
216	133.20	69.16	17.59	6.12	40.24	43.62
301	251.16	79.52	42.30	42.30	75.35	75.35
302	198.33	196.72	21.95	21.95	59.50	59.50
303	116.10	115.41	15.79	11.10	42.08	23.28
304	116.10	111.33	15.79	11.10	42.08	23.28
305	116.10	114.23	15.79	11.10	42.08	23.28
306	116.10	115.41	15.79	11.10	42.08	23.28
307	116.10	114.23	15.79	11.10	42.08	23.28
308	133.20	20.65	32.87	6.12	40.24	43.62
309	66.48	58.89	3.82	3.82	19.94	19.94
310	116.10	111.33	15.79	11.10	42.08	23.28
311	133.20	20.65	32.87	6.12	40.24	43.62
312	198.33	196.72	21.95	21.95	59.50	59.50
313	133.20	85.85	24.69	6.12	40.24	43.62
314	133.20	85.85	24.75	6.12	40.24	43.62
315	133.20	69.16	19.38	6.12	40.24	43.62
316	133.20	69.16	19.02	6.12	40.24	43.62

Design Ratio

Member ID	P	M_z	M_y	V_y	V_z	(P,M_z,M_y)	Worst LC	KL/r	δ	Status
1	0.152	0.679	0.054	0.026	0.004	0.687	#16	0.630	Not Required	Pass
2	0.003	0.523	0.108	0.107	0.017	0.614	#21	0.035	Not Required	Pass
3	0.007	0.731	0.047	0.074	0.005	0.781	#21	0.045	Not Required	Pass
4	0.008	0.709	0.144	0.071	0.028	0.782	#21	0.080	Not Required	Pass
5	0.007	0.453	0.160	0.073	0.041	0.495	#21	0.074	Not Required	Pass
6	0.006	0.630	0.033	0.062	0.006	0.648	#21	0.045	Not Required	Pass
7	0.006	0.391	0.117	0.063	0.030	0.425	#21	0.074	Not Required	Pass
8	0.001	0.097	0.107	0.046	0.011	0.204	#21	0.095	Not Required	Pass
9	0.014	0.090	0.051	0.002	0.002	0.148	#21	0.204	Not Required	Pass
10	0.006	0.619	0.130	0.062	0.029	0.717	#21	0.080	Not Required	Pass
11	0.002	0.094	0.105	0.046	0.011	0.201	#21	0.063	Not Required	Pass
12	0.004	0.419	0.097	0.091	0.016	0.499	#21	0.035	Not Required	Pass
13	0.007	0.343	0.370	0.058	0.014	0.709	#21	0.190	Not Required	Pass
14	0.008	0.345	0.370	0.058	0.014	0.701	#21	0.190	Not Required	Pass
15	0.000	0.154	0.222	0.042	0.010	0.375	#21	Not Required	Not Required	Pass
16	0.000	0.150	0.222	0.041	0.010	0.371	#21	Not Required	Not Required	Pass
101	0.145	0.675	0.010	0.024	0.001	0.683	#16	0.630	Not Required	Pass
102	0.002	0.430	0.091	0.093	0.015	0.501	#21	0.035	Not Required	Pass
103	0.007	0.642	0.038	0.064	0.006	0.682	#21	0.045	Not Required	Pass
104	0.006	0.615	0.101	0.062	0.022	0.687	#21	0.080	Not Required	Pass
105	0.007	0.398	0.102	0.064	0.026	0.423	#21	0.074	Not Required	Pass
106	0.007	0.658	0.044	0.066	0.008	0.705	#21	0.045	Not Required	Pass
107	0.007	0.408	0.106	0.065	0.027	0.435	#21	0.074	Not Required	Pass
108	0.001	0.066	0.093	0.042	0.011	0.114	#21	0.095	Not Required	Pass
109	0.008	0.064	0.032	0.001	0.000	0.100	#21	0.204	Not Required	Pass
110	0.007	0.637	0.101	0.064	0.021	0.706	#21	0.080	Not Required	Pass

110	0.007	0.637	0.101	0.064	0.021	0.706	#21	0.080	Not Required	Pass
111	0.002	0.061	0.094	0.043	0.011	0.123	#21	0.063	Not Required	Pass
112	0.002	0.450	0.093	0.096	0.015	0.523	#21	0.035	Not Required	Pass
113	0.006	0.205	0.245	0.055	0.014	0.427	#21	0.190	Not Required	Pass
114	0.007	0.194	0.244	0.054	0.014	0.411	#21	0.286	Not Required	Pass
115	0.002	0.169	0.136	0.039	0.010	0.303	#21	0.316	Not Required	Pass
116	0.002	0.170	0.137	0.037	0.010	0.303	#21	0.473	Not Required	Pass
201	0.145	0.675	0.010	0.024	0.001	0.683	#16	0.630	Not Required	Pass
202	0.002	0.450	0.093	0.096	0.015	0.523	#21	0.035	Not Required	Pass
203	0.007	0.658	0.044	0.066	0.008	0.705	#21	0.045	Not Required	Pass
204	0.007	0.637	0.101	0.064	0.021	0.706	#21	0.080	Not Required	Pass
205	0.007	0.408	0.106	0.065	0.027	0.435	#21	0.074	Not Required	Pass
206	0.007	0.642	0.038	0.064	0.006	0.682	#21	0.045	Not Required	Pass
207	0.007	0.398	0.102	0.064	0.026	0.423	#21	0.074	Not Required	Pass
208	0.001	0.044	0.088	0.037	0.010	0.113	#24	0.095	Not Required	Pass
209	0.008	0.064	0.032	0.001	0.000	0.100	#21	0.204	Not Required	Pass
210	0.006	0.615	0.101	0.062	0.022	0.687	#21	0.080	Not Required	Pass
211	0.002	0.041	0.088	0.039	0.010	0.122	#21	0.063	Not Required	Pass
212	0.002	0.430	0.091	0.093	0.015	0.501	#21	0.035	Not Required	Pass
213	0.006	0.205	0.245	0.055	0.014	0.427	#21	0.190	Not Required	Pass
214	0.007	0.194	0.244	0.054	0.014	0.411	#21	0.286	Not Required	Pass
215	0.002	0.244	0.135	0.043	0.011	0.380	#21	0.316	Not Required	Pass
216	0.002	0.253	0.138	0.042	0.011	0.390	#21	0.473	Not Required	Pass
301	0.152	0.679	0.054	0.026	0.004	0.687	#16	0.630	Not Required	Pass
302	0.004	0.419	0.097	0.091	0.016	0.499	#21	0.035	Not Required	Pass
303	0.006	0.630	0.033	0.062	0.006	0.648	#21	0.045	Not Required	Pass
304	0.006	0.619	0.130	0.062	0.029	0.717	#21	0.080	Not Required	Pass
305	0.006	0.391	0.117	0.063	0.030	0.425	#21	0.074	Not Required	Pass
306	0.007	0.731	0.047	0.074	0.005	0.781	#21	0.045	Not Required	Pass
307	0.007	0.453	0.160	0.073	0.041	0.495	#21	0.074	Not Required	Pass
308	0.000	0.150	0.222	0.041	0.010	0.371	#21	Not Required	Not Required	Pass
309	0.014	0.090	0.051	0.002	0.002	0.148	#21	0.204	Not Required	Pass
310	0.008	0.709	0.144	0.071	0.028	0.782	#21	0.080	Not Required	Pass
311	0.000	0.154	0.222	0.042	0.010	0.375	#21	Not Required	Not Required	Pass
312	0.003	0.523	0.108	0.107	0.017	0.614	#21	0.035	Not Required	Pass
313	0.007	0.343	0.370	0.058	0.014	0.709	#21	0.190	Not Required	Pass
314	0.008	0.345	0.370	0.058	0.014	0.701	#21	0.286	Not Required	Pass
315	0.002	0.154	0.137	0.046	0.011	0.291	#21	0.316	Not Required	Pass
316	0.002	0.153	0.138	0.046	0.011	0.284	#21	0.473	Not Required	Pass

Definitions

Φ_t	Safety factor for tensile
Φ_c	Safety factor for compression
Φ_b	Safety factor for flexure
Φ_v	Safety factor for shear
E	Modulus of elasticity
F_y	Specified minimum yield stress
F_u	Specified minimum tensile strength
A	Cross-sectional area
J	Torsional constant
I_{yp}	Moment of inertia about the Y axes
I_{zp}	Moment of inertia about the Z axes
I_w	Warping constant
S_{yp}	Plastic section modulus about the Y axis

S_{zp}	Plastic section modulus about the Z axis
KL	Effective length
C_b	Buckling modification factor (from all load combinations)
L_b	Length between braced points
LST	Limited slenderness for tension
LSC	Limited slenderness for compression
LD	Limited deflection
P_n	Nominal axial strength (tension/comression)
M_n	Nominal flexural strength (about Z/Y axis)
V_n	Nominal shear strength (along Z/Y axis)
P	Design ratio in case of axial force
M_z	Design ratio in case of bending about Z axis
M_y	Design ratio in case of bending about Y axis
V_y	Design ratio in case of shear along Y axis
V_z	Design ratio in case of shear along Z axis
(P,M_z,M_y)	Design ratio in case of axial force and bending action
KL/r	Design ratio in case of section slenderness
δ	Design ratio in case of member deflection
OK	Capacity is provided
NG	Capacity is not provided

REFERENCES	CALCULATIONS	RESULTS

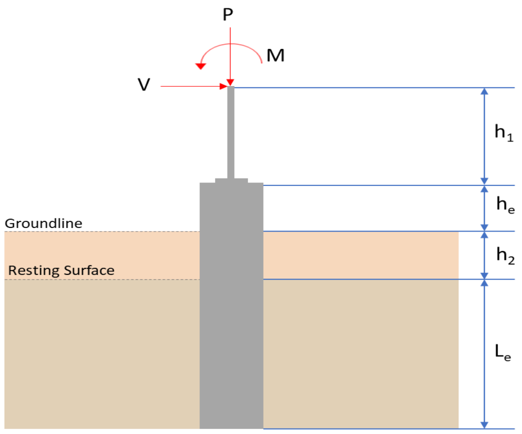
SkyCiv Foundation Design

Pile Foundation

Design Information :

Design code : IBC 2021 (International Building Code)
Unit System : Imperial

Pile Input



Geometry

Pile shape: rectangular

$b = 48$ in - Pile width

$D = 48$ in - Pile depth

$L = 5.75$ ft - Total pile length

$h_1 = 0$ ft - Lateral load height from the top of the pile,

$h_2 = 0$ ft - Depth to resisting surface

$h_e = 0$ ft - Length of pile above the ground

Tabulation of Soil Parameters

Layer	Label	Allowable Bearing Pressure (q_a) (psf)	Allowable Lateral Pressure (R) (psf/ft)
1	Sand, silty sand, clayey sand, silty gravel & clayey gravel	2000.000	150.000

Tabulation of Loads

Load Component	ASD	LRFD
P (kip)	7.932	12.050
V_x (kip)	-1.172	-1.958
V_z (kip)	-0.195	-0.298
M_x (kipft)	-0.686	-1.058
M_z (kipft)	16.582	28.734

Material Properties

$f'_{ck} = 2.5$ ksi - Concrete strength,

Required depth to resist lateral loads (ASD)

H - Point of application of the lateral load

$$H = h_1 + h_2 + h_e$$

$$H = (0 \text{ ft}) + (0 \text{ ft}) + (0 \text{ ft})$$

$$H = 0 \text{ ft}$$

Considering x-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\,D}$$

$$H_o = \frac{(-1.172 \text{ kip})}{1.57 \times (48 \text{ in})}$$

$$H_o = -0.18662 \text{ kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\,H)}{1.57\,D}$$

$$M_o = \frac{(16.582\text{ kipft}) + ((-1.172\text{ kip}) \times (0\text{ ft}))}{1.57 \times (48\text{ in})}$$

$$M_o = 2.6404\text{ kipft/ft}$$

Required depth of embedment in earth:

$$L_x^3 - \left(14.14 \times \frac{H_o \times L_x}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,x} = 5.3314\text{ ft}$ - Required depth in x-direction,

Considering z-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\,b}$$

$$H_o = \frac{(-0.195\text{ kip})}{1.57 \times (48\text{ in})}$$

$$H_o = -0.031051\text{ kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\,H)}{1.57\,b}$$

$$M_o = \frac{(0.686\text{ kipft}) + ((-0.195\text{ kip}) \times (0\text{ ft}))}{1.57 \times (48\text{ in})}$$

$$M_o = 0.10924\text{ kipft/ft}$$

Required depth of embedment in earth:

$$L_z^3 - \left(14.14 \times \frac{H_o \times L_z}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,z} = 1.7608\text{ ft}$ - Required depth in z-direction,

Minimum embedded depth required:

$L_{e,req}$ - Depth of pile required,

$$L_{e,req} = MAX[L_{e,x}, L_{e,z}]$$

$$L_{e,req} = MAX[(5.3314\text{ ft}), (1.7608\text{ ft})]$$

$$L_{e,req} = 5.331\text{ ft}$$

L_e - Actual embedded length of pile,

$$L_e = L - h_e - h_2$$

$$L_e = (5.75\text{ ft}) - (0\text{ ft}) - (0\text{ ft})$$

$$L_e = 5.75\text{ ft}$$

$Ratio$ - Embedded depth

$$Ratio = \frac{L_{e,req}}{L_e}$$

$$Ratio = \frac{(5.331\text{ ft})}{(5.75\text{ ft})}$$

$$Ratio = 0.92713$$

Status: **PASS**
Ratio: **0.930**

End-bearing Capacity (ASD)

A - Pile cross-section area

$$A = b\,D$$

$$A = (48\text{ in}) \times (48\text{ in})$$

$$A = 16\text{ ft}^2$$

q - End-bearing pressure

$$q = \frac{P_v}{A}$$

$$q = \frac{(7.932 \text{ kip})}{(16 \text{ ft}^2)}$$

$$q = 0.49575 \text{ kip/ft}^2$$

Check bearing capacity ratio:

$Ratio$ - Capacity

$$Ratio = \frac{q}{q_a}$$

$$Ratio = \frac{\left(0.49575 \text{ kip/ft}^2\right)}{(2000 \text{ psf})}$$

$$Ratio = 0.24787$$

Status: **PASS**
Ratio: **0.250**

Czerniak

Lateral Soil Pressure (ASD):

L/D - Length to least lateral dimension ratio,

$$L/D = \frac{L}{D}$$

$$L/D = \frac{(5.75 \text{ ft})}{(48 \text{ in})}$$

$$L/D = 1.4375$$

Since L/D $\leq$ 10,

Pile is short.

Considering x-direction:

$H_o = -0.18662 \text{ kip/ft}$ - Lateral force per length of pile,
$M_o = 2.6404 \text{ kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\, M_o\, L_e) + \left(3\, H_o\, L_e^2\right)}{(6\, M_o) + (4\, H_o\, L_e)}$$

$$a = \frac{(4 \times (2.6404 \text{ kipft/ft}) \times (5.75 \text{ ft})) + \left(3 \times (-0.18662 \text{ kip/ft}) \times (5.75 \text{ ft})^2\right)}{(6 \times (2.6404 \text{ kipft/ft})) + (4 \times (-0.18662 \text{ kip/ft}) \times (5.75 \text{ ft}))}$$

$$a = 3.9355 \text{ ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\, [(4\, M_o) + (3\, H_o\, L_e)]^2}{L_e^2\, [(3\, M_o) + (2\, H_o\, L_e)]}$$

$$p = \frac{0.75 \times [(4 \times (2.6404 \text{ kipft/ft})) + (3 \times (-0.18662 \text{ kip/ft}) \times (5.75 \text{ ft}))]^{\,2}}{(5.75 \text{ ft})^2 \times [(3 \times (2.6404 \text{ kipft/ft})) + (2 \times (-0.18662 \text{ kip/ft}) \times (5.75 \text{ ft}))]}$$

$$p = 0.21176 \text{ kip/ft}^2$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\, [(2\, M_o) + (H_o\, L_e)]}{L_e^2}$$

$$s = \frac{6 \times [(2 \times (2.6404 \text{ kipft/ft})) + ((-0.18662 \text{ kip/ft}) \times (5.75 \text{ ft}))]}{(5.75 \text{ ft})^2}$$

$$s = 0.76361 \text{ kip/ft}^2$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\, \frac{a}{2}$$

$$p_a = (150 \text{ psf/ft}) \times \frac{(3.9355 \text{ ft})}{2}$$

$$p_a = 0.29516 \text{ kip/ft}^2$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{\left(0.21176 \text{ kip/ft}^2\right)}{\left(0.29516 \text{ kip/ft}^2\right)}$$

$$Ratio = 0.71745$$

Status: **PASS**
Ratio: **0.720**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\, L_e$$

$$p_s = (150 \text{ psf/ft}) \times (5.75 \text{ ft})$$

$$p_s = 0.8625 \text{ kip/ft}^2$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{\left(0.76361 \text{ kip/ft}^2\right)}{\left(0.8625 \text{ kip/ft}^2\right)}$$

$$Ratio = 0.88534$$

Status: **PASS**
Ratio: **0.890**

Considering z-direction:

$H_o = -0.031051 \text{ kip/ft}$ - Lateral force per length of pile,
$M_o = 0.10924 \text{ kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\, M_o\, L_e) + \left(3\, H_o\, L_e^2\right)}{(6\, M_o) + (4\, H_o\, L_e)}$$

$$a = \frac{(4 \times (0.10924 \text{ kipft/ft}) \times (5.75 \text{ ft})) + \left(3 \times (-0.031051 \text{ kip/ft}) \times (5.75 \text{ ft})^2\right)}{(6 \times (0.10924 \text{ kipft/ft})) + (4 \times (-0.031051 \text{ kip/ft}) \times (5.75 \text{ ft}))}$$

$$a = 4.0832 \text{ ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\, [(4\, M_o) + (3\, H_o\, L_e)]^2}{L_e^2\, [(3\, M_o) + (2\, H_o\, L_e)]}$$

$$p = \frac{0.75 \times [(4 \times (0.10924 \text{ kipft/ft})) + (3 \times (-0.031051 \text{ kip/ft}) \times (5.75 \text{ ft}))]^2}{(5.75 \text{ ft})^2 \times [(3 \times (0.10924 \text{ kipft/ft})) + (2 \times (-0.031051 \text{ kip/ft}) \times (5.75 \text{ ft}))]}$$

$$p = -0.0075197 \text{ kip/ft}^2$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\, [(2\, M_o) + (H_o\, L_e)]}{L_e^2}$$

$$s = \frac{6 \times [(2 \times (0.10924 \text{ kipft/ft})) + ((-0.031051 \text{ kip/ft}) \times (5.75 \text{ ft}))]}{(5.75 \text{ ft})^2}$$

$$s = 0.007246 \text{ kip/ft}^2$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\, \frac{a}{2}$$

$$p_a = (150 \text{ psf/ft}) \times \frac{(4.0832 \text{ ft})}{2}$$

$$p_a = 0.30624 \text{ kip/ft}^2$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{\left(-0.0075197 \text{ kip/ft}^2\right)}{\left(0.30624 \text{ kip/ft}^2\right)}$$

$$Ratio = -0.024555$$

Status: **PASS**
Ratio: **-0.020**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\,L_e$$

$$p_s = (150\ \mathrm{psf/ft}) \times (5.75\ \mathrm{ft})$$

$$p_s = 0.8625\ \mathrm{kip/ft^2}$$

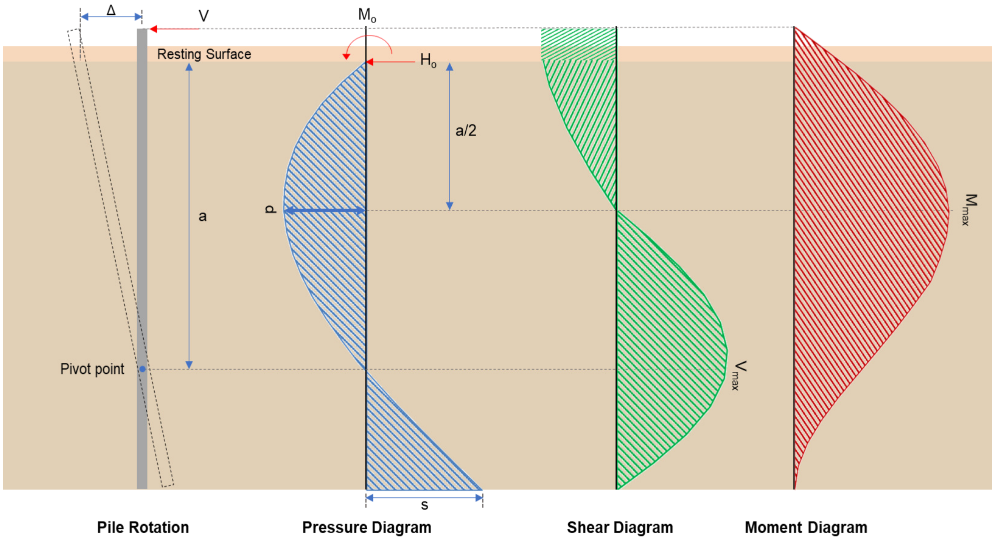
$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{\left(0.007246\ \mathrm{kip/ft^2}\right)}{\left(0.8625\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = 0.0084011$$

Status: **PASS**
Ratio: **0.010**



Shear force and Bending moment (x-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\,D}$$

$$H_o = \frac{(-1.958\ \mathrm{kip})}{1.57 \times (48\ \mathrm{in})}$$

$$H_o = -0.31178\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\,H)}{1.57\,D}$$

$$M_o = \frac{(28.734\ \mathrm{kipft}) + ((-1.958\ \mathrm{kip}) \times (0\ \mathrm{ft}))}{1.57 \times (48\ \mathrm{in})}$$

$$M_o = 4.5755\ \mathrm{kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(4.5755\ \mathrm{kipft/ft})}{(-0.31178\ \mathrm{kip/ft})}$$

$$E = 14.675\ \mathrm{ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (4.5755\ \mathrm{kipft/ft}) \times (5.75\ \mathrm{ft})) + \left(3 \times (-0.31178\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft})^2\right)}{\ldots}$$

$$a = \frac{}{(6 \times (4.5755\ \text{kipft/ft})) + (4 \times (-0.31178\ \text{kip/ft}) \times (5.75\ \text{ft}))}$$

$$a = 3.9326\ \text{ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\, D) \left[1 - \left[3 \left(\frac{4\, E}{L_e} + 3\right) \left(\frac{a}{L_e}\right)^2\right] + \left[4 \left(\frac{3\, E}{L_e} + 2\right) \left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((-0.31178\ \text{kip/ft}) \times (48\ \text{in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (14.675\ \text{ft})}{(5.75\ \text{ft})} + 3\right) \times \left(\frac{(3.9326\ \text{ft})}{(5.75\ \text{ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (14.675\ \text{ft})}{(5.75\ \text{ft})} + 2\right) \times \left(\frac{(3.9326\ \text{ft})}{(5.75\ \text{ft})}\right)^3\right]\right]$$

$$V_{max} = 6.4583\ \text{kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, D\, L_e) \left[\left(\frac{E}{L_e} + \frac{a}{2\, L_e}\right) - \left[\left(\frac{4\, E}{L_e} + 3\right) \left(\frac{a}{2\, L_e}\right)^3\right] + \left[\left(\frac{3\, E}{L_e} + 2\right) \left(\frac{a}{2\, L_e}\right)^4\right]\right]$$

$$M_{max} = ((-0.31178\ \text{kip/ft}) \times (48\ \text{in}) \times (5.75\ \text{ft})) \times \left[\left(\frac{(14.675\ \text{ft})}{(5.75\ \text{ft})} + \frac{(3.9326\ \text{ft})}{2 \times (5.75\ \text{ft})}\right) - \left[\left(\frac{4 \times (14.675\ \text{ft})}{(5.75\ \text{ft})} + 3\right) \times \left(\frac{(3.9326\ \text{ft})}{2 \times (5.75\ \text{ft})}\right)^3\right] + \left[\left(\frac{3 \times (14.675\ \text{ft})}{(5.75\ \text{ft})} + 2\right) \times \left(\frac{(3.9326\ \text{ft})}{2 \times (5.75\ \text{ft})}\right)^4\right]\right]$$

$$M_{max} = 17.913\ \text{kipft}$$

Shear force and Bending moment (z-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\, b}$$

$$H_o = \frac{(-0.298\ \text{kip})}{1.57 \times (48\ \text{in})}$$

$$H_o = -0.047452\ \text{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\, H)}{1.57\, b}$$

$$M_o = \frac{(1.058\ \text{kipft}) + ((-0.298\ \text{kip}) \times (0\ \text{ft}))}{1.57 \times (48\ \text{in})}$$

$$M_o = 0.16847\ \text{kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(0.16847\ \text{kipft/ft})}{(-0.047452\ \text{kip/ft})}$$

$$E = 3.5503\ \text{ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\, M_o\, L_e) + \left(3\, H_o\, L_e^2\right)}{(6\, M_o) + (4\, H_o\, L_e)}$$

$$a = \frac{(4 \times (0.16847\ \text{kipft/ft}) \times (5.75\ \text{ft})) + \left(3 \times (-0.047452\ \text{kip/ft}) \times (5.75\ \text{ft})^2\right)}{(6 \times (0.16847\ \text{kipft/ft})) + (4 \times (-0.047452\ \text{kip/ft}) \times (5.75\ \text{ft}))}$$

$$a = 4.0821\ \text{ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\, b) \left[1 - \left[3 \left(\frac{4\, E}{L_e} + 3\right) \left(\frac{a}{L_e}\right)^2\right] + \left[4 \left(\frac{3\, E}{L_e} + 2\right) \left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((-0.047452\ \mathrm{kip/ft}) \times (48\ \mathrm{in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (3.5503\ \mathrm{ft})}{(5.75\ \mathrm{ft})} + 3\right) \times \left(\frac{(4.0821\ \mathrm{ft})}{(5.75\ \mathrm{ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (3.5503\ \mathrm{ft})}{(5.75\ \mathrm{ft})} + 2\right) \times \left(\frac{(4.0821\ \mathrm{ft})}{(5.75\ \mathrm{ft})}\right)^3\right]\right]$$

$$V_{max} = 0.33345\ \mathrm{kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, b\, L_e) \left[\left(\frac{E}{L_e} + \frac{a}{2\, L_e}\right) - \left[\left(\frac{4\, E}{L_e} + 3\right) \left(\frac{a}{2\, L_e}\right)^3\right] + \left[\left(\frac{3\, E}{L_e} + 2\right) \left(\frac{a}{2\, L_e}\right)^4\right]\right]$$

$$M_{max} = ((-0.047452\ \mathrm{kip/ft}) \times (48\ \mathrm{in}) \times (5.75\ \mathrm{ft})) \times \left[\left(\frac{(3.5503\ \mathrm{ft})}{(5.75\ \mathrm{ft})} + \frac{(4.0821\ \mathrm{ft})}{2 \times (5.75\ \mathrm{ft})}\right) - \left[\left(\frac{4 \times (3.5503\ \mathrm{ft})}{(5.75\ \mathrm{ft})} + 3\right) \times \left(\frac{(4.0821\ \mathrm{ft})}{2 \times (5.75\ \mathrm{ft})}\right)^3\right] + \left[\left(\frac{3 \times (3.5503\ \mathrm{ft})}{(5.75\ \mathrm{ft})} + 2\right) \times \left(\frac{(4.0821\ \mathrm{ft})}{2 \times (5.75\ \mathrm{ft})}\right)^4\right]\right]$$

$$M_{max} = 0.86104\ \mathrm{kipft}$$

Minimum Reinforcement Check (LRFD)

Parameters:

$f'_{ck} = 2.5\ \mathrm{ksi}$ - Concrete strength,
$f_{yk} = 60\ \mathrm{ksi}$ - Longitudinal reinforcement strength,
$\phi = 0.65$ - Reduction factor for axial strength,
Table 22.4.2.1 $\alpha = 0.8$ - Alpha factor for axial strength,
$A_g = 2304\ \mathrm{in}^2$ - Gross area of concrete,

Longitudinal reinforcement:

Required reinforcement due to axial load, $A_{st,required}$
22.4.2.2, 10.6.1.1 $A_{st,required}$

$$A_{st,required} = Min\left[\frac{\frac{P}{\phi\, \alpha} - (0.85\, f'_{ck}\, A_g)}{f_{yk} - (0.85\, f'_{ck})}, (0.08\, A_g)\right]$$

$$A_{st,required} = Min\left[\frac{\frac{(12.05\ \mathrm{kip})}{(0.65)\times(0.8)} - (0.85 \times (2.5\ \mathrm{ksi}) \times (2304\ \mathrm{in}^2))}{(60\ \mathrm{ksi}) - (0.85 \times (2.5\ \mathrm{ksi}))}, (0.08 \times (2304\ \mathrm{in}^2))\right]$$

$$A_{st,required} = -84.196\ \mathrm{in}^2$$

A_{min} - Governing minimum reinforcement area,

$$A_{min} = Max[A_{st,required}, (0.0018\, A_g)]$$

$$A_{min} = Max\left[(-84.196\ \mathrm{in}^2), (0.0018 \times (2304\ \mathrm{in}^2))\right]$$

$$A_{min} = 4.1472\ \mathrm{in}^2$$

n_{rebar} - Required number of reinforcement,

$$n_{rebar} = \frac{A_{min}}{A_{rebar}}$$

$$n_{rebar} = \frac{(4.1472\ \mathrm{in}^2)}{(0.3068\ \mathrm{in}^2)}$$

$$n_{rebar} = 14$$

A_{st} - Actual total reinforcement area,

$$A_{st} = n_{rebar}\, \frac{\pi\, d_{bar}^2}{4}$$

$$A_{st} = (14) \times \frac{\pi \times (0.625\ \mathrm{in})^2}{4}$$

$$A_{st} = 4.2951\ \mathrm{in}^2$$

$Ratio$ - Capacity

$$Ratio = \frac{A_{min}}{A_{st}}$$

$$Ratio = \frac{(4.1472\ \mathrm{in}^2)}{(4.2951\ \mathrm{in}^2)}$$

$$Ratio = 0.96556$$

Status: PASS
Ratio: 0.970

25.2.3 s_{rebar} - Minimum spacing of reinforcement,

$$s_{rebar} = Max\left[1.5, (1.5\, d_{bar})\right]$$

$$s_{rebar} = Max\left[1.5, (1.5 \times (0.625 \text{ in}))\right]$$

$$s_{rebar} = 1.5 \text{ in}$$

Ties:

25.7.2.2 Since longitudinal reinforcement is $\leq$ No. 10ø: Use #3(0.375 in)

25.7.2.1 s_{ties} - Maximum spacing of ties,

$$s_{ties} = Min\left[(16\, d_{bar}), (48\, d_{ties}), Min\,(D, b)\right]$$

$$s_{ties} = Min\left[(16 \times (0.625 \text{ in})), (48 \times (0.375 \text{ in})), Min\,((48 \text{ in}), (48 \text{ in}))\right]$$

$$s_{ties} = 10 \text{ in}$$

Summary:

Main reinforcement: **14 - #5 (0.625 in)**
Ties: **#3(0.375 in) - 10 in**

Axial Compression Strength (ACI 318-19, LRFD)

22.4.2.2 ϕP_N - Allowable axial compressive strength

$$\phi P_N = \phi\, 0.80\, \left[\left(0.85\, f'_{ck}\, \left[A_g - A_{st}\right]\right) + \left(f_{yk}\, A_{st}\right)\right]$$

$$\phi P_N = (0.65) \times 0.80 \times \left[\left(0.85 \times (2.5 \text{ ksi}) \times \left[\left(2304 \text{ in}^2\right) - \left(4.2951 \text{ in}^2\right)\right]\right) + \left((60 \text{ ksi}) \times \left(4.2951 \text{ in}^2\right)\right)\right]$$

$$\phi P_N = 2675.2 \text{ kip}$$

$Ratio$ - Capacity

$$Ratio = \frac{P}{\phi P_N}$$

$$Ratio = \frac{(12.05 \text{ kip})}{(2675.2 \text{ kip})}$$

$$Ratio = 0.0045044$$

Status: PASS
Ratio: 0.000

Shear Strength (ACI 318-19, LRFD)

Parameters:

$b_w = 48 \text{ in}$ - Effective width,

22.5.2.2 d - Effective depth

$$d = 0.80\, D$$

$$d = 0.80 \times (48 \text{ in})$$

$$d = 38.4 \text{ in}$$

22.5.5.1.3 λ_s - size effect modification factor

$$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{d}{10}}}, 1\right]$$

$$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{(38.4 \text{ in})}{10}}}, 1\right]$$

$$\lambda_s = 0.64282$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$,

22.5.5.1.1 $V_{c,max}$ - Max shear strength of concrete

$$V_{c,max} = 5\, \lambda_s\, \sqrt{f'_{ck}}\, b_w\, d$$

$$V_{c,max} = 5 \times (0.64282) \times \sqrt{(2500 \text{ psi})} \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{c,max} = 296.21\ \text{kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5\ \text{ksi} \rightarrow 2500\ \text{psi}$, $P = 12.05\ \text{kip} \rightarrow 12050\ \text{lbf}$,

22.5.5.1.1(a) $V_{c,a}$ - Shear strength of concrete (a)

$$V_{c,a} = \left[2\ \lambda_s\ \sqrt{f'_{ck}} + \frac{P}{6\ A_g}\right]\ b_w\ d$$

$$V_{c,a} = \left[2 \times (0.64282) \times \sqrt{(2500\ \text{psi})} + \frac{(12050\ \text{lbf})}{6 \times (2304\ \text{in}^2)}\right] \times (48\ \text{in}) \times (38.4\ \text{in})$$

$$V_{c,a} = 120.09\ \text{kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5\ \text{ksi} \rightarrow 2500\ \text{psi}$,

22.5.5.1.2 $V_{c,b}$ - Shear strength of concrete (b)

$$V_{c,b} = \left[2\ \lambda_s\ \sqrt{f'_{ck}} + \left(0.05\ f'_{ck}\right)\right]\ b_w\ d$$

$$V_{c,b} = \left[2 \times (0.64282) \times \sqrt{(2500\ \text{psi})} + (0.05 \times (2500\ \text{psi}))\right] \times (48\ \text{in}) \times (38.4\ \text{in})$$

$$V_{c,b} = 348.89\ \text{kip}$$

V_c - Governing shear strength of concrete

$$V_c = Min\left[V_{c,max}, V_{c,a}, V_{c,b}\right]$$

$$V_c = Min\left[(296.21\ \text{kip}), (120.09\ \text{kip}), (348.89\ \text{kip})\right]$$

$$V_c = 120.09\ \text{kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5\ \text{ksi} \rightarrow 2500\ \text{psi}$,

22.5.1.2 $V_{s,a}$ - Shear strength of steel (a)

$$V_{s,a} = 8\ \sqrt{f'_{ck}}\ b_w\ d$$

$$V_{s,a} = 8 \times \sqrt{(2500\ \text{psi})} \times (48\ \text{in}) \times (38.4\ \text{in})$$

$$V_{s,a} = 737.28\ \text{kip}$$

A_v - Ties rebar area,

$$A_v = \frac{\pi\ d_{ties}^2}{4}$$

$$A_v = \frac{\pi \times (0.375\ \text{in})^2}{4}$$

$$A_v = 0.11045\ \text{in}^2$$

22.5.8.5.3 $V_{s,b}$ - Shear strength of steel (b)

$$V_{s,b} = \frac{2\ A_v\ f_{ywk}\ d}{s_{ties}}$$

$$V_{s,b} = \frac{2 \times \left(0.11045\ \text{in}^2\right) \times (60\ \text{ksi}) \times (38.4\ \text{in})}{(10\ \text{in})}$$

$$V_{s,b} = 50.894\ \text{kip}$$

V_s - Governing shear strength of steel

$$V_s = MIN\left[V_{s,a}, V_{s,b}\right]$$

$$V_s = MIN\left[(737.28\ \text{kip}), (50.894\ \text{kip})\right]$$

$$V_s = 50.894\ \text{kip}$$

22.5.1.1 ϕV_n - Allowable shear strength

$$\phi V_n = \phi\ (V_c + V_s)$$

$$\phi V_n = (0.65) \times ((120.09\ \text{kip}) + (50.894\ \text{kip}))$$

$$\phi V_n = 111.14\ \text{kip}$$

Considering x-direction:

$V_{max} = 6.4583\ \text{kip}$ - Maximum shear force in the x-direction,
$Ratio$ - Capacity

$$Ratio = \frac{V_{max}}{\phi V_n}$$

	$$Ratio = \frac{(6.4583\ \text{kip})}{(111.14\ \text{kip})}$$ $$Ratio = 0.058109$$		Status: **PASS** Ratio: **0.060**
	Considering z-direction: $V_{max} = 0.33345\ \text{kip}$ - Maximum shear force in the z-direction, $Ratio$ - Capacity $$Ratio = \frac{V_{max}}{\phi V_n}$$ $$Ratio = \frac{(0.33345\ \text{kip})}{(111.14\ \text{kip})}$$ $$Ratio = 0.0030003$$		Status: **PASS** Ratio: **0.000**
14.5.2.1b	**Flexural Strength (ACI 318-19, LRFD)** S_m - Section modulus $$S_m = \frac{b\ D^2}{6}$$ $$S_m = \frac{(48\ \text{in}) \times (48\ \text{in})^2}{6}$$ $$S_m = 18432\ \text{in}^3$$ $\lambda = 1$ - Concrete modification factor (Normal concrete), Allowable flexural strength: M_n shall be the lesser of: $\phi M_{n,1}$ $$\phi M_{n,1} = \phi \times 5 \times \lambda \times \sqrt{(f_c')} \times S_m$$ $$\phi M_{n,1} = 0.65 \times 5 \times 1 \times \sqrt{(2.5ksi)} \times 18432.001in^3$$ $$\phi M_{n,1} = 249.600kipft$$ $\phi M_{n,2}$ $$\phi M_{n,2} = \phi\ 0.85\ f_{ck}'\ S_m$$ $$\phi M_{n,2} = (0.65) \times 0.85 \times (2.5\ \text{ksi}) \times \left(18432\ \text{in}^3\right)$$ $$\phi M_{n,2} = 2121.6\ \text{kipft}$$ Therefore, ϕM_n - Allowable flexural strength, $$\phi M_n = MIN[\phi M_{n,1}, \phi M_{n,2}]$$ $$\phi M_n = MIN[(249.6\ \text{kipft}), (2121.6\ \text{kipft})]$$ $$\phi M_n = 249.6\ \text{kipft}$$ **Considering x-direction:** $M_{max} = 17.913\ \text{kipft}$ - Maximum moment in the x-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\phi M_n}$$ $$Ratio = \frac{(17.913\ \text{kipft})}{(249.6\ \text{kipft})}$$ $$Ratio = 0.071768$$		Status: **PASS** Ratio: **0.070**
	Considering z-direction: $M_{max} = 0.86104\ \text{kipft}$ - Maximum moment in the z-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\phi M_n}$$		

	$$Ratio = \frac{(0.86104\ \mathrm{kipft})}{(249.6\ \mathrm{kipft})}$$ $$Ratio = 0.0034497$$	Status: **PASS** Ratio: **0.000**

REFERENCES	CALCULATIONS	RESULTS

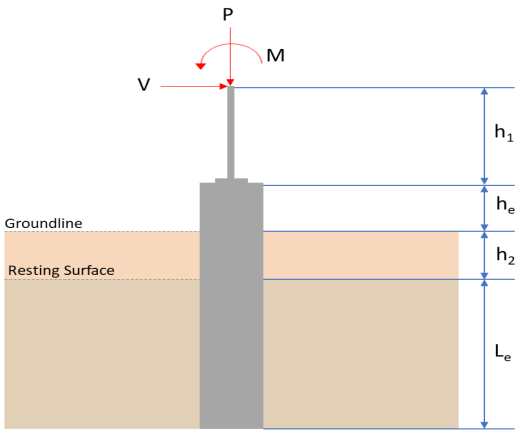
SkyCiv Foundation Design

Pile Foundation

Design Information :

Design code : IBC 2021 (International Building Code)
Unit System : Imperial

Pile Input



Geometry

Pile shape: rectangular
$b = 48\ \mathrm{in}$ - Pile width
$D = 48\ \mathrm{in}$ - Pile depth
$L = 5.75\ \mathrm{ft}$ - Total pile length
$h_1 = 0\ \mathrm{ft}$ - Lateral load height from the top of the pile,
$h_2 = 0\ \mathrm{ft}$ - Depth to resisting surface
$h_e = 0\ \mathrm{ft}$ - Length of pile above the ground

Tabulation of Soil Parameters

Layer	Label	Allowable Bearing Pressure (q_a) (psf)	Allowable Lateral Pressure (R) (psf/ft)
1	Sand, silty sand, clayey sand, silty gravel & clayey gravel	2000.000	150.000

Tabulation of Loads

Load Component	ASD	LRFD
P (kip)	7.571	11.503
V_x (kip)	-1.101	-1.850
V_z (kip)	0.038	0.058
M_x (kipft)	0.137	0.208
M_z (kipft)	16.350	28.560

Material Properties

$f'_{ck} = 2.5\ \mathrm{ksi}$ - Concrete strength,

Required depth to resist lateral loads (ASD)

H - Point of application of the lateral load

$$H = h_1 + h_2 + h_e$$

$$H = (0\ \mathrm{ft}) + (0\ \mathrm{ft}) + (0\ \mathrm{ft})$$

$$H = 0\ \mathrm{ft}$$

Considering x-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\ D}$$

$$H_o = \frac{(-1.101\ \mathrm{kip})}{1.57 \times (48\ \mathrm{in})}$$

$$H_o = -0.17532\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\,H)}{1.57\,D}$$

$$M_o = \frac{(16.35 \text{ kipft}) + ((-1.101 \text{ kip}) \times (0 \text{ ft}))}{1.57 \times (48 \text{ in})}$$

$$M_o = 2.6035 \text{ kipft/ft}$$

Required depth of embedment in earth:

$$L_x^3 - \left(14.14 \times \frac{H_o \times L_x}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,x} = 5.3383 \text{ ft}$ - Required depth in x-direction,

Considering z-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\,b}$$

$$H_o = \frac{(0.038 \text{ kip})}{1.57 \times (48 \text{ in})}$$

$$H_o = 0.006051 \text{ kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\,H)}{1.57\,b}$$

$$M_o = \frac{(0.137 \text{ kipft}) + ((0.038 \text{ kip}) \times (0 \text{ ft}))}{1.57 \times (48 \text{ in})}$$

$$M_o = 0.021815 \text{ kipft/ft}$$

Required depth of embedment in earth:

$$L_z^3 - \left(14.14 \times \frac{H_o \times L_z}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,z} = 1.3043 \text{ ft}$ - Required depth in z-direction,

Minimum embedded depth required:

$L_{e,req}$ - Depth of pile required,

$$L_{e,req} = MAX[L_{e,x}, L_{e,z}]$$

$$L_{e,req} = MAX[(5.3383 \text{ ft}), (1.3043 \text{ ft})]$$

$$L_{e,req} = 5.338 \text{ ft}$$

L_e - Actual embedded length of pile,

$$L_e = L - h_e - h_2$$

$$L_e = (5.75 \text{ ft}) - (0 \text{ ft}) - (0 \text{ ft})$$

$$L_e = 5.75 \text{ ft}$$

$Ratio$ - Embedded depth

$$Ratio = \frac{L_{e,req}}{L_e}$$

$$Ratio = \frac{(5.338 \text{ ft})}{(5.75 \text{ ft})}$$

$$Ratio = 0.92835$$

Status: **PASS**
Ratio: **0.930**

End-bearing Capacity (ASD)

A - Pile cross-section area

$$A = b\,D$$

$$A = (48 \text{ in}) \times (48 \text{ in})$$

$$A = 16 \text{ ft}^2$$

q - End-bearing pressure

$$q = \frac{P_v}{A}$$

$$q = \frac{(7.571 \text{ kip})}{(16 \text{ ft}^2)}$$

$$q = 0.47319 \text{ kip/ft}^2$$

Check bearing capacity ratio:

$Ratio$ - Capacity

$$Ratio = \frac{q}{q_a}$$

$$Ratio = \frac{\left(0.47319 \text{ kip/ft}^2\right)}{(2000 \text{ psf})}$$

$$Ratio = 0.23659$$

Status: **PASS**
Ratio: **0.240**

Czerniak

Lateral Soil Pressure (ASD):

L/D - Length to least lateral dimension ratio,

$$L/D = \frac{L}{D}$$

$$L/D = \frac{(5.75 \text{ ft})}{(48 \text{ in})}$$

$$L/D = 1.4375$$

Since L/D $\leq$ 10,

Pile is short.

Considering x-direction:

$H_o = -0.17532 \text{ kip/ft}$ - Lateral force per length of pile,
$M_o = 2.6035 \text{ kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\, M_o\, L_e) + \left(3\, H_o\, L_e^2\right)}{(6\, M_o) + (4\, H_o\, L_e)}$$

$$a = \frac{(4 \times (2.6035 \text{ kipft/ft}) \times (5.75 \text{ ft})) + \left(3 \times (-0.17532 \text{ kip/ft}) \times (5.75 \text{ ft})^2\right)}{(6 \times (2.6035 \text{ kipft/ft})) + (4 \times (-0.17532 \text{ kip/ft}) \times (5.75 \text{ ft}))}$$

$$a = 3.9316 \text{ ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\, [(4\, M_o) + (3\, H_o\, L_e)]^2}{L_e^2\, [(3\, M_o) + (2\, H_o\, L_e)]}$$

$$p = \frac{0.75 \times [(4 \times (2.6035 \text{ kipft/ft})) + (3 \times (-0.17532 \text{ kip/ft}) \times (5.75 \text{ ft}))]^2}{(5.75 \text{ ft})^2 \times [(3 \times (2.6035 \text{ kipft/ft})) + (2 \times (-0.17532 \text{ kip/ft}) \times (5.75 \text{ ft}))]}$$

$$p = 0.21379 \text{ kip/ft}^2$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\, [(2\, M_o) + (H_o\, L_e)]}{L_e^2}$$

$$s = \frac{6 \times [(2 \times (2.6035 \text{ kipft/ft})) + ((-0.17532 \text{ kip/ft}) \times (5.75 \text{ ft}))]}{(5.75 \text{ ft})^2}$$

$$s = 0.762 \text{ kip/ft}^2$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\, \frac{a}{2}$$

$$p_a = (150 \text{ psf/ft}) \times \frac{(3.9316 \text{ ft})}{2}$$

$$p_a = 0.29487 \text{ kip/ft}^2$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{\left(0.21379\ \mathrm{kip/ft^2}\right)}{\left(0.29487\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = 0.72502$$

Status: **PASS**
Ratio: **0.730**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\,L_e$$

$$p_s = \left(150\ \mathrm{psf/ft}\right) \times \left(5.75\ \mathrm{ft}\right)$$

$$p_s = 0.8625\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{\left(0.762\ \mathrm{kip/ft^2}\right)}{\left(0.8625\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = 0.88348$$

Status: **PASS**
Ratio: **0.880**

Considering z-direction:

$H_o = 0.006051\ \mathrm{kip/ft}$ - Lateral force per length of pile,
$M_o = 0.021815\ \mathrm{kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{\left(4\,M_o\,L_e\right) + \left(3\,H_o\,L_e^2\right)}{\left(6\,M_o\right) + \left(4\,H_o\,L_e\right)}$$

$$a = \frac{\left(4 \times \left(0.021815\ \mathrm{kipft/ft}\right) \times \left(5.75\ \mathrm{ft}\right)\right) + \left(3 \times \left(0.006051\ \mathrm{kip/ft}\right) \times \left(5.75\ \mathrm{ft}\right)^2\right)}{\left(6 \times \left(0.021815\ \mathrm{kipft/ft}\right)\right) + \left(4 \times \left(0.006051\ \mathrm{kip/ft}\right) \times \left(5.75\ \mathrm{ft}\right)\right)}$$

$$a = 4.0803\ \mathrm{ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\left[\left(4\,M_o\right) + \left(3\,H_o\,L_e\right)\right]^2}{L_e^2\left[\left(3\,M_o\right) + \left(2\,H_o\,L_e\right)\right]}$$

$$p = \frac{0.75 \times \left[\left(4 \times \left(0.021815\ \mathrm{kipft/ft}\right)\right) + \left(3 \times \left(0.006051\ \mathrm{kip/ft}\right) \times \left(5.75\ \mathrm{ft}\right)\right)\right]^2}{\left(5.75\ \mathrm{ft}\right)^2 \times \left[\left(3 \times \left(0.021815\ \mathrm{kipft/ft}\right)\right) + \left(2 \times \left(0.006051\ \mathrm{kip/ft}\right) \times \left(5.75\ \mathrm{ft}\right)\right)\right]}$$

$$p = 0.0061697\ \mathrm{kip/ft^2}$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\left[\left(2\,M_o\right) + \left(H_o\,L_e\right)\right]}{L_e^2}$$

$$s = \frac{6 \times \left[\left(2 \times \left(0.021815\ \mathrm{kipft/ft}\right)\right) + \left(\left(0.006051\ \mathrm{kip/ft}\right) \times \left(5.75\ \mathrm{ft}\right)\right)\right]}{\left(5.75\ \mathrm{ft}\right)^2}$$

$$s = 0.014232\ \mathrm{kip/ft^2}$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\,\frac{a}{2}$$

$$p_a = \left(150\ \mathrm{psf/ft}\right) \times \frac{\left(4.0803\ \mathrm{ft}\right)}{2}$$

$$p_a = 0.30602\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{\left(0.0061697\ \mathrm{kip/ft^2}\right)}{\left(0.30602\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = 0.020161$$

Status: **PASS**
Ratio: **0.020**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\,L_e$$

$$p_s = (150\ \mathrm{psf/ft}) \times (5.75\ \mathrm{ft})$$

$$p_s = 0.8625\ \mathrm{kip/ft^2}$$

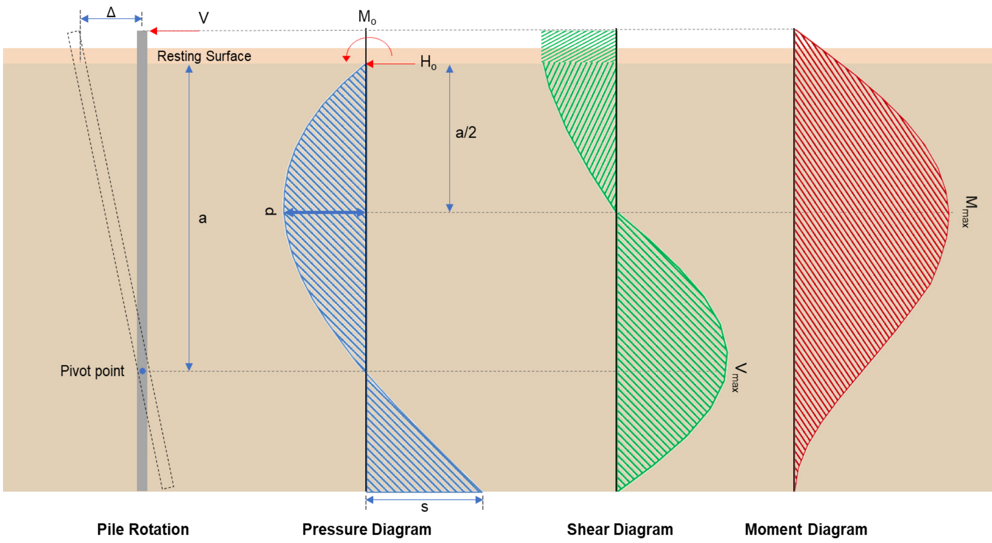
$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{\left(0.014232\ \mathrm{kip/ft^2}\right)}{\left(0.8625\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = 0.016501$$

Status: **PASS**
Ratio: **0.020**



Shear force and Bending moment (x-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\,D}$$

$$H_o = \frac{(-1.85\ \mathrm{kip})}{1.57 \times (48\ \mathrm{in})}$$

$$H_o = -0.29459\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\,H)}{1.57\,D}$$

$$M_o = \frac{(28.56\ \mathrm{kipft}) + ((-1.85\ \mathrm{kip}) \times (0\ \mathrm{ft}))}{1.57 \times (48\ \mathrm{in})}$$

$$M_o = 4.5478\ \mathrm{kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(4.5478\ \mathrm{kipft/ft})}{(-0.29459\ \mathrm{kip/ft})}$$

$$E = 15.438\ \mathrm{ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (4.5478\ \mathrm{kipft/ft}) \times (5.75\ \mathrm{ft})) + \left(3 \times (-0.29459\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft})^2\right)}{}$$

$$a = \frac{}{(6 \times (4.5478\ \text{kipft/ft})) + (4 \times (-0.29459\ \text{kip/ft}) \times (5.75\ \text{ft}))}$$

$$a = 3.9286\ \text{ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\, D)\left[1 - \left[3\left(\frac{4\,E}{L_e} + 3\right)\left(\frac{a}{L_e}\right)^2\right] + \left[4\left(\frac{3\,E}{L_e} + 2\right)\left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((-0.29459\ \text{kip/ft}) \times (48\ \text{in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (15.438\ \text{ft})}{(5.75\ \text{ft})} + 3\right) \times \left(\frac{(3.9286\ \text{ft})}{(5.75\ \text{ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (15.438\ \text{ft})}{(5.75\ \text{ft})} + 2\right) \times \left(\frac{(3.9286\ \text{ft})}{(5.75\ \text{ft})}\right)^3\right]\right]$$

$$V_{max} = 6.3793\ \text{kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, D\, L_e)\left[\left(\frac{E}{L_e} + \frac{a}{2\,L_e}\right) - \left[\left(\frac{4\,E}{L_e} + 3\right)\left(\frac{a}{2\,L_e}\right)^3\right] + \left[\left(\frac{3\,E}{L_e} + 2\right)\left(\frac{a}{2\,L_e}\right)^4\right]\right]$$

$$M_{max} = ((-0.29459\ \text{kip/ft}) \times (48\ \text{in}) \times (5.75\ \text{ft})) \times \left[\left(\frac{(15.438\ \text{ft})}{(5.75\ \text{ft})} + \frac{(3.9286\ \text{ft})}{2 \times (5.75\ \text{ft})}\right) - \left[\left(\frac{4 \times (15.438\ \text{ft})}{(5.75\ \text{ft})} + 3\right) \times \left(\frac{(3.9286\ \text{ft})}{2 \times (5.75\ \text{ft})}\right)^3\right] + \left[\left(\frac{3 \times (15.438\ \text{ft})}{(5.75\ \text{ft})} + 2\right) \times \left(\frac{(3.9286\ \text{ft})}{2 \times (5.75\ \text{ft})}\right)^4\right]\right]$$

$$M_{max} = 17.722\ \text{kipft}$$

Shear force and Bending moment (z-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\, b}$$

$$H_o = \frac{(0.058\ \text{kip})}{1.57 \times (48\ \text{in})}$$

$$H_o = 0.0092357\ \text{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\, H)}{1.57\, b}$$

$$M_o = \frac{(0.208\ \text{kipft}) + ((0.058\ \text{kip}) \times (0\ \text{ft}))}{1.57 \times (48\ \text{in})}$$

$$M_o = 0.033121\ \text{kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(0.033121\ \text{kipft/ft})}{(0.0092357\ \text{kip/ft})}$$

$$E = 3.5862\ \text{ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\, M_o\, L_e) + \left(3\, H_o\, L_e^2\right)}{(6\, M_o) + (4\, H_o\, L_e)}$$

$$a = \frac{(4 \times (0.033121\ \text{kipft/ft}) \times (5.75\ \text{ft})) + \left(3 \times (0.0092357\ \text{kip/ft}) \times (5.75\ \text{ft})^2\right)}{(6 \times (0.033121\ \text{kipft/ft})) + (4 \times (0.0092357\ \text{kip/ft}) \times (5.75\ \text{ft}))}$$

$$a = 4.0809\ \text{ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\, b)\left[1 - \left[3\left(\frac{4\,E}{L_e} + 3\right)\left(\frac{a}{L_e}\right)^2\right] + \left[4\left(\frac{3\,E}{L_e} + 2\right)\left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((0.0092357\ \text{kip/ft}) \times (48\ \text{in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (3.5862\ \text{ft})}{(5.75\ \text{ft})} + 3\right) \times \left(\frac{(4.0809\ \text{ft})}{(5.75\ \text{ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (3.5862\ \text{ft})}{(5.75\ \text{ft})} + 2\right) \times \left(\frac{(4.0809\ \text{ft})}{(5.75\ \text{ft})}\right)^3\right]\right]$$

$$V_{max} = 0.065305\ \text{kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, b\, L_e) \left[\left(\frac{E}{L_e} + \frac{a}{2\,L_e}\right) - \left[\left(\frac{4\,E}{L_e} + 3\right)\left(\frac{a}{2\,L_e}\right)^3\right] + \left[\left(\frac{3\,E}{L_e} + 2\right)\left(\frac{a}{2\,L_e}\right)^4\right]\right]$$

$$M_{max} = ((0.0092357\ \text{kip/ft}) \times (48\ \text{in}) \times (5.75\ \text{ft})) \times \left[\left(\frac{(3.5862\ \text{ft})}{(5.75\ \text{ft})} + \frac{(4.0809\ \text{ft})}{2 \times (5.75\ \text{ft})}\right) - \left[\left(\frac{4 \times (3.5862\ \text{ft})}{(5.75\ \text{ft})} + 3\right) \times \left(\frac{(4.0809\ \text{ft})}{2 \times (5.75\ \text{ft})}\right)^3\right] + \left[\left(\frac{3 \times (3.5862\ \text{ft})}{(5.75\ \text{ft})} + 2\right) \times \left(\frac{(4.0809\ \text{ft})}{2 \times (5.75\ \text{ft})}\right)^4\right]\right]$$

$$M_{max} = 0.16875\ \text{kipft}$$

Minimum Reinforcement Check (LRFD)

Parameters:

$f'_{ck} = 2.5\ \text{ksi}$ - Concrete strength,
$f_{yk} = 60\ \text{ksi}$ - Longitudinal reinforcement strength,
$\phi = 0.65$ - Reduction factor for axial strength,
Table 22.4.2.1 $\alpha = 0.8$ - Alpha factor for axial strength,
$A_g = 2304\ \text{in}^2$ - Gross area of concrete,

Longitudinal reinforcement:

Required reinforcement due to axial load, $A_{st,required}$

22.4.2.2, 10.6.1.1 $A_{st,required}$

$$A_{st,required} = Min\left[\frac{\frac{P}{\phi\,\alpha} - (0.85\, f'_{ck}\, A_g)}{f_{yk} - (0.85\, f'_{ck})}, (0.08\, A_g)\right]$$

$$A_{st,required} = Min\left[\frac{\frac{(11.503\ \text{kip})}{(0.65)\times(0.8)} - (0.85 \times (2.5\ \text{ksi}) \times (2304\ \text{in}^2))}{(60\ \text{ksi}) - (0.85 \times (2.5\ \text{ksi}))}, (0.08 \times (2304\ \text{in}^2))\right]$$

$$A_{st,required} = -84.214\ \text{in}^2$$

A_{min} - Governing minimum reinforcement area,

$$A_{min} = Max\,[A_{st,required}, (0.0018\, A_g)]$$

$$A_{min} = Max\left[(-84.214\ \text{in}^2), (0.0018 \times (2304\ \text{in}^2))\right]$$

$$A_{min} = 4.1472\ \text{in}^2$$

n_{rebar} - Required number of reinforcement,

$$n_{rebar} = \frac{A_{min}}{A_{rebar}}$$

$$n_{rebar} = \frac{(4.1472\ \text{in}^2)}{(0.3068\ \text{in}^2)}$$

$$n_{rebar} = 14$$

A_{st} - Actual total reinforcement area,

$$A_{st} = n_{rebar}\,\frac{\pi\, d_{bar}^2}{4}$$

$$A_{st} = (14) \times \frac{\pi \times (0.625\ \text{in})^2}{4}$$

$$A_{st} = 4.2951\ \text{in}^2$$

$Ratio$ - Capacity

$$Ratio = \frac{A_{min}}{A_{st}}$$

$$Ratio = \frac{(4.1472\ \text{in}^2)}{(4.2951\ \text{in}^2)}$$

$$Ratio = 0.96556$$

Status: **PASS**
Ratio: **0.970**

25.2.3 s_{rebar} - Minimum spacing of reinforcement,

$$s_{rebar} = Max\left[1.5, \left(1.5\ d_{bar}\right)\right]$$

$$s_{rebar} = Max\left[1.5, \left(1.5 \times (0.625\ \text{in})\right)\right]$$

$$s_{rebar} = 1.5\ \text{in}$$

Ties:

25.7.2.2 Since longitudinal reinforcement is $\leq$ No. 10ø: Use #3(0.375 in)

25.7.2.1 s_{ties} - Maximum spacing of ties,

$$s_{ties} = Min\left[\left(16\ d_{bar}\right), \left(48\ d_{ties}\right), Min\left(D, b\right)\right]$$

$$s_{ties} = Min\left[\left(16 \times (0.625\ \text{in})\right), \left(48 \times (0.375\ \text{in})\right), Min\left((48\ \text{in}), (48\ \text{in})\right)\right]$$

$$s_{ties} = 10\ \text{in}$$

Summary:

Main reinforcement: **14 - #5 (0.625 in)**
Ties: **#3(0.375 in) - 10 in**

Axial Compression Strength (ACI 318-19, LRFD)

22.4.2.2 ϕP_N - Allowable axial compressive strength

$$\phi P_N = \phi\ 0.80\ \left[\left(0.85\ f'_{ck}\ \left[A_g - A_{st}\right]\right) + \left(f_{yk}\ A_{st}\right)\right]$$

$$\phi P_N = (0.65) \times 0.80 \times \left[\left(0.85 \times (2.5\ \text{ksi}) \times \left[\left(2304\ \text{in}^2\right) - \left(4.2951\ \text{in}^2\right)\right]\right) + \left((60\ \text{ksi}) \times \left(4.2951\ \text{in}^2\right)\right)\right]$$

$$\phi P_N = 2675.2\ \text{kip}$$

$Ratio$ - Capacity

$$Ratio = \frac{P}{\phi P_N}$$

$$Ratio = \frac{(11.503\ \text{kip})}{(2675.2\ \text{kip})}$$

$$Ratio = 0.0043$$

Status: **PASS**
Ratio: **0.000**

Shear Strength (ACI 318-19, LRFD)

Parameters:

$b_w = 48\ \text{in}$ - Effective width,

22.5.2.2 d - Effective depth

$$d = 0.80\ D$$

$$d = 0.80 \times (48\ \text{in})$$

$$d = 38.4\ \text{in}$$

22.5.5.1.3 λ_s - size effect modification factor

$$\lambda_s = MIN\left[\sqrt{\frac{2}{1+\frac{d}{10}}}, 1\right]$$

$$\lambda_s = MIN\left[\sqrt{\frac{2}{1+\frac{(38.4\ \text{in})}{10}}}, 1\right]$$

$$\lambda_s = 0.64282$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5\ \text{ksi} \rightarrow 2500\ \text{psi}$,

22.5.5.1.1 $V_{c,max}$ - Max shear strength of concrete

$$V_{c,max} = 5\ \lambda_s\ \sqrt{f'_{ck}}\ b_w\ d$$

$$V_{c,max} = 5 \times (0.64282) \times \sqrt{(2500\ \text{psi})} \times (48\ \text{in}) \times (38.4\ \text{in})$$

$$V_{c,max} = 296.21 \text{ kip}$$

22.5.5.1.1(a) The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$, $P = 11.503 \text{ kip} \rightarrow 11503 \text{ lbf}$,
$V_{c,a}$ - Shear strength of concrete (a)

$$V_{c,a} = \left[2\,\lambda_s\,\sqrt{f'_{ck}} + \frac{P}{6\,A_g}\right]\,b_w\,d$$

$$V_{c,a} = \left[2 \times (0.64282) \times \sqrt{(2500 \text{ psi})} + \frac{(11503 \text{ lbf})}{6 \times (2304 \text{ in}^2)}\right] \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{c,a} = 120.02 \text{ kip}$$

22.5.5.1.2 The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$,
$V_{c,b}$ - Shear strength of concrete (b)

$$V_{c,b} = \left[2\,\lambda_s\,\sqrt{f'_{ck}} + \left(0.05\,f'_{ck}\right)\right]\,b_w\,d$$

$$V_{c,b} = \left[2 \times (0.64282) \times \sqrt{(2500 \text{ psi})} + (0.05 \times (2500 \text{ psi}))\right] \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{c,b} = 348.89 \text{ kip}$$

V_c - Governing shear strength of concrete

$$V_c = Min\left[V_{c,max}, V_{c,a}, V_{c,b}\right]$$

$$V_c = Min\left[(296.21 \text{ kip}), (120.02 \text{ kip}), (348.89 \text{ kip})\right]$$

$$V_c = 120.02 \text{ kip}$$

22.5.1.2 The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$,
$V_{s,a}$ - Shear strength of steel (a)

$$V_{s,a} = 8\,\sqrt{f'_{ck}}\,b_w\,d$$

$$V_{s,a} = 8 \times \sqrt{(2500 \text{ psi})} \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{s,a} = 737.28 \text{ kip}$$

A_v - Ties rebar area,

$$A_v = \frac{\pi\,d_{ties}^2}{4}$$

$$A_v = \frac{\pi \times (0.375 \text{ in})^2}{4}$$

$$A_v = 0.11045 \text{ in}^2$$

22.5.8.5.3 $V_{s,b}$ - Shear strength of steel (b)

$$V_{s,b} = \frac{2\,A_v\,f_{ywk}\,d}{s_{ties}}$$

$$V_{s,b} = \frac{2 \times \left(0.11045 \text{ in}^2\right) \times (60 \text{ ksi}) \times (38.4 \text{ in})}{(10 \text{ in})}$$

$$V_{s,b} = 50.894 \text{ kip}$$

V_s - Governing shear strength of steel

$$V_s = MIN\left[V_{s,a}, V_{s,b}\right]$$

$$V_s = MIN\left[(737.28 \text{ kip}), (50.894 \text{ kip})\right]$$

$$V_s = 50.894 \text{ kip}$$

22.5.1.1 ϕV_n - Allowable shear strength

$$\phi V_n = \phi\,\left(V_c + V_s\right)$$

$$\phi V_n = (0.65) \times ((120.02 \text{ kip}) + (50.894 \text{ kip}))$$

$$\phi V_n = 111.09 \text{ kip}$$

Considering x-direction:

$V_{max} = 6.3793 \text{ kip}$ - Maximum shear force in the x-direction,
$Ratio$ - Capacity

$$Ratio = \frac{V_{max}}{\phi V_n}$$

	$$Ratio = \frac{(6.3793 \text{ kip})}{(111.09 \text{ kip})}$$ $$Ratio = 0.057423$$	Status: **PASS** Ratio: **0.060**
	Considering z-direction: $V_{max} = 0.065305 \text{ kip}$ - Maximum shear force in the z-direction, $Ratio$ - Capacity $$Ratio = \frac{V_{max}}{\phi V_n}$$ $$Ratio = \frac{(0.065305 \text{ kip})}{(111.09 \text{ kip})}$$ $$Ratio = 0.00058784$$	Status: **PASS** Ratio: **0.000**
14.5.2.1b	**Flexural Strength (ACI 318-19, LRFD)** S_m - Section modulus $$S_m = \frac{b\,D^2}{6}$$ $$S_m = \frac{(48 \text{ in}) \times (48 \text{ in})^2}{6}$$ $$S_m = 18432 \text{ in}^3$$ $\lambda = 1$ - Concrete modification factor (Normal concrete), Allowable flexural strength: M_n shall be the lesser of: $\phi M_{n,1}$ $$\phi M_{n,1} = \phi \times 5 \times \lambda \times \sqrt{(f_c')} \times S_m$$ $$\phi M_{n,1} = 0.65 \times 5 \times 1 \times \sqrt{(2.5ksi)} \times 18432.001in^3$$ $$\phi M_{n,1} = 249.600kipft$$ $\phi M_{n,2}$ $$\phi M_{n,2} = \phi\, 0.85\, f_{ck}'\, S_m$$ $$\phi M_{n,2} = (0.65) \times 0.85 \times (2.5 \text{ ksi}) \times \left(18432 \text{ in}^3\right)$$ $$\phi M_{n,2} = 2121.6 \text{ kipft}$$ Therefore, ϕM_n - Allowable flexural strength, $$\phi M_n = MIN[\phi M_{n,1}, \phi M_{n,2}]$$ $$\phi M_n = MIN[(249.6 \text{ kipft}), (2121.6 \text{ kipft})]$$ $$\phi M_n = 249.6 \text{ kipft}$$ **Considering x-direction:** $M_{max} = 17.722 \text{ kipft}$ - Maximum moment in the x-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\phi M_n}$$ $$Ratio = \frac{(17.722 \text{ kipft})}{(249.6 \text{ kipft})}$$ $$Ratio = 0.071002$$	Status: **PASS** Ratio: **0.070**
	Considering z-direction: $M_{max} = 0.16875 \text{ kipft}$ - Maximum moment in the z-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\phi M_n}$$	

	$$Ratio = \frac{(0.16875 \text{ kipft})}{(249.6 \text{ kipft})}$$ $$Ratio = 0.00067606$$	Status: **PASS** Ratio: **0.000**

REFERENCES	CALCULATIONS	RESULTS

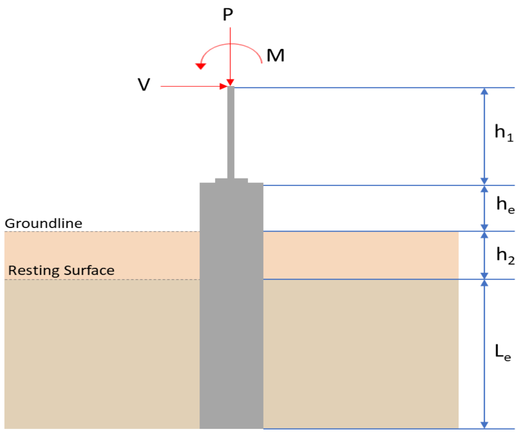
SkyCiv Foundation Design

Pile Foundation

Design Information :

Design code : IBC 2021 (International Building Code)
Unit System : Imperial

Pile Input



Geometry

Pile shape: rectangular

$b = 48\ \text{in}$ - Pile width

$D = 48\ \text{in}$ - Pile depth

$L = 5.75\ \text{ft}$ - Total pile length

$h_1 = 0\ \text{ft}$ - Lateral load height from the top of the pile,

$h_2 = 0\ \text{ft}$ - Depth to resisting surface

$h_e = 0\ \text{ft}$ - Length of pile above the ground

Tabulation of Soil Parameters

Layer	Label	Allowable Bearing Pressure (q_a) (psf)	Allowable Lateral Pressure (R) (psf/ft)
1	Sand, silty sand, clayey sand, silty gravel & clayey gravel	2000.000	150.000

Tabulation of Loads

Load Component	ASD	LRFD
P (kip)	7.571	11.503
V_x (kip)	-1.101	-1.850
V_z (kip)	-0.038	-0.058
M_x (kipft)	-0.137	-0.208
M_z (kipft)	16.350	28.560

Material Properties

$f'_{ck} = 2.5\ \text{ksi}$ - Concrete strength,

Required depth to resist lateral loads (ASD)

H - Point of application of the lateral load

$$H = h_1 + h_2 + h_e$$

$$H = (0\ \text{ft}) + (0\ \text{ft}) + (0\ \text{ft})$$

$$H = 0\ \text{ft}$$

Considering x-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\ D}$$

$$H_o = \frac{(-1.101\ \text{kip})}{1.57 \times (48\ \text{in})}$$

$$H_o = -0.17532\ \text{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\, H)}{1.57\, D}$$

$$M_o = \frac{(16.35 \text{ kipft}) + ((-1.101 \text{ kip}) \times (0 \text{ ft}))}{1.57 \times (48 \text{ in})}$$

$$M_o = 2.6035 \text{ kipft/ft}$$

Required depth of embedment in earth:

$$L_x^3 - \left(14.14 \times \frac{H_o \times L_x}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,x} = 5.3383 \text{ ft}$ - Required depth in x-direction,

Considering z-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\, b}$$

$$H_o = \frac{(-0.038 \text{ kip})}{1.57 \times (48 \text{ in})}$$

$$H_o = -0.006051 \text{ kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\, H)}{1.57\, b}$$

$$M_o = \frac{(0.137 \text{ kipft}) + ((-0.038 \text{ kip}) \times (0 \text{ ft}))}{1.57 \times (48 \text{ in})}$$

$$M_o = 0.021815 \text{ kipft/ft}$$

Required depth of embedment in earth:

$$L_z^3 - \left(14.14 \times \frac{H_o \times L_z}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,z} = 1.1038 \text{ ft}$ - Required depth in z-direction,

Minimum embedded depth required:

$L_{e,req}$ - Depth of pile required,

$$L_{e,req} = MAX[L_{e,x}, L_{e,z}]$$

$$L_{e,req} = MAX[(5.3383 \text{ ft}), (1.1038 \text{ ft})]$$

$$L_{e,req} = 5.338 \text{ ft}$$

L_e - Actual embedded length of pile,

$$L_e = L - h_e - h_2$$

$$L_e = (5.75 \text{ ft}) - (0 \text{ ft}) - (0 \text{ ft})$$

$$L_e = 5.75 \text{ ft}$$

$Ratio$ - Embedded depth

$$Ratio = \frac{L_{e,req}}{L_e}$$

$$Ratio = \frac{(5.338 \text{ ft})}{(5.75 \text{ ft})}$$

$$Ratio = 0.92835$$

Status: **PASS**
Ratio: **0.930**

End-bearing Capacity (ASD)

A - Pile cross-section area

$$A = b\, D$$

$$A = (48 \text{ in}) \times (48 \text{ in})$$

$$A = 16 \text{ ft}^2$$

q - End-bearing pressure

$$q = \frac{P_v}{A}$$

$$q = \frac{(7.571\ \text{kip})}{(16\ \text{ft}^2)}$$

$$q = 0.47319\ \text{kip/ft}^2$$

Check bearing capacity ratio:

$Ratio$ - Capacity

$$Ratio = \frac{q}{q_a}$$

$$Ratio = \frac{\left(0.47319\ \text{kip/ft}^2\right)}{(2000\ \text{psf})}$$

$$Ratio = 0.23659$$

Status: **PASS**
Ratio: **0.240**

Czerniak

Lateral Soil Pressure (ASD):

L/D - Length to least lateral dimension ratio,

$$L/D = \frac{L}{D}$$

$$L/D = \frac{(5.75\ \text{ft})}{(48\ \text{in})}$$

$$L/D = 1.4375$$

Since L/D $\leq$ 10,

Pile is short.

Considering x-direction:

$H_o = -0.17532\ \text{kip/ft}$ - Lateral force per length of pile,
$M_o = 2.6035\ \text{kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (2.6035\ \text{kipft/ft}) \times (5.75\ \text{ft})) + \left(3 \times (-0.17532\ \text{kip/ft}) \times (5.75\ \text{ft})^2\right)}{(6 \times (2.6035\ \text{kipft/ft})) + (4 \times (-0.17532\ \text{kip/ft}) \times (5.75\ \text{ft}))}$$

$$a = 3.9316\ \text{ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\,[(4\,M_o) + (3\,H_o\,L_e)]^2}{L_e^2\,[(3\,M_o) + (2\,H_o\,L_e)]}$$

$$p = \frac{0.75 \times [(4 \times (2.6035\ \text{kipft/ft})) + (3 \times (-0.17532\ \text{kip/ft}) \times (5.75\ \text{ft}))]^2}{(5.75\ \text{ft})^2 \times [(3 \times (2.6035\ \text{kipft/ft})) + (2 \times (-0.17532\ \text{kip/ft}) \times (5.75\ \text{ft}))]}$$

$$p = 0.21379\ \text{kip/ft}^2$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\,[(2\,M_o) + (H_o\,L_e)]}{L_e^2}$$

$$s = \frac{6 \times [(2 \times (2.6035\ \text{kipft/ft})) + ((-0.17532\ \text{kip/ft}) \times (5.75\ \text{ft}))]}{(5.75\ \text{ft})^2}$$

$$s = 0.762\ \text{kip/ft}^2$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\,\frac{a}{2}$$

$$p_a = (150\ \text{psf/ft}) \times \frac{(3.9316\ \text{ft})}{2}$$

$$p_a = 0.29487\ \text{kip/ft}^2$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{\left(0.21379\ \mathrm{kip/ft^2}\right)}{\left(0.29487\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = 0.72502$$

Status: **PASS**
Ratio: **0.730**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\,L_e$$

$$p_s = (150\ \mathrm{psf/ft}) \times (5.75\ \mathrm{ft})$$

$$p_s = 0.8625\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{\left(0.762\ \mathrm{kip/ft^2}\right)}{\left(0.8625\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = 0.88348$$

Status: **PASS**
Ratio: **0.880**

Considering z-direction:

$H_o = -0.006051\ \mathrm{kip/ft}$ - Lateral force per length of pile,
$M_o = 0.021815\ \mathrm{kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (0.021815\ \mathrm{kipft/ft}) \times (5.75\ \mathrm{ft})) + \left(3 \times (-0.006051\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft})^2\right)}{(6 \times (0.021815\ \mathrm{kipft/ft})) + (4 \times (-0.006051\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))}$$

$$a = 4.0803\ \mathrm{ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\,[(4\,M_o) + (3\,H_o\,L_e)]^2}{L_e^2\,[(3\,M_o) + (2\,H_o\,L_e)]}$$

$$p = \frac{0.75 \times [(4 \times (0.021815\ \mathrm{kipft/ft})) + (3 \times (-0.006051\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))]^{\,2}}{(5.75\ \mathrm{ft})^2 \times [(3 \times (0.021815\ \mathrm{kipft/ft})) + (2 \times (-0.006051\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))]}$$

$$p = -0.0016055\ \mathrm{kip/ft^2}$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\,[(2\,M_o) + (H_o\,L_e)]}{L_e^2}$$

$$s = \frac{6 \times [(2 \times (0.021815\ \mathrm{kipft/ft})) + ((-0.006051\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))]}{(5.75\ \mathrm{ft})^2}$$

$$s = 0.0016038\ \mathrm{kip/ft^2}$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\,\frac{a}{2}$$

$$p_a = (150\ \mathrm{psf/ft}) \times \frac{(4.0803\ \mathrm{ft})}{2}$$

$$p_a = 0.30602\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{\left(-0.0016055\ \mathrm{kip/ft^2}\right)}{\left(0.30602\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = -0.0052464$$

Status: **PASS**
Ratio: **-0.010**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\,L_e$$

$$p_s = (150\ \mathrm{psf/ft}) \times (5.75\ \mathrm{ft})$$

$$p_s = 0.8625\ \mathrm{kip/ft^2}$$

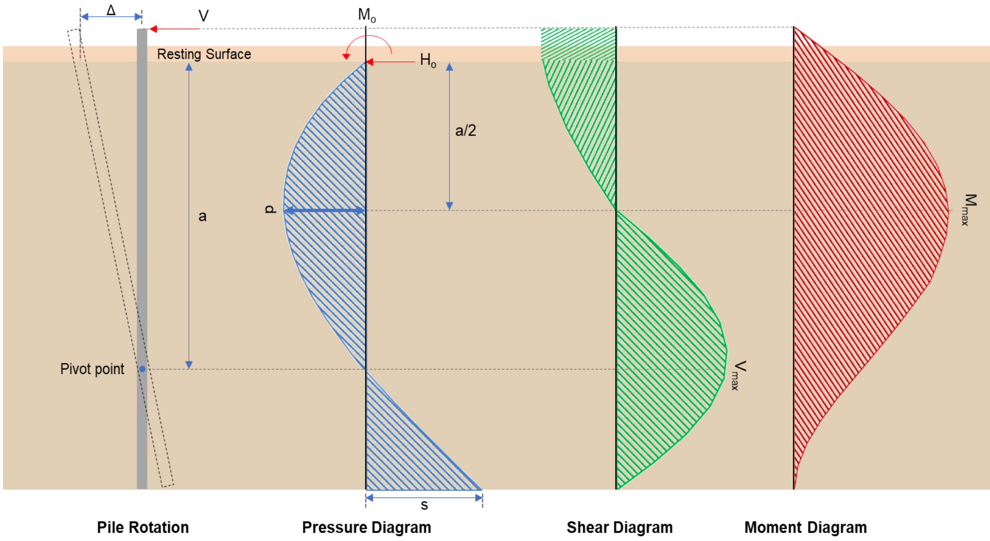
$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{(0.0016038\ \mathrm{kip/ft^2})}{(0.8625\ \mathrm{kip/ft^2})}$$

$$Ratio = 0.0018595$$

Status: **PASS**
Ratio: **0.000**



Shear force and Bending moment (x-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\,D}$$

$$H_o = \frac{(-1.85\ \mathrm{kip})}{1.57 \times (48\ \mathrm{in})}$$

$$H_o = -0.29459\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\,H)}{1.57\,D}$$

$$M_o = \frac{(28.56\ \mathrm{kipft}) + ((-1.85\ \mathrm{kip}) \times (0\ \mathrm{ft}))}{1.57 \times (48\ \mathrm{in})}$$

$$M_o = 4.5478\ \mathrm{kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(4.5478\ \mathrm{kipft/ft})}{(-0.29459\ \mathrm{kip/ft})}$$

$$E = 15.438\ \mathrm{ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (4.5478\ \mathrm{kipft/ft}) \times (5.75\ \mathrm{ft})) + \left(3 \times (-0.29459\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft})^2\right)}{}$$

$$a = \frac{}{(6 \times (4.5478\ \text{kipft/ft})) + (4 \times (-0.29459\ \text{kip/ft}) \times (5.75\ \text{ft}))}$$

$$a = 3.9286\ \text{ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\, D) \left[1 - \left[3 \left(\frac{4\,E}{L_e} + 3\right) \left(\frac{a}{L_e}\right)^2\right] + \left[4 \left(\frac{3\,E}{L_e} + 2\right) \left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((-0.29459\ \text{kip/ft}) \times (48\ \text{in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (15.438\ \text{ft})}{(5.75\ \text{ft})} + 3\right) \times \left(\frac{(3.9286\ \text{ft})}{(5.75\ \text{ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (15.438\ \text{ft})}{(5.75\ \text{ft})} + 2\right) \times \left(\frac{(3.9286\ \text{ft})}{(5.75\ \text{ft})}\right)^3\right]\right]$$

$$V_{max} = 6.3793\ \text{kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, D\, L_e) \left[\left(\frac{E}{L_e} + \frac{a}{2\,L_e}\right) - \left[\left(\frac{4\,E}{L_e} + 3\right) \left(\frac{a}{2\,L_e}\right)^3\right] + \left[\left(\frac{3\,E}{L_e} + 2\right) \left(\frac{a}{2\,L_e}\right)^4\right]\right]$$

$$M_{max} = ((-0.29459\ \text{kip/ft}) \times (48\ \text{in}) \times (5.75\ \text{ft})) \times \left[\left(\frac{(15.438\ \text{ft})}{(5.75\ \text{ft})} + \frac{(3.9286\ \text{ft})}{2 \times (5.75\ \text{ft})}\right) - \left[\left(\frac{4 \times (15.438\ \text{ft})}{(5.75\ \text{ft})} + 3\right) \times \left(\frac{(3.9286\ \text{ft})}{2 \times (5.75\ \text{ft})}\right)^3\right] + \left[\left(\frac{3 \times (15.438\ \text{ft})}{(5.75\ \text{ft})} + 2\right) \times \left(\frac{(3.9286\ \text{ft})}{2 \times (5.75\ \text{ft})}\right)^4\right]\right]$$

$$M_{max} = 17.722\ \text{kipft}$$

Shear force and Bending moment (z-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\, b}$$

$$H_o = \frac{(-0.058\ \text{kip})}{1.57 \times (48\ \text{in})}$$

$$H_o = -0.0092357\ \text{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\, H)}{1.57\, b}$$

$$M_o = \frac{(0.208\ \text{kipft}) + ((-0.058\ \text{kip}) \times (0\ \text{ft}))}{1.57 \times (48\ \text{in})}$$

$$M_o = 0.033121\ \text{kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(0.033121\ \text{kipft/ft})}{(-0.0092357\ \text{kip/ft})}$$

$$E = 3.5862\ \text{ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\, M_o\, L_e) + \left(3\, H_o\, L_e^2\right)}{(6\, M_o) + (4\, H_o\, L_e)}$$

$$a = \frac{(4 \times (0.033121\ \text{kipft/ft}) \times (5.75\ \text{ft})) + \left(3 \times (-0.0092357\ \text{kip/ft}) \times (5.75\ \text{ft})^2\right)}{(6 \times (0.033121\ \text{kipft/ft})) + (4 \times (-0.0092357\ \text{kip/ft}) \times (5.75\ \text{ft}))}$$

$$a = 4.0809\ \text{ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\, b) \left[1 - \left[3 \left(\frac{4\,E}{L_e} + 3\right) \left(\frac{a}{L_e}\right)^2\right] + \left[4 \left(\frac{3\,E}{L_e} + 2\right) \left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((-0.0092357 \text{ kip/ft}) \times (48 \text{ in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (3.5862 \text{ ft})}{(5.75 \text{ ft})} + 3\right) \times \left(\frac{(4.0809 \text{ ft})}{(5.75 \text{ ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (3.5862 \text{ ft})}{(5.75 \text{ ft})} + 2\right) \times \left(\frac{(4.0809 \text{ ft})}{(5.75 \text{ ft})}\right)^3\right]\right]$$

$$V_{max} = 0.065305 \text{ kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, b\, L_e) \left[\left(\frac{E}{L_e} + \frac{a}{2\, L_e}\right) - \left[\left(\frac{4\, E}{L_e} + 3\right) \left(\frac{a}{2\, L_e}\right)^3\right] + \left[\left(\frac{3\, E}{L_e} + 2\right) \left(\frac{a}{2\, L_e}\right)^4\right]\right]$$

$$M_{max} = ((-0.0092357 \text{ kip/ft}) \times (48 \text{ in}) \times (5.75 \text{ ft})) \times \left[\left(\frac{(3.5862 \text{ ft})}{(5.75 \text{ ft})} + \frac{(4.0809 \text{ ft})}{2 \times (5.75 \text{ ft})}\right) - \left[\left(\frac{4 \times (3.5862 \text{ ft})}{(5.75 \text{ ft})} + 3\right) \times \left(\frac{(4.0809 \text{ ft})}{2 \times (5.75 \text{ ft})}\right)^3\right] + \left[\left(\frac{3 \times (3.5862 \text{ ft})}{(5.75 \text{ ft})} + 2\right) \times \left(\frac{(4.0809 \text{ ft})}{2 \times (5.75 \text{ ft})}\right)^4\right]\right]$$

$$M_{max} = 0.16875 \text{ kipft}$$

Minimum Reinforcement Check (LRFD)

Parameters:

$f'_{ck} = 2.5 \text{ ksi}$ - Concrete strength,
$f_{yk} = 60 \text{ ksi}$ - Longitudinal reinforcement strength,
$\phi = 0.65$ - Reduction factor for axial strength,
Table 22.4.2.1 $\alpha = 0.8$ - Alpha factor for axial strength,
$A_g = 2304 \text{ in}^2$ - Gross area of concrete,

Longitudinal reinforcement:

Required reinforcement due to axial load, $A_{st,required}$
22.4.2.2, 10.6.1.1 $A_{st,required}$

$$A_{st,required} = Min\left[\frac{\frac{P}{\phi\, \alpha} - (0.85\, f'_{ck}\, A_g)}{f_{yk} - (0.85\, f'_{ck})}, (0.08\, A_g)\right]$$

$$A_{st,required} = Min\left[\frac{\frac{(11.503 \text{ kip})}{(0.65)\times(0.8)} - (0.85 \times (2.5 \text{ ksi}) \times (2304 \text{ in}^2))}{(60 \text{ ksi}) - (0.85 \times (2.5 \text{ ksi}))}, (0.08 \times (2304 \text{ in}^2))\right]$$

$$A_{st,required} = -84.214 \text{ in}^2$$

A_{min} - Governing minimum reinforcement area,

$$A_{min} = Max\left[A_{st,required}, (0.0018\, A_g)\right]$$

$$A_{min} = Max\left[(-84.214 \text{ in}^2), (0.0018 \times (2304 \text{ in}^2))\right]$$

$$A_{min} = 4.1472 \text{ in}^2$$

n_{rebar} - Required number of reinforcement,

$$n_{rebar} = \frac{A_{min}}{A_{rebar}}$$

$$n_{rebar} = \frac{(4.1472 \text{ in}^2)}{(0.3068 \text{ in}^2)}$$

$$n_{rebar} = 14$$

A_{st} - Actual total reinforcement area,

$$A_{st} = n_{rebar}\, \frac{\pi\, d_{bar}^2}{4}$$

$$A_{st} = (14) \times \frac{\pi \times (0.625 \text{ in})^2}{4}$$

$$A_{st} = 4.2951 \text{ in}^2$$

$Ratio$ - Capacity

$$Ratio = \frac{A_{min}}{A_{st}}$$

$$Ratio = \frac{(4.1472 \text{ in}^2)}{(4.2951 \text{ in}^2)}$$

$$Ratio = 0.96556$$

Status: PASS
Ratio: 0.970

25.2.3 s_{rebar} - Minimum spacing of reinforcement,

$$s_{rebar} = Max\left[1.5, \left(1.5\ d_{bar}\right)\right]$$

$$s_{rebar} = Max\left[1.5, \left(1.5 \times (0.625\ \text{in})\right)\right]$$

$$s_{rebar} = 1.5\ \text{in}$$

Ties:

25.7.2.2 Since longitudinal reinforcement is $\leq$ No. 10ø: Use #3(0.375 in)

25.7.2.1 s_{ties} - Maximum spacing of ties,

$$s_{ties} = Min\left[\left(16\ d_{bar}\right), \left(48\ d_{ties}\right), Min\left(D, b\right)\right]$$

$$s_{ties} = Min\left[\left(16 \times (0.625\ \text{in})\right), \left(48 \times (0.375\ \text{in})\right), Min\left((48\ \text{in}), (48\ \text{in})\right)\right]$$

$$s_{ties} = 10\ \text{in}$$

Summary:

Main reinforcement: **14 - #5 (0.625 in)**
Ties: **#3(0.375 in) - 10 in**

Axial Compression Strength (ACI 318-19, LRFD)

22.4.2.2 ϕP_N - Allowable axial compressive strength

$$\phi P_N = \phi\ 0.80\ \left[\left(0.85\ f'_{ck}\ \left[A_g - A_{st}\right]\right) + \left(f_{yk}\ A_{st}\right)\right]$$

$$\phi P_N = (0.65) \times 0.80 \times \left[\left(0.85 \times (2.5\ \text{ksi}) \times \left[\left(2304\ \text{in}^2\right) - \left(4.2951\ \text{in}^2\right)\right]\right) + \left((60\ \text{ksi}) \times \left(4.2951\ \text{in}^2\right)\right)\right]$$

$$\phi P_N = 2675.2\ \text{kip}$$

$Ratio$ - Capacity

$$Ratio = \frac{P}{\phi P_N}$$

$$Ratio = \frac{(11.503\ \text{kip})}{(2675.2\ \text{kip})}$$

$$Ratio = 0.0043$$

Status: PASS
Ratio: 0.000

Shear Strength (ACI 318-19, LRFD)

Parameters:

$b_w = 48\ \text{in}$ - Effective width,

22.5.2.2 d - Effective depth

$$d = 0.80\ D$$

$$d = 0.80 \times (48\ \text{in})$$

$$d = 38.4\ \text{in}$$

22.5.5.1.3 λ_s - size effect modification factor

$$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{d}{10}}}, 1\right]$$

$$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{(38.4\ \text{in})}{10}}}, 1\right]$$

$$\lambda_s = 0.64282$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5\ \text{ksi} \rightarrow 2500\ \text{psi}$,

22.5.5.1.1 $V_{c,max}$ - Max shear strength of concrete

$$V_{c,max} = 5\ \lambda_s\ \sqrt{f'_{ck}}\ b_w\ d$$

$$V_{c,max} = 5 \times (0.64282) \times \sqrt{(2500\ \text{psi})} \times (48\ \text{in}) \times (38.4\ \text{in})$$

$$V_{c,max} = 296.21 \text{ kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$, $P = 11.503 \text{ kip} \rightarrow 11503 \text{ lbf}$,

22.5.5.1.1(a) $V_{c,a}$ - Shear strength of concrete (a)

$$V_{c,a} = \left[2\, \lambda_s \, \sqrt{f'_{ck}} + \frac{P}{6\, A_g}\right]\, b_w\, d$$

$$V_{c,a} = \left[2 \times (0.64282) \times \sqrt{(2500 \text{ psi})} + \frac{(11503 \text{ lbf})}{6 \times (2304 \text{ in}^2)}\right] \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{c,a} = 120.02 \text{ kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$,

22.5.5.1.2 $V_{c,b}$ - Shear strength of concrete (b)

$$V_{c,b} = \left[2\, \lambda_s \, \sqrt{f'_{ck}} + \left(0.05\, f'_{ck}\right)\right]\, b_w\, d$$

$$V_{c,b} = \left[2 \times (0.64282) \times \sqrt{(2500 \text{ psi})} + (0.05 \times (2500 \text{ psi}))\right] \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{c,b} = 348.89 \text{ kip}$$

V_c - Governing shear strength of concrete

$$V_c = Min\left[V_{c,max}, V_{c,a}, V_{c,b}\right]$$

$$V_c = Min\left[(296.21 \text{ kip}), (120.02 \text{ kip}), (348.89 \text{ kip})\right]$$

$$V_c = 120.02 \text{ kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$,

22.5.1.2 $V_{s,a}$ - Shear strength of steel (a)

$$V_{s,a} = 8\, \sqrt{f'_{ck}}\, b_w\, d$$

$$V_{s,a} = 8 \times \sqrt{(2500 \text{ psi})} \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{s,a} = 737.28 \text{ kip}$$

A_v - Ties rebar area,

$$A_v = \frac{\pi\, d_{ties}^2}{4}$$

$$A_v = \frac{\pi \times (0.375 \text{ in})^2}{4}$$

$$A_v = 0.11045 \text{ in}^2$$

22.5.8.5.3 $V_{s,b}$ - Shear strength of steel (b)

$$V_{s,b} = \frac{2\, A_v\, f_{ywk}\, d}{s_{ties}}$$

$$V_{s,b} = \frac{2 \times \left(0.11045 \text{ in}^2\right) \times (60 \text{ ksi}) \times (38.4 \text{ in})}{(10 \text{ in})}$$

$$V_{s,b} = 50.894 \text{ kip}$$

V_s - Governing shear strength of steel

$$V_s = MIN\left[V_{s,a}, V_{s,b}\right]$$

$$V_s = MIN\left[(737.28 \text{ kip}), (50.894 \text{ kip})\right]$$

$$V_s = 50.894 \text{ kip}$$

22.5.1.1 ϕV_n - Allowable shear strength

$$\phi V_n = \phi\, \left(V_c + V_s\right)$$

$$\phi V_n = (0.65) \times ((120.02 \text{ kip}) + (50.894 \text{ kip}))$$

$$\phi V_n = 111.09 \text{ kip}$$

Considering x-direction:

$V_{max} = 6.3793 \text{ kip}$ - Maximum shear force in the x-direction,
$Ratio$ - Capacity

$$Ratio = \frac{V_{max}}{\phi V_n}$$

	$$Ratio = \frac{(6.3793\ \text{kip})}{(111.09\ \text{kip})}$$ $$Ratio = 0.057423$$	Status: **PASS** Ratio: **0.060**
	Considering z-direction: $V_{max} = 0.065305\ \text{kip}$ - Maximum shear force in the z-direction, $Ratio$ - Capacity $$Ratio = \frac{V_{max}}{\phi V_n}$$ $$Ratio = \frac{(0.065305\ \text{kip})}{(111.09\ \text{kip})}$$ $$Ratio = 0.00058784$$	Status: **PASS** Ratio: **0.000**
14.5.2.1b	**Flexural Strength (ACI 318-19, LRFD)** S_m - Section modulus $$S_m = \frac{b\,D^2}{6}$$ $$S_m = \frac{(48\ \text{in}) \times (48\ \text{in})^2}{6}$$ $$S_m = 18432\ \text{in}^3$$ $\lambda = 1$ - Concrete modification factor (Normal concrete), Allowable flexural strength: M_n shall be the lesser of: $\phi M_{n,1}$ $$\phi M_{n,1} = \phi \times 5 \times \lambda \times \sqrt{(f_c')} \times S_m$$ $$\phi M_{n,1} = 0.65 \times 5 \times 1 \times \sqrt{(2.5ksi)} \times 18432.001in^3$$ $$\phi M_{n,1} = 249.600kipft$$ $\phi M_{n,2}$ $$\phi M_{n,2} = \phi\ 0.85\ f_{ck}'\ S_m$$ $$\phi M_{n,2} = (0.65) \times 0.85 \times (2.5\ \text{ksi}) \times \left(18432\ \text{in}^3\right)$$ $$\phi M_{n,2} = 2121.6\ \text{kipft}$$ Therefore, ϕM_n - Allowable flexural strength, $$\phi M_n = MIN[\phi M_{n,1}, \phi M_{n,2}]$$ $$\phi M_n = MIN[(249.6\ \text{kipft}), (2121.6\ \text{kipft})]$$ $$\phi M_n = 249.6\ \text{kipft}$$ **Considering x-direction:** $M_{max} = 17.722\ \text{kipft}$ - Maximum moment in the x-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\phi M_n}$$ $$Ratio = \frac{(17.722\ \text{kipft})}{(249.6\ \text{kipft})}$$ $$Ratio = 0.071002$$	Status: **PASS** Ratio: **0.070**
	Considering z-direction: $M_{max} = 0.16875\ \text{kipft}$ - Maximum moment in the z-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\phi M_n}$$	

	$$Ratio = \frac{(0.16875 \text{ kipft})}{(249.6 \text{ kipft})}$$ $$Ratio = 0.00067606$$	Status: **PASS** Ratio: **0.000**

REFERENCES	CALCULATIONS	RESULTS

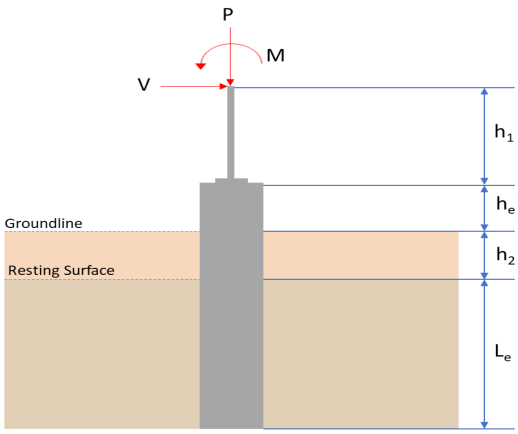
SkyCiv Foundation Design

Pile Foundation

Design Information :

Design code : IBC 2021 (International Building Code)
Unit System : Imperial

Pile Input



Geometry

Pile shape: rectangular
$b = 48\ \text{in}$ - Pile width
$D = 48\ \text{in}$ - Pile depth
$L = 5.75\ \text{ft}$ - Total pile length
$h_1 = 0\ \text{ft}$ - Lateral load height from the top of the pile,
$h_2 = 0\ \text{ft}$ - Depth to resisting surface
$h_e = 0\ \text{ft}$ - Length of pile above the ground

Tabulation of Soil Parameters

Layer	Label	Allowable Bearing Pressure (q_a) (psf)	Allowable Lateral Pressure (R) (psf/ft)
1	Sand, silty sand, clayey sand, silty gravel & clayey gravel	2000.000	150.000

Tabulation of Loads

Load Component	ASD	LRFD
P (kip)	7.932	12.050
V_x (kip)	-1.172	-1.958
V_z (kip)	0.195	0.298
M_x (kipft)	0.686	1.059
M_z (kipft)	16.582	28.735

Material Properties

$f'_{ck} = 2.5\ \text{ksi}$ - Concrete strength,

Required depth to resist lateral loads (ASD)

H - Point of application of the lateral load

$$H = h_1 + h_2 + h_e$$

$$H = (0\ \text{ft}) + (0\ \text{ft}) + (0\ \text{ft})$$

$$H = 0\ \text{ft}$$

Considering x-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\ D}$$

$$H_o = \frac{(-1.172\ \text{kip})}{1.57 \times (48\ \text{in})}$$

$$H_o = -0.18662\ \text{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\, H)}{1.57\, D}$$

$$M_o = \frac{(16.582 \text{ kipft}) + ((-1.172 \text{ kip}) \times (0 \text{ ft}))}{1.57 \times (48 \text{ in})}$$

$$M_o = 2.6404 \text{ kipft/ft}$$

Required depth of embedment in earth:

$$L_x^3 - \left(14.14 \times \frac{H_o \times L_x}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,x} = 5.3314 \text{ ft}$ - Required depth in x-direction,

Considering z-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\, b}$$

$$H_o = \frac{(0.195 \text{ kip})}{1.57 \times (48 \text{ in})}$$

$$H_o = 0.031051 \text{ kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\, H)}{1.57\, b}$$

$$M_o = \frac{(0.686 \text{ kipft}) + ((0.195 \text{ kip}) \times (0 \text{ ft}))}{1.57 \times (48 \text{ in})}$$

$$M_o = 0.10924 \text{ kipft/ft}$$

Required depth of embedment in earth:

$$L_z^3 - \left(14.14 \times \frac{H_o \times L_z}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,z} = 2.3594 \text{ ft}$ - Required depth in z-direction,

Minimum embedded depth required:

$L_{e,req}$ - Depth of pile required,

$$L_{e,req} = MAX[L_{e,x}, L_{e,z}]$$

$$L_{e,req} = MAX[(5.3314 \text{ ft}), (2.3594 \text{ ft})]$$

$$L_{e,req} = 5.331 \text{ ft}$$

L_e - Actual embedded length of pile,

$$L_e = L - h_e - h_2$$

$$L_e = (5.75 \text{ ft}) - (0 \text{ ft}) - (0 \text{ ft})$$

$$L_e = 5.75 \text{ ft}$$

$Ratio$ - Embedded depth

$$Ratio = \frac{L_{e,req}}{L_e}$$

$$Ratio = \frac{(5.331 \text{ ft})}{(5.75 \text{ ft})}$$

$$Ratio = 0.92713$$

Status: **PASS**
Ratio: **0.930**

End-bearing Capacity (ASD)

A - Pile cross-section area

$$A = b\, D$$

$$A = (48 \text{ in}) \times (48 \text{ in})$$

$$A = 16 \text{ ft}^2$$

q - End-bearing pressure

$$q = \frac{P_v}{A}$$

$$q = \frac{(7.932\ \mathrm{kip})}{(16\ \mathrm{ft}^2)}$$

$$q = 0.49575\ \mathrm{kip/ft}^2$$

Check bearing capacity ratio:

$Ratio$ - Capacity

$$Ratio = \frac{q}{q_a}$$

$$Ratio = \frac{\left(0.49575\ \mathrm{kip/ft}^2\right)}{(2000\ \mathrm{psf})}$$

$$Ratio = 0.24787$$

Status: **PASS**
Ratio: **0.250**

Czerniak

Lateral Soil Pressure (ASD):

L/D - Length to least lateral dimension ratio,

$$L/D = \frac{L}{D}$$

$$L/D = \frac{(5.75\ \mathrm{ft})}{(48\ \mathrm{in})}$$

$$L/D = 1.4375$$

Since L/D $\leq$ 10,

Pile is short.

Considering x-direction:

$H_o = -0.18662\ \mathrm{kip/ft}$ - Lateral force per length of pile,
$M_o = 2.6404\ \mathrm{kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (2.6404\ \mathrm{kipft/ft}) \times (5.75\ \mathrm{ft})) + \left(3 \times (-0.18662\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft})^2\right)}{(6 \times (2.6404\ \mathrm{kipft/ft})) + (4 \times (-0.18662\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))}$$

$$a = 3.9355\ \mathrm{ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\,[(4\,M_o) + (3\,H_o\,L_e)]^2}{L_e^2\,[(3\,M_o) + (2\,H_o\,L_e)]}$$

$$p = \frac{0.75 \times [(4 \times (2.6404\ \mathrm{kipft/ft})) + (3 \times (-0.18662\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))]^2}{(5.75\ \mathrm{ft})^2 \times [(3 \times (2.6404\ \mathrm{kipft/ft})) + (2 \times (-0.18662\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))]}$$

$$p = 0.21176\ \mathrm{kip/ft}^2$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\,[(2\,M_o) + (H_o\,L_e)]}{L_e^2}$$

$$s = \frac{6 \times [(2 \times (2.6404\ \mathrm{kipft/ft})) + ((-0.18662\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))]}{(5.75\ \mathrm{ft})^2}$$

$$s = 0.76361\ \mathrm{kip/ft}^2$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\,\frac{a}{2}$$

$$p_a = (150\ \mathrm{psf/ft}) \times \frac{(3.9355\ \mathrm{ft})}{2}$$

$$p_a = 0.29516\ \mathrm{kip/ft}^2$$

$Ratio$ - Lateral soil capacity

	$$Ratio = \frac{p}{p_a}$$ $$Ratio = \frac{(0.21176\ \mathrm{kip/ft^2})}{(0.29516\ \mathrm{kip/ft^2})}$$ $$Ratio = 0.71745$$	Status: **PASS** Ratio: **0.720**
	p_s - Allowable lateral soil pressure at depth L_e, $$p_s = R\, L_e$$ $$p_s = (150\ \mathrm{psf/ft}) \times (5.75\ \mathrm{ft})$$ $$p_s = 0.8625\ \mathrm{kip/ft^2}$$ $Ratio$ - Lateral soil capacity $$Ratio = \frac{s}{p_s}$$ $$Ratio = \frac{(0.76361\ \mathrm{kip/ft^2})}{(0.8625\ \mathrm{kip/ft^2})}$$ $$Ratio = 0.88534$$	Status: **PASS** Ratio: **0.890**

Considering z-direction:

$H_o = 0.031051\ \mathrm{kip/ft}$ - Lateral force per length of pile,
$M_o = 0.10924\ \mathrm{kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\, M_o\, L_e) + \left(3\, H_o\, L_e^2\right)}{(6\, M_o) + (4\, H_o\, L_e)}$$

$$a = \frac{(4 \times (0.10924\ \mathrm{kipft/ft}) \times (5.75\ \mathrm{ft})) + \left(3 \times (0.031051\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft})^2\right)}{(6 \times (0.10924\ \mathrm{kipft/ft})) + (4 \times (0.031051\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))}$$

$$a = 4.0832\ \mathrm{ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\,[(4\, M_o) + (3\, H_o\, L_e)]^2}{L_e^2\,[(3\, M_o) + (2\, H_o\, L_e)]}$$

$$p = \frac{0.75 \times [(4 \times (0.10924\ \mathrm{kipft/ft})) + (3 \times (0.031051\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))]^{\ 2}}{(5.75\ \mathrm{ft})^2 \times [(3 \times (0.10924\ \mathrm{kipft/ft})) + (2 \times (0.031051\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))]}$$

$$p = 0.031334\ \mathrm{kip/ft^2}$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\,[(2\, M_o) + (H_o\, L_e)]}{L_e^2}$$

$$s = \frac{6 \times [(2 \times (0.10924\ \mathrm{kipft/ft})) + ((0.031051\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))]}{(5.75\ \mathrm{ft})^2}$$

$$s = 0.072048\ \mathrm{kip/ft^2}$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\,\frac{a}{2}$$

$$p_a = (150\ \mathrm{psf/ft}) \times \frac{(4.0832\ \mathrm{ft})}{2}$$

$$p_a = 0.30624\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{(0.031334\ \mathrm{kip/ft^2})}{(0.30624\ \mathrm{kip/ft^2})}$$

$$Ratio = 0.10232$$

Status: **PASS**
Ratio: **0.100**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\, L_e$$

$$p_s = (150\ \mathrm{psf/ft}) \times (5.75\ \mathrm{ft})$$

$$p_s = 0.8625\ \mathrm{kip/ft^2}$$

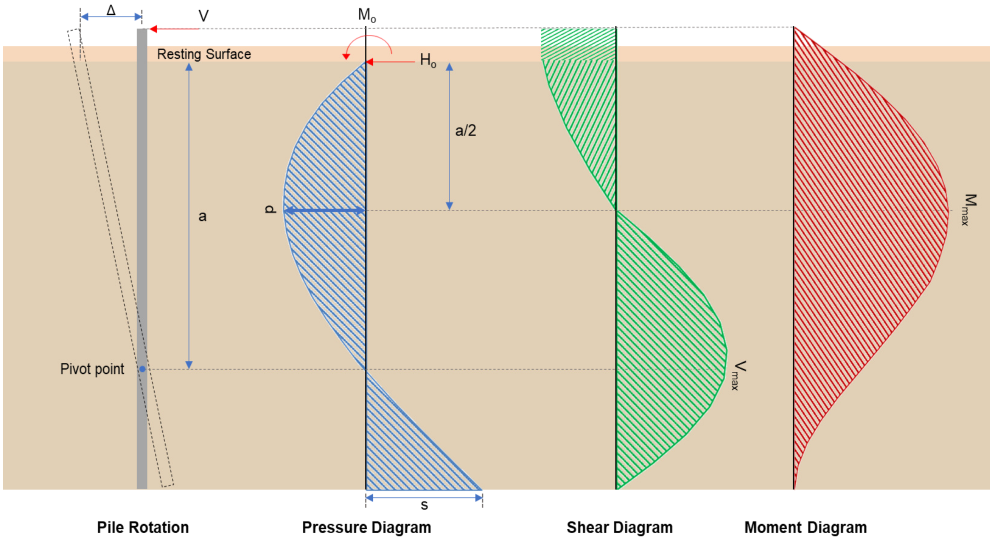
$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{\left(0.072048\ \mathrm{kip/ft^2}\right)}{\left(0.8625\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = 0.083534$$

Status: **PASS**
Ratio: **0.080**



Shear force and Bending moment (x-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\, D}$$

$$H_o = \frac{(-1.958\ \mathrm{kip})}{1.57 \times (48\ \mathrm{in})}$$

$$H_o = -0.31178\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\, H)}{1.57\, D}$$

$$M_o = \frac{(28.735\ \mathrm{kipft}) + ((-1.958\ \mathrm{kip}) \times (0\ \mathrm{ft}))}{1.57 \times (48\ \mathrm{in})}$$

$$M_o = 4.5756\ \mathrm{kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(4.5756\ \mathrm{kipft/ft})}{(-0.31178\ \mathrm{kip/ft})}$$

$$E = 14.676\ \mathrm{ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\, M_o\, L_e) + \left(3\, H_o\, L_e^2\right)}{(6\, M_o) + (4\, H_o\, L_e)}$$

$$a = \frac{(4 \times (4.5756\ \mathrm{kipft/ft}) \times (5.75\ \mathrm{ft})) + \left(3 \times (-0.31178\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft})^2\right)}{\ldots}$$

$$a = \frac{}{(6 \times (4.5756\ \mathrm{kipft/ft})) + (4 \times (-0.31178\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft})}$$

$$a = 3.9326\ \mathrm{ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\, D) \left[1 - \left[3 \left(\frac{4\, E}{L_e} + 3\right) \left(\frac{a}{L_e}\right)^2\right] + \left[4 \left(\frac{3\, E}{L_e} + 2\right) \left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((-0.31178\ \mathrm{kip/ft}) \times (48\ \mathrm{in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (14.676\ \mathrm{ft})}{(5.75\ \mathrm{ft})} + 3\right) \times \left(\frac{(3.9326\ \mathrm{ft})}{(5.75\ \mathrm{ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (14.676\ \mathrm{ft})}{(5.75\ \mathrm{ft})} + 2\right) \times \left(\frac{(3.9326\ \mathrm{ft})}{(5.75\ \mathrm{ft})}\right)^3\right]\right]$$

$$V_{max} = 6.4585\ \mathrm{kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, D\, L_e) \left[\left(\frac{E}{L_e} + \frac{a}{2\, L_e}\right) - \left[\left(\frac{4\, E}{L_e} + 3\right) \left(\frac{a}{2\, L_e}\right)^3\right] + \left[\left(\frac{3\, E}{L_e} + 2\right) \left(\frac{a}{2\, L_e}\right)^4\right]\right]$$

$$M_{max} = ((-0.31178\ \mathrm{kip/ft}) \times (48\ \mathrm{in}) \times (5.75\ \mathrm{ft})) \times \left[\left(\frac{(14.676\ \mathrm{ft})}{(5.75\ \mathrm{ft})} + \frac{(3.9326\ \mathrm{ft})}{2 \times (5.75\ \mathrm{ft})}\right) - \left[\left(\frac{4 \times (14.676\ \mathrm{ft})}{(5.75\ \mathrm{ft})} + 3\right) \times \left(\frac{(3.9326\ \mathrm{ft})}{2 \times (5.75\ \mathrm{ft})}\right)^3\right] + \left[\left(\frac{3 \times (14.676\ \mathrm{ft})}{(5.75\ \mathrm{ft})} + 2\right) \times \left(\frac{(3.9326\ \mathrm{ft})}{2 \times (5.75\ \mathrm{ft})}\right)^4\right]\right]$$

$$M_{max} = 17.914\ \mathrm{kipft}$$

Shear force and Bending moment (z-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\, b}$$

$$H_o = \frac{(0.298\ \mathrm{kip})}{1.57 \times (48\ \mathrm{in})}$$

$$H_o = 0.047452\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\, H)}{1.57\, b}$$

$$M_o = \frac{(1.059\ \mathrm{kipft}) + ((0.298\ \mathrm{kip}) \times (0\ \mathrm{ft}))}{1.57 \times (48\ \mathrm{in})}$$

$$M_o = 0.16863\ \mathrm{kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(0.16863\ \mathrm{kipft/ft})}{(0.047452\ \mathrm{kip/ft})}$$

$$E = 3.5537\ \mathrm{ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\, M_o\, L_e) + \left(3\, H_o\, L_e^2\right)}{(6\, M_o) + (4\, H_o\, L_e)}$$

$$a = \frac{(4 \times (0.16863\ \mathrm{kipft/ft}) \times (5.75\ \mathrm{ft})) + \left(3 \times (0.047452\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft})^2\right)}{(6 \times (0.16863\ \mathrm{kipft/ft})) + (4 \times (0.047452\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))}$$

$$a = 4.082\ \mathrm{ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\, b) \left[1 - \left[3 \left(\frac{4\, E}{L_e} + 3\right) \left(\frac{a}{L_e}\right)^2\right] + \left[4 \left(\frac{3\, E}{L_e} + 2\right) \left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((0.047452 \text{ kip/ft}) \times (48 \text{ in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (3.5537 \text{ ft})}{(5.75 \text{ ft})} + 3\right) \times \left(\frac{(4.082 \text{ ft})}{(5.75 \text{ ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (3.5537 \text{ ft})}{(5.75 \text{ ft})} + 2\right) \times \left(\frac{(4.082 \text{ ft})}{(5.75 \text{ ft})}\right)^3\right]\right]$$

$$V_{max} = 0.33365 \text{ kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, b\, L_e) \left[\left(\frac{E}{L_e} + \frac{a}{2\, L_e}\right) - \left[\left(\frac{4\, E}{L_e} + 3\right) \left(\frac{a}{2\, L_e}\right)^3\right] + \left[\left(\frac{3\, E}{L_e} + 2\right) \left(\frac{a}{2\, L_e}\right)^4\right]\right]$$

$$M_{max} = ((0.047452 \text{ kip/ft}) \times (48 \text{ in}) \times (5.75 \text{ ft})) \times \left[\left(\frac{(3.5537 \text{ ft})}{(5.75 \text{ ft})} + \frac{(4.082 \text{ ft})}{2 \times (5.75 \text{ ft})}\right) - \left[\left(\frac{4 \times (3.5537 \text{ ft})}{(5.75 \text{ ft})} + 3\right) \times \left(\frac{(4.082 \text{ ft})}{2 \times (5.75 \text{ ft})}\right)^3\right] + \left[\left(\frac{3 \times (3.5537 \text{ ft})}{(5.75 \text{ ft})} + 2\right) \times \left(\frac{(4.082 \text{ ft})}{2 \times (5.75 \text{ ft})}\right)^4\right]\right]$$

$$M_{max} = 0.8616 \text{ kipft}$$

Minimum Reinforcement Check (LRFD)

Parameters:

$f'_{ck} = 2.5 \text{ ksi}$ - Concrete strength,
$f_{yk} = 60 \text{ ksi}$ - Longitudinal reinforcement strength,
$\phi = 0.65$ - Reduction factor for axial strength,
Table 22.4.2.1 $\alpha = 0.8$ - Alpha factor for axial strength,
$A_g = 2304 \text{ in}^2$ - Gross area of concrete,

Longitudinal reinforcement:

Required reinforcement due to axial load, $A_{st,required}$
22.4.2.2, 10.6.1.1 $A_{st,required}$

$$A_{st,required} = Min\left[\frac{\frac{P}{\phi\, \alpha} - (0.85\, f'_{ck}\, A_g)}{f_{yk} - (0.85\, f'_{ck})}, (0.08\, A_g)\right]$$

$$A_{st,required} = Min\left[\frac{\frac{(12.05 \text{ kip})}{(0.65) \times (0.8)} - (0.85 \times (2.5 \text{ ksi}) \times (2304 \text{ in}^2))}{(60 \text{ ksi}) - (0.85 \times (2.5 \text{ ksi}))}, (0.08 \times (2304 \text{ in}^2))\right]$$

$$A_{st,required} = -84.196 \text{ in}^2$$

A_{min} - Governing minimum reinforcement area,

$$A_{min} = Max\left[A_{st,required}, (0.0018\, A_g)\right]$$

$$A_{min} = Max\left[(-84.196 \text{ in}^2), (0.0018 \times (2304 \text{ in}^2))\right]$$

$$A_{min} = 4.1472 \text{ in}^2$$

n_{rebar} - Required number of reinforcement,

$$n_{rebar} = \frac{A_{min}}{A_{rebar}}$$

$$n_{rebar} = \frac{(4.1472 \text{ in}^2)}{(0.3068 \text{ in}^2)}$$

$$n_{rebar} = 14$$

A_{st} - Actual total reinforcement area,

$$A_{st} = n_{rebar}\, \frac{\pi\, d_{bar}^2}{4}$$

$$A_{st} = (14) \times \frac{\pi \times (0.625 \text{ in})^2}{4}$$

$$A_{st} = 4.2951 \text{ in}^2$$

$Ratio$ - Capacity

$$Ratio = \frac{A_{min}}{A_{st}}$$

$$Ratio = \frac{(4.1472 \text{ in}^2)}{(4.2951 \text{ in}^2)}$$

$$Ratio = 0.96556$$

Status: PASS
Ratio: 0.970

25.2.3 s_{rebar} - Minimum spacing of reinforcement,

$$s_{rebar} = Max\left[1.5, \left(1.5\, d_{bar}\right)\right]$$

$$s_{rebar} = Max\left[1.5, \left(1.5 \times (0.625\text{ in})\right)\right]$$

$$s_{rebar} = 1.5\text{ in}$$

Ties:

25.7.2.2 Since longitudinal reinforcement is $\leq$ No. 10ø: Use #3(0.375 in)

25.7.2.1 s_{ties} - Maximum spacing of ties,

$$s_{ties} = Min\left[\left(16\, d_{bar}\right), \left(48\, d_{ties}\right), Min\left(D, b\right)\right]$$

$$s_{ties} = Min\left[\left(16 \times (0.625\text{ in})\right), \left(48 \times (0.375\text{ in})\right), Min\left((48\text{ in}), (48\text{ in})\right)\right]$$

$$s_{ties} = 10\text{ in}$$

Summary:

Main reinforcement: **14 - #5 (0.625 in)**
Ties: **#3(0.375 in) - 10 in**

Axial Compression Strength (ACI 318-19, LRFD)

22.4.2.2 ϕP_N - Allowable axial compressive strength

$$\phi P_N = \phi\, 0.80\, \left[\left(0.85\, f'_{ck}\, \left[A_g - A_{st}\right]\right) + \left(f_{yk}\, A_{st}\right)\right]$$

$$\phi P_N = (0.65) \times 0.80 \times \left[\left(0.85 \times (2.5\text{ ksi}) \times \left[\left(2304\text{ in}^2\right) - \left(4.2951\text{ in}^2\right)\right]\right) + \left((60\text{ ksi}) \times \left(4.2951\text{ in}^2\right)\right)\right]$$

$$\phi P_N = 2675.2\text{ kip}$$

$Ratio$ - Capacity

$$Ratio = \frac{P}{\phi P_N}$$

$$Ratio = \frac{(12.05\text{ kip})}{(2675.2\text{ kip})}$$

$$Ratio = 0.0045044$$

Status: PASS
Ratio: 0.000

Shear Strength (ACI 318-19, LRFD)

Parameters:

$b_w = 48\text{ in}$ - Effective width,

22.5.2.2 d - Effective depth

$$d = 0.80\, D$$

$$d = 0.80 \times (48\text{ in})$$

$$d = 38.4\text{ in}$$

22.5.5.1.3 λ_s - size effect modification factor

$$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{d}{10}}}, 1\right]$$

$$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{(38.4\text{ in})}{10}}}, 1\right]$$

$$\lambda_s = 0.64282$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5\text{ ksi} \rightarrow 2500\text{ psi}$,

22.5.5.1.1 $V_{c,max}$ - Max shear strength of concrete

$$V_{c,max} = 5\, \lambda_s\, \sqrt{f'_{ck}}\, b_w\, d$$

$$V_{c,max} = 5 \times (0.64282) \times \sqrt{(2500\text{ psi})} \times (48\text{ in}) \times (38.4\text{ in})$$

$$V_{c,max} = 296.21 \text{ kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$, $P = 12.05 \text{ kip} \rightarrow 12050 \text{ lbf}$,

22.5.5.1.1(a) $V_{c,a}$ - Shear strength of concrete (a)

$$V_{c,a} = \left[2\,\lambda_s\,\sqrt{f'_{ck}} + \frac{P}{6\,A_g}\right]\,b_w\,d$$

$$V_{c,a} = \left[2 \times (0.64282) \times \sqrt{(2500 \text{ psi})} + \frac{(12050 \text{ lbf})}{6 \times (2304 \text{ in}^2)}\right] \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{c,a} = 120.09 \text{ kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$,

22.5.5.1.2 $V_{c,b}$ - Shear strength of concrete (b)

$$V_{c,b} = \left[2\,\lambda_s\,\sqrt{f'_{ck}} + \left(0.05\,f'_{ck}\right)\right]\,b_w\,d$$

$$V_{c,b} = \left[2 \times (0.64282) \times \sqrt{(2500 \text{ psi})} + (0.05 \times (2500 \text{ psi}))\right] \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{c,b} = 348.89 \text{ kip}$$

V_c - Governing shear strength of concrete

$$V_c = Min\left[V_{c,max}, V_{c,a}, V_{c,b}\right]$$

$$V_c = Min\left[(296.21 \text{ kip}), (120.09 \text{ kip}), (348.89 \text{ kip})\right]$$

$$V_c = 120.09 \text{ kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$,

22.5.1.2 $V_{s,a}$ - Shear strength of steel (a)

$$V_{s,a} = 8\,\sqrt{f'_{ck}}\,b_w\,d$$

$$V_{s,a} = 8 \times \sqrt{(2500 \text{ psi})} \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{s,a} = 737.28 \text{ kip}$$

A_v - Ties rebar area,

$$A_v = \frac{\pi\,d_{ties}^2}{4}$$

$$A_v = \frac{\pi \times (0.375 \text{ in})^2}{4}$$

$$A_v = 0.11045 \text{ in}^2$$

22.5.8.5.3 $V_{s,b}$ - Shear strength of steel (b)

$$V_{s,b} = \frac{2\,A_v\,f_{ywk}\,d}{s_{ties}}$$

$$V_{s,b} = \frac{2 \times \left(0.11045 \text{ in}^2\right) \times (60 \text{ ksi}) \times (38.4 \text{ in})}{(10 \text{ in})}$$

$$V_{s,b} = 50.894 \text{ kip}$$

V_s - Governing shear strength of steel

$$V_s = MIN\left[V_{s,a}, V_{s,b}\right]$$

$$V_s = MIN\left[(737.28 \text{ kip}), (50.894 \text{ kip})\right]$$

$$V_s = 50.894 \text{ kip}$$

22.5.1.1 ϕV_n - Allowable shear strength

$$\phi V_n = \phi\,\left(V_c + V_s\right)$$

$$\phi V_n = (0.65) \times ((120.09 \text{ kip}) + (50.894 \text{ kip}))$$

$$\phi V_n = 111.14 \text{ kip}$$

Considering x-direction:

$V_{max} = 6.4585 \text{ kip}$ - Maximum shear force in the x-direction,
$Ratio$ - Capacity

$$Ratio = \frac{V_{max}}{\phi V_n}$$

	$$Ratio = \frac{(6.4585 \text{ kip})}{(111.14 \text{ kip})}$$ $$Ratio = 0.058111$$	Status: **PASS** Ratio: **0.060**
	Considering z-direction: $V_{max} = 0.33365 \text{ kip}$ - Maximum shear force in the z-direction, $Ratio$ - Capacity $$Ratio = \frac{V_{max}}{\phi V_n}$$ $$Ratio = \frac{(0.33365 \text{ kip})}{(111.14 \text{ kip})}$$ $$Ratio = 0.003002$$	Status: **PASS** Ratio: **0.000**
14.5.2.1b	**Flexural Strength (ACI 318-19, LRFD)** S_m - Section modulus $$S_m = \frac{b\, D^2}{6}$$ $$S_m = \frac{(48 \text{ in}) \times (48 \text{ in})^2}{6}$$ $$S_m = 18432 \text{ in}^3$$ $\lambda = 1$ - Concrete modification factor (Normal concrete), Allowable flexural strength: M_n shall be the lesser of: $\phi M_{n,1}$ $$\phi M_{n,1} = \phi \times 5 \times \lambda \times \sqrt{(f_c')} \times S_m$$ $$\phi M_{n,1} = 0.65 \times 5 \times 1 \times \sqrt{(2.5 ksi)} \times 18432.001 in^3$$ $$\phi M_{n,1} = 249.600 kipft$$ $\phi M_{n,2}$ $$\phi M_{n,2} = \phi\, 0.85\, f_{ck}'\, S_m$$ $$\phi M_{n,2} = (0.65) \times 0.85 \times (2.5 \text{ ksi}) \times \left(18432 \text{ in}^3\right)$$ $$\phi M_{n,2} = 2121.6 \text{ kipft}$$ Therefore, ϕM_n - Allowable flexural strength, $$\phi M_n = MIN[\phi M_{n,1}, \phi M_{n,2}]$$ $$\phi M_n = MIN[(249.6 \text{ kipft}), (2121.6 \text{ kipft})]$$ $$\phi M_n = 249.6 \text{ kipft}$$ **Considering x-direction:** $M_{max} = 17.914 \text{ kipft}$ - Maximum moment in the x-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\phi M_n}$$ $$Ratio = \frac{(17.914 \text{ kipft})}{(249.6 \text{ kipft})}$$ $$Ratio = 0.07177$$	Status: **PASS** Ratio: **0.070**
	Considering z-direction: $M_{max} = 0.8616 \text{ kipft}$ - Maximum moment in the z-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\phi M_n}$$	

	$$Ratio = \frac{(0.8616\ \mathrm{kipft})}{(249.6\ \mathrm{kipft})}$$ $$Ratio = 0.0034519$$	Status: **PASS** Ratio: **0.000**