Your Project Calculations

Project Name: BusStationGarage-JB-RevC

S3D Model Link:
https://platform.skyciv.com/structural?preload_name=BusStationGarage-JB-RevC&preload_path=Shared%20Enterprise%20Folder/MT_Solar_Projects/8_2023

Public Model Link:
https://platform.skyciv.com/structural-viewer?project_id=YTiX815BmB3ZfcSwLokYf8ipxF3mHIolE65EihozduaZXWlI5GAZwiuPQaBp12Zw

Array Specification

Product:	Beam
Unique ID:	4P-22.5-6TOP-XD-72-L-5Hx15W-CGBB
Duty Classification:	XD
Module Width:	41.20 in
Module Length:	69.37in
Number of Rows:	5
Number of Columns:	15
Total Number of Modules:	75
Desired Tilt Angle:	5
Front Edge Clearance:	10
Total Array Height at Tilt:	11.51 ft
Total Frame Length:	87.00 ft
Frame Weight:	3996 lbs
Array Dimensions N/S:	17.38 ft
Array Dimensions E/W:	87.96 ft
Rail Length:	208.50 in
Rail Spacing:	2.89 ft
Rail Check:	Not Checked

Support Specifications

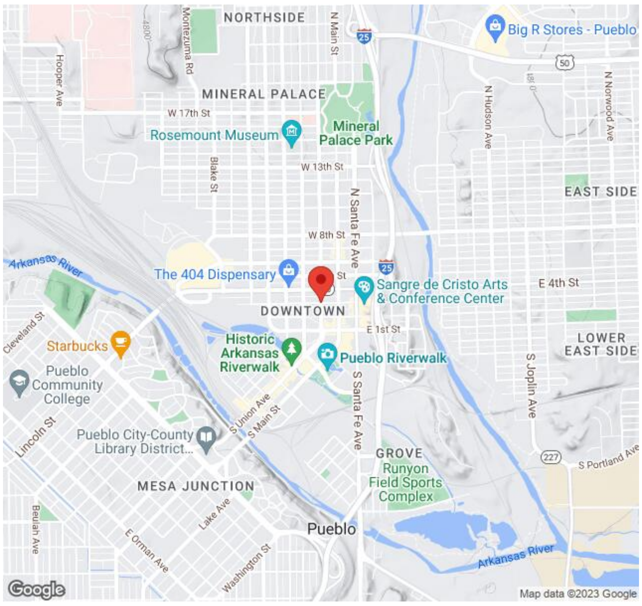
Pole Size:	6in Pipe Sch 40
Pole Length above Grade:	10.76 ft
Number of Poles:	4
Pole Spacing:	22.5 ft

Foundation Specifications

Foundation Type:	Square
Foundation Dimensions:	48 x 48 in
Foundation Depth (below grade):	Pile 1: 4.25 ft Pile 2: 4.25 ft Pile 3: 4.25 ft Pile 4: 4.25 ft
Foundation Volume:	10.074 y^3
Foundation Result:	PASSED
Mount Twist:	0.026447 kip

Site Info

Risk Category:	I
Exposure:	B
Soil Classification:	sand
Site Location:	300 W 3rd St, Pueblo, CO 81003, USA
Wind Speed:	100 mph
Snow Load:	30 psf
Design Uplift Pressure:	Multiple pressures
Design Downforce Pressure:	Multiple pressures
Design Snow Pressure:	0.018144 ksf



AutoDesigner Input

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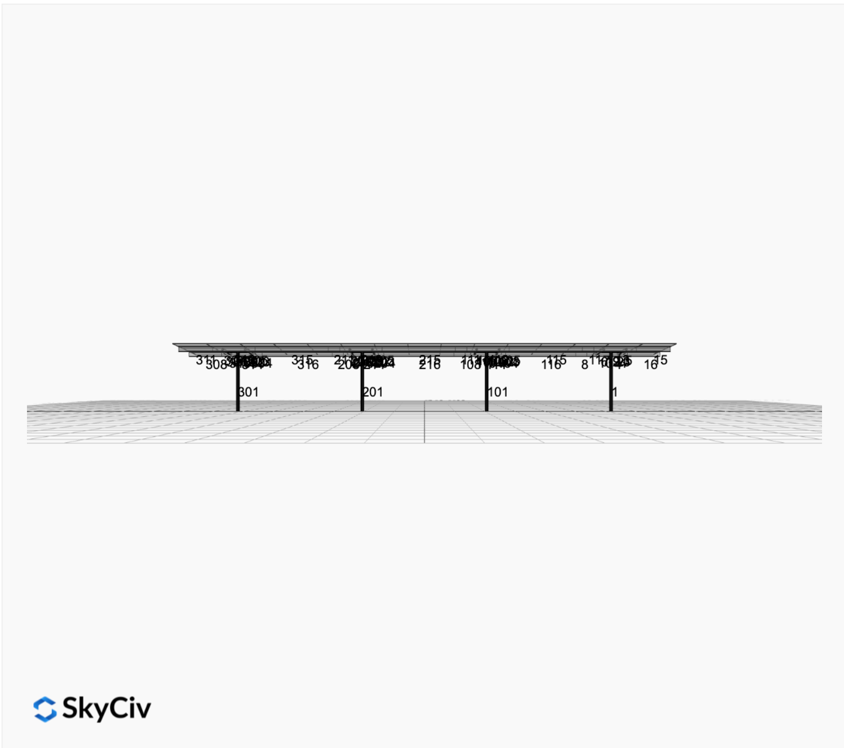
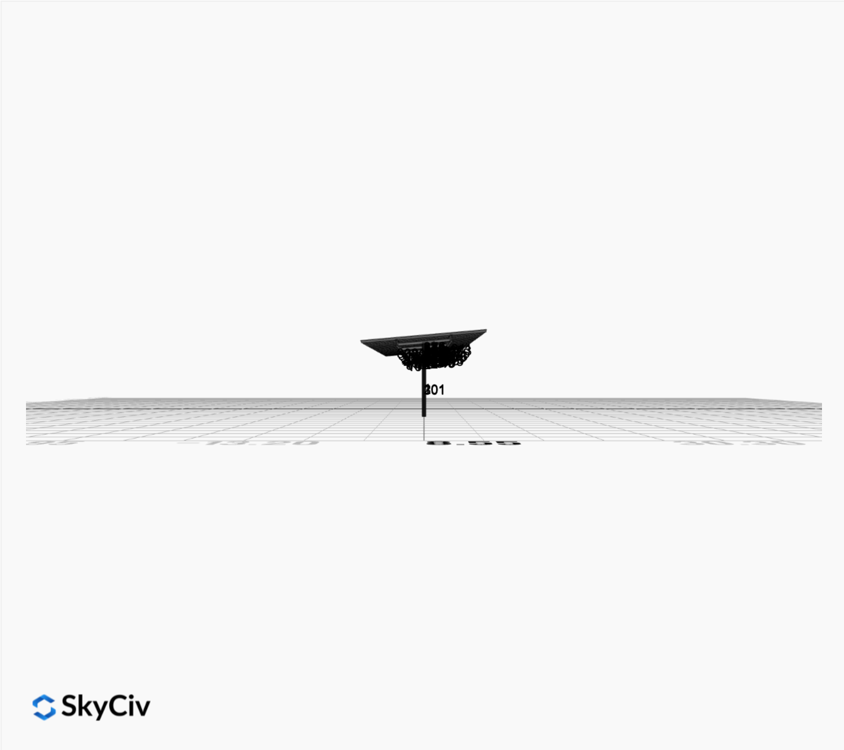
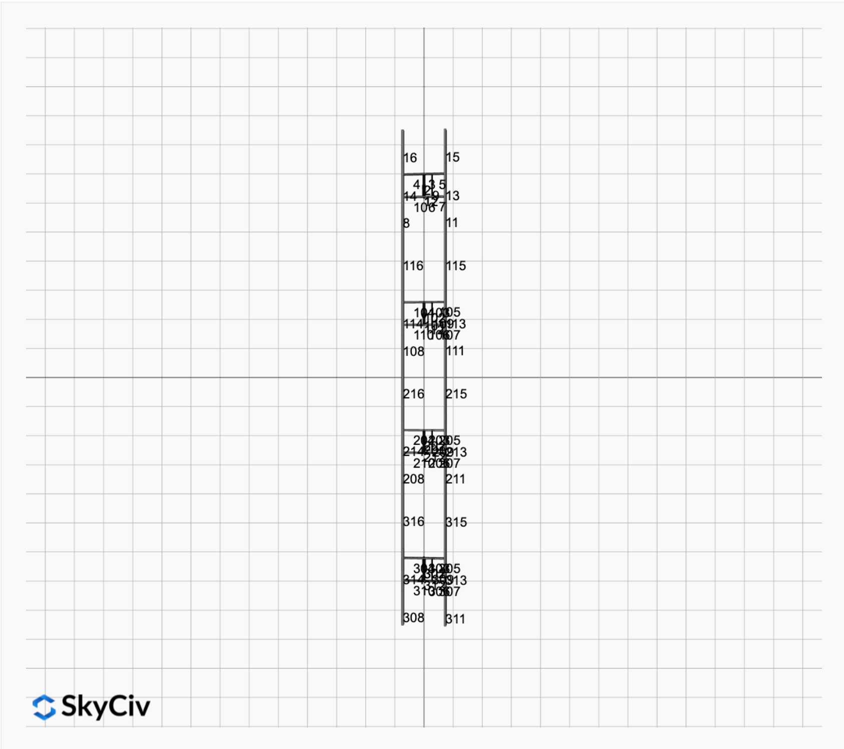
Design Notes:

- AISC Deflection checks are set to L/1 due to structure design intent
- Foundation Design and Sizing is approximate only



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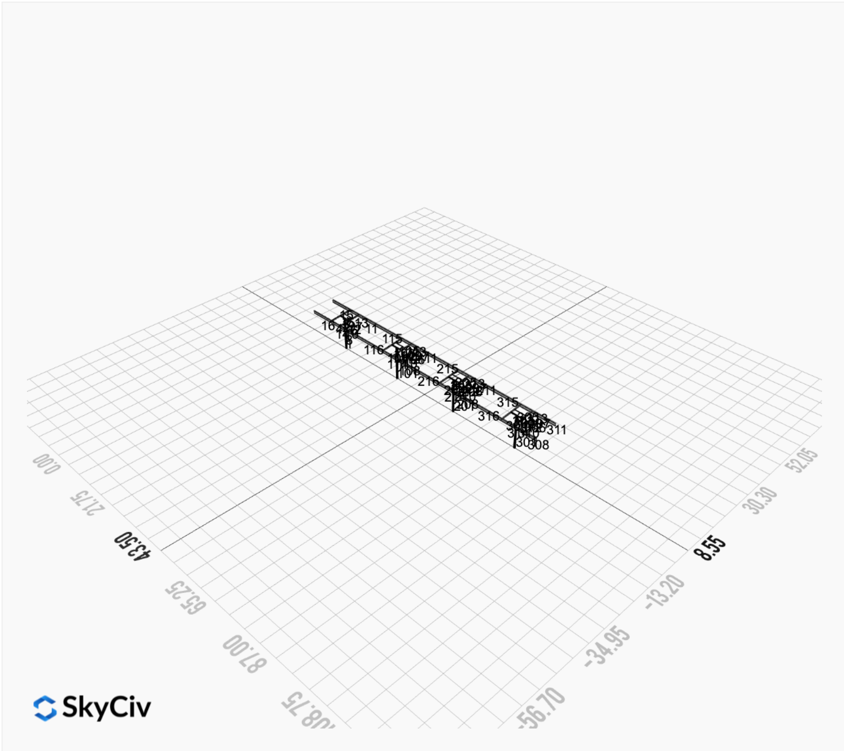
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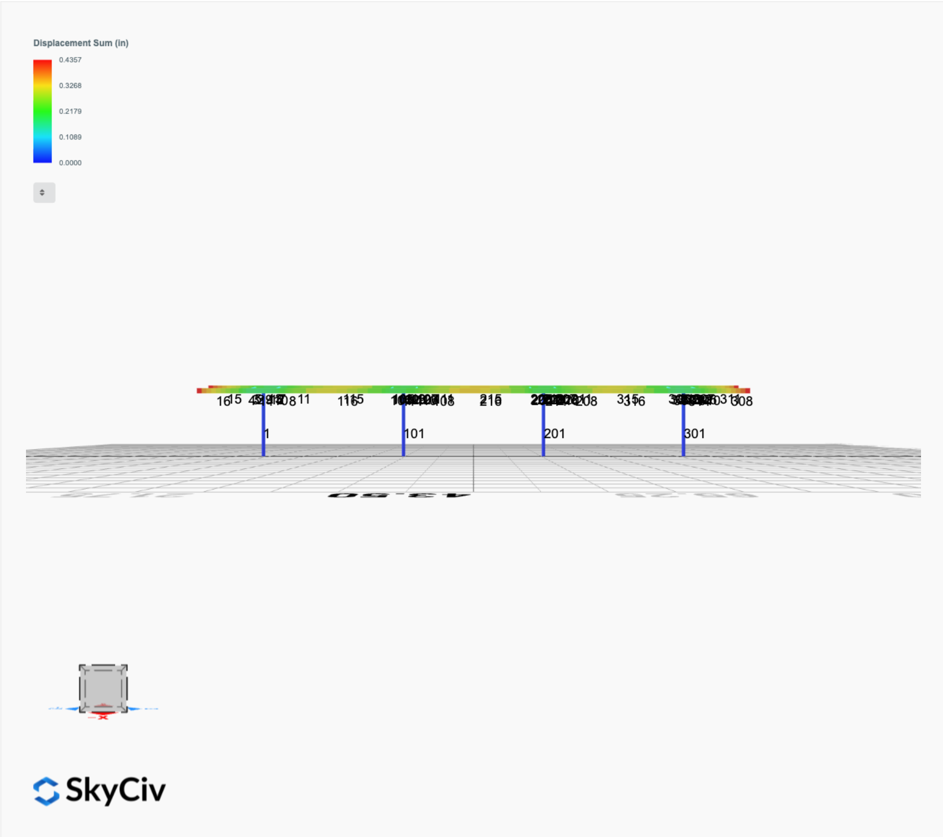
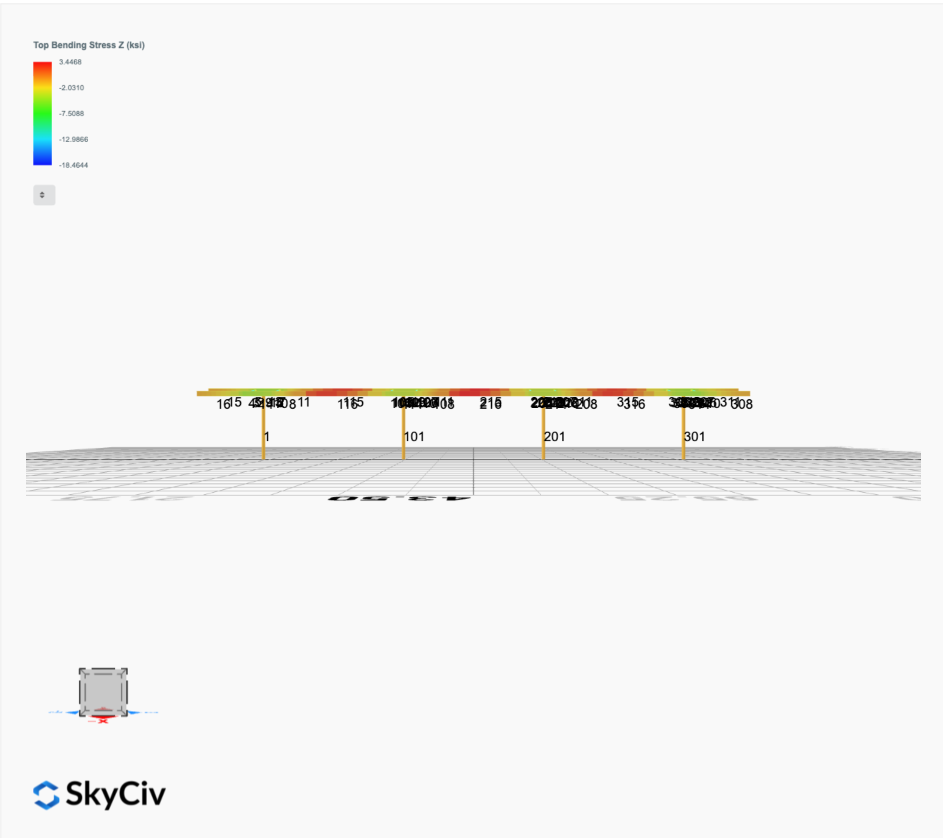
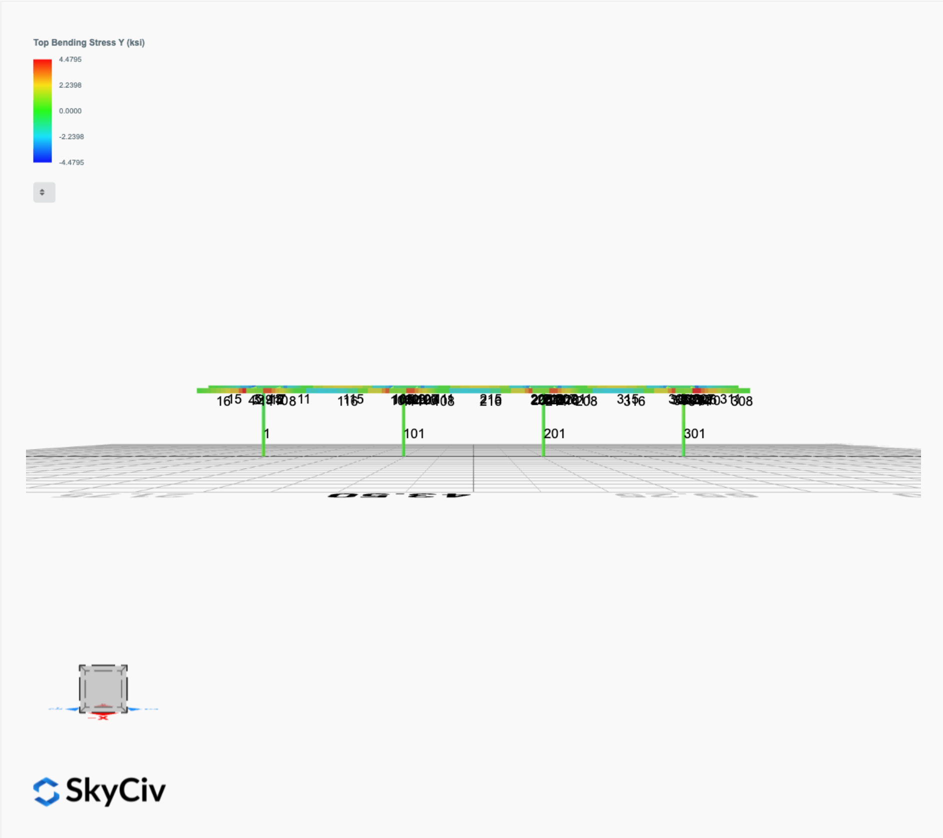
21.75
43.50
65.25
87.00
52.05
30.30
8.55
-13.20
-34.95
-56.70
SkyCiv

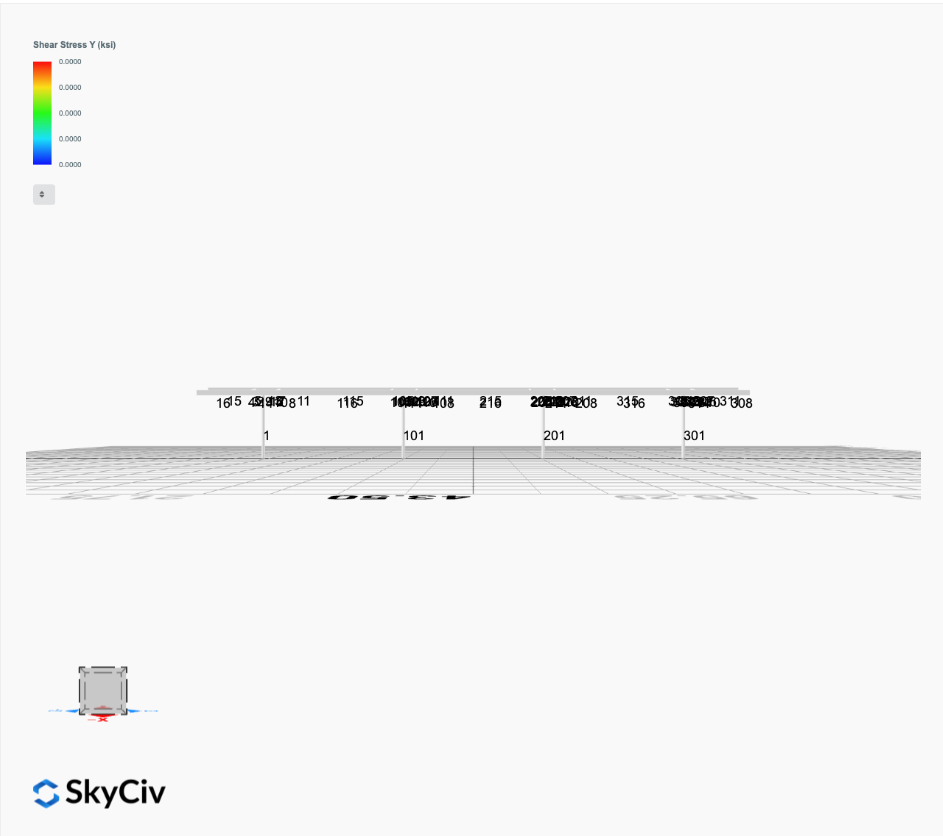

301
201
101
1
SkyCiv

FEM Results (Envelope Worst Case for each member)






Top Bending Stress Y (ksi)
4.4795
2.2398
0.0000
-2.2398
-4.4795
1
101
201
301


Shear Stress Y (ksi)
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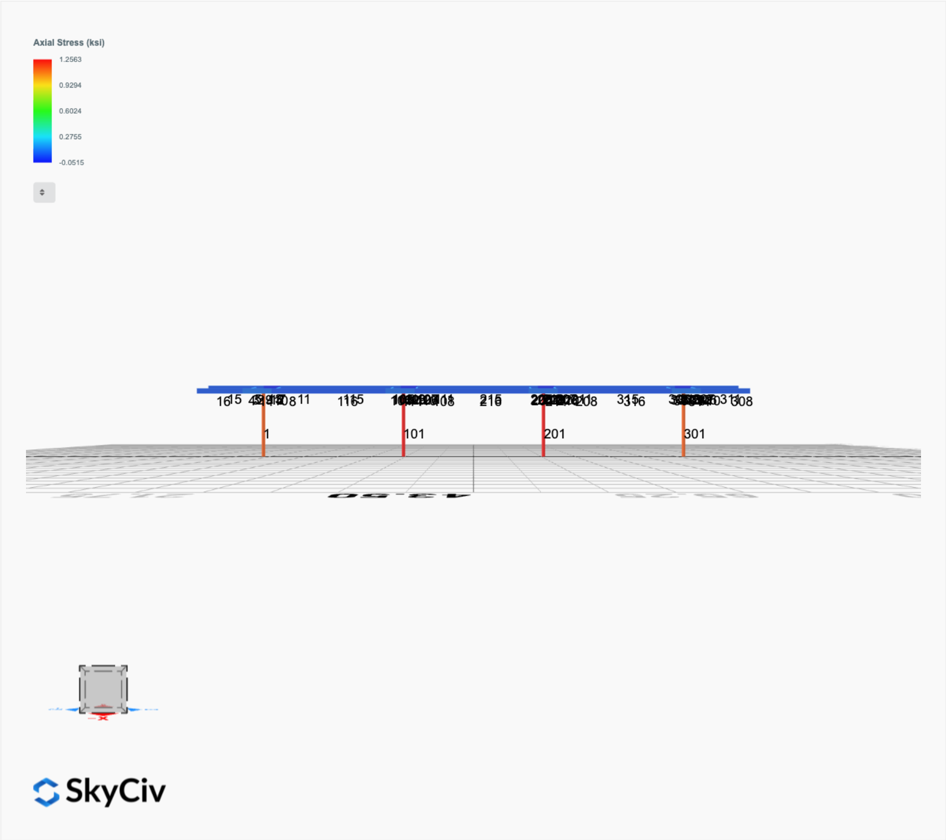
0.0000
0.0000
0.0000
0.0000
1
101
201
301


Axial Stress (ksi)
1.2563
0.9294
0.6024
0.2755
-0.0515
1
101
201
301
SkyCiv

Reaction Forces for Foundation 1 (Node ID#1), (kip, kip-ft)

ASD Load Combination Results

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 1. D	-0.0014	2.6921	-0.0111	-0.0375	0.0042	0.0457
ULS: 2. D + L	-0.0014	2.6921	-0.0111	-0.0375	0.0042	0.0457
ULS: 3. D + (S or Lr or R)	-0.0055	9.3415	-0.0441	-0.1489	0.0165	0.0931
ULS: 3. D + (S or Lr or R)	-0.0014	2.6921	-0.0111	-0.0375	0.0042	0.0457
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	-0.0044	7.6792	-0.0359	-0.1211	0.0134	0.0812
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	-0.0014	2.6921	-0.0111	-0.0375	0.0042	0.0457
ULS: 5b. D + 0.7E	-0.0014	2.6921	-0.0111	-0.0375	0.0042	0.0457
ULS: 6b. D + 0.75L + 0.75(0.7)E + 0.75S	-0.0044	7.6792	-0.0359	-0.1211	0.0134	0.0812
ULS: 8. 0.6D + 0.7E	-0.0008	1.6152	-0.0067	-0.0225	0.0025	0.0274
ULS: 5a. D + 0.6W_Wind downforce Case A only	-0.1858	4.7888	-0.0213	-0.0719	0.0061	2.4761
ULS: 5a. D + 0.6W_Wind downforce Case B only	-0.1858	4.7888	-0.0213	-0.0719	0.0061	2.4761
ULS: 5a. D + 0.6W_Wind uplift Case A only	0.0472	2.1266	-0.0080	-0.0269	0.0031	1.6475
ULS: 5a. D + 0.6W_Wind uplift Case B only	0.1180	1.3604	-0.0054	-0.0181	0.0040	-5.9056
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-0.1428	9.2517	-0.0435	-0.1469	0.0149	1.9040
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-0.1428	9.2517	-0.0435	-0.1469	0.0149	1.9040
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	0.0320	7.2550	-0.0335	-0.1132	0.0126	1.2826
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	0.0851	6.6804	-0.0316	-0.1066	0.0133	-4.3822
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-0.1397	4.2646	-0.0187	-0.0633	0.0056	1.8685
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-0.1397	4.2646	-0.0187	-0.0633	0.0056	1.8685
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	0.0351	2.2679	-0.0088	-0.0296	0.0033	1.2471
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	0.0882	1.6933	-0.0068	-0.0230	0.0040	-4.4177
ULS: 7. 0.6D + 0.6W_Wind downforce Case A only	-0.1853	3.7119	-0.0168	-0.0569	0.0045	2.4578
ULS: 7. 0.6D + 0.6W_Wind downforce Case B only	-0.1853	3.7119	-0.0168	-0.0569	0.0045	2.4578
ULS: 7. 0.6D + 0.6W_Wind uplift Case A only	0.0478	1.0497	-0.0035	-0.0119	0.0014	1.6292
ULS: 7. 0.6D + 0.6W_Wind uplift Case B only	0.1186	0.2835	-0.0010	-0.0031	0.0023	-5.9239

Worst Case Reactions LRFD

These calculations are taken directly from the FEA via SkyCiv and are used in the Concrete Checks of the Foundatino Module. Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	15.6170
Shear X	-0.3113
Shear Z	-0.0748
Moment X	-0.2544
Moment Y (Twist)	0.0264
Moment Z	10.4908

Worst Case Reactions ASD

These results are taken from the worst case values in the above table and are used in the Soil Checks in the Foundation Module. Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	9.3415
Shear X	-0.1858
Shear Z	-0.0441
Moment X	-0.1489
Moment Y (Twist)	0.0165
Moment Z	5.9239

Reaction Forces for Foundation 2 (Node ID#101), (kip, kip-ft)

ASD Load Combination Results

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 1. D	0.0014	2.8125	0.0046	0.0157	-0.0011	0.0203
ULS: 2. D + L	0.0014	2.8125	0.0046	0.0157	-0.0011	0.0203
ULS: 3. D + (S or Lr or R)	0.0055	9.8243	0.0183	0.0625	-0.0042	-0.0094
ULS: 3. D + (S or Lr or R)	0.0014	2.8125	0.0046	0.0157	-0.0011	0.0203
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	0.0044	8.0714	0.0149	0.0508	-0.0035	-0.0020
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	0.0014	2.8125	0.0046	0.0157	-0.0011	0.0203
ULS: 5b. D + 0.7E	0.0014	2.8125	0.0046	0.0157	-0.0011	0.0203

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 6b. D + 0.75L + 0.75(0.7)E + 0.75S	0.0044	8.0714	0.0149	0.0508	-0.0035	-0.0020
ULS: 8. 0.6D + 0.7E	0.0008	1.6875	0.0028	0.0094	-0.0006	0.0122
ULS: 5a. D + 0.6W_Wind downforce Case A only	-0.1910	5.0227	0.0092	0.0313	-0.0025	2.5465
ULS: 5a. D + 0.6W_Wind downforce Case B only	-0.1910	5.0227	0.0092	0.0313	-0.0025	2.5465
ULS: 5a. D + 0.6W_Wind uplift Case A only	0.0545	2.2159	0.0036	0.0123	-0.0012	1.6638
ULS: 5a. D + 0.6W_Wind uplift Case B only	0.1212	1.4097	0.0012	0.0043	0.0008	-6.1184
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-0.1398	9.7290	0.0183	0.0625	-0.0045	1.8927
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-0.1398	9.7290	0.0183	0.0625	-0.0045	1.8927
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	0.0443	7.6239	0.0141	0.0483	-0.0035	1.2306
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	0.0943	7.0193	0.0123	0.0423	-0.0021	-4.6060
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-0.1429	4.4701	0.0080	0.0274	-0.0021	1.9149
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-0.1429	4.4701	0.0080	0.0274	-0.0021	1.9149
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	0.0412	2.3650	0.0039	0.0132	-0.0011	1.2529
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	0.0913	1.7604	0.0021	0.0071	0.0003	-4.5837
ULS: 7. 0.6D + 0.6W_Wind downforce Case A only	-0.1915	3.8977	0.0073	0.0250	-0.0021	2.5384
ULS: 7. 0.6D + 0.6W_Wind downforce Case B only	-0.1915	3.8977	0.0073	0.0250	-0.0021	2.5384
ULS: 7. 0.6D + 0.6W_Wind uplift Case A only	0.0539	1.0909	0.0018	0.0060	-0.0007	1.6557
ULS: 7. 0.6D + 0.6W_Wind uplift Case B only	0.1207	0.2847	-0.0006	-0.0020	0.0012	-6.1265

Worst Case Reactions LRFD

These calculations are taken directly from the FEA via SkyCiv and are used in the Concrete Checks of the Foundatino Module. Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	16.4356
Shear X	-0.3206
Shear Z	0.0314
Moment X	0.1080
Moment Y (Twist)	0.0076
Moment Z	10.9096

Worst Case Reactions ASD

These results are taken from the worst case values in the above table and are used in the Soil Checks in the Foundation Module. Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	9.8243
Shear X	-0.1915
Shear Z	0.0183
Moment X	0.0625
Moment Y (Twist)	0.0045
Moment Z	6.1265

Reaction Forces for Foundation 3 (Node ID#201), (kip, kip-ft)

ASD Load Combination Results

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 1. D	0.0014	2.8125	-0.0046	-0.0157	0.0011	0.0203
ULS: 2. D + L	0.0014	2.8125	-0.0046	-0.0157	0.0011	0.0203
ULS: 3. D + (S or Lr or R)	0.0055	9.8243	-0.0183	-0.0625	0.0042	-0.0094
ULS: 3. D + (S or Lr or R)	0.0014	2.8125	-0.0046	-0.0157	0.0011	0.0203
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	0.0044	8.0714	-0.0149	-0.0508	0.0034	-0.0020
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	0.0014	2.8125	-0.0046	-0.0157	0.0011	0.0203
ULS: 5b. D + 0.7E	0.0014	2.8125	-0.0046	-0.0157	0.0011	0.0203
ULS: 6b. D + 0.75L + 0.75(0.7)E + 0.75S	0.0044	8.0714	-0.0149	-0.0508	0.0034	-0.0020
ULS: 8. 0.6D + 0.7E	0.0008	1.6875	-0.0028	-0.0094	0.0006	0.0122
ULS: 5a. D + 0.6W_Wind downforce Case A only	-0.1910	5.0227	-0.0092	-0.0313	0.0025	2.5465
ULS: 5a. D + 0.6W_Wind downforce Case B only	-0.1910	5.0227	-0.0092	-0.0313	0.0025	2.5465
ULS: 5a. D + 0.6W_Wind uplift Case A only	0.0545	2.2159	-0.0036	-0.0123	0.0012	1.6638
ULS: 5a. D + 0.6W_Wind uplift Case B only	0.1212	1.4097	-0.0012	-0.0043	-0.0008	-6.1184
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-0.1398	9.7290	-0.0183	-0.0625	0.0045	1.8927
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-0.1398	9.7290	-0.0183	-0.0625	0.0045	1.8927
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	0.0443	7.6239	-0.0141	-0.0483	0.0035	1.2306
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	0.0943	7.0193	-0.0123	-0.0423	0.0021	-4.6060

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-0.1429	4.4701	-0.0080	-0.0274	0.0021	1.9149
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-0.1429	4.4701	-0.0080	-0.0274	0.0021	1.9149
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	0.0412	2.3650	-0.0039	-0.0132	0.0011	1.2529
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	0.0913	1.7604	-0.0021	-0.0071	-0.0003	-4.5837
ULS: 7. 0.6D + 0.6W_Wind downforce Case A only	-0.1915	3.8977	-0.0073	-0.0250	0.0021	2.5384
ULS: 7. 0.6D + 0.6W_Wind downforce Case B only	-0.1915	3.8977	-0.0073	-0.0250	0.0021	2.5384
ULS: 7. 0.6D + 0.6W_Wind uplift Case A only	0.0539	1.0909	-0.0018	-0.0060	0.0007	1.6557
ULS: 7. 0.6D + 0.6W_Wind uplift Case B only	0.1207	0.2847	0.0006	0.0020	-0.0012	-6.1265

Worst Case Reactions LRFD

These calculations are taken directly from the FEA via SkyCiv and are used in the Concrete Checks of the Foundatino Module. Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	16.4356
Shear X	-0.3206
Shear Z	-0.0314
Moment X	-0.1080
Moment Y (Twist)	0.0076
Moment Z	10.9096

Worst Case Reactions ASD

These results are taken from the worst case values in the above table and are used in the Soil Checks in the Foundation Module. Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	9.8243
Shear X	-0.1915
Shear Z	-0.0183
Moment X	-0.0625
Moment Y (Twist)	0.0045
Moment Z	6.1265

Reaction Forces for Foundation 4 (Node ID#301), (kip, kip-ft)

ASD Load Combination Results

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 1. D	-0.0014	2.6921	0.0111	0.0375	-0.0042	0.0457
ULS: 2. D + L	-0.0014	2.6921	0.0111	0.0375	-0.0042	0.0457
ULS: 3. D + (S or Lr or R)	-0.0055	9.3415	0.0441	0.1489	-0.0165	0.0931
ULS: 3. D + (S or Lr or R)	-0.0014	2.6921	0.0111	0.0375	-0.0042	0.0457
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	-0.0044	7.6792	0.0358	0.1211	-0.0134	0.0812
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	-0.0014	2.6921	0.0111	0.0375	-0.0042	0.0457
ULS: 5b. D + 0.7E	-0.0014	2.6921	0.0111	0.0375	-0.0042	0.0457
ULS: 6b. D + 0.75L + 0.75(0.7)E + 0.75S	-0.0044	7.6792	0.0358	0.1211	-0.0134	0.0812
ULS: 8. 0.6D + 0.7E	-0.0008	1.6152	0.0067	0.0225	-0.0025	0.0274
ULS: 5a. D + 0.6W_Wind downforce Case A only	-0.1858	4.7888	0.0213	0.0719	-0.0061	2.4761
ULS: 5a. D + 0.6W_Wind downforce Case B only	-0.1858	4.7888	0.0213	0.0719	-0.0061	2.4761
ULS: 5a. D + 0.6W_Wind uplift Case A only	0.0472	2.1266	0.0080	0.0269	-0.0031	1.6475
ULS: 5a. D + 0.6W_Wind uplift Case B only	0.1180	1.3604	0.0054	0.0181	-0.0040	-5.9056
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-0.1428	9.2517	0.0435	0.1469	-0.0149	1.9040
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-0.1428	9.2517	0.0435	0.1469	-0.0149	1.9040
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	0.0320	7.2550	0.0335	0.1132	-0.0126	1.2826
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	0.0851	6.6804	0.0316	0.1066	-0.0133	-4.3822
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-0.1397	4.2646	0.0187	0.0633	-0.0056	1.8685
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-0.1397	4.2646	0.0187	0.0633	-0.0056	1.8685
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	0.0351	2.2679	0.0088	0.0296	-0.0033	1.2470
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	0.0882	1.6933	0.0068	0.0230	-0.0040	-4.4178
ULS: 7. 0.6D + 0.6W_Wind downforce Case A only	-0.1853	3.7119	0.0168	0.0569	-0.0045	2.4578
ULS: 7. 0.6D + 0.6W_Wind downforce Case B only	-0.1853	3.7119	0.0168	0.0569	-0.0045	2.4578
ULS: 7. 0.6D + 0.6W_Wind uplift Case A only	0.0478	1.0497	0.0035	0.0119	-0.0014	1.6292
ULS: 7. 0.6D + 0.6W_Wind uplift Case B only	0.1186	0.2835	0.0010	0.0031	-0.0023	-5.9239

Worst Case Reactions LRFD

Worst Case Reactions ASD

These calculations are taken directly from the FEA via SkyCiv and are used in the Concrete Checks of the Foundatino Module.
Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	15.6170
Shear X	-0.3113
Shear Z	0.0748
Moment X	0.2544
Moment Y (Twist)	0.0264
Moment Z	10.4909

These results are taken from the worst case values in the above table and are used in the Soil Checks in the Foundation Module.
Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	9.3415
Shear X	-0.1858
Shear Z	0.0441
Moment X	0.1489
Moment Y (Twist)	0.0165
Moment Z	5.9239

Project Details

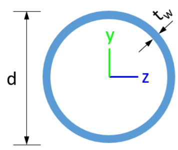
Design Code:	AISC 360-16 LRFD
Provision:	LRFD
Country:	United States
User Name:	sales@mtsolar.us
Unit System:	imperial

Design Input Information

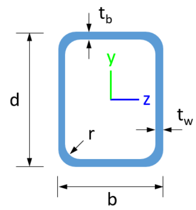
Design Factors			
Φ_t	Φ_c	Φ_b	Φ_v
0.9	0.9	0.9	0.9

Design Materials			
ID	E (ksi)	F_y (ksi)	F_u (ksi)
1	29000	50	65

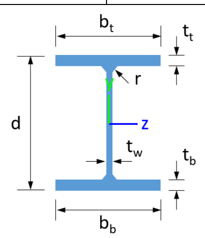
Section Dimensions



ID	Name	d (in)	t_w (in)
3	2in Pipe Sch 120	2.38	0.25
6	4in Pipe Sch 120	4.50	0.44
7	6in Pipe Sch 40	6.63	0.28



ID	Name	d (in)	b (in)	t_w (in)	t_b (in)	r (in)
17	HSS5x3x1/4	5.00	3.00	0.23	0.23	0.23



ID	Name	d (in)	t_w (in)	b_t (in)	b_b (in)	t_t (in)	t_b (in)	r (in)
20	W10x12	9.87	0.19	3.96	3.96	0.21	0.21	0.30

Section Properties								
ID	Name	A (in^2)	J (in^4)	I_{yp} (in^4)	I_{zp} (in^4)	I_w (in^6)	S_{yp} (in^3)	S_{zp} (in^3)
3	2in Pipe Sch 120	1.67	1.91	0.96	0.96	0.00	1.13	1.13
6	4in Pipe Sch 120	5.58	23.29	11.64	11.64	0.00	7.24	7.24
7	6in Pipe Sch 40	5.58	56.28	28.14	28.14	0.00	11.28	11.28

17	HSS5x3x1/4	3.37	11.00	4.81	10.70	62.42	3.77	5.38
20	W10x12	3.54	0.05	2.18	53.80	50.90	1.74	12.60

Member Properties								
Member ID	Section ID	K_zL (ft)	K_yL (ft)	L_b(ft)	C_b	LST	LSC	LD
1	7	22.59	22.59	10.76	-	300	200	1
2	6	1.30	1.30	2.00	-	300	200	1
3	17	0.92	0.92	1.42	1.19,1.18,1.19,1.18,1.18,1.19,1.18,1.18,1.18,1.19,1.18,1.18,1.19,1.16,1.18,1.18,1.18,1.18,1.18,1.18,1.19,1.22,1.18,1.18,1.19,1.17	300	200	1
4	17	2.44	2.44	3.75	1.68,1.67,1.68,1.67,1.68,1.68,1.67,1.67,1.68,1.67,1.67,1.67,1.72,1.69,1.67,1.67,1.67,1.67,1.68,1.68,1.69,1.68,1.67,1.67,1.89,1.69	300	200	1
5	17	1.52	1.52	2.33	1.68,1.67,1.68,1.67,1.67,1.68,1.67,1.67,1.67,1.68,1.67,1.67,1.68,1.65,1.67,1.67,1.67,1.67,1.67,1.67,1.68,1.72,1.67,1.67,1.68,1.66	300	200	1
6	17	0.92	0.92	1.42	1.19,1.18,1.19,1.18,1.18,1.19,1.18,1.18,1.18,1.19,1.18,1.18,1.19,1.16,1.18,1.18,1.18,1.18,1.18,1.18,1.19,1.22,1.18,1.18,1.19,1.17	300	200	1
7	17	1.52	1.52	2.33	1.68,1.67,1.68,1.67,1.67,1.68,1.67,1.67,1.67,1.69,1.67,1.67,1.68,1.65,1.67,1.67,1.67,1.67,1.67,1.67,1.68,1.73,1.67,1.67,1.68,1.66	300	200	1
8	20	1.33	1.33	2.05	2.13,2.13,2.13,2.13,2.13,2.13,2.14,2.14,2.17,2.08,2.15,2.15,2.23,2.02,2.13,2.13,2.14,2.12,2.14,2.14,2.18,2.07,2.15,2.15,2.00,1.98	300	200	1
9	3	2.60	2.60	4.00	-	300	200	1
10	17	2.44	2.44	3.75	1.68,1.67,1.68,1.67,1.68,1.68,1.67,1.67,1.68,1.67,1.67,1.67,1.73,1.69,1.67,1.67,1.67,1.67,1.68,1.68,1.69,1.68,1.67,1.67,1.94,1.69	300	200	1
11	20	1.33	1.33	2.05	2.14,2.13,2.14,2.13,2.14,2.14,2.12,2.12,2.11,2.24,2.12,2.12,2.09,1.94,2.13,2.13,2.13,2.15,2.12,2.12,2.11,2.28,2.12,2.12,2.07,1.99	300	200	1
12	6	1.30	1.30	2.00	-	300	200	1
13	20	4.88	4.00	7.50	1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.08,1.07,1.07,1.07,1.13,1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.08,1.07,1.07,1.07,1.10	300	200	1
14	20	4.88	4.00	7.50	1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.08,1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.06,1.10	300	200	1
15	20	12.60	12.60	6.00	2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33	300	200	1
16	20	12.60	12.60	6.00	2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33	300	200	1
101	7	22.59	22.59	10.76	-	300	200	1
102	6	1.30	1.30	2.00	-	300	200	1
103	17	0.92	0.92	1.42	1.19,1.18,1.19,1.18,1.18,1.19,1.18,1.18,1.18,1.19,1.18,1.18,1.19,1.16,1.18,1.18,1.18,1.18,1.18,1.18,1.19,1.22,1.18,1.18,1.19,1.17	300	200	1
104	17	2.44	2.44	3.75	1.68,1.67,1.68,1.67,1.68,1.68,1.67,1.67,1.68,1.67,1.67,1.67,1.72,1.68,1.67,1.67,1.67,1.67,1.68,1.68,1.69,1.68,1.67,1.67,1.89,1.68	300	200	1
105	17	1.52	1.52	2.33	1.68,1.67,1.68,1.67,1.67,1.68,1.67,1.67,1.67,1.68,1.67,1.67,1.68,1.65,1.67,1.67,1.67,1.67,1.67,1.67,1.68,1.72,1.67,1.67,1.68,1.66	300	200	1
106	17	0.92	0.92	1.42	1.19,1.18,1.19,1.18,1.18,1.19,1.18,1.18,1.18,1.19,1.18,1.18,1.19,1.16,1.18,1.18,1.18,1.18,1.18,1.18,1.19,1.22,1.18,1.18,1.19,1.17	300	200	1
107	17	1.52	1.52	2.33	1.68,1.67,1.68,1.67,1.67,1.68,1.67,1.67,1.67,1.68,1.67,1.67,1.68,1.65,1.67,1.67,1.67,1.67,1.67,1.67,1.68,1.72,1.67,1.67,1.68,1.66	300	200	1

108	20	1.33	1.33	2.05	2.32,2.32,2.32,2.32,2.32,2.32,2.31,2.31,2.31,2.33,2.31,2.31,2.30,2.35,2.32,2.32,2.32,2.32,2.31,2.31,2.32,2.33,2.31,2.31,2.23,2.36	300	200	1
109	3	2.60	2.60	4.00	-	300	200	1
110	17	2.44	2.44	3.75	1.68,1.67,1.68,1.67,1.68,1.68,1.67,1.67,1.68,1.67,1.67,1.67,1.72,1.68,1.67,1.67,1.67,1.67,1.68,1.68,1.69,1.68,1.67,1.67,1.89,1.68	300	200	1
111	20	1.33	1.33	2.05	2.31,2.31,2.31,2.31,2.31,2.31,2.31,2.31,2.31,2.25,2.32,2.32,2.32,2.27,2.31,2.31,2.31,2.30,2.31,2.31,2.32,2.22,2.32,2.32,2.33,2.36	300	200	1
112	6	1.30	1.30	2.00	-	300	200	1
113	20	4.88	4.00	7.50	1.03,1.03,1.03,1.03,1.03,1.03,1.03,1.03,1.03,1.02,1.03,1.03,1.03,1.14,1.03,1.03,1.03,1.03,1.03,1.03,1.03,1.02,1.03,1.03,1.03,1.08	300	200	1
114	20	4.88	4.00	7.50	1.03,1.03,1.03,1.03,1.03,1.03,1.03,1.03,1.03,1.03,1.03,1.03,1.02,1.04,1.03,1.03,1.03,1.03,1.03,1.03,1.03,1.03,1.03,1.03,1.01,1.09	300	200	1
115	20	8.42	8.42	12.95	1.16,1.16,1.16,1.17,1.16,1.16,1.16,1.16,1.16,1.19,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.17,1.16,1.16,1.16,1.20,1.16,1.16,1.16,1.16	300	200	1
116	20	8.42	8.42	12.95	1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.17,1.16,1.16,1.16,1.17,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.17,1.16,1.16,1.16,1.24,1.16	300	200	1
201	7	22.59	22.59	10.76	-	300	200	1
202	6	1.30	1.30	2.00	-	300	200	1
203	17	0.92	0.92	1.42	1.19,1.18,1.19,1.18,1.18,1.19,1.18,1.18,1.18,1.19,1.18,1.18,1.19,1.16,1.18,1.18,1.18,1.18,1.18,1.18,1.19,1.22,1.18,1.18,1.19,1.17	300	200	1
204	17	2.44	2.44	3.75	1.68,1.67,1.68,1.67,1.68,1.68,1.67,1.67,1.68,1.67,1.67,1.67,1.72,1.68,1.67,1.67,1.67,1.67,1.68,1.68,1.69,1.68,1.67,1.67,1.89,1.68	300	200	1
205	17	1.52	1.52	2.33	1.68,1.67,1.68,1.67,1.67,1.68,1.67,1.67,1.67,1.68,1.67,1.67,1.68,1.65,1.67,1.67,1.67,1.67,1.67,1.67,1.68,1.72,1.67,1.67,1.68,1.66	300	200	1
206	17	0.92	0.92	1.42	1.19,1.18,1.19,1.18,1.18,1.19,1.18,1.18,1.18,1.19,1.18,1.18,1.19,1.16,1.18,1.18,1.18,1.18,1.18,1.18,1.19,1.22,1.18,1.18,1.19,1.17	300	200	1
207	17	1.52	1.52	2.33	1.68,1.67,1.68,1.67,1.67,1.68,1.67,1.67,1.67,1.68,1.67,1.67,1.68,1.65,1.67,1.67,1.67,1.67,1.67,1.67,1.68,1.72,1.67,1.67,1.68,1.66	300	200	1
208	20	1.33	1.33	2.05	2.29,2.29,2.29,2.29,2.29,2.29,2.28,2.28,2.28,2.31,2.28,2.28,2.26,2.34,2.29,2.29,2.29,2.29,2.29,2.29,2.28,2.31,2.28,2.28,1.74,2.36	300	200	1
209	3	2.60	2.60	4.00	-	300	200	1
210	17	2.44	2.44	3.75	1.68,1.67,1.68,1.67,1.68,1.68,1.67,1.67,1.68,1.67,1.67,1.67,1.72,1.68,1.67,1.67,1.67,1.67,1.68,1.68,1.69,1.68,1.67,1.67,1.89,1.68	300	200	1
211	20	1.33	1.33	2.05	2.28,2.28,2.28,2.28,2.28,2.28,2.29,2.29,2.29,2.21,2.29,2.29,2.30,2.27,2.28,2.28,2.28,2.27,2.29,2.29,2.29,2.01,2.29,2.29,2.31,2.35	300	200	1
212	6	1.30	1.30	2.00	-	300	200	1
213	20	4.88	4.00	7.50	1.03,1.03,1.03,1.03,1.03,1.03,1.03,1.03,1.03,1.02,1.03,1.03,1.03,1.14,1.03,1.03,1.03,1.03,1.03,1.03,1.03,1.02,1.03,1.03,1.03,1.08	300	200	1
214	20	4.88	4.00	7.50	1.03,1.03,1.03,1.03,1.03,1.03,1.03,1.03,1.03,1.03,1.03,1.03,1.02,1.04,1.03,1.03,1.03,1.03,1.03,1.03,1.03,1.03,1.03,1.03,1.01,1.09	300	200	1
215	20	8.42	8.42	12.95	1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.18,1.16,1.16,1.16,1.15,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.18,1.16,1.16,1.16,1.16	300	200	1
216	20	8.42	8.42	12.95	1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.18,1.16	300	200	1
301	7	22.59	22.59	10.76	-	300	200	1

302	6	1.30	1.30	2.00	-	300	200	1
303	17	0.92	0.92	1.42	1.19,1.18,1.19,1.18,1.18,1.19,1.18,1.18,1.18,1.19,1.18,1.18,1.19,1.16,1.18,1.18,1.18,1.18,1.18,1.18,1.19,1.22,1.18,1.18,1.19,1.17	300	200	1
304	17	2.44	2.44	3.75	1.68,1.67,1.68,1.67,1.68,1.68,1.67,1.67,1.68,1.67,1.67,1.67,1.73,1.69,1.67,1.67,1.67,1.67,1.68,1.68,1.69,1.68,1.67,1.67,1.94,1.69	300	200	1
305	17	1.52	1.52	2.33	1.68,1.67,1.68,1.67,1.67,1.68,1.67,1.67,1.67,1.69,1.67,1.67,1.68,1.65,1.67,1.67,1.67,1.67,1.67,1.67,1.68,1.73,1.67,1.67,1.68,1.66	300	200	1
306	17	0.92	0.92	1.42	1.19,1.18,1.19,1.18,1.18,1.19,1.18,1.18,1.18,1.19,1.18,1.18,1.19,1.16,1.18,1.18,1.18,1.18,1.18,1.18,1.19,1.22,1.18,1.18,1.19,1.17	300	200	1
307	17	1.52	1.52	2.33	1.68,1.67,1.68,1.67,1.67,1.68,1.67,1.67,1.67,1.68,1.67,1.67,1.68,1.65,1.67,1.67,1.67,1.67,1.67,1.67,1.68,1.72,1.67,1.67,1.68,1.66	300	200	1
308	20	12.60	12.60	6.00	2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33	300	200	1
309	3	2.60	2.60	4.00	-	300	200	1
310	17	2.44	2.44	3.75	1.68,1.67,1.68,1.67,1.68,1.68,1.67,1.67,1.68,1.67,1.67,1.67,1.72,1.69,1.67,1.67,1.67,1.67,1.68,1.68,1.69,1.68,1.67,1.67,1.89,1.69	300	200	1
311	20	12.60	12.60	6.00	2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33	300	200	1
312	6	1.30	1.30	2.00	-	300	200	1
313	20	4.88	4.00	7.50	1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.08,1.07,1.07,1.07,1.13,1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.08,1.07,1.07,1.07,1.10	300	200	1
314	20	4.88	4.00	7.50	1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.08,1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.06,1.10	300	200	1
315	20	8.42	8.42	12.95	1.20,1.20,1.20,1.20,1.20,1.20,1.20,1.20,1.18,1.20,1.20,1.20,1.21,1.20,1.20,1.20,1.19,1.20,1.20,1.20,1.16,1.20,1.20,1.20,1.21	300	200	1
316	20	8.42	8.42	12.95	1.20,1.20,1.20,1.20,1.20,1.20,1.20,1.19,1.20,1.20,1.20,1.18,1.21,1.20,1.20,1.20,1.20,1.20,1.20,1.19,1.20,1.20,1.20,1.15,1.21	300	200	1

Member Design Capacity

Member ID	$\Phi_t P_n$ (kip)	$\Phi_c P_n$ (kip)	$\Phi_b M_{zn}$ (k-ft)	$\Phi_b M_{yn}$ (k-ft)	$\Phi_v V_{yn}$ (kip)	$\Phi_v V_{zn}$ (kip)
1	251.16	86.52	42.30	42.30	75.35	75.35
2	251.01	248.88	27.16	27.16	75.30	75.30
3	151.65	150.70	20.17	14.14	54.12	28.95
4	151.65	145.15	20.17	14.14	54.12	28.95
5	151.65	149.10	20.17	14.14	54.12	28.95
6	151.65	150.70	20.17	14.14	54.12	28.95
7	151.65	149.10	20.17	14.14	54.12	28.95
8	159.30	142.47	46.90	6.46	56.26	44.91
9	75.10	66.32	4.25	4.25	22.53	22.53
10	151.65	145.15	20.17	14.14	54.12	28.95
11	159.30	142.47	46.90	6.46	56.26	44.91
12	251.01	248.88	27.16	27.16	75.30	75.30
13	159.30	116.35	32.70	6.46	56.26	44.91
14	159.30	116.35	32.39	6.46	56.26	44.91
15	159.30	21.54	46.90	6.46	56.26	44.91
16	159.30	21.54	46.90	6.46	56.26	44.91
101	251.16	86.52	42.30	42.30	75.35	75.35
102	251.01	248.88	27.16	27.16	75.30	75.30
103	151.65	150.70	20.17	14.14	54.12	28.95

103	151.65	150.70	20.17	14.14	54.12	28.95
104	151.65	145.15	20.17	14.14	54.12	28.95
105	151.65	149.10	20.17	14.14	54.12	28.95
106	151.65	150.70	20.17	14.14	54.12	28.95
107	151.65	149.10	20.17	14.14	54.12	28.95
108	159.30	142.47	46.90	6.46	56.26	44.91
109	75.10	66.32	4.25	4.25	22.53	22.53
110	151.65	145.15	20.17	14.14	54.12	28.95
111	159.30	142.47	46.90	6.46	56.26	44.91
112	251.01	248.88	27.16	27.16	75.30	75.30
113	159.30	116.35	31.17	6.46	56.26	44.91
114	159.30	116.35	30.87	6.46	56.26	44.91
115	159.30	48.27	15.41	6.46	56.26	44.91
116	159.30	48.27	15.41	6.46	56.26	44.91
201	251.16	86.52	42.30	42.30	75.35	75.35
202	251.01	248.88	27.16	27.16	75.30	75.30
203	151.65	150.70	20.17	14.14	54.12	28.95
204	151.65	145.15	20.17	14.14	54.12	28.95
205	151.65	149.10	20.17	14.14	54.12	28.95
206	151.65	150.70	20.17	14.14	54.12	28.95
207	151.65	149.10	20.17	14.14	54.12	28.95
208	159.30	142.47	46.90	6.46	56.26	44.91
209	75.10	66.32	4.25	4.25	22.53	22.53
210	151.65	145.15	20.17	14.14	54.12	28.95
211	159.30	142.47	46.90	6.46	56.26	44.91
212	251.01	248.88	27.16	27.16	75.30	75.30
213	159.30	116.35	31.17	6.46	56.26	44.91
214	159.30	116.35	30.87	6.46	56.26	44.91
215	159.30	48.27	15.28	6.46	56.26	44.91
216	159.30	48.27	15.41	6.46	56.26	44.91
301	251.16	86.52	42.30	42.30	75.35	75.35
302	251.01	248.88	27.16	27.16	75.30	75.30
303	151.65	150.70	20.17	14.14	54.12	28.95
304	151.65	145.15	20.17	14.14	54.12	28.95
305	151.65	149.10	20.17	14.14	54.12	28.95
306	151.65	150.70	20.17	14.14	54.12	28.95
307	151.65	149.10	20.17	14.14	54.12	28.95
308	159.30	21.54	46.90	6.46	56.26	44.91
309	75.10	66.32	4.25	4.25	22.53	22.53
310	151.65	145.15	20.17	14.14	54.12	28.95
311	159.30	21.54	46.90	6.46	56.26	44.91
312	251.01	248.88	27.16	27.16	75.30	75.30
313	159.30	116.35	32.70	6.46	56.26	44.91
314	159.30	116.35	32.39	6.46	56.26	44.91
315	159.30	48.27	15.41	6.46	56.26	44.91
316	159.30	48.27	15.28	6.46	56.26	44.91

Design Ratio

Member ID	P	M_z	M_y	V_y	V_z	(P,M_z,M_y)	Worst LC	KL/r	δ	Status
1	0.181	0.248	0.013	0.004	0.001	0.275	#16	0.604	Not Required	Pass
2	0.001	0.510	0.019	0.103	0.002	0.530	#21	0.036	Not Required	Pass
3	0.002	0.716	0.012	0.072	0.001	0.727	#21	0.046	Not Required	Pass
4	0.002	0.707	0.042	0.071	0.000	0.730	#21	0.082	Not Required	Pass

4	0.002	0.707	0.043	0.071	0.009	0.730	#21	0.082	Not Required	Pass
5	0.002	0.444	0.048	0.071	0.013	0.457	#21	0.076	Not Required	Pass
6	0.002	0.699	0.011	0.070	0.001	0.706	#21	0.046	Not Required	Pass
7	0.002	0.433	0.046	0.069	0.012	0.445	#21	0.076	Not Required	Pass
8	0.000	0.082	0.058	0.048	0.005	0.138	#21	0.068	Not Required	Pass
9	0.006	0.085	0.011	0.001	0.000	0.099	#21	0.206	Not Required	Pass
10	0.002	0.689	0.047	0.069	0.010	0.721	#21	0.082	Not Required	Pass
11	0.000	0.083	0.058	0.049	0.005	0.139	#21	0.102	Not Required	Pass
12	0.001	0.490	0.018	0.101	0.002	0.509	#21	0.036	Not Required	Pass
13	0.002	0.344	0.127	0.060	0.006	0.454	#21	0.306	Not Required	Pass
14	0.002	0.345	0.127	0.059	0.006	0.451	#21	0.204	Not Required	Pass
15	0.000	0.137	0.076	0.038	0.004	0.213	#21	Not Required	Not Required	Pass
16	0.000	0.135	0.076	0.038	0.004	0.212	#21	Not Required	Not Required	Pass
101	0.190	0.258	0.005	0.004	0.000	0.285	#16	0.604	Not Required	Pass
102	0.001	0.518	0.018	0.107	0.002	0.537	#21	0.036	Not Required	Pass
103	0.002	0.742	0.010	0.074	0.000	0.752	#21	0.046	Not Required	Pass
104	0.002	0.731	0.043	0.073	0.009	0.760	#21	0.082	Not Required	Pass
105	0.002	0.460	0.044	0.074	0.011	0.471	#21	0.076	Not Required	Pass
106	0.002	0.750	0.011	0.075	0.000	0.762	#21	0.046	Not Required	Pass
107	0.002	0.465	0.045	0.074	0.012	0.477	#21	0.076	Not Required	Pass
108	0.000	0.060	0.055	0.047	0.005	0.114	#21	0.102	Not Required	Pass
109	0.005	0.082	0.010	0.001	0.000	0.095	#21	0.206	Not Required	Pass
110	0.002	0.739	0.042	0.074	0.009	0.765	#21	0.082	Not Required	Pass
111	0.000	0.061	0.056	0.047	0.005	0.117	#21	0.102	Not Required	Pass
112	0.001	0.526	0.019	0.108	0.002	0.545	#21	0.036	Not Required	Pass
113	0.002	0.281	0.118	0.059	0.006	0.374	#21	0.306	Not Required	Pass
114	0.002	0.281	0.117	0.058	0.006	0.370	#21	0.204	Not Required	Pass
115	0.000	0.421	0.063	0.046	0.005	0.484	#21	0.644	Not Required	Pass
116	0.000	0.417	0.063	0.046	0.005	0.480	#21	0.429	Not Required	Pass
201	0.190	0.258	0.005	0.004	0.000	0.285	#16	0.604	Not Required	Pass
202	0.001	0.526	0.019	0.108	0.002	0.545	#21	0.036	Not Required	Pass
203	0.002	0.750	0.011	0.075	0.000	0.762	#21	0.046	Not Required	Pass
204	0.002	0.739	0.042	0.074	0.009	0.765	#21	0.082	Not Required	Pass
205	0.002	0.465	0.045	0.074	0.012	0.477	#21	0.076	Not Required	Pass
206	0.002	0.742	0.010	0.074	0.000	0.752	#21	0.046	Not Required	Pass
207	0.002	0.460	0.044	0.074	0.011	0.471	#21	0.076	Not Required	Pass
208	0.000	0.062	0.055	0.046	0.005	0.117	#21	0.068	Not Required	Pass
209	0.005	0.082	0.010	0.001	0.000	0.095	#21	0.206	Not Required	Pass
210	0.002	0.731	0.043	0.073	0.009	0.760	#21	0.082	Not Required	Pass
211	0.000	0.063	0.055	0.046	0.005	0.119	#21	0.102	Not Required	Pass
212	0.001	0.518	0.018	0.107	0.002	0.537	#21	0.036	Not Required	Pass
213	0.002	0.281	0.118	0.059	0.006	0.374	#21	0.306	Not Required	Pass
214	0.002	0.281	0.117	0.058	0.006	0.370	#21	0.306	Not Required	Pass
215	0.000	0.461	0.063	0.047	0.005	0.523	#21	0.644	Not Required	Pass
216	0.000	0.457	0.064	0.047	0.005	0.521	#21	0.644	Not Required	Pass
301	0.181	0.248	0.013	0.004	0.001	0.275	#16	0.604	Not Required	Pass
302	0.001	0.490	0.018	0.101	0.002	0.509	#21	0.036	Not Required	Pass
303	0.002	0.699	0.011	0.070	0.001	0.706	#21	0.046	Not Required	Pass
304	0.002	0.689	0.047	0.069	0.010	0.721	#21	0.082	Not Required	Pass
305	0.002	0.433	0.046	0.069	0.012	0.445	#21	0.076	Not Required	Pass
306	0.002	0.716	0.012	0.072	0.001	0.727	#21	0.046	Not Required	Pass
307	0.002	0.444	0.048	0.071	0.013	0.457	#21	0.076	Not Required	Pass
308	0.000	0.135	0.076	0.038	0.004	0.212	#21	Not Required	Not Required	Pass
309	0.006	0.085	0.011	0.001	0.000	0.099	#21	0.206	Not Required	Pass

310	0.002	0.707	0.043	0.071	0.009	0.730	#21	0.082	Not Required	Pass
311	0.000	0.137	0.076	0.038	0.004	0.213	#21	Not Required	Not Required	Pass
312	0.001	0.510	0.019	0.103	0.002	0.530	#21	0.036	Not Required	Pass
313	0.002	0.344	0.127	0.060	0.006	0.454	#21	0.204	Not Required	Pass
314	0.002	0.345	0.127	0.059	0.006	0.451	#21	0.204	Not Required	Pass
315	0.000	0.411	0.063	0.049	0.005	0.474	#21	0.644	Not Required	Pass
316	0.000	0.408	0.064	0.048	0.005	0.471	#21	0.429	Not Required	Pass

Definitions

Φ_t	Safety factor for tensile
Φ_c	Safety factor for compression
Φ_b	Safety factor for flexure
Φ_v	Safety factor for shear
E	Modulus of elasticity
F_y	Specified minimum yield stress
F_u	Specified minimum tensile strength
A	Cross-sectional area
J	Torsional constant
I_{yp}	Moment of inertia about the Y axes
I_{zp}	Moment of inertia about the Z axes
I_w	Warping constant
S_{yp}	Plastic section modulus about the Y axis
S_{zp}	Plastic section modulus about the Z axis
KL	Effective length
C_b	Buckling modification factor (from all load combinations)
L_b	Length between braced points
LST	Limited slenderness for tension
LSC	Limited slenderness for compression
LD	Limited deflection
P_n	Nominal axial strength (tension/comression)
M_n	Nominal flexural strength (about Z/Y axis)
V_n	Nominal shear strength (along Z/Y axis)
P	Design ratio in case of axial force
M_z	Design ratio in case of bending about Z axis
M_y	Design ratio in case of bending about Y axis
V_y	Design ratio in case of shear along Y axis
V_z	Design ratio in case of shear along Z axis
(P,M_z,M_y)	Design ratio in case of axial force and bending action
KL/r	Design ratio in case of section slenderness
δ	Design ratio in case of member deflection
OK	Capacity is provided
NG	Capacity is not provided

REFERENCES	CALCULATIONS	RESULTS

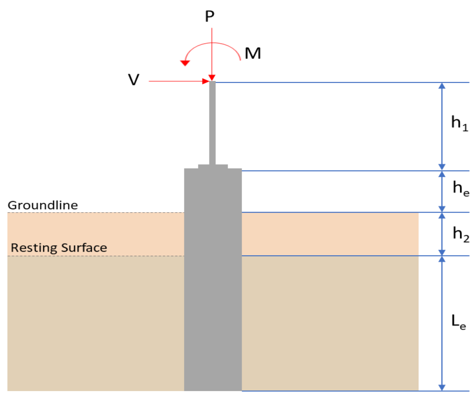
SkyCiv Foundation Design

Pile Foundation

Design Information :

Design code : IBC 2021 (International Building Code)
Unit System : Imperial

Pile Input



Geometry

Pile shape: rectangular
$b = 48\ \text{in}$ - Pile width
$D = 48\ \text{in}$ - Pile depth
$L = 4.25\ \text{ft}$ - Total pile length
$h_1 = 0\ \text{ft}$ - Lateral load height from the top of the pile,
$h_2 = 0\ \text{ft}$ - Depth to resisting surface
$h_e = 0\ \text{ft}$ - Length of pile above the ground

Tabulation of Soil Parameters

Layer	Label	Allowable Bearing Pressure (q_a) (psf)	Allowable Lateral Pressure (R) (psf/ft)
1	Sand, silty sand, clayey sand, silty gravel & clayey gravel	2000.000	150.000

Tabulation of Loads

Load Component	ASD	LRFD
P (kip)	9.342	15.617
V_x (kip)	-0.186	-0.311
V_z (kip)	-0.044	-0.075
M_x (kipft)	-0.149	-0.254
M_z (kipft)	5.924	10.491

Material Properties

$f'_{ck} = 2.5\ \text{ksi}$ - Concrete strength,

Required depth to resist lateral loads (ASD)

H - Point of application of the lateral load

$$H = h_1 + h_2 + h_e$$

$$H = (0\ \text{ft}) + (0\ \text{ft}) + (0\ \text{ft})$$

$$H = 0\ \text{ft}$$

Considering x-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\,D}$$

$$H_o = \frac{(-0.186\ \text{kip})}{1.57 \times (48\ \text{in})}$$

$$H_o = -0.029618\ \text{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\,H)}{1.57\,D}$$

$$M_o = \frac{(5.924\ \text{kipft}) + ((-0.186\ \text{kip}) \times (0\ \text{ft}))}{1.57 \times (48\ \text{in})}$$

$$M_o = 0.94331\ \text{kipft/ft}$$

Required depth of embedment in earth:

$$L_x^3 - \left(14.14 \times \frac{H_o \times L_x}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,x} = 4.0857\ \text{ft}$ - Required depth in x-direction,

Considering z-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\, b}$$

$$H_o = \frac{(-0.044\ \text{kip})}{1.57 \times (48\ \text{in})}$$

$$H_o = -0.0070064\ \text{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\, H)}{1.57\, b}$$

$$M_o = \frac{(0.149\ \text{kipft}) + ((-0.044\ \text{kip}) \times (0\ \text{ft}))}{1.57 \times (48\ \text{in})}$$

$$M_o = 0.023726\ \text{kipft/ft}$$

Required depth of embedment in earth:

$$L_z^3 - \left(14.14 \times \frac{H_o \times L_z}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,z} = 1.1253\ \text{ft}$ - Required depth in z-direction,

Minimum embedded depth required:

$L_{e,req}$ - Depth of pile required,

$$L_{e,req} = MAX[L_{e,x}, L_{e,z}]$$

$$L_{e,req} = MAX[(4.0857\ \text{ft}), (1.1253\ \text{ft})]$$

$$L_{e,req} = 4.086\ \text{ft}$$

L_e - Actual embedded length of pile,

$$L_e = L - h_e - h_2$$

$$L_e = (4.25\ \text{ft}) - (0\ \text{ft}) - (0\ \text{ft})$$

$$L_e = 4.25\ \text{ft}$$

$Ratio$ - Embedded depth

$$Ratio = \frac{L_{e,req}}{L_e}$$

$$Ratio = \frac{(4.086\ \text{ft})}{(4.25\ \text{ft})}$$

$$Ratio = 0.96141$$

Status: **PASS**
Ratio: **0.960**

End-bearing Capacity (ASD)

A - Pile cross-section area

$$A = b\, D$$

$$A = (48\ \text{in}) \times (48\ \text{in})$$

$$A = 16\ \text{ft}^2$$

q - End-bearing pressure

$$q = \frac{P_v}{A}$$

$$q = \frac{(9.342\ \text{kip})}{(16\ \text{ft}^2)}$$

$$q = 0.58388\ \text{kip/ft}^2$$

$$q = 0.58388\ \mathrm{kip/ft}$$

Check bearing capacity ratio:

$Ratio$ - Capacity

$$Ratio = \frac{q}{q_a}$$

$$Ratio = \frac{\left(0.58388\ \mathrm{kip/ft^2}\right)}{(2000\ \mathrm{psf})}$$

$$Ratio = 0.29194$$

Status: **PASS**
Ratio: **0.290**

Czerniak

Lateral Soil Pressure (ASD):

L/D - Length to least lateral dimension ratio,

$$L/D = \frac{L}{D}$$

$$L/D = \frac{(4.25\ \mathrm{ft})}{(48\ \mathrm{in})}$$

$$L/D = 1.0625$$

Since L/D $\leq$ 10,

Pile is short.

Considering x-direction:

$H_o = -0.029618\ \mathrm{kip/ft}$ - Lateral force per length of pile,
$M_o = 0.94331\ \mathrm{kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (0.94331\ \mathrm{kipft/ft}) \times (4.25\ \mathrm{ft})) + \left(3 \times (-0.029618\ \mathrm{kip/ft}) \times (4.25\ \mathrm{ft})^2\right)}{(6 \times (0.94331\ \mathrm{kipft/ft})) + (4 \times (-0.029618\ \mathrm{kip/ft}) \times (4.25\ \mathrm{ft}))}$$

$$a = 2.8623\ \mathrm{ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\,[(4\,M_o) + (3\,H_o\,L_e)]^2}{L_e^2\,[(3\,M_o) + (2\,H_o\,L_e)]}$$

$$p = \frac{0.75 \times [(4 \times (0.94331\ \mathrm{kipft/ft})) + (3 \times (-0.029618\ \mathrm{kip/ft}) \times (4.25\ \mathrm{ft}))]^{\ 2}}{(4.25\ \mathrm{ft})^2 \times [(3 \times (0.94331\ \mathrm{kipft/ft})) + (2 \times (-0.029618\ \mathrm{kip/ft}) \times (4.25\ \mathrm{ft}))]}$$

$$p = 0.1857\ \mathrm{kip/ft^2}$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\,[(2\,M_o) + (H_o\,L_e)]}{L_e^2}$$

$$s = \frac{6 \times [(2 \times (0.94331\ \mathrm{kipft/ft})) + ((-0.029618\ \mathrm{kip/ft}) \times (4.25\ \mathrm{ft}))]}{(4.25\ \mathrm{ft})^2}$$

$$s = 0.58489\ \mathrm{kip/ft^2}$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\,\frac{a}{2}$$

$$p_a = (150\ \mathrm{psf/ft}) \times \frac{(2.8623\ \mathrm{ft})}{2}$$

$$p_a = 0.21467\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{\left(0.1857\ \mathrm{kip/ft^2}\right)}{\left(0.21467\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = 0.86504$$

Status: **PASS**
Ratio: **0.870**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\, L_e$$

$$p_s = (150 \text{ psf/ft}) \times (4.25 \text{ ft})$$

$$p_s = 0.6375 \text{ kip/ft}^2$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{\left(0.58489 \text{ kip/ft}^2\right)}{\left(0.6375 \text{ kip/ft}^2\right)}$$

$$Ratio = 0.91747$$

Status: **PASS**
Ratio: **0.920**

Considering z-direction:

$H_o = -0.0070064 \text{ kip/ft}$ - Lateral force per length of pile,
$M_o = 0.023726 \text{ kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\, M_o\, L_e) + \left(3\, H_o\, L_e^2\right)}{(6\, M_o) + (4\, H_o\, L_e)}$$

$$a = \frac{(4 \times (0.023726 \text{ kipft/ft}) \times (4.25 \text{ ft})) + \left(3 \times (-0.0070064 \text{ kip/ft}) \times (4.25 \text{ ft})^2\right)}{(6 \times (0.023726 \text{ kipft/ft})) + (4 \times (-0.0070064 \text{ kip/ft}) \times (4.25 \text{ ft}))}$$

$$a = 2.9947 \text{ ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\, [(4\, M_o) + (3\, H_o\, L_e)]^2}{L_e^2\, [(3\, M_o) + (2\, H_o\, L_e)]}$$

$$p = \frac{0.75 \times [(4 \times (0.023726 \text{ kipft/ft})) + (3 \times (-0.0070064 \text{ kip/ft}) \times (4.25 \text{ ft}))]^{\ 2}}{(4.25 \text{ ft})^2 \times [(3 \times (0.023726 \text{ kipft/ft})) + (2 \times (-0.0070064 \text{ kip/ft}) \times (4.25 \text{ ft}))]}$$

$$p = 0.00011095 \text{ kip/ft}^2$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\, [(2\, M_o) + (H_o\, L_e)]}{L_e^2}$$

$$s = \frac{6 \times [(2 \times (0.023726 \text{ kipft/ft})) + ((-0.0070064 \text{ kip/ft}) \times (4.25 \text{ ft}))]}{(4.25 \text{ ft})^2}$$

$$s = 0.0058713 \text{ kip/ft}^2$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\, \frac{a}{2}$$

$$p_a = (150 \text{ psf/ft}) \times \frac{(2.9947 \text{ ft})}{2}$$

$$p_a = 0.2246 \text{ kip/ft}^2$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{\left(0.00011095 \text{ kip/ft}^2\right)}{\left(0.2246 \text{ kip/ft}^2\right)}$$

$$Ratio = 0.000494$$

Status: **PASS**
Ratio: **0.000**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\, L_e$$

$$p_s = (150 \text{ psf/ft}) \times (4.25 \text{ ft})$$

$$p_s = 0.6375 \text{ kip/ft}^2$$

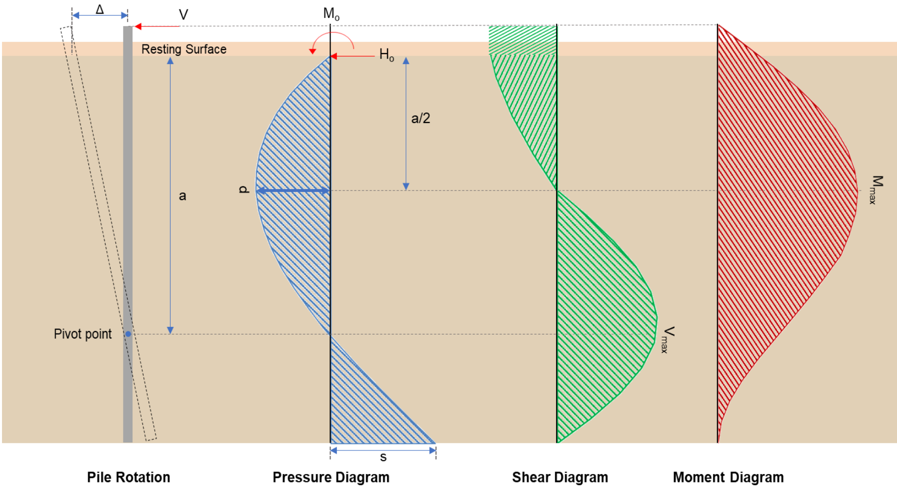
$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{(0.0058713\ \mathrm{kip/ft^2})}{(0.6375\ \mathrm{kip/ft^2})}$$

$$Ratio = 0.00921$$

Status: **PASS**
Ratio: **0.010**



Shear force and Bending moment (x-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\ D}$$

$$H_o = \frac{(-0.311\ \mathrm{kip})}{1.57 \times (48\ \mathrm{in})}$$

$$H_o = -0.049522\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\ H)}{1.57\ D}$$

$$M_o = \frac{(10.491\ \mathrm{kipft}) + ((-0.311\ \mathrm{kip}) \times (0\ \mathrm{ft}))}{1.57 \times (48\ \mathrm{in})}$$

$$M_o = 1.6705\ \mathrm{kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(1.6705\ \mathrm{kipft/ft})}{(-0.049522\ \mathrm{kip/ft})}$$

$$E = 33.733\ \mathrm{ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\ M_o\ L_e) + \left(3\ H_o\ L_e^2\right)}{(6\ M_o) + (4\ H_o\ L_e)}$$

$$a = \frac{(4 \times (1.6705\ \mathrm{kipft/ft}) \times (4.25\ \mathrm{ft})) + \left(3 \times (-0.049522\ \mathrm{kip/ft}) \times (4.25\ \mathrm{ft})^2\right)}{(6 \times (1.6705\ \mathrm{kipft/ft})) + (4 \times (-0.049522\ \mathrm{kip/ft}) \times (4.25\ \mathrm{ft}))}$$

$$a = 2.8608\ \mathrm{ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\ D) \left[1 - \left[3 \left(\frac{4\ E}{L_e} + 3\right) \left(\frac{a}{L_e}\right)^2\right] + \left[4 \left(\frac{3\ E}{L_e} + 2\right) \left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((-0.049522\ \mathrm{kip/ft}) \times (48\ \mathrm{in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (33.733\ \mathrm{ft})}{(4.25\ \mathrm{ft})} + 3\right) \times \left(\frac{(2.8608\ \mathrm{ft})}{(4.25\ \mathrm{ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (33.733\ \mathrm{ft})}{(4.25\ \mathrm{ft})} + 2\right) \times \left(\frac{(2.8608\ \mathrm{ft})}{(4.25\ \mathrm{ft})}\right)^3\right]\right]$$

$$V_{max} = 2.9207\ \mathrm{kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, D\, L_e) \left[\left(\frac{E}{L_e} + \frac{a}{2\, L_e} \right) - \left[\left(\frac{4\, E}{L_e} + 3 \right) \left(\frac{a}{2\, L_e} \right)^3 \right] + \left[\left(\frac{3\, E}{L_e} + 2 \right) \left(\frac{a}{2\, L_e} \right)^4 \right] \right]$$

$$M_{max} = ((-0.049522 \text{ kip/ft}) \times (48 \text{ in}) \times (4.25 \text{ ft})) \times \left[\left(\frac{(33.733 \text{ ft})}{(4.25 \text{ ft})} + \frac{(2.8608 \text{ ft})}{2 \times (4.25 \text{ ft})} \right) - \left[\left(\frac{4 \times (33.733 \text{ ft})}{(4.25 \text{ ft})} + 3 \right) \times \left(\frac{(2.8608 \text{ ft})}{2 \times (4.25 \text{ ft})} \right)^3 \right] + \left[\left(\frac{3 \times (33.733 \text{ ft})}{(4.25 \text{ ft})} + 2 \right) \times \left(\frac{(2.8608 \text{ ft})}{2 \times (4.25 \text{ ft})} \right)^4 \right] \right]$$

$$M_{max} = 6.129 \text{ kipft}$$

Shear force and Bending moment (z-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\, b}$$

$$H_o = \frac{(-0.075 \text{ kip})}{1.57 \times (48 \text{ in})}$$

$$H_o = -0.011943 \text{ kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\, H)}{1.57\, b}$$

$$M_o = \frac{(0.254 \text{ kipft}) + ((-0.075 \text{ kip}) \times (0 \text{ ft}))}{1.57 \times (48 \text{ in})}$$

$$M_o = 0.040446 \text{ kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(0.040446 \text{ kipft/ft})}{(-0.011943 \text{ kip/ft})}$$

$$E = 3.3867 \text{ ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\, M_o\, L_e) + \left(3\, H_o\, L_e^2\right)}{(6\, M_o) + (4\, H_o\, L_e)}$$

$$a = \frac{(4 \times (0.040446 \text{ kipft/ft}) \times (4.25 \text{ ft})) + \left(3 \times (-0.011943 \text{ kip/ft}) \times (4.25 \text{ ft})^2\right)}{(6 \times (0.040446 \text{ kipft/ft})) + (4 \times (-0.011943 \text{ kip/ft}) \times (4.25 \text{ ft}))}$$

$$a = 2.9947 \text{ ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\, b) \left[1 - \left[3 \left(\frac{4\, E}{L_e} + 3 \right) \left(\frac{a}{L_e} \right)^2 \right] + \left[4 \left(\frac{3\, E}{L_e} + 2 \right) \left(\frac{a}{L_e} \right)^3 \right] \right]$$

$$V_{max} = ((-0.011943 \text{ kip/ft}) \times (48 \text{ in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (3.3867 \text{ ft})}{(4.25 \text{ ft})} + 3 \right) \times \left(\frac{(2.9947 \text{ ft})}{(4.25 \text{ ft})} \right)^2 \right] + \left[4 \times \left(\frac{3 \times (3.3867 \text{ ft})}{(4.25 \text{ ft})} + 2 \right) \times \left(\frac{(2.9947 \text{ ft})}{(4.25 \text{ ft})} \right)^3 \right] \right]$$

$$V_{max} = 0.098984 \text{ kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, b\, L_e) \left[\left(\frac{E}{L_e} + \frac{a}{2\, L_e} \right) - \left[\left(\frac{4\, E}{L_e} + 3 \right) \left(\frac{a}{2\, L_e} \right)^3 \right] + \left[\left(\frac{3\, E}{L_e} + 2 \right) \left(\frac{a}{2\, L_e} \right)^4 \right] \right]$$

$$M_{max} = ((-0.011943 \text{ kip/ft}) \times (48 \text{ in}) \times (4.25 \text{ ft})) \times \left[\left(\frac{(3.3867 \text{ ft})}{(4.25 \text{ ft})} + \frac{(2.9947 \text{ ft})}{2 \times (4.25 \text{ ft})} \right) - \left[\left(\frac{4 \times (3.3867 \text{ ft})}{(4.25 \text{ ft})} + 3 \right) \times \left(\frac{(2.9947 \text{ ft})}{2 \times (4.25 \text{ ft})} \right)^3 \right] + \left[\left(\frac{3 \times (3.3867 \text{ ft})}{(4.25 \text{ ft})} + 2 \right) \times \left(\frac{(2.9947 \text{ ft})}{2 \times (4.25 \text{ ft})} \right)^4 \right] \right]$$

$$M_{max} = 0.19211 \text{ kipft}$$

Minimum Reinforcement Check (LRFD)

Parameters:

$f'_{ck} = 2.5 \text{ ksi}$ - Concrete strength,
$f_{yk} = 60 \text{ ksi}$ - Longitudinal reinforcement strength,
$\phi = 0.65$ - Reduction factor for axial strength,
Table 22.4.2.1 $\alpha = 0.8$ - Alpha factor for axial strength,
$A_g = 2304 \text{ in}^2$ - Gross area of concrete,

Longitudinal reinforcement:

Required reinforcement due to axial load, $A_{st,required}$
22.4.2.2, 10.6.1.1 $A_{st,required}$

$$A_{st,required} = Min\left[\frac{\frac{P}{\phi\ \alpha} - (0.85\ f'_{ck}\ A_g)}{f_{yk} - (0.85\ f'_{ck})}, (0.08\ A_g)\right]$$

$$A_{st,required} = Min\left[\frac{\frac{(15.617 \text{ kip})}{(0.65)\times(0.8)} - \left(0.85 \times (2.5 \text{ ksi}) \times \left(2304 \text{ in}^2\right)\right)}{(60 \text{ ksi}) - (0.85 \times (2.5 \text{ ksi}))}, \left(0.08 \times \left(2304 \text{ in}^2\right)\right)\right]$$

$$A_{st,required} = -84.077 \text{ in}^2$$

A_{min} - Governing minimum reinforcement area,

$$A_{min} = Max\left[A_{st,required}, (0.0018\ A_g)\right]$$

$$A_{min} = Max\left[\left(-84.077 \text{ in}^2\right), \left(0.0018 \times \left(2304 \text{ in}^2\right)\right)\right]$$

$$A_{min} = 4.1472 \text{ in}^2$$

n_{rebar} - Required number of reinforcement,

$$n_{rebar} = \frac{A_{min}}{A_{rebar}}$$

$$n_{rebar} = \frac{\left(4.1472 \text{ in}^2\right)}{\left(0.3068 \text{ in}^2\right)}$$

$$n_{rebar} = 14$$

A_{st} - Actual total reinforcement area,

$$A_{st} = n_{rebar}\ \frac{\pi\ d_{bar}^2}{4}$$

$$A_{st} = (14) \times \frac{\pi \times (0.625 \text{ in})^2}{4}$$

$$A_{st} = 4.2951 \text{ in}^2$$

$Ratio$ - Capacity

$$Ratio = \frac{A_{min}}{A_{st}}$$

$$Ratio = \frac{\left(4.1472 \text{ in}^2\right)}{\left(4.2951 \text{ in}^2\right)}$$

$$Ratio = 0.96556$$

Status: **PASS**
Ratio: **0.970**

25.2.3 s_{rebar} - Minimum spacing of reinforcement,

$$s_{rebar} = Max\left[1.5, (1.5\ d_{bar})\right]$$

$$s_{rebar} = Max\left[1.5, (1.5 \times (0.625 \text{ in}))\right]$$

$$s_{rebar} = 1.5 \text{ in}$$

Ties:

25.7.2.2 Since longitudinal reinforcement is $\leq$ No. 10ø: Use #3(0.375 in)
25.7.2.1 s_{ties} - Maximum spacing of ties,

$$s_{ties} = Min\left[(16\ d_{bar}), (48\ d_{ties}), Min(D, b)\right]$$

$$s_{ties} = Min\left[(16 \times (0.625 \text{ in})), (48 \times (0.375 \text{ in})), Min((48 \text{ in}), (48 \text{ in}))\right]$$

$$s_{ties} = 10 \text{ in}$$

Summary:

Main reinforcement: **14 - #5 (0.625 in)**

	Ties: **#3(0.375 in) - 10 in**	

Axial Compression Strength (ACI 318-19, LRFD)

22.4.2.2 ϕP_N - Allowable axial compressive strength

$$\phi P_N = \phi\, 0.80\, \left[\left(0.85\, f'_{ck}\, \left[A_g - A_{st}\right]\right) + \left(f_{yk}\, A_{st}\right)\right]$$

$$\phi P_N = (0.65) \times 0.80 \times \left[\left(0.85 \times (2.5\ \text{ksi}) \times \left[\left(2304\ \text{in}^2\right) - \left(4.2951\ \text{in}^2\right)\right]\right) + \left((60\ \text{ksi}) \times \left(4.2951\ \text{in}^2\right)\right)\right]$$

$$\phi P_N = 2675.2\ \text{kip}$$

$Ratio$ - Capacity

$$Ratio = \frac{P}{\phi P_N}$$

$$Ratio = \frac{(15.617\ \text{kip})}{(2675.2\ \text{kip})}$$

$$Ratio = 0.0058377$$

Status: **PASS**
Ratio: **0.010**

Shear Strength (ACI 318-19, LRFD)

Parameters:

$b_w = 48\ \text{in}$ - Effective width,

22.5.2.2 d - Effective depth

$$d = 0.80\, D$$

$$d = 0.80 \times (48\ \text{in})$$

$$d = 38.4\ \text{in}$$

22.5.5.1.3 λ_s - size effect modification factor

$$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{d}{10}}}, 1\right]$$

$$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{(38.4\ \text{in})}{10}}}, 1\right]$$

$$\lambda_s = 0.64282$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5\ \text{ksi} \rightarrow 2500\ \text{psi}$,

22.5.5.1.1 $V_{c,max}$ - Max shear strength of concrete

$$V_{c,max} = 5\, \lambda_s\, \sqrt{f'_{ck}}\, b_w\, d$$

$$V_{c,max} = 5 \times (0.64282) \times \sqrt{(2500\ \text{psi})} \times (48\ \text{in}) \times (38.4\ \text{in})$$

$$V_{c,max} = 296.21\ \text{kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5\ \text{ksi} \rightarrow 2500\ \text{psi}$, $P = 15.617\ \text{kip} \rightarrow 15617\ \text{lbf}$,

22.5.5.1.1(a) $V_{c,a}$ - Shear strength of concrete (a)

$$V_{c,a} = \left[2\, \lambda_s\, \sqrt{f'_{ck}} + \frac{P}{6\, A_g}\right]\, b_w\, d$$

$$V_{c,a} = \left[2 \times (0.64282) \times \sqrt{(2500\ \text{psi})} + \frac{(15617\ \text{lbf})}{6 \times (2304\ \text{in}^2)}\right] \times (48\ \text{in}) \times (38.4\ \text{in})$$

$$V_{c,a} = 120.57\ \text{kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5\ \text{ksi} \rightarrow 2500\ \text{psi}$,

22.5.5.1.2 $V_{c,b}$ - Shear strength of concrete (b)

$$V_{c,b} = \left[2\, \lambda_s\, \sqrt{f'_{ck}} + \left(0.05\, f'_{ck}\right)\right]\, b_w\, d$$

$$V_{c,b} = \left[2 \times (0.64282) \times \sqrt{(2500\ \text{psi})} + (0.05 \times (2500\ \text{psi}))\right] \times (48\ \text{in}) \times (38.4\ \text{in})$$

$$V_{c,b} = 348.89\ \text{kip}$$

V_c - Governing shear strength of concrete

$$V_c = Min\left[V_{c,max}, V_{c,a}, V_{c,b}\right]$$

$$V_c = Min\left[(296.21\ \text{kip}), (120.57\ \text{kip}), (348.89\ \text{kip})\right]$$

$$V_c = 120.57\ \text{kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$,

22.5.1.2 $V_{s,a}$ - Shear strength of steel (a)

$$V_{s,a} = 8 \sqrt{f'_{ck}}\, b_w\, d$$

$$V_{s,a} = 8 \times \sqrt{(2500 \text{ psi})} \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{s,a} = 737.28 \text{ kip}$$

A_v - Ties rebar area,

$$A_v = \frac{\pi\, d_{ties}^2}{4}$$

$$A_v = \frac{\pi \times (0.375 \text{ in})^2}{4}$$

$$A_v = 0.11045 \text{ in}^2$$

22.5.8.5.3 $V_{s,b}$ - Shear strength of steel (b)

$$V_{s,b} = \frac{2\, A_v\, f_{ywk}\, d}{s_{ties}}$$

$$V_{s,b} = \frac{2 \times \left(0.11045 \text{ in}^2\right) \times (60 \text{ ksi}) \times (38.4 \text{ in})}{(10 \text{ in})}$$

$$V_{s,b} = 50.894 \text{ kip}$$

V_s - Governing shear strength of steel

$$V_s = MIN\left[V_{s,a}, V_{s,b}\right]$$

$$V_s = MIN\left[(737.28 \text{ kip}), (50.894 \text{ kip})\right]$$

$$V_s = 50.894 \text{ kip}$$

22.5.1.1 ϕV_n - Allowable shear strength

$$\phi V_n = \phi\ (V_c + V_s)$$

$$\phi V_n = (0.65) \times ((120.57 \text{ kip}) + (50.894 \text{ kip}))$$

$$\phi V_n = 111.45 \text{ kip}$$

Considering x-direction:

$V_{max} = 2.9207 \text{ kip}$ - Maximum shear force in the x-direction,
$Ratio$ - Capacity

$$Ratio = \frac{V_{max}}{\phi V_n}$$

$$Ratio = \frac{(2.9207 \text{ kip})}{(111.45 \text{ kip})}$$

$$Ratio = 0.026207$$

Status: **PASS**
Ratio: **0.030**

Considering z-direction:

$V_{max} = 0.098984 \text{ kip}$ - Maximum shear force in the z-direction,
$Ratio$ - Capacity

$$Ratio = \frac{V_{max}}{\phi V_n}$$

$$Ratio = \frac{(0.098984 \text{ kip})}{(111.45 \text{ kip})}$$

$$Ratio = 0.00088815$$

Status: **PASS**
Ratio: **0.000**

Flexural Strength (ACI 318-19, LRFD)

S_m - Section modulus

$$S_m = \frac{b\, D^2}{6}$$

$$S_m = \frac{(48 \text{ in}) \times (48 \text{ in})^2}{6}$$

$$S_m = 18432 \text{ in}^3$$

	$\lambda = 1$ - Concrete modification factor (Normal concrete), Allowable flexural strength: M_n shall be the lesser of: $\phi M_{n,1}$ $$\phi M_{n,1} = \phi \times 5 \times \lambda \times \sqrt{(f_c')} \times S_m$$ $$\phi M_{n,1} = 0.65 \times 5 \times 1 \times \sqrt{(2.5ksi)} \times 18432.001in^3$$ $$\phi M_{n,1} = 249.600kipft$$	
14.5.2.1b	$\phi M_{n,2}$ $$\phi M_{n,2} = \phi\, 0.85\, f_{ck}'\, S_m$$ $$\phi M_{n,2} = (0.65) \times 0.85 \times (2.5 \text{ ksi}) \times \left(18432 \text{ in}^3\right)$$ $$\phi M_{n,2} = 2121.6 \text{ kipft}$$ Therefore, ϕM_n - Allowable flexural strength, $$\phi M_n = MIN[\phi M_{n,1}, \phi M_{n,2}]$$ $$\phi M_n = MIN[(249.6 \text{ kipft}), (2121.6 \text{ kipft})]$$ $$\phi M_n = 249.6 \text{ kipft}$$ **Considering x-direction:** $M_{max} = 6.129 \text{ kipft}$ - Maximum moment in the x-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\phi M_n}$$ $$Ratio = \frac{(6.129 \text{ kipft})}{(249.6 \text{ kipft})}$$ $$Ratio = 0.024555$$	Status: **PASS** Ratio: **0.020**
	Considering z-direction: $M_{max} = 0.19211 \text{ kipft}$ - Maximum moment in the z-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\phi M_n}$$ $$Ratio = \frac{(0.19211 \text{ kipft})}{(249.6 \text{ kipft})}$$ $$Ratio = 0.00076967$$	Status: **PASS** Ratio: **0.000**

REFERENCES	CALCULATIONS	RESULTS

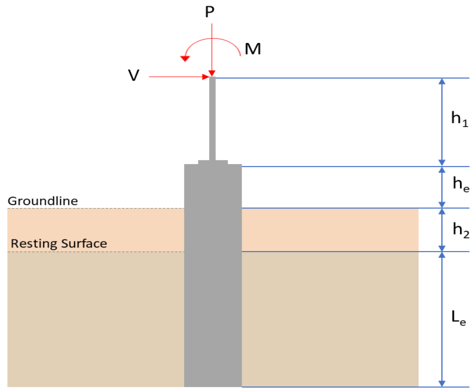
SkyCiv Foundation Design

Pile Foundation

Design Information :

Design code : IBC 2021 (International Building Code)
Unit System : Imperial

Pile Input



Geometry

Pile shape: rectangular
$b = 48\ \mathrm{in}$ - Pile width
$D = 48\ \mathrm{in}$ - Pile depth
$L = 4.25\ \mathrm{ft}$ - Total pile length
$h_1 = 0\ \mathrm{ft}$ - Lateral load height from the top of the pile,
$h_2 = 0\ \mathrm{ft}$ - Depth to resisting surface
$h_e = 0\ \mathrm{ft}$ - Length of pile above the ground

Tabulation of Soil Parameters

Layer	Label	Allowable Bearing Pressure (q_a) (psf)	Allowable Lateral Pressure (R) (psf/ft)
1	Sand, silty sand, clayey sand, silty gravel & clayey gravel	2000.000	150.000

Tabulation of Loads

Load Component	ASD	LRFD
P (kip)	9.824	16.436
V_x (kip)	-0.192	-0.321
V_z (kip)	0.018	0.031
M_x (kipft)	0.063	0.108
M_z (kipft)	6.127	10.910

Material Properties

$f'_{ck} = 2.5\ \mathrm{ksi}$ - Concrete strength,

Required depth to resist lateral loads (ASD)

H - Point of application of the lateral load

$$H = h_1 + h_2 + h_e$$

$$H = (0\ \mathrm{ft}) + (0\ \mathrm{ft}) + (0\ \mathrm{ft})$$

$$H = 0\ \mathrm{ft}$$

Considering x-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\,D}$$

$$H_o = \frac{(-0.192\ \mathrm{kip})}{1.57 \times (48\ \mathrm{in})}$$

$$H_o = -0.030573\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\,H)}{1.57\,D}$$

$$M_o = \frac{(6.127\ \mathrm{kipft}) + ((-0.192\ \mathrm{kip}) \times (0\ \mathrm{ft}))}{1.57 \times (48\ \mathrm{in})}$$

$$M_o = 0.97564\ \mathrm{kipft/ft}$$

Required depth of embedment in earth:

$$L_x^3 - \left(14.14 \times \frac{H_o \times L_x}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,x} = 4.1306\ \mathrm{ft}$ - Required depth in x-direction,

Considering z-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\, b}$$

$$H_o = \frac{(0.018\ \mathrm{kip})}{1.57 \times (48\ \mathrm{in})}$$

$$H_o = 0.0028662\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\, H)}{1.57\, b}$$

$$M_o = \frac{(0.063\ \mathrm{kipft}) + ((0.018\ \mathrm{kip}) \times (0\ \mathrm{ft}))}{1.57 \times (48\ \mathrm{in})}$$

$$M_o = 0.010032\ \mathrm{kipft/ft}$$

Required depth of embedment in earth:

$$L_z^3 - \left(14.14 \times \frac{H_o \times L_z}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,z} = 0.99093\ \mathrm{ft}$ - Required depth in z-direction,

Minimum embedded depth required:

$L_{e,req}$ - Depth of pile required,

$$L_{e,req} = MAX[L_{e,x}, L_{e,z}]$$

$$L_{e,req} = MAX[(4.1306\ \mathrm{ft}), (0.99093\ \mathrm{ft})]$$

$$L_{e,req} = 4.131\ \mathrm{ft}$$

L_e - Actual embedded length of pile,

$$L_e = L - h_e - h_2$$

$$L_e = (4.25\ \mathrm{ft}) - (0\ \mathrm{ft}) - (0\ \mathrm{ft})$$

$$L_e = 4.25\ \mathrm{ft}$$

$Ratio$ - Embedded depth

$$Ratio = \frac{L_{e,req}}{L_e}$$

$$Ratio = \frac{(4.131\ \mathrm{ft})}{(4.25\ \mathrm{ft})}$$

$$Ratio = 0.972$$

Status: PASS
Ratio: 0.970

End-bearing Capacity (ASD)

A - Pile cross-section area

$$A = b\, D$$

$$A = (48\ \mathrm{in}) \times (48\ \mathrm{in})$$

$$A = 16\ \mathrm{ft}^2$$

q - End-bearing pressure

$$q = \frac{P_v}{A}$$

$$q = \frac{(9.824\ \mathrm{kip})}{(16\ \mathrm{ft}^2)}$$

$$q = 0.614\ \mathrm{kip/ft}^2$$

$$q = 0.614\ \mathrm{kip/ft}$$

Check bearing capacity ratio:

$Ratio$ - Capacity

$$Ratio = \frac{q}{q_a}$$

$$Ratio = \frac{(0.614\ \mathrm{kip/ft^2})}{(2000\ \mathrm{psf})}$$

$$Ratio = 0.307$$

Status: **PASS**
Ratio: **0.310**

Czerniak

Lateral Soil Pressure (ASD):

L/D - Length to least lateral dimension ratio,

$$L/D = \frac{L}{D}$$

$$L/D = \frac{(4.25\ \mathrm{ft})}{(48\ \mathrm{in})}$$

$$L/D = 1.0625$$

Since L/D $\leq$ 10,

Pile is short.

Considering x-direction:

$H_o = -0.030573\ \mathrm{kip/ft}$ - Lateral force per length of pile,
$M_o = 0.97564\ \mathrm{kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\, M_o\, L_e) + \left(3\, H_o\, L_e^2\right)}{(6\, M_o) + (4\, H_o\, L_e)}$$

$$a = \frac{(4 \times (0.97564\ \mathrm{kipft/ft}) \times (4.25\ \mathrm{ft})) + \left(3 \times (-0.030573\ \mathrm{kip/ft}) \times (4.25\ \mathrm{ft})^2\right)}{(6 \times (0.97564\ \mathrm{kipft/ft})) + (4 \times (-0.030573\ \mathrm{kip/ft}) \times (4.25\ \mathrm{ft}))}$$

$$a = 2.8622\ \mathrm{ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\ [(4\, M_o) + (3\, H_o\, L_e)]^2}{L_e^2\ [(3\, M_o) + (2\, H_o\, L_e)]}$$

$$p = \frac{0.75 \times [(4 \times (0.97564\ \mathrm{kipft/ft})) + (3 \times (-0.030573\ \mathrm{kip/ft}) \times (4.25\ \mathrm{ft}))]^2}{(4.25\ \mathrm{ft})^2 \times [(3 \times (0.97564\ \mathrm{kipft/ft})) + (2 \times (-0.030573\ \mathrm{kip/ft}) \times (4.25\ \mathrm{ft}))]}$$

$$p = 0.19211\ \mathrm{kip/ft^2}$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\ [(2\, M_o) + (H_o\, L_e)]}{L_e^2}$$

$$s = \frac{6 \times [(2 \times (0.97564\ \mathrm{kipft/ft})) + ((-0.030573\ \mathrm{kip/ft}) \times (4.25\ \mathrm{ft}))]}{(4.25\ \mathrm{ft})^2}$$

$$s = 0.60501\ \mathrm{kip/ft^2}$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\, \frac{a}{2}$$

$$p_a = (150\ \mathrm{psf/ft}) \times \frac{(2.8622\ \mathrm{ft})}{2}$$

$$p_a = 0.21467\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{(0.19211\ \mathrm{kip/ft^2})}{(0.21467\ \mathrm{kip/ft^2})}$$

$$Ratio = 0.89492$$

Status: **PASS**
Ratio: **0.890**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\,L_e$$

$$p_s = (150\ \mathrm{psf/ft}) \times (4.25\ \mathrm{ft})$$

$$p_s = 0.6375\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{\left(0.60501\ \mathrm{kip/ft^2}\right)}{\left(0.6375\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = 0.94904$$

Status: PASS
Ratio: 0.950

Considering z-direction:

$H_o = 0.0028662\ \mathrm{kip/ft}$ - Lateral force per length of pile,
$M_o = 0.010032\ \mathrm{kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (0.010032\ \mathrm{kipft/ft}) \times (4.25\ \mathrm{ft})) + \left(3 \times (0.0028662\ \mathrm{kip/ft}) \times (4.25\ \mathrm{ft})^2\right)}{(6 \times (0.010032\ \mathrm{kipft/ft})) + (4 \times (0.0028662\ \mathrm{kip/ft}) \times (4.25\ \mathrm{ft}))}$$

$$a = 2.9918\ \mathrm{ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\,[(4\,M_o) + (3\,H_o\,L_e)]^2}{L_e^2\,[(3\,M_o) + (2\,H_o\,L_e)]}$$

$$p = \frac{0.75 \times [(4 \times (0.010032\ \mathrm{kipft/ft})) + (3 \times (0.0028662\ \mathrm{kip/ft}) \times (4.25\ \mathrm{ft}))]^2}{(4.25\ \mathrm{ft})^2 \times [(3 \times (0.010032\ \mathrm{kipft/ft})) + (2 \times (0.0028662\ \mathrm{kip/ft}) \times (4.25\ \mathrm{ft}))]}$$

$$p = 0.0044822\ \mathrm{kip/ft^2}$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\,[(2\,M_o) + (H_o\,L_e)]}{L_e^2}$$

$$s = \frac{6 \times [(2 \times (0.010032\ \mathrm{kipft/ft})) + ((0.0028662\ \mathrm{kip/ft}) \times (4.25\ \mathrm{ft}))]}{(4.25\ \mathrm{ft})^2}$$

$$s = 0.010711\ \mathrm{kip/ft^2}$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\,\frac{a}{2}$$

$$p_a = (150\ \mathrm{psf/ft}) \times \frac{(2.9918\ \mathrm{ft})}{2}$$

$$p_a = 0.22438\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{\left(0.0044822\ \mathrm{kip/ft^2}\right)}{\left(0.22438\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = 0.019976$$

Status: PASS
Ratio: 0.020

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\,L_e$$

$$p_s = (150\ \mathrm{psf/ft}) \times (4.25\ \mathrm{ft})$$

$$p_s = 0.6375\ \mathrm{kip/ft^2}$$

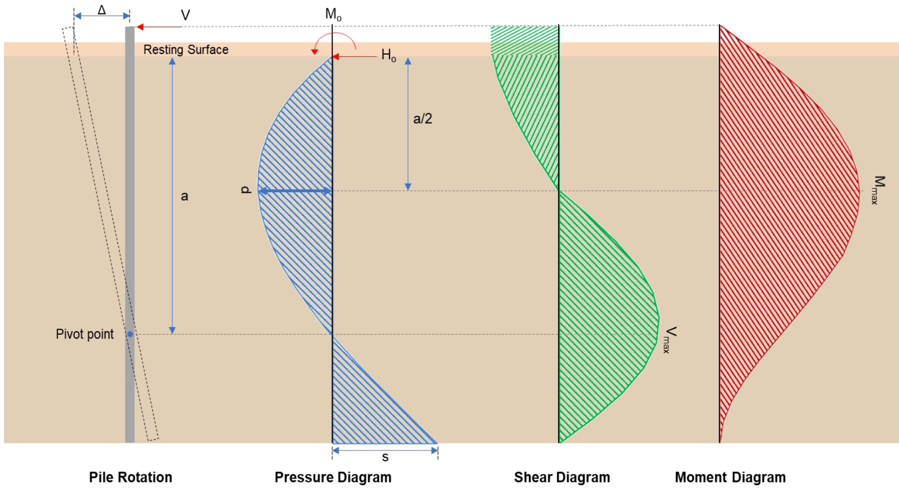
$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{(0.010711\ \mathrm{kip/ft^2})}{(0.6375\ \mathrm{kip/ft^2})}$$

$$Ratio = 0.016802$$

Status: **PASS**
Ratio: **0.020**



Shear force and Bending moment (x-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\,D}$$

$$H_o = \frac{(-0.321\ \mathrm{kip})}{1.57 \times (48\ \mathrm{in})}$$

$$H_o = -0.051115\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\,H)}{1.57\,D}$$

$$M_o = \frac{(10.91\ \mathrm{kipft}) + ((-0.321\ \mathrm{kip}) \times (0\ \mathrm{ft}))}{1.57 \times (48\ \mathrm{in})}$$

$$M_o = 1.7373\ \mathrm{kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(1.7373\ \mathrm{kipft/ft})}{(-0.051115\ \mathrm{kip/ft})}$$

$$E = 33.988\ \mathrm{ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (1.7373\ \mathrm{kipft/ft}) \times (4.25\ \mathrm{ft})) + \left(3 \times (-0.051115\ \mathrm{kip/ft}) \times (4.25\ \mathrm{ft})^2\right)}{(6 \times (1.7373\ \mathrm{kipft/ft})) + (4 \times (-0.051115\ \mathrm{kip/ft}) \times (4.25\ \mathrm{ft}))}$$

$$a = 2.8606\ \mathrm{ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\,D)\left[1 - \left[3\left(\frac{4\,E}{L_e} + 3\right)\left(\frac{a}{L_e}\right)^2\right] + \left[4\left(\frac{3\,E}{L_e} + 2\right)\left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((-0.051115\ \mathrm{kip/ft}) \times (48\ \mathrm{in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (33.988\ \mathrm{ft})}{(4.25\ \mathrm{ft})} + 3\right) \times \left(\frac{(2.8606\ \mathrm{ft})}{(4.25\ \mathrm{ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (33.988\ \mathrm{ft})}{(4.25\ \mathrm{ft})} + 2\right) \times \left(\frac{(2.8606\ \mathrm{ft})}{(4.25\ \mathrm{ft})}\right)^3\right]\right]$$

$$V_{max} = 3.0364\ \mathrm{kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, D\, L_e)\left[\left(\frac{E}{L_e} + \frac{a}{2\,L_e}\right) - \left[\left(\frac{4\,E}{L_e} + 3\right)\left(\frac{a}{2\,L_e}\right)^3\right] + \left[\left(\frac{3\,E}{L_e} + 2\right)\left(\frac{a}{2\,L_e}\right)^4\right]\right]$$

$$M_{max} = ((-0.051115\ \mathrm{kip/ft}) \times (48\ \mathrm{in}) \times (4.25\ \mathrm{ft})) \times \left[\left(\frac{(33.988\ \mathrm{ft})}{(4.25\ \mathrm{ft})} + \frac{(2.8606\ \mathrm{ft})}{2 \times (4.25\ \mathrm{ft})}\right) - \left[\left(\frac{4 \times (33.988\ \mathrm{ft})}{(4.25\ \mathrm{ft})} + 3\right) \times \left(\frac{(2.8606\ \mathrm{ft})}{2 \times (4.25\ \mathrm{ft})}\right)^3\right] + \left[\left(\frac{3 \times (33.988\ \mathrm{ft})}{(4.25\ \mathrm{ft})} + 2\right) \times \left(\frac{(2.8606\ \mathrm{ft})}{2 \times (4.25\ \mathrm{ft})}\right)^4\right]\right]$$

$$M_{max} = 6.3723\ \mathrm{kipft}$$

Shear force and Bending moment (z-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\,b}$$

$$H_o = \frac{(0.031\ \mathrm{kip})}{1.57 \times (48\ \mathrm{in})}$$

$$H_o = 0.0049363\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\, H)}{1.57\,b}$$

$$M_o = \frac{(0.108\ \mathrm{kipft}) + ((0.031\ \mathrm{kip}) \times (0\ \mathrm{ft}))}{1.57 \times (48\ \mathrm{in})}$$

$$M_o = 0.017197\ \mathrm{kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(0.017197\ \mathrm{kipft/ft})}{(0.0049363\ \mathrm{kip/ft})}$$

$$E = 3.4839\ \mathrm{ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (0.017197\ \mathrm{kipft/ft}) \times (4.25\ \mathrm{ft})) + \left(3 \times (0.0049363\ \mathrm{kip/ft}) \times (4.25\ \mathrm{ft})^2\right)}{(6 \times (0.017197\ \mathrm{kipft/ft})) + (4 \times (0.0049363\ \mathrm{kip/ft}) \times (4.25\ \mathrm{ft}))}$$

$$a = 2.9922\ \mathrm{ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\, b)\left[1 - \left[3\left(\frac{4\,E}{L_e} + 3\right)\left(\frac{a}{L_e}\right)^2\right] + \left[4\left(\frac{3\,E}{L_e} + 2\right)\left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((0.0049363\ \mathrm{kip/ft}) \times (48\ \mathrm{in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (3.4839\ \mathrm{ft})}{(4.25\ \mathrm{ft})} + 3\right) \times \left(\frac{(2.9922\ \mathrm{ft})}{(4.25\ \mathrm{ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (3.4839\ \mathrm{ft})}{(4.25\ \mathrm{ft})} + 2\right) \times \left(\frac{(2.9922\ \mathrm{ft})}{(4.25\ \mathrm{ft})}\right)^3\right]\right]$$

$$V_{max} = 0.041708\ \mathrm{kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, b\, L_e)\left[\left(\frac{E}{L_e} + \frac{a}{2\,L_e}\right) - \left[\left(\frac{4\,E}{L_e} + 3\right)\left(\frac{a}{2\,L_e}\right)^3\right] + \left[\left(\frac{3\,E}{L_e} + 2\right)\left(\frac{a}{2\,L_e}\right)^4\right]\right]$$

$$M_{max} = ((0.0049363\ \mathrm{kip/ft}) \times (48\ \mathrm{in}) \times (4.25\ \mathrm{ft})) \times \left[\left(\frac{(3.4839\ \mathrm{ft})}{(4.25\ \mathrm{ft})} + \frac{(2.9922\ \mathrm{ft})}{2 \times (4.25\ \mathrm{ft})}\right) - \left[\left(\frac{4 \times (3.4839\ \mathrm{ft})}{(4.25\ \mathrm{ft})} + 3\right) \times \left(\frac{(2.9922\ \mathrm{ft})}{2 \times (4.25\ \mathrm{ft})}\right)^3\right] + \left[\left(\frac{3 \times (3.4839\ \mathrm{ft})}{(4.25\ \mathrm{ft})} + 2\right) \times \left(\frac{(2.9922\ \mathrm{ft})}{2 \times (4.25\ \mathrm{ft})}\right)^4\right]\right]$$

$$M_{max} = 0.081092 \text{ kipft}$$

Minimum Reinforcement Check (LRFD)

Parameters:

$f'_{ck} = 2.5 \text{ ksi}$ - Concrete strength,
$f_{yk} = 60 \text{ ksi}$ - Longitudinal reinforcement strength,
$\phi = 0.65$ - Reduction factor for axial strength,
Table 22.4.2.1 $\alpha = 0.8$ - Alpha factor for axial strength,
$A_g = 2304 \text{ in}^2$ - Gross area of concrete,

Longitudinal reinforcement:

Required reinforcement due to axial load, $A_{st,required}$
22.4.2.2, 10.6.1.1 $A_{st,required}$

$$A_{st,required} = Min\left[\frac{\frac{P}{\phi\ \alpha} - (0.85\ f'_{ck}\ A_g)}{f_{yk} - (0.85\ f'_{ck})}, (0.08\ A_g)\right]$$

$$A_{st,required} = Min\left[\frac{\frac{(16.436 \text{ kip})}{(0.65)\times(0.8)} - (0.85 \times (2.5 \text{ ksi}) \times (2304 \text{ in}^2))}{(60 \text{ ksi}) - (0.85 \times (2.5 \text{ ksi}))}, (0.08 \times (2304 \text{ in}^2))\right]$$

$$A_{st,required} = -84.05 \text{ in}^2$$

A_{min} - Governing minimum reinforcement area,

$$A_{min} = Max\left[A_{st,required}, (0.0018\ A_g)\right]$$

$$A_{min} = Max\left[(-84.05 \text{ in}^2), (0.0018 \times (2304 \text{ in}^2))\right]$$

$$A_{min} = 4.1472 \text{ in}^2$$

n_{rebar} - Required number of reinforcement,

$$n_{rebar} = \frac{A_{min}}{A_{rebar}}$$

$$n_{rebar} = \frac{(4.1472 \text{ in}^2)}{(0.3068 \text{ in}^2)}$$

$$n_{rebar} = 14$$

A_{st} - Actual total reinforcement area,

$$A_{st} = n_{rebar}\ \frac{\pi\ d_{bar}^2}{4}$$

$$A_{st} = (14) \times \frac{\pi \times (0.625 \text{ in})^2}{4}$$

$$A_{st} = 4.2951 \text{ in}^2$$

$Ratio$ - Capacity

$$Ratio = \frac{A_{min}}{A_{st}}$$

$$Ratio = \frac{(4.1472 \text{ in}^2)}{(4.2951 \text{ in}^2)}$$

$$Ratio = 0.96556$$

Status: **PASS**
Ratio: **0.970**

25.2.3 s_{rebar} - Minimum spacing of reinforcement,

$$s_{rebar} = Max\left[1.5, (1.5\ d_{bar})\right]$$

$$s_{rebar} = Max\left[1.5, (1.5 \times (0.625 \text{ in}))\right]$$

$$s_{rebar} = 1.5 \text{ in}$$

Ties:

25.7.2.2 Since longitudinal reinforcement is $\leq$ No. 10ø: Use #3(0.375 in)
25.7.2.1 s_{ties} - Maximum spacing of ties,

$$s_{ties} = Min\left[(16\ d_{bar}), (48\ d_{ties}), Min(D, b)\right]$$

$$s_{ties} = Min\left[(16 \times (0.625 \text{ in})), (48 \times (0.375 \text{ in})), Min((48 \text{ in}), (48 \text{ in}))\right]$$

$$s_{ties} = 10 \text{ in}$$

Summary:

Main reinforcement: **14 - #5 (0.625 in)**

	Ties: **#3(0.375 in) - 10 in**	
	Axial Compression Strength (ACI 318-19, LRFD)	
22.4.2.2	ϕP_N - Allowable axial compressive strength $$\phi P_N = \phi\, 0.80\, \left[\left(0.85\, f'_{ck}\, [A_g - A_{st}]\right) + (f_{yk}\, A_{st})\right]$$ $$\phi P_N = (0.65) \times 0.80 \times \left[\left(0.85 \times (2.5\text{ ksi}) \times \left[\left(2304\text{ in}^2\right) - \left(4.2951\text{ in}^2\right)\right]\right) + \left((60\text{ ksi}) \times \left(4.2951\text{ in}^2\right)\right)\right]$$ $$\phi P_N = 2675.2\text{ kip}$$ $Ratio$ - Capacity $$Ratio = \frac{P}{\phi P_N}$$ $$Ratio = \frac{(16.436\text{ kip})}{(2675.2\text{ kip})}$$ $$Ratio = 0.0061439$$	Status: **PASS** Ratio: **0.010**
	Shear Strength (ACI 318-19, LRFD) **Parameters:** $b_w = 48\text{ in}$ - Effective width,	
22.5.2.2	d - Effective depth $$d = 0.80\, D$$ $$d = 0.80 \times (48\text{ in})$$ $$d = 38.4\text{ in}$$	
22.5.5.1.3	λ_s - size effect modification factor $$\lambda_s = MIN\left[\sqrt{\frac{2}{1+\frac{d}{10}}}, 1\right]$$ $$\lambda_s = MIN\left[\sqrt{\frac{2}{1+\frac{(38.4\text{ in})}{10}}}, 1\right]$$ $$\lambda_s = 0.64282$$ The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5\text{ ksi} \rightarrow 2500\text{ psi}$,	
22.5.5.1.1	$V_{c,max}$ - Max shear strength of concrete $$V_{c,max} = 5\, \lambda_s\, \sqrt{f'_{ck}}\, b_w\, d$$ $$V_{c,max} = 5 \times (0.64282) \times \sqrt{(2500\text{ psi})} \times (48\text{ in}) \times (38.4\text{ in})$$ $$V_{c,max} = 296.21\text{ kip}$$ The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5\text{ ksi} \rightarrow 2500\text{ psi}$, $P = 16.436\text{ kip} \rightarrow 16436\text{ lbf}$,	
22.5.5.1.1(a)	$V_{c,a}$ - Shear strength of concrete (a) $$V_{c,a} = \left[2\, \lambda_s\, \sqrt{f'_{ck}} + \frac{P}{6\, A_g}\right]\, b_w\, d$$ $$V_{c,a} = \left[2 \times (0.64282) \times \sqrt{(2500\text{ psi})} + \frac{(16436\text{ lbf})}{6 \times (2304\text{ in}^2)}\right] \times (48\text{ in}) \times (38.4\text{ in})$$ $$V_{c,a} = 120.68\text{ kip}$$ The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5\text{ ksi} \rightarrow 2500\text{ psi}$,	
22.5.5.1.2	$V_{c,b}$ - Shear strength of concrete (b) $$V_{c,b} = \left[2\, \lambda_s\, \sqrt{f'_{ck}} + \left(0.05\, f'_{ck}\right)\right]\, b_w\, d$$ $$V_{c,b} = \left[2 \times (0.64282) \times \sqrt{(2500\text{ psi})} + (0.05 \times (2500\text{ psi}))\right] \times (48\text{ in}) \times (38.4\text{ in})$$ $$V_{c,b} = 348.89\text{ kip}$$ V_c - Governing shear strength of concrete $$V_c = Min\left[V_{c,max}, V_{c,a}, V_{c,b}\right]$$ $$V_c = Min\left[(296.21\text{ kip}), (120.68\text{ kip}), (348.89\text{ kip})\right]$$ $$V_c = 120.68\text{ kip}$$	

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$,

22.5.1.2 $V_{s,a}$ - Shear strength of steel (a)

$$V_{s,a} = 8 \sqrt{f'_{ck}}\, b_w\, d$$

$$V_{s,a} = 8 \times \sqrt{(2500 \text{ psi})} \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{s,a} = 737.28 \text{ kip}$$

A_v - Ties rebar area,

$$A_v = \frac{\pi\, d_{ties}^2}{4}$$

$$A_v = \frac{\pi \times (0.375 \text{ in})^2}{4}$$

$$A_v = 0.11045 \text{ in}^2$$

22.5.8.5.3 $V_{s,b}$ - Shear strength of steel (b)

$$V_{s,b} = \frac{2\, A_v\, f_{ywk}\, d}{s_{ties}}$$

$$V_{s,b} = \frac{2 \times \left(0.11045 \text{ in}^2\right) \times (60 \text{ ksi}) \times (38.4 \text{ in})}{(10 \text{ in})}$$

$$V_{s,b} = 50.894 \text{ kip}$$

V_s - Governing shear strength of steel

$$V_s = MIN\left[V_{s,a}, V_{s,b}\right]$$

$$V_s = MIN\left[(737.28 \text{ kip}), (50.894 \text{ kip})\right]$$

$$V_s = 50.894 \text{ kip}$$

22.5.1.1 ϕV_n - Allowable shear strength

$$\phi V_n = \phi\ (V_c + V_s)$$

$$\phi V_n = (0.65) \times ((120.68 \text{ kip}) + (50.894 \text{ kip}))$$

$$\phi V_n = 111.52 \text{ kip}$$

Considering x-direction:

$V_{max} = 3.0364 \text{ kip}$ - Maximum shear force in the x-direction,
$Ratio$ - Capacity

$$Ratio = \frac{V_{max}}{\phi V_n}$$

$$Ratio = \frac{(3.0364 \text{ kip})}{(111.52 \text{ kip})}$$

$$Ratio = 0.027227$$

Status: **PASS**
Ratio: **0.030**

Considering z-direction:

$V_{max} = 0.041708 \text{ kip}$ - Maximum shear force in the z-direction,
$Ratio$ - Capacity

$$Ratio = \frac{V_{max}}{\phi V_n}$$

$$Ratio = \frac{(0.041708 \text{ kip})}{(111.52 \text{ kip})}$$

$$Ratio = 0.00037399$$

Status: **PASS**
Ratio: **0.000**

Flexural Strength (ACI 318-19, LRFD)

S_m - Section modulus

$$S_m = \frac{b\, D^2}{6}$$

$$S_m = \frac{(48 \text{ in}) \times (48 \text{ in})^2}{6}$$

$$S_m = 18432 \text{ in}^3$$

	$\lambda = 1$ - Concrete modification factor (Normal concrete), Allowable flexural strength: M_n shall be the lesser of: $\phi M_{n,1}$ $$\phi M_{n,1} = \phi \times 5 \times \lambda \times \sqrt{(f_c')} \times S_m$$ $$\phi M_{n,1} = 0.65 \times 5 \times 1 \times \sqrt{(2.5ksi)} \times 18432.001in^3$$ $$\phi M_{n,1} = 249.600kipft$$	
14.5.2.1b	$\phi M_{n,2}$ $$\phi M_{n,2} = \phi\, 0.85\, f'_{ck}\, S_m$$ $$\phi M_{n,2} = (0.65) \times 0.85 \times (2.5\ \mathrm{ksi}) \times \left(18432\ \mathrm{in}^3\right)$$ $$\phi M_{n,2} = 2121.6\ \mathrm{kipft}$$ Therefore, ϕM_n - Allowable flexural strength, $$\phi M_n = MIN[\phi M_{n,1}, \phi M_{n,2}]$$ $$\phi M_n = MIN[(249.6\ \mathrm{kipft}), (2121.6\ \mathrm{kipft})]$$ $$\phi M_n = 249.6\ \mathrm{kipft}$$ **Considering x-direction:** $M_{max} = 6.3723\ \mathrm{kipft}$ - Maximum moment in the x-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\phi M_n}$$ $$Ratio = \frac{(6.3723\ \mathrm{kipft})}{(249.6\ \mathrm{kipft})}$$ $$Ratio = 0.02553$$	Status: **PASS** Ratio: **0.030**
	Considering z-direction: $M_{max} = 0.081092\ \mathrm{kipft}$ - Maximum moment in the z-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\phi M_n}$$ $$Ratio = \frac{(0.081092\ \mathrm{kipft})}{(249.6\ \mathrm{kipft})}$$ $$Ratio = 0.00032489$$	Status: **PASS** Ratio: **0.000**

REFERENCES	CALCULATIONS	RESULTS

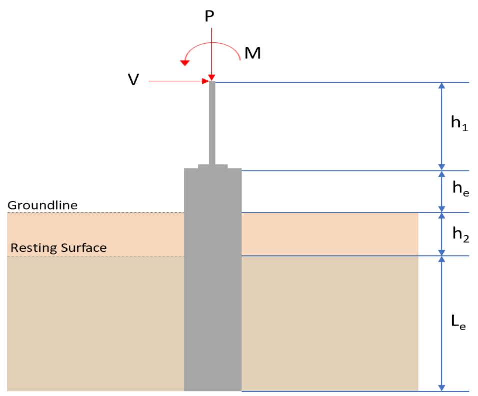
SkyCiv Foundation Design

Pile Foundation

Design Information :

Design code : IBC 2021 (International Building Code)
Unit System : Imperial

Pile Input



Geometry

Pile shape: rectangular
$b = 48\ \mathrm{in}$ - Pile width
$D = 48\ \mathrm{in}$ - Pile depth
$L = 4.25\ \mathrm{ft}$ - Total pile length
$h_1 = 0\ \mathrm{ft}$ - Lateral load height from the top of the pile,
$h_2 = 0\ \mathrm{ft}$ - Depth to resisting surface
$h_e = 0\ \mathrm{ft}$ - Length of pile above the ground

Tabulation of Soil Parameters

Layer	Label	Allowable Bearing Pressure (q_a) (psf)	Allowable Lateral Pressure (R) $(\mathrm{psf/ft})$
1	Sand, silty sand, clayey sand, silty gravel & clayey gravel	2000.000	150.000

Tabulation of Loads

Load Component	ASD	LRFD
P (kip)	9.824	16.436
V_x (kip)	-0.192	-0.321
V_z (kip)	-0.018	-0.031
M_x (kipft)	-0.063	-0.108
M_z (kipft)	6.127	10.910

Material Properties

$f'_{ck} = 2.5\ \mathrm{ksi}$ - Concrete strength,

Required depth to resist lateral loads (ASD)

H - Point of application of the lateral load

$$H = h_1 + h_2 + h_e$$

$$H = (0\ \mathrm{ft}) + (0\ \mathrm{ft}) + (0\ \mathrm{ft})$$

$$H = 0\ \mathrm{ft}$$

Considering x-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\ D}$$

$$H_o = \frac{(-0.192\ \mathrm{kip})}{1.57 \times (48\ \mathrm{in})}$$

$$H_o = -0.030573\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\ H)}{1.57\ D}$$

$$M_o = \frac{(6.127\ \text{kipft}) + ((-0.192\ \text{kip}) \times (0\ \text{ft}))}{1.57 \times (48\ \text{in})}$$

$$M_o = 0.97564\ \text{kipft/ft}$$

Required depth of embedment in earth:

$$L_x^3 - \left(14.14 \times \frac{H_o \times L_x}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,x} = 4.1306\ \text{ft}$ - Required depth in x-direction,

Considering z-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\, b}$$

$$H_o = \frac{(-0.018\ \text{kip})}{1.57 \times (48\ \text{in})}$$

$$H_o = -0.0028662\ \text{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\, H)}{1.57\, b}$$

$$M_o = \frac{(0.063\ \text{kipft}) + ((-0.018\ \text{kip}) \times (0\ \text{ft}))}{1.57 \times (48\ \text{in})}$$

$$M_o = 0.010032\ \text{kipft/ft}$$

Required depth of embedment in earth:

$$L_z^3 - \left(14.14 \times \frac{H_o \times L_z}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,z} = 0.86757\ \text{ft}$ - Required depth in z-direction,

Minimum embedded depth required:

$L_{e,req}$ - Depth of pile required,

$$L_{e,req} = MAX[L_{e,x}, L_{e,z}]$$

$$L_{e,req} = MAX[(4.1306\ \text{ft}), (0.86757\ \text{ft})]$$

$$L_{e,req} = 4.131\ \text{ft}$$

L_e - Actual embedded length of pile,

$$L_e = L - h_e - h_2$$

$$L_e = (4.25\ \text{ft}) - (0\ \text{ft}) - (0\ \text{ft})$$

$$L_e = 4.25\ \text{ft}$$

$Ratio$ - Embedded depth

$$Ratio = \frac{L_{e,req}}{L_e}$$

$$Ratio = \frac{(4.131\ \text{ft})}{(4.25\ \text{ft})}$$

$$Ratio = 0.972$$

Status: **PASS**
Ratio: **0.970**

End-bearing Capacity (ASD)

A - Pile cross-section area

$$A = b\, D$$

$$A = (48\ \text{in}) \times (48\ \text{in})$$

$$A = 16\ \text{ft}^2$$

q - End-bearing pressure

$$q = \frac{P_v}{A}$$

$$q = \frac{(9.824\ \text{kip})}{(16\ \text{ft}^2)}$$

$$q = 0.614\ \text{kip/ft}^2$$

$q = 0.614\ \mathrm{kip/ft}$

Check bearing capacity ratio:

$Ratio$ - Capacity

$$Ratio = \frac{q}{q_a}$$

$$Ratio = \frac{\left(0.614\ \mathrm{kip/ft^2}\right)}{\left(2000\ \mathrm{psf}\right)}$$

$$Ratio = 0.307$$

Status: **PASS**
Ratio: **0.310**

Czerniak

Lateral Soil Pressure (ASD):

L/D - Length to least lateral dimension ratio,

$$L/D = \frac{L}{D}$$

$$L/D = \frac{(4.25\ \mathrm{ft})}{(48\ \mathrm{in})}$$

$$L/D = 1.0625$$

Since L/D $\leq$ 10,

Pile is short.

Considering x-direction:

$H_o = -0.030573\ \mathrm{kip/ft}$ - Lateral force per length of pile,
$M_o = 0.97564\ \mathrm{kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (0.97564\ \mathrm{kipft/ft}) \times (4.25\ \mathrm{ft})) + \left(3 \times (-0.030573\ \mathrm{kip/ft}) \times (4.25\ \mathrm{ft})^2\right)}{(6 \times (0.97564\ \mathrm{kipft/ft})) + (4 \times (-0.030573\ \mathrm{kip/ft}) \times (4.25\ \mathrm{ft}))}$$

$$a = 2.8622\ \mathrm{ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\,[(4\,M_o) + (3\,H_o\,L_e)]^2}{L_e^2\,[(3\,M_o) + (2\,H_o\,L_e)]}$$

$$p = \frac{0.75 \times [(4 \times (0.97564\ \mathrm{kipft/ft})) + (3 \times (-0.030573\ \mathrm{kip/ft}) \times (4.25\ \mathrm{ft}))]^2}{(4.25\ \mathrm{ft})^2 \times [(3 \times (0.97564\ \mathrm{kipft/ft})) + (2 \times (-0.030573\ \mathrm{kip/ft}) \times (4.25\ \mathrm{ft}))]}$$

$$p = 0.19211\ \mathrm{kip/ft^2}$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\,[(2\,M_o) + (H_o\,L_e)]}{L_e^2}$$

$$s = \frac{6 \times [(2 \times (0.97564\ \mathrm{kipft/ft})) + ((-0.030573\ \mathrm{kip/ft}) \times (4.25\ \mathrm{ft}))]}{(4.25\ \mathrm{ft})^2}$$

$$s = 0.60501\ \mathrm{kip/ft^2}$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\,\frac{a}{2}$$

$$p_a = (150\ \mathrm{psf/ft}) \times \frac{(2.8622\ \mathrm{ft})}{2}$$

$$p_a = 0.21467\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{\left(0.19211\ \mathrm{kip/ft^2}\right)}{\left(0.21467\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = 0.89492$$

Status: **PASS**
Ratio: **0.890**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\, L_e$$

$$p_s = (150 \text{ psf/ft}) \times (4.25 \text{ ft})$$

$$p_s = 0.6375 \text{ kip/ft}^2$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{\left(0.60501 \text{ kip/ft}^2\right)}{\left(0.6375 \text{ kip/ft}^2\right)}$$

$$Ratio = 0.94904$$

Status: PASS
Ratio: 0.950

Considering z-direction:

$H_o = -0.0028662 \text{ kip/ft}$ - Lateral force per length of pile,
$M_o = 0.010032 \text{ kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\, M_o\, L_e) + \left(3\, H_o\, L_e^2\right)}{(6\, M_o) + (4\, H_o\, L_e)}$$

$$a = \frac{(4 \times (0.010032 \text{ kipft/ft}) \times (4.25 \text{ ft})) + \left(3 \times (-0.0028662 \text{ kip/ft}) \times (4.25 \text{ ft})^2\right)}{(6 \times (0.010032 \text{ kipft/ft})) + (4 \times (-0.0028662 \text{ kip/ft}) \times (4.25 \text{ ft}))}$$

$$a = 2.9918 \text{ ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\, [(4\, M_o) + (3\, H_o\, L_e)]^2}{L_e^2\, [(3\, M_o) + (2\, H_o\, L_e)]}$$

$$p = \frac{0.75 \times [(4 \times (0.010032 \text{ kipft/ft})) + (3 \times (-0.0028662 \text{ kip/ft}) \times (4.25 \text{ ft}))]^{\,2}}{(4.25 \text{ ft})^2 \times [(3 \times (0.010032 \text{ kipft/ft})) + (2 \times (-0.0028662 \text{ kip/ft}) \times (4.25 \text{ ft}))]}$$

$$p = 0.00009298 \text{ kip/ft}^2$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\, [(2\, M_o) + (H_o\, L_e)]}{L_e^2}$$

$$s = \frac{6 \times [(2 \times (0.010032 \text{ kipft/ft})) + ((-0.0028662 \text{ kip/ft}) \times (4.25 \text{ ft}))]}{(4.25 \text{ ft})^2}$$

$$s = 0.0026183 \text{ kip/ft}^2$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\, \frac{a}{2}$$

$$p_a = (150 \text{ psf/ft}) \times \frac{(2.9918 \text{ ft})}{2}$$

$$p_a = 0.22438 \text{ kip/ft}^2$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{\left(0.00009298 \text{ kip/ft}^2\right)}{\left(0.22438 \text{ kip/ft}^2\right)}$$

$$Ratio = 0.00041438$$

Status: PASS
Ratio: 0.000

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\, L_e$$

$$p_s = (150 \text{ psf/ft}) \times (4.25 \text{ ft})$$

$$p_s = 0.6375 \text{ kip/ft}^2$$

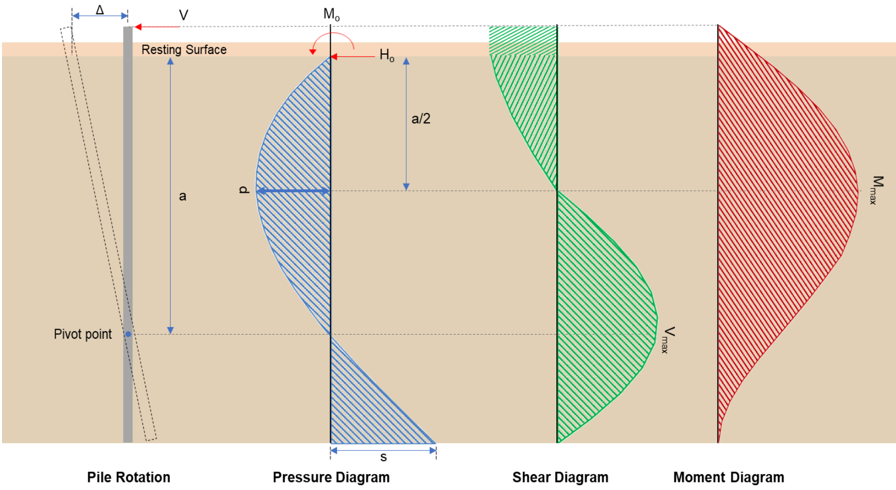
$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{(0.0026183\ \text{kip/ft}^2)}{(0.6375\ \text{kip/ft}^2)}$$

$$Ratio = 0.0041071$$

Status: **PASS**
Ratio: **0.000**



Shear force and Bending moment (x-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\ D}$$

$$H_o = \frac{(-0.321\ \text{kip})}{1.57 \times (48\ \text{in})}$$

$$H_o = -0.051115\ \text{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\ H)}{1.57\ D}$$

$$M_o = \frac{(10.91\ \text{kipft}) + ((-0.321\ \text{kip}) \times (0\ \text{ft}))}{1.57 \times (48\ \text{in})}$$

$$M_o = 1.7373\ \text{kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(1.7373\ \text{kipft/ft})}{(-0.051115\ \text{kip/ft})}$$

$$E = 33.988\ \text{ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\ M_o\ L_e) + \left(3\ H_o\ L_e^2\right)}{(6\ M_o) + (4\ H_o\ L_e)}$$

$$a = \frac{(4 \times (1.7373\ \text{kipft/ft}) \times (4.25\ \text{ft})) + \left(3 \times (-0.051115\ \text{kip/ft}) \times (4.25\ \text{ft})^2\right)}{(6 \times (1.7373\ \text{kipft/ft})) + (4 \times (-0.051115\ \text{kip/ft}) \times (4.25\ \text{ft}))}$$

$$a = 2.8606\ \text{ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\ D)\left[1 - \left[3\left(\frac{4\ E}{L_e} + 3\right)\left(\frac{a}{L_e}\right)^2\right] + \left[4\left(\frac{3\ E}{L_e} + 2\right)\left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((-0.051115\ \text{kip/ft}) \times (48\ \text{in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (33.988\ \text{ft})}{(4.25\ \text{ft})} + 3\right) \times \left(\frac{(2.8606\ \text{ft})}{(4.25\ \text{ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (33.988\ \text{ft})}{(4.25\ \text{ft})} + 2\right) \times \left(\frac{(2.8606\ \text{ft})}{(4.25\ \text{ft})}\right)^3\right]\right]$$

$$V_{max} = 3.0364\ \text{kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, D\, L_e) \left[\left(\frac{E}{L_e} + \frac{a}{2\, L_e} \right) - \left[\left(\frac{4\, E}{L_e} + 3 \right) \left(\frac{a}{2\, L_e} \right)^3 \right] + \left[\left(\frac{3\, E}{L_e} + 2 \right) \left(\frac{a}{2\, L_e} \right)^4 \right] \right]$$

$$M_{max} = ((-0.051115 \text{ kip/ft}) \times (48 \text{ in}) \times (4.25 \text{ ft})) \times \left[\left(\frac{(33.988 \text{ ft})}{(4.25 \text{ ft})} + \frac{(2.8606 \text{ ft})}{2 \times (4.25 \text{ ft})} \right) - \left[\left(\frac{4 \times (33.988 \text{ ft})}{(4.25 \text{ ft})} + 3 \right) \times \left(\frac{(2.8606 \text{ ft})}{2 \times (4.25 \text{ ft})} \right)^3 \right] + \left[\left(\frac{3 \times (33.988 \text{ ft})}{(4.25 \text{ ft})} + 2 \right) \times \left(\frac{(2.8606 \text{ ft})}{2 \times (4.25 \text{ ft})} \right)^4 \right] \right]$$

$$M_{max} = 6.3723 \text{ kipft}$$

Shear force and Bending moment (z-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\, b}$$

$$H_o = \frac{(-0.031 \text{ kip})}{1.57 \times (48 \text{ in})}$$

$$H_o = -0.0049363 \text{ kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\, H)}{1.57\, b}$$

$$M_o = \frac{(0.108 \text{ kipft}) + ((-0.031 \text{ kip}) \times (0 \text{ ft}))}{1.57 \times (48 \text{ in})}$$

$$M_o = 0.017197 \text{ kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(0.017197 \text{ kipft/ft})}{(-0.0049363 \text{ kip/ft})}$$

$$E = 3.4839 \text{ ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\, M_o\, L_e) + \left(3\, H_o\, L_e^2\right)}{(6\, M_o) + (4\, H_o\, L_e)}$$

$$a = \frac{(4 \times (0.017197 \text{ kipft/ft}) \times (4.25 \text{ ft})) + \left(3 \times (-0.0049363 \text{ kip/ft}) \times (4.25 \text{ ft})^2\right)}{(6 \times (0.017197 \text{ kipft/ft})) + (4 \times (-0.0049363 \text{ kip/ft}) \times (4.25 \text{ ft}))}$$

$$a = 2.9922 \text{ ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\, b) \left[1 - \left[3 \left(\frac{4\, E}{L_e} + 3 \right) \left(\frac{a}{L_e} \right)^2 \right] + \left[4 \left(\frac{3\, E}{L_e} + 2 \right) \left(\frac{a}{L_e} \right)^3 \right] \right]$$

$$V_{max} = ((-0.0049363 \text{ kip/ft}) \times (48 \text{ in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (3.4839 \text{ ft})}{(4.25 \text{ ft})} + 3 \right) \times \left(\frac{(2.9922 \text{ ft})}{(4.25 \text{ ft})} \right)^2 \right] + \left[4 \times \left(\frac{3 \times (3.4839 \text{ ft})}{(4.25 \text{ ft})} + 2 \right) \times \left(\frac{(2.9922 \text{ ft})}{(4.25 \text{ ft})} \right)^3 \right] \right]$$

$$V_{max} = 0.041708 \text{ kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, b\, L_e) \left[\left(\frac{E}{L_e} + \frac{a}{2\, L_e} \right) - \left[\left(\frac{4\, E}{L_e} + 3 \right) \left(\frac{a}{2\, L_e} \right)^3 \right] + \left[\left(\frac{3\, E}{L_e} + 2 \right) \left(\frac{a}{2\, L_e} \right)^4 \right] \right]$$

$$M_{max} = ((-0.0049363 \text{ kip/ft}) \times (48 \text{ in}) \times (4.25 \text{ ft})) \times \left[\left(\frac{(3.4839 \text{ ft})}{(4.25 \text{ ft})} + \frac{(2.9922 \text{ ft})}{2 \times (4.25 \text{ ft})} \right) - \left[\left(\frac{4 \times (3.4839 \text{ ft})}{(4.25 \text{ ft})} + 3 \right) \times \left(\frac{(2.9922 \text{ ft})}{2 \times (4.25 \text{ ft})} \right)^3 \right] + \left[\left(\frac{3 \times (3.4839 \text{ ft})}{(4.25 \text{ ft})} + 2 \right) \times \left(\frac{(2.9922 \text{ ft})}{2 \times (4.25 \text{ ft})} \right)^4 \right] \right]$$

$$M_{max} = 0.081092 \text{ kipft}$$

Minimum Reinforcement Check (LRFD)

Parameters:

$f'_{ck} = 2.5 \text{ ksi}$ - Concrete strength,
$f_{yk} = 60 \text{ ksi}$ - Longitudinal reinforcement strength,
$\phi = 0.65$ - Reduction factor for axial strength,
Table 22.4.2.1 $\alpha = 0.8$ - Alpha factor for axial strength,
$A_g = 2304 \text{ in}^2$ - Gross area of concrete,

Longitudinal reinforcement:

Required reinforcement due to axial load, $A_{st,required}$
22.4.2.2, 10.6.1.1 $A_{st,required}$

$$A_{st,required} = Min\left[\frac{\frac{P}{\phi\ \alpha} - (0.85\ f'_{ck}\ A_g)}{f_{yk} - (0.85\ f'_{ck})}, (0.08\ A_g)\right]$$

$$A_{st,required} = Min\left[\frac{\frac{(16.436 \text{ kip})}{(0.65)\times(0.8)} - \left(0.85 \times (2.5 \text{ ksi}) \times \left(2304 \text{ in}^2\right)\right)}{(60 \text{ ksi}) - (0.85 \times (2.5 \text{ ksi}))}, \left(0.08 \times \left(2304 \text{ in}^2\right)\right)\right]$$

$$A_{st,required} = -84.05 \text{ in}^2$$

A_{min} - Governing minimum reinforcement area,

$$A_{min} = Max\left[A_{st,required}, (0.0018\ A_g)\right]$$

$$A_{min} = Max\left[\left(-84.05 \text{ in}^2\right), \left(0.0018 \times \left(2304 \text{ in}^2\right)\right)\right]$$

$$A_{min} = 4.1472 \text{ in}^2$$

n_{rebar} - Required number of reinforcement,

$$n_{rebar} = \frac{A_{min}}{A_{rebar}}$$

$$n_{rebar} = \frac{\left(4.1472 \text{ in}^2\right)}{\left(0.3068 \text{ in}^2\right)}$$

$$n_{rebar} = 14$$

A_{st} - Actual total reinforcement area,

$$A_{st} = n_{rebar}\ \frac{\pi\ d_{bar}^2}{4}$$

$$A_{st} = (14) \times \frac{\pi \times (0.625 \text{ in})^2}{4}$$

$$A_{st} = 4.2951 \text{ in}^2$$

$Ratio$ - Capacity

$$Ratio = \frac{A_{min}}{A_{st}}$$

$$Ratio = \frac{\left(4.1472 \text{ in}^2\right)}{\left(4.2951 \text{ in}^2\right)}$$

$$Ratio = 0.96556$$

Status: **PASS**
Ratio: **0.970**

25.2.3 s_{rebar} - Minimum spacing of reinforcement,

$$s_{rebar} = Max\left[1.5, (1.5\ d_{bar})\right]$$

$$s_{rebar} = Max\left[1.5, (1.5 \times (0.625 \text{ in}))\right]$$

$$s_{rebar} = 1.5 \text{ in}$$

Ties:

25.7.2.2 Since longitudinal reinforcement is $\leq$ No. 10ø: Use #3(0.375 in)
25.7.2.1 s_{ties} - Maximum spacing of ties,

$$s_{ties} = Min\left[(16\ d_{bar}), (48\ d_{ties}), Min(D, b)\right]$$

$$s_{ties} = Min\left[(16 \times (0.625 \text{ in})), (48 \times (0.375 \text{ in})), Min((48 \text{ in}), (48 \text{ in}))\right]$$

$$s_{ties} = 10 \text{ in}$$

Summary:

Main reinforcement: **14 - #5 (0.625 in)**

	Ties: **#3(0.375 in) - 10 in**	

Axial Compression Strength (ACI 318-19, LRFD)

22.4.2.2 ϕP_N - Allowable axial compressive strength

$$\phi P_N = \phi\, 0.80\, \left[\left(0.85\, f'_{ck}\, [A_g - A_{st}]\right) + (f_{yk}\, A_{st})\right]$$

$$\phi P_N = (0.65) \times 0.80 \times \left[\left(0.85 \times (2.5 \text{ ksi}) \times \left[\left(2304 \text{ in}^2\right) - \left(4.2951 \text{ in}^2\right)\right]\right) + \left((60 \text{ ksi}) \times \left(4.2951 \text{ in}^2\right)\right)\right]$$

$$\phi P_N = 2675.2 \text{ kip}$$

$Ratio$ - Capacity

$$Ratio = \frac{P}{\phi P_N}$$

$$Ratio = \frac{(16.436 \text{ kip})}{(2675.2 \text{ kip})}$$

$$Ratio = 0.0061439$$

Status: **PASS**
Ratio: **0.010**

Shear Strength (ACI 318-19, LRFD)

Parameters:

$b_w = 48 \text{ in}$ - Effective width,

22.5.2.2 d - Effective depth

$$d = 0.80\, D$$

$$d = 0.80 \times (48 \text{ in})$$

$$d = 38.4 \text{ in}$$

22.5.5.1.3 λ_s - size effect modification factor

$$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{d}{10}}}, 1\right]$$

$$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{(38.4 \text{ in})}{10}}}, 1\right]$$

$$\lambda_s = 0.64282$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$,

22.5.5.1.1 $V_{c,max}$ - Max shear strength of concrete

$$V_{c,max} = 5\, \lambda_s\, \sqrt{f'_{ck}}\, b_w\, d$$

$$V_{c,max} = 5 \times (0.64282) \times \sqrt{(2500 \text{ psi})} \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{c,max} = 296.21 \text{ kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$, $P = 16.436 \text{ kip} \rightarrow 16436 \text{ lbf}$,

22.5.5.1.1(a) $V_{c,a}$ - Shear strength of concrete (a)

$$V_{c,a} = \left[2\, \lambda_s\, \sqrt{f'_{ck}} + \frac{P}{6\, A_g}\right]\, b_w\, d$$

$$V_{c,a} = \left[2 \times (0.64282) \times \sqrt{(2500 \text{ psi})} + \frac{(16436 \text{ lbf})}{6 \times (2304 \text{ in}^2)}\right] \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{c,a} = 120.68 \text{ kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$,

22.5.5.1.2 $V_{c,b}$ - Shear strength of concrete (b)

$$V_{c,b} = \left[2\, \lambda_s\, \sqrt{f'_{ck}} + \left(0.05\, f'_{ck}\right)\right]\, b_w\, d$$

$$V_{c,b} = \left[2 \times (0.64282) \times \sqrt{(2500 \text{ psi})} + (0.05 \times (2500 \text{ psi}))\right] \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{c,b} = 348.89 \text{ kip}$$

V_c - Governing shear strength of concrete

$$V_c = Min\left[V_{c,max}, V_{c,a}, V_{c,b}\right]$$

$$V_c = Min\left[(296.21 \text{ kip}), (120.68 \text{ kip}), (348.89 \text{ kip})\right]$$

$$V_c = 120.68 \text{ kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$,

22.5.1.2 $V_{s,a}$ - Shear strength of steel (a)

$$V_{s,a} = 8 \sqrt{f'_{ck}}\, b_w\, d$$

$$V_{s,a} = 8 \times \sqrt{(2500 \text{ psi})} \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{s,a} = 737.28 \text{ kip}$$

A_v - Ties rebar area,

$$A_v = \frac{\pi\, d_{ties}^2}{4}$$

$$A_v = \frac{\pi \times (0.375 \text{ in})^2}{4}$$

$$A_v = 0.11045 \text{ in}^2$$

22.5.8.5.3 $V_{s,b}$ - Shear strength of steel (b)

$$V_{s,b} = \frac{2\, A_v\, f_{ywk}\, d}{s_{ties}}$$

$$V_{s,b} = \frac{2 \times \left(0.11045 \text{ in}^2\right) \times (60 \text{ ksi}) \times (38.4 \text{ in})}{(10 \text{ in})}$$

$$V_{s,b} = 50.894 \text{ kip}$$

V_s - Governing shear strength of steel

$$V_s = MIN\left[V_{s,a}, V_{s,b}\right]$$

$$V_s = MIN\left[(737.28 \text{ kip}), (50.894 \text{ kip})\right]$$

$$V_s = 50.894 \text{ kip}$$

22.5.1.1 ϕV_n - Allowable shear strength

$$\phi V_n = \phi\, (V_c + V_s)$$

$$\phi V_n = (0.65) \times ((120.68 \text{ kip}) + (50.894 \text{ kip}))$$

$$\phi V_n = 111.52 \text{ kip}$$

Considering x-direction:

$V_{max} = 3.0364 \text{ kip}$ - Maximum shear force in the x-direction,
$Ratio$ - Capacity

$$Ratio = \frac{V_{max}}{\phi V_n}$$

$$Ratio = \frac{(3.0364 \text{ kip})}{(111.52 \text{ kip})}$$

$$Ratio = 0.027227$$

Status: **PASS**
Ratio: **0.030**

Considering z-direction:

$V_{max} = 0.041708 \text{ kip}$ - Maximum shear force in the z-direction,
$Ratio$ - Capacity

$$Ratio = \frac{V_{max}}{\phi V_n}$$

$$Ratio = \frac{(0.041708 \text{ kip})}{(111.52 \text{ kip})}$$

$$Ratio = 0.00037399$$

Status: **PASS**
Ratio: **0.000**

Flexural Strength (ACI 318-19, LRFD)

S_m - Section modulus

$$S_m = \frac{b\, D^2}{6}$$

$$S_m = \frac{(48 \text{ in}) \times (48 \text{ in})^2}{6}$$

$$S_m = 18432 \text{ in}^3$$

	$\lambda = 1$ - Concrete modification factor (Normal concrete), Allowable flexural strength: M_n shall be the lesser of: $\phi M_{n,1}$ $$\phi M_{n,1} = \phi \times 5 \times \lambda \times \sqrt{(f_c')} \times S_m$$ $$\phi M_{n,1} = 0.65 \times 5 \times 1 \times \sqrt{(2.5ksi)} \times 18432.001in^3$$ $$\phi M_{n,1} = 249.600kipft$$	
14.5.2.1b	$\phi M_{n,2}$ $$\phi M_{n,2} = \phi\, 0.85\, f'_{ck}\, S_m$$ $$\phi M_{n,2} = (0.65) \times 0.85 \times (2.5\text{ ksi}) \times \left(18432\text{ in}^3\right)$$ $$\phi M_{n,2} = 2121.6\text{ kipft}$$ Therefore, ϕM_n - Allowable flexural strength, $$\phi M_n = MIN[\phi M_{n,1}, \phi M_{n,2}]$$ $$\phi M_n = MIN[(249.6\text{ kipft}), (2121.6\text{ kipft})]$$ $$\phi M_n = 249.6\text{ kipft}$$ **Considering x-direction:** $M_{max} = 6.3723\text{ kipft}$ - Maximum moment in the x-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\phi M_n}$$ $$Ratio = \frac{(6.3723\text{ kipft})}{(249.6\text{ kipft})}$$ $$Ratio = 0.02553$$	Status: **PASS** Ratio: **0.030**
	Considering z-direction: $M_{max} = 0.081092\text{ kipft}$ - Maximum moment in the z-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\phi M_n}$$ $$Ratio = \frac{(0.081092\text{ kipft})}{(249.6\text{ kipft})}$$ $$Ratio = 0.00032489$$	Status: **PASS** Ratio: **0.000**

REFERENCES	CALCULATIONS	RESULTS

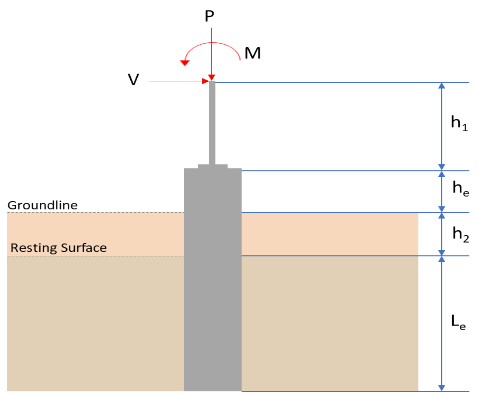
SkyCiv Foundation Design

Pile Foundation

Design Information :

Design code : IBC 2021 (International Building Code)
Unit System : Imperial

Pile Input



Geometry

Pile shape: rectangular
$b = 48\ \text{in}$ - Pile width
$D = 48\ \text{in}$ - Pile depth
$L = 4.25\ \text{ft}$ - Total pile length
$h_1 = 0\ \text{ft}$ - Lateral load height from the top of the pile,
$h_2 = 0\ \text{ft}$ - Depth to resisting surface
$h_e = 0\ \text{ft}$ - Length of pile above the ground

Tabulation of Soil Parameters

Layer	Label	Allowable Bearing Pressure (q_a) (psf)	Allowable Lateral Pressure (R) (psf/ft)
1	Sand, silty sand, clayey sand, silty gravel & clayey gravel	2000.000	150.000

Tabulation of Loads

Load Component	ASD	LRFD
P (kip)	9.342	15.617
V_x (kip)	-0.186	-0.311
V_z (kip)	0.044	0.075
M_x (kipft)	0.149	0.254
M_z (kipft)	5.924	10.491

Material Properties

$f'_{ck} = 2.5\ \text{ksi}$ - Concrete strength,

Required depth to resist lateral loads (ASD)

H - Point of application of the lateral load

$$H = h_1 + h_2 + h_e$$

$$H = (0\ \text{ft}) + (0\ \text{ft}) + (0\ \text{ft})$$

$$H = 0\ \text{ft}$$

Considering x-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\,D}$$

$$H_o = \frac{(-0.186\ \text{kip})}{1.57 \times (48\ \text{in})}$$

$$H_o = -0.029618\ \text{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\,H)}{1.57\,D}$$

$$M_o = \frac{(5.924\ \mathrm{kipft}) + ((-0.186\ \mathrm{kip}) \times (0\ \mathrm{ft}))}{1.57 \times (48\ \mathrm{in})}$$

$$M_o = 0.94331\ \mathrm{kipft/ft}$$

Required depth of embedment in earth:

$$L_x^3 - \left(14.14 \times \frac{H_o \times L_x}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,x} = 4.0857\ \mathrm{ft}$ - Required depth in x-direction,

Considering z-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\, b}$$

$$H_o = \frac{(0.044\ \mathrm{kip})}{1.57 \times (48\ \mathrm{in})}$$

$$H_o = 0.0070064\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\, H)}{1.57\, b}$$

$$M_o = \frac{(0.149\ \mathrm{kipft}) + ((0.044\ \mathrm{kip}) \times (0\ \mathrm{ft}))}{1.57 \times (48\ \mathrm{in})}$$

$$M_o = 0.023726\ \mathrm{kipft/ft}$$

Required depth of embedment in earth:

$$L_z^3 - \left(14.14 \times \frac{H_o \times L_z}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,z} = 1.3512\ \mathrm{ft}$ - Required depth in z-direction,

Minimum embedded depth required:

$L_{e,req}$ - Depth of pile required,

$$L_{e,req} = MAX[L_{e,x}, L_{e,z}]$$

$$L_{e,req} = MAX[(4.0857\ \mathrm{ft}), (1.3512\ \mathrm{ft})]$$

$$L_{e,req} = 4.086\ \mathrm{ft}$$

L_e - Actual embedded length of pile,

$$L_e = L - h_e - h_2$$

$$L_e = (4.25\ \mathrm{ft}) - (0\ \mathrm{ft}) - (0\ \mathrm{ft})$$

$$L_e = 4.25\ \mathrm{ft}$$

$Ratio$ - Embedded depth

$$Ratio = \frac{L_{e,req}}{L_e}$$

$$Ratio = \frac{(4.086\ \mathrm{ft})}{(4.25\ \mathrm{ft})}$$

$$Ratio = 0.96141$$

Status: **PASS**
Ratio: **0.960**

End-bearing Capacity (ASD)

A - Pile cross-section area

$$A = b\, D$$

$$A = (48\ \mathrm{in}) \times (48\ \mathrm{in})$$

$$A = 16\ \mathrm{ft}^2$$

q - End-bearing pressure

$$q = \frac{P_v}{A}$$

$$q = \frac{(9.342\ \mathrm{kip})}{(16\ \mathrm{ft}^2)}$$

$$q = 0.58388\ \mathrm{kip/ft}^2$$

$$q = 0.58388\ \mathrm{kip/ft}$$

Check bearing capacity ratio:

$Ratio$ - Capacity

$$Ratio = \frac{q}{q_a}$$

$$Ratio = \frac{\left(0.58388\ \mathrm{kip/ft^2}\right)}{(2000\ \mathrm{psf})}$$

$$Ratio = 0.29194$$

Status: **PASS**
Ratio: **0.290**

Czerniak

Lateral Soil Pressure (ASD):

L/D - Length to least lateral dimension ratio,

$$L/D = \frac{L}{D}$$

$$L/D = \frac{(4.25\ \mathrm{ft})}{(48\ \mathrm{in})}$$

$$L/D = 1.0625$$

Since L/D $\leq$ 10,

Pile is short.

Considering x-direction:

$H_o = -0.029618\ \mathrm{kip/ft}$ - Lateral force per length of pile,
$M_o = 0.94331\ \mathrm{kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (0.94331\ \mathrm{kipft/ft}) \times (4.25\ \mathrm{ft})) + \left(3 \times (-0.029618\ \mathrm{kip/ft}) \times (4.25\ \mathrm{ft})^2\right)}{(6 \times (0.94331\ \mathrm{kipft/ft})) + (4 \times (-0.029618\ \mathrm{kip/ft}) \times (4.25\ \mathrm{ft}))}$$

$$a = 2.8623\ \mathrm{ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\,[(4\,M_o) + (3\,H_o\,L_e)]^2}{L_e^2\,[(3\,M_o) + (2\,H_o\,L_e)]}$$

$$p = \frac{0.75 \times [(4 \times (0.94331\ \mathrm{kipft/ft})) + (3 \times (-0.029618\ \mathrm{kip/ft}) \times (4.25\ \mathrm{ft}))]^{\ 2}}{(4.25\ \mathrm{ft})^2 \times [(3 \times (0.94331\ \mathrm{kipft/ft})) + (2 \times (-0.029618\ \mathrm{kip/ft}) \times (4.25\ \mathrm{ft}))]}$$

$$p = 0.1857\ \mathrm{kip/ft^2}$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\,[(2\,M_o) + (H_o\,L_e)]}{L_e^2}$$

$$s = \frac{6 \times [(2 \times (0.94331\ \mathrm{kipft/ft})) + ((-0.029618\ \mathrm{kip/ft}) \times (4.25\ \mathrm{ft}))]}{(4.25\ \mathrm{ft})^2}$$

$$s = 0.58489\ \mathrm{kip/ft^2}$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\,\frac{a}{2}$$

$$p_a = (150\ \mathrm{psf/ft}) \times \frac{(2.8623\ \mathrm{ft})}{2}$$

$$p_a = 0.21467\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{\left(0.1857\ \mathrm{kip/ft^2}\right)}{\left(0.21467\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = 0.86504$$

Status: **PASS**
Ratio: **0.870**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\,L_e$$

$$p_s = (150\ \mathrm{psf/ft}) \times (4.25\ \mathrm{ft})$$

$$p_s = 0.6375\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{(0.58489\ \mathrm{kip/ft^2})}{(0.6375\ \mathrm{kip/ft^2})}$$

$$Ratio = 0.91747$$

Status: **PASS**
Ratio: **0.920**

Considering z-direction:

$H_o = 0.0070064\ \mathrm{kip/ft}$ - Lateral force per length of pile,
$M_o = 0.023726\ \mathrm{kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + (3\,H_o\,L_e^2)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (0.023726\ \mathrm{kipft/ft}) \times (4.25\ \mathrm{ft})) + \left(3 \times (0.0070064\ \mathrm{kip/ft}) \times (4.25\ \mathrm{ft})^2\right)}{(6 \times (0.023726\ \mathrm{kipft/ft})) + (4 \times (0.0070064\ \mathrm{kip/ft}) \times (4.25\ \mathrm{ft}))}$$

$$a = 2.9947\ \mathrm{ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\,[(4\,M_o) + (3\,H_o\,L_e)]^2}{L_e^2\,[(3\,M_o) + (2\,H_o\,L_e)]}$$

$$p = \frac{0.75 \times [(4 \times (0.023726\ \mathrm{kipft/ft})) + (3 \times (0.0070064\ \mathrm{kip/ft}) \times (4.25\ \mathrm{ft}))]^2}{(4.25\ \mathrm{ft})^2 \times [(3 \times (0.023726\ \mathrm{kipft/ft})) + (2 \times (0.0070064\ \mathrm{kip/ft}) \times (4.25\ \mathrm{ft}))]}$$

$$p = 0.010781\ \mathrm{kip/ft^2}$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\,[(2\,M_o) + (H_o\,L_e)]}{L_e^2}$$

$$s = \frac{6 \times [(2 \times (0.023726\ \mathrm{kipft/ft})) + ((0.0070064\ \mathrm{kip/ft}) \times (4.25\ \mathrm{ft}))]}{(4.25\ \mathrm{ft})^2}$$

$$s = 0.025654\ \mathrm{kip/ft^2}$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\,\frac{a}{2}$$

$$p_a = (150\ \mathrm{psf/ft}) \times \frac{(2.9947\ \mathrm{ft})}{2}$$

$$p_a = 0.2246\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{(0.010781\ \mathrm{kip/ft^2})}{(0.2246\ \mathrm{kip/ft^2})}$$

$$Ratio = 0.048$$

Status: **PASS**
Ratio: **0.050**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\,L_e$$

$$p_s = (150\ \mathrm{psf/ft}) \times (4.25\ \mathrm{ft})$$

$$p_s = 0.6375\ \mathrm{kip/ft^2}$$

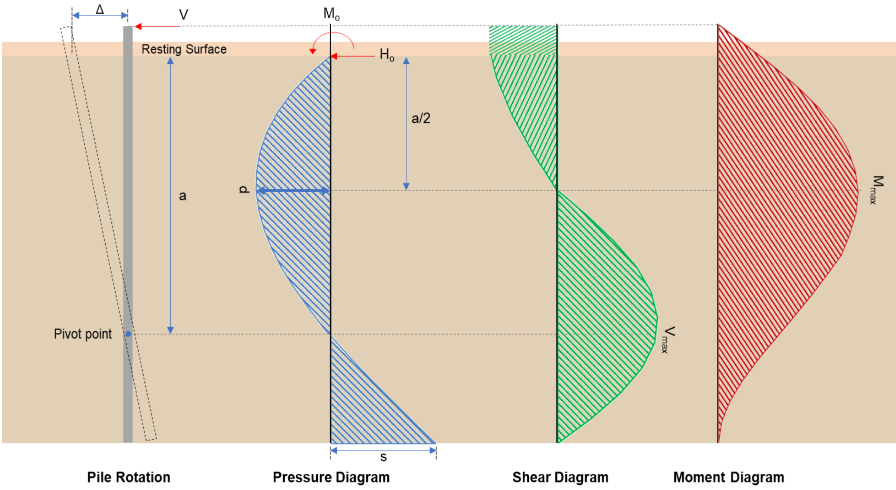
$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{\left(0.025654 \text{ kip/ft}^2\right)}{\left(0.6375 \text{ kip/ft}^2\right)}$$

$$Ratio = 0.040242$$

Status: **PASS**
Ratio: **0.040**



Shear force and Bending moment (x-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\,D}$$

$$H_o = \frac{(-0.311 \text{ kip})}{1.57 \times (48 \text{ in})}$$

$$H_o = -0.049522 \text{ kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\,H)}{1.57\,D}$$

$$M_o = \frac{(10.491 \text{ kipft}) + ((-0.311 \text{ kip}) \times (0 \text{ ft}))}{1.57 \times (48 \text{ in})}$$

$$M_o = 1.6705 \text{ kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(1.6705 \text{ kipft/ft})}{(-0.049522 \text{ kip/ft})}$$

$$E = 33.733 \text{ ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (1.6705 \text{ kipft/ft}) \times (4.25 \text{ ft})) + \left(3 \times (-0.049522 \text{ kip/ft}) \times (4.25 \text{ ft})^2\right)}{(6 \times (1.6705 \text{ kipft/ft})) + (4 \times (-0.049522 \text{ kip/ft}) \times (4.25 \text{ ft}))}$$

$$a = 2.8608 \text{ ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\,D)\left[1 - \left[3\left(\frac{4\,E}{L_e} + 3\right)\left(\frac{a}{L_e}\right)^2\right] + \left[4\left(\frac{3\,E}{L_e} + 2\right)\left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((-0.049522 \text{ kip/ft}) \times (48 \text{ in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (33.733 \text{ ft})}{(4.25 \text{ ft})} + 3\right) \times \left(\frac{(2.8608 \text{ ft})}{(4.25 \text{ ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (33.733 \text{ ft})}{(4.25 \text{ ft})} + 2\right) \times \left(\frac{(2.8608 \text{ ft})}{(4.25 \text{ ft})}\right)^3\right]\right]$$

$$V_{max} = 2.9207 \text{ kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, D\, L_e)\left[\left(\frac{E}{L_e} + \frac{a}{2\,L_e}\right) - \left[\left(\frac{4\,E}{L_e} + 3\right)\left(\frac{a}{2\,L_e}\right)^3\right] + \left[\left(\frac{3\,E}{L_e} + 2\right)\left(\frac{a}{2\,L_e}\right)^4\right]\right]$$

$$M_{max} = ((-0.049522\ \mathrm{kip/ft}) \times (48\ \mathrm{in}) \times (4.25\ \mathrm{ft})) \times \left[\left(\frac{(33.733\ \mathrm{ft})}{(4.25\ \mathrm{ft})} + \frac{(2.8608\ \mathrm{ft})}{2 \times (4.25\ \mathrm{ft})}\right) - \left[\left(\frac{4 \times (33.733\ \mathrm{ft})}{(4.25\ \mathrm{ft})} + 3\right) \times \left(\frac{(2.8608\ \mathrm{ft})}{2 \times (4.25\ \mathrm{ft})}\right)^3\right] + \left[\left(\frac{3 \times (33.733\ \mathrm{ft})}{(4.25\ \mathrm{ft})} + 2\right) \times \left(\frac{(2.8608\ \mathrm{ft})}{2 \times (4.25\ \mathrm{ft})}\right)^4\right]\right]$$

$$M_{max} = 6.129\ \mathrm{kipft}$$

Shear force and Bending moment (z-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\,b}$$

$$H_o = \frac{(0.075\ \mathrm{kip})}{1.57 \times (48\ \mathrm{in})}$$

$$H_o = 0.011943\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\, H)}{1.57\,b}$$

$$M_o = \frac{(0.254\ \mathrm{kipft}) + ((0.075\ \mathrm{kip}) \times (0\ \mathrm{ft}))}{1.57 \times (48\ \mathrm{in})}$$

$$M_o = 0.040446\ \mathrm{kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(0.040446\ \mathrm{kipft/ft})}{(0.011943\ \mathrm{kip/ft})}$$

$$E = 3.3867\ \mathrm{ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (0.040446\ \mathrm{kipft/ft}) \times (4.25\ \mathrm{ft})) + \left(3 \times (0.011943\ \mathrm{kip/ft}) \times (4.25\ \mathrm{ft})^2\right)}{(6 \times (0.040446\ \mathrm{kipft/ft})) + (4 \times (0.011943\ \mathrm{kip/ft}) \times (4.25\ \mathrm{ft}))}$$

$$a = 2.9947\ \mathrm{ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\, b)\left[1 - \left[3\left(\frac{4\,E}{L_e} + 3\right)\left(\frac{a}{L_e}\right)^2\right] + \left[4\left(\frac{3\,E}{L_e} + 2\right)\left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((0.011943\ \mathrm{kip/ft}) \times (48\ \mathrm{in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (3.3867\ \mathrm{ft})}{(4.25\ \mathrm{ft})} + 3\right) \times \left(\frac{(2.9947\ \mathrm{ft})}{(4.25\ \mathrm{ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (3.3867\ \mathrm{ft})}{(4.25\ \mathrm{ft})} + 2\right) \times \left(\frac{(2.9947\ \mathrm{ft})}{(4.25\ \mathrm{ft})}\right)^3\right]\right]$$

$$V_{max} = 0.098984\ \mathrm{kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, b\, L_e)\left[\left(\frac{E}{L_e} + \frac{a}{2\,L_e}\right) - \left[\left(\frac{4\,E}{L_e} + 3\right)\left(\frac{a}{2\,L_e}\right)^3\right] + \left[\left(\frac{3\,E}{L_e} + 2\right)\left(\frac{a}{2\,L_e}\right)^4\right]\right]$$

$$M_{max} = ((0.011943\ \mathrm{kip/ft}) \times (48\ \mathrm{in}) \times (4.25\ \mathrm{ft})) \times \left[\left(\frac{(3.3867\ \mathrm{ft})}{(4.25\ \mathrm{ft})} + \frac{(2.9947\ \mathrm{ft})}{2 \times (4.25\ \mathrm{ft})}\right) - \left[\left(\frac{4 \times (3.3867\ \mathrm{ft})}{(4.25\ \mathrm{ft})} + 3\right) \times \left(\frac{(2.9947\ \mathrm{ft})}{2 \times (4.25\ \mathrm{ft})}\right)^3\right] + \left[\left(\frac{3 \times (3.3867\ \mathrm{ft})}{(4.25\ \mathrm{ft})} + 2\right) \times \left(\frac{(2.9947\ \mathrm{ft})}{2 \times (4.25\ \mathrm{ft})}\right)^4\right]\right]$$

$$M_{max} = 0.19211 \text{ kipft}$$

Minimum Reinforcement Check (LRFD)

Parameters:

$f'_{ck} = 2.5 \text{ ksi}$ - Concrete strength,
$f_{yk} = 60 \text{ ksi}$ - Longitudinal reinforcement strength,
$\phi = 0.65$ - Reduction factor for axial strength,
Table 22.4.2.1 $\alpha = 0.8$ - Alpha factor for axial strength,
$A_g = 2304 \text{ in}^2$ - Gross area of concrete,

Longitudinal reinforcement:

Required reinforcement due to axial load, $A_{st,required}$
22.4.2.2, 10.6.1.1 $A_{st,required}$

$$A_{st,required} = Min\left[\frac{\frac{P}{\phi\ \alpha} - (0.85\ f'_{ck}\ A_g)}{f_{yk} - (0.85\ f'_{ck})}, (0.08\ A_g)\right]$$

$$A_{st,required} = Min\left[\frac{\frac{(15.617 \text{ kip})}{(0.65)\times(0.8)} - \left(0.85 \times (2.5 \text{ ksi}) \times \left(2304 \text{ in}^2\right)\right)}{(60 \text{ ksi}) - (0.85 \times (2.5 \text{ ksi}))}, \left(0.08 \times \left(2304 \text{ in}^2\right)\right)\right]$$

$$A_{st,required} = -84.077 \text{ in}^2$$

A_{min} - Governing minimum reinforcement area,

$$A_{min} = Max\left[A_{st,required}, (0.0018\ A_g)\right]$$

$$A_{min} = Max\left[\left(-84.077 \text{ in}^2\right), \left(0.0018 \times \left(2304 \text{ in}^2\right)\right)\right]$$

$$A_{min} = 4.1472 \text{ in}^2$$

n_{rebar} - Required number of reinforcement,

$$n_{rebar} = \frac{A_{min}}{A_{rebar}}$$

$$n_{rebar} = \frac{\left(4.1472 \text{ in}^2\right)}{\left(0.3068 \text{ in}^2\right)}$$

$$n_{rebar} = 14$$

A_{st} - Actual total reinforcement area,

$$A_{st} = n_{rebar}\ \frac{\pi\ d_{bar}^2}{4}$$

$$A_{st} = (14) \times \frac{\pi \times (0.625 \text{ in})^2}{4}$$

$$A_{st} = 4.2951 \text{ in}^2$$

$Ratio$ - Capacity

$$Ratio = \frac{A_{min}}{A_{st}}$$

$$Ratio = \frac{\left(4.1472 \text{ in}^2\right)}{\left(4.2951 \text{ in}^2\right)}$$

$$Ratio = 0.96556$$

Status: **PASS**
Ratio: **0.970**

25.2.3 s_{rebar} - Minimum spacing of reinforcement,

$$s_{rebar} = Max\left[1.5, (1.5\ d_{bar})\right]$$

$$s_{rebar} = Max\left[1.5, (1.5 \times (0.625 \text{ in}))\right]$$

$$s_{rebar} = 1.5 \text{ in}$$

Ties:

25.7.2.2 Since longitudinal reinforcement is $\leq$ No. 10ø: Use #3(0.375 in)
25.7.2.1 s_{ties} - Maximum spacing of ties,

$$s_{ties} = Min\left[(16\ d_{bar}), (48\ d_{ties}), Min\,(D, b)\right]$$

$$s_{ties} = Min\left[(16 \times (0.625 \text{ in})), (48 \times (0.375 \text{ in})), Min\,((48 \text{ in}), (48 \text{ in}))\right]$$

$$s_{ties} = 10 \text{ in}$$

Summary:

Main reinforcement: **14 - #5 (0.625 in)**

	Ties: **#3(0.375 in) - 10 in**	

Axial Compression Strength (ACI 318-19, LRFD)

22.4.2.2 ϕP_N - Allowable axial compressive strength

$$\phi P_N = \phi\, 0.80\, \left[\left(0.85\, f'_{ck}\, \left[A_g - A_{st}\right]\right) + \left(f_{yk}\, A_{st}\right)\right]$$

$$\phi P_N = (0.65) \times 0.80 \times \left[\left(0.85 \times (2.5\ \mathrm{ksi}) \times \left[\left(2304\ \mathrm{in}^2\right) - \left(4.2951\ \mathrm{in}^2\right)\right]\right) + \left((60\ \mathrm{ksi}) \times \left(4.2951\ \mathrm{in}^2\right)\right)\right]$$

$$\phi P_N = 2675.2\ \mathrm{kip}$$

$Ratio$ - Capacity

$$Ratio = \frac{P}{\phi P_N}$$

$$Ratio = \frac{(15.617\ \mathrm{kip})}{(2675.2\ \mathrm{kip})}$$

$$Ratio = 0.0058377$$

Status: **PASS**
Ratio: **0.010**

Shear Strength (ACI 318-19, LRFD)

Parameters:

$b_w = 48\ \mathrm{in}$ - Effective width,

22.5.2.2 d - Effective depth

$$d = 0.80\, D$$

$$d = 0.80 \times (48\ \mathrm{in})$$

$$d = 38.4\ \mathrm{in}$$

22.5.5.1.3 λ_s - size effect modification factor

$$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{d}{10}}}, 1\right]$$

$$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{(38.4\ \mathrm{in})}{10}}}, 1\right]$$

$$\lambda_s = 0.64282$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5\ \mathrm{ksi} \rightarrow 2500\ \mathrm{psi}$,

22.5.5.1.1 $V_{c,max}$ - Max shear strength of concrete

$$V_{c,max} = 5\, \lambda_s\, \sqrt{f'_{ck}}\, b_w\, d$$

$$V_{c,max} = 5 \times (0.64282) \times \sqrt{(2500\ \mathrm{psi})} \times (48\ \mathrm{in}) \times (38.4\ \mathrm{in})$$

$$V_{c,max} = 296.21\ \mathrm{kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5\ \mathrm{ksi} \rightarrow 2500\ \mathrm{psi}$, $P = 15.617\ \mathrm{kip} \rightarrow 15617\ \mathrm{lbf}$,

22.5.5.1.1(a) $V_{c,a}$ - Shear strength of concrete (a)

$$V_{c,a} = \left[2\, \lambda_s\, \sqrt{f'_{ck}} + \frac{P}{6\, A_g}\right]\, b_w\, d$$

$$V_{c,a} = \left[2 \times (0.64282) \times \sqrt{(2500\ \mathrm{psi})} + \frac{(15617\ \mathrm{lbf})}{6 \times \left(2304\ \mathrm{in}^2\right)}\right] \times (48\ \mathrm{in}) \times (38.4\ \mathrm{in})$$

$$V_{c,a} = 120.57\ \mathrm{kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5\ \mathrm{ksi} \rightarrow 2500\ \mathrm{psi}$,

22.5.5.1.2 $V_{c,b}$ - Shear strength of concrete (b)

$$V_{c,b} = \left[2\, \lambda_s\, \sqrt{f'_{ck}} + \left(0.05\, f'_{ck}\right)\right]\, b_w\, d$$

$$V_{c,b} = \left[2 \times (0.64282) \times \sqrt{(2500\ \mathrm{psi})} + (0.05 \times (2500\ \mathrm{psi}))\right] \times (48\ \mathrm{in}) \times (38.4\ \mathrm{in})$$

$$V_{c,b} = 348.89\ \mathrm{kip}$$

V_c - Governing shear strength of concrete

$$V_c = Min\left[V_{c,max}, V_{c,a}, V_{c,b}\right]$$

$$V_c = Min\left[(296.21\ \mathrm{kip}), (120.57\ \mathrm{kip}), (348.89\ \mathrm{kip})\right]$$

$$V_c = 120.57\ \mathrm{kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5\ \mathrm{ksi} \rightarrow 2500\ \mathrm{psi}$,

22.5.1.2 $V_{s,a}$ - Shear strength of steel (a)

$$V_{s,a} = 8\sqrt{f'_{ck}}\, b_w\, d$$

$$V_{s,a} = 8 \times \sqrt{(2500\ \mathrm{psi})} \times (48\ \mathrm{in}) \times (38.4\ \mathrm{in})$$

$$V_{s,a} = 737.28\ \mathrm{kip}$$

A_v - Ties rebar area,

$$A_v = \frac{\pi\, d_{ties}^2}{4}$$

$$A_v = \frac{\pi \times (0.375\ \mathrm{in})^2}{4}$$

$$A_v = 0.11045\ \mathrm{in}^2$$

22.5.8.5.3 $V_{s,b}$ - Shear strength of steel (b)

$$V_{s,b} = \frac{2\, A_v\, f_{ywk}\, d}{s_{ties}}$$

$$V_{s,b} = \frac{2 \times \left(0.11045\ \mathrm{in}^2\right) \times (60\ \mathrm{ksi}) \times (38.4\ \mathrm{in})}{(10\ \mathrm{in})}$$

$$V_{s,b} = 50.894\ \mathrm{kip}$$

V_s - Governing shear strength of steel

$$V_s = MIN\,[V_{s,a}, V_{s,b}]$$

$$V_s = MIN\,[(737.28\ \mathrm{kip}), (50.894\ \mathrm{kip})]$$

$$V_s = 50.894\ \mathrm{kip}$$

22.5.1.1 ϕV_n - Allowable shear strength

$$\phi V_n = \phi\ (V_c + V_s)$$

$$\phi V_n = (0.65) \times ((120.57\ \mathrm{kip}) + (50.894\ \mathrm{kip}))$$

$$\phi V_n = 111.45\ \mathrm{kip}$$

Considering x-direction:

$V_{max} = 2.9207\ \mathrm{kip}$ - Maximum shear force in the x-direction,
$Ratio$ - Capacity

$$Ratio = \frac{V_{max}}{\phi V_n}$$

$$Ratio = \frac{(2.9207\ \mathrm{kip})}{(111.45\ \mathrm{kip})}$$

$$Ratio = 0.026207$$

Status: **PASS**
Ratio: **0.030**

Considering z-direction:

$V_{max} = 0.098984\ \mathrm{kip}$ - Maximum shear force in the z-direction,
$Ratio$ - Capacity

$$Ratio = \frac{V_{max}}{\phi V_n}$$

$$Ratio = \frac{(0.098984\ \mathrm{kip})}{(111.45\ \mathrm{kip})}$$

$$Ratio = 0.00088815$$

Status: **PASS**
Ratio: **0.000**

Flexural Strength (ACI 318-19, LRFD)

S_m - Section modulus

$$S_m = \frac{b\, D^2}{6}$$

$$S_m = \frac{(48\ \mathrm{in}) \times (48\ \mathrm{in})^2}{6}$$

$$S_m = 18432\ \mathrm{in}^3$$

	$\lambda = 1$ - Concrete modification factor (Normal concrete), Allowable flexural strength: M_n shall be the lesser of: $\phi M_{n,1}$ $$\phi M_{n,1} = \phi \times 5 \times \lambda \times \sqrt{(f_c')} \times S_m$$ $$\phi M_{n,1} = 0.65 \times 5 \times 1 \times \sqrt{(2.5ksi)} \times 18432.001in^3$$ $$\phi M_{n,1} = 249.600kipft$$	
14.5.2.1b	$\phi M_{n,2}$ $$\phi M_{n,2} = \phi\, 0.85\, f'_{ck}\, S_m$$ $$\phi M_{n,2} = (0.65) \times 0.85 \times (2.5\ \mathrm{ksi}) \times \left(18432\ \mathrm{in}^3\right)$$ $$\phi M_{n,2} = 2121.6\ \mathrm{kipft}$$ Therefore, ϕM_n - Allowable flexural strength, $$\phi M_n = MIN[\phi M_{n,1}, \phi M_{n,2}]$$ $$\phi M_n = MIN[(249.6\ \mathrm{kipft}), (2121.6\ \mathrm{kipft})]$$ $$\phi M_n = 249.6\ \mathrm{kipft}$$ **Considering x-direction:** $M_{max} = 6.129\ \mathrm{kipft}$ - Maximum moment in the x-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\phi M_n}$$ $$Ratio = \frac{(6.129\ \mathrm{kipft})}{(249.6\ \mathrm{kipft})}$$ $$Ratio = 0.024555$$	Status: **PASS** Ratio: **0.020**
	Considering z-direction: $M_{max} = 0.19211\ \mathrm{kipft}$ - Maximum moment in the z-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\phi M_n}$$ $$Ratio = \frac{(0.19211\ \mathrm{kipft})}{(249.6\ \mathrm{kipft})}$$ $$Ratio = 0.00076967$$	Status: **PASS** Ratio: **0.000**