Your Project Calculations

Project Name: Wentworth Plymouth Ener - 32 mods - Jb

S3D Model Link:
https://platform.skyciv.com/structural?preload_name=Wentworth%20Plymouth%20Ener%20-%2032%20mods%20-%20Jb&preload_path=Shared%20Enterprise%20Folder/MT_Solar_Projects/3_2024

Public Model Link:
https://platform.skyciv.com/structural-viewer?project_id=mECZp32oK4TV7X2lzDdeGZsCtPvqEqJtUf57ro7uPfJ4hB4WB5UtOhV8TYboqF62

Array Specification

Product:	Beam
Unique ID:	2P-22.5-6TOP-XD-72-L-4Hx8W-STRUTS-DKI3
Duty Classification:	XD
Module Width:	42.10 in
Module Length:	64.00in
Number of Rows:	4
Number of Columns:	8
Total Number of Modules:	32
Desired Tilt Angle:	46
Front Edge Clearance:	3
Total Array Height at Tilt:	13.15 ft
Total Frame Length:	42.00 ft
Frame Weight:	2003 lbs
Array Dimensions N/S:	14.20 ft
Array Dimensions E/W:	43.33 ft
Rail Length:	170.40 in
Rail Spacing:	2.67 ft
Rail Check:	Not Checked

Support Specifications

Pole Size:	6in Pipe Sch 40
Pole Length above Grade:	8.11 ft
Number of Poles:	2
Pole Spacing:	22.5 ft

Foundation Specifications

Foundation Type:	Square
Foundation Dimensions:	48 x 48 in
Foundation Depth (below grade):	Pile 1: 5.25 ft Pile 2: 5.25 ft
Foundation Volume:	6.222 y^3
Foundation Result:	PASSED
Mount Twist:	0.284608 kip

Site Info

Risk Category:	I
Exposure:	B
Soil Classification:	sand
Site Location:	19 Turner Rd, Wentworth, NH 03282, USA
Wind Speed:	100 mph
Snow Load:	96 psf
Design Uplift Pressure:	0.013024 ksf
Design Downforce Pressure:	-0.013024 ksf
Design Snow Pressure:	0.025336 ksf

Design Disclaimer

This software should be used for preliminary designs and should not be used as a final design unless reviewed, verified and designed by a qualified structural engineer.

AutoDesigner Input


```
{"wind_speed_override":100,"snow_load_override":96,"direct_snow_load":false,"add_angle_brace":true,"product_type":"Beam","project_id":"Wentworth Plymouth Ener - 32 mods - Jb","site_address":"19 Turner Rd, Wentworth, NH 03282, USA","module_width":42.1,"module_length":64,"number_rows":"4","number_columns":8,"pole_mount_section":"4_40","core_pipe_width":65,"core_pipe_section":"2_40","adjuster_section":"2_40","core_beam_height":65,"core_beam_section":"HSS3x2x1/8","main_pipe_section":"2_12GA","pole_spacing":"15","tilt_angle":46,"ground_clearance":3,"risk_category":"I","exposure_category":"B","frame_duty_override":"auto","pole_override":"auto","soil_type":"sand","customer_foundation_override":"48_Square","foundation_type":"Square","foundation_size":48,"check_rails":false}
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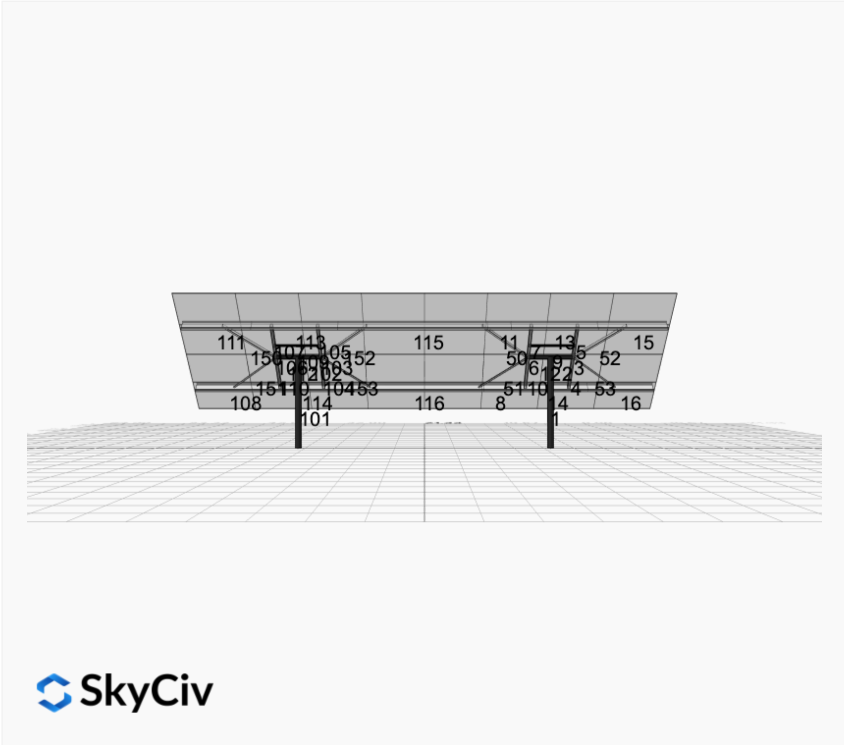
Design Notes:

- AISC Deflection checks are set to L/1 due to structure design intent
- Foundation Soil Parameters used in this Autodesign are all estimates, proper geotechnical reports are required to confirm soil profiles
- Wind speeds, snow loads and other site specific results are based on ASCE 7 2016
- Steel frame design checks are based on AISC 360 2016 (LRFD)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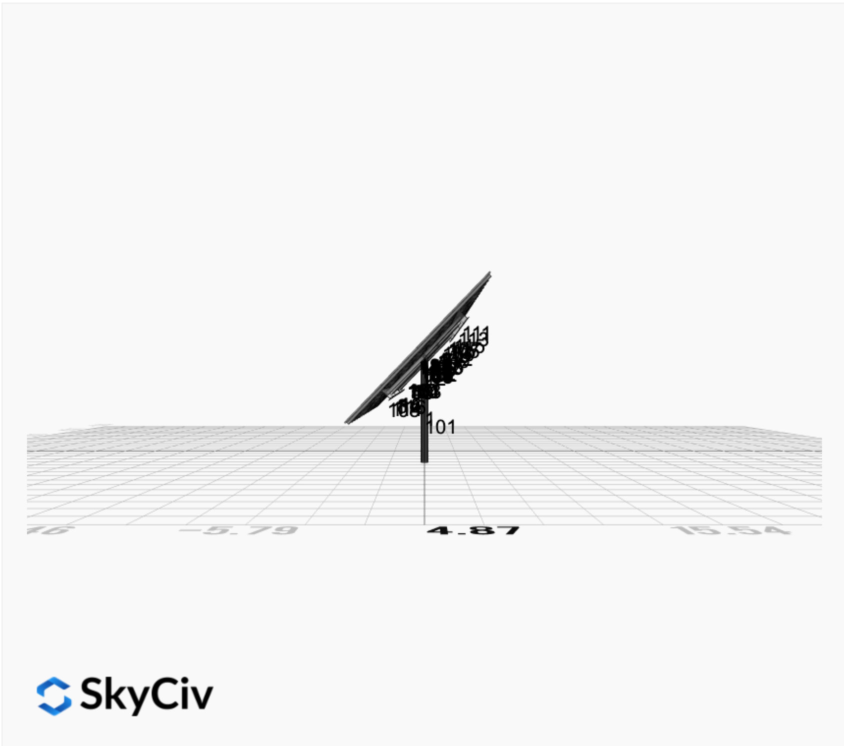
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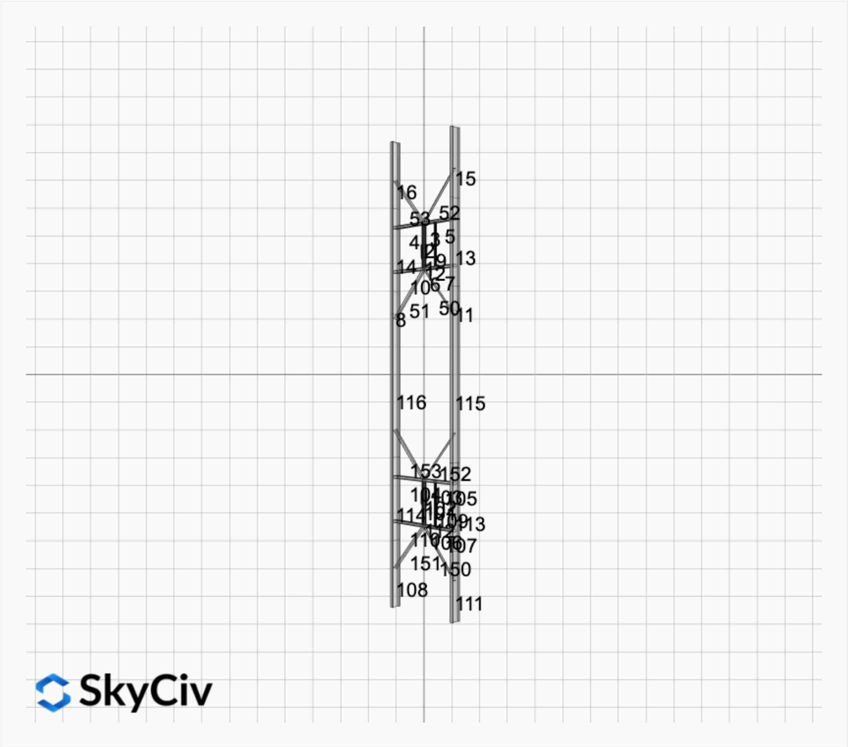
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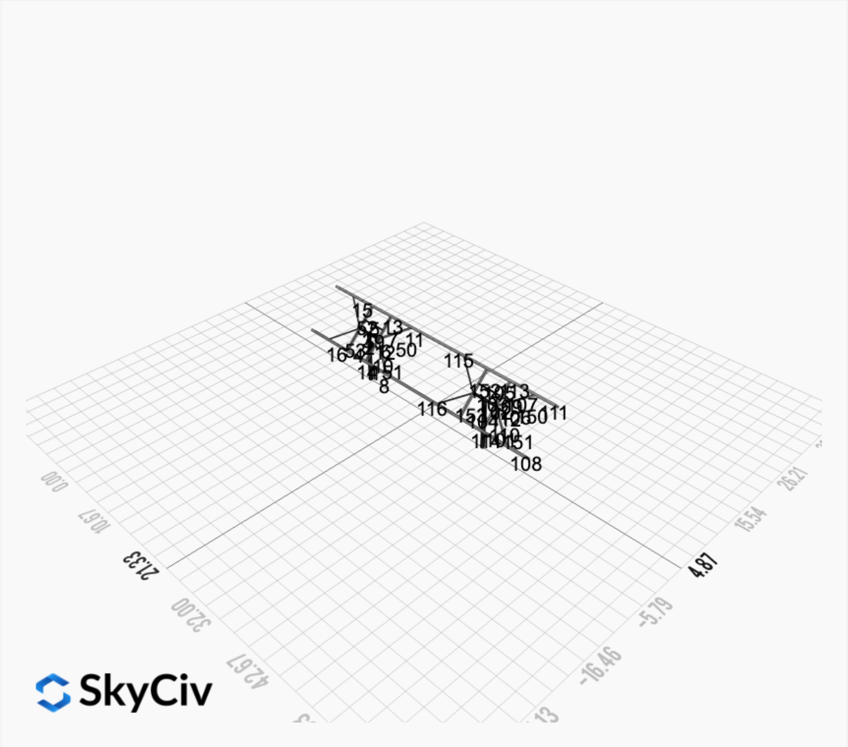
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42.67
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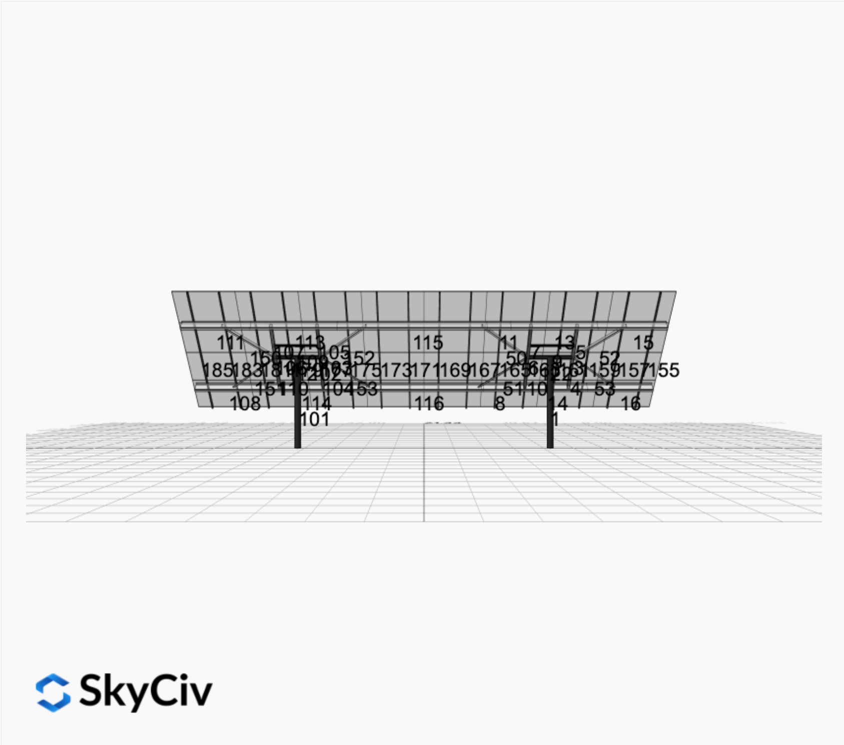
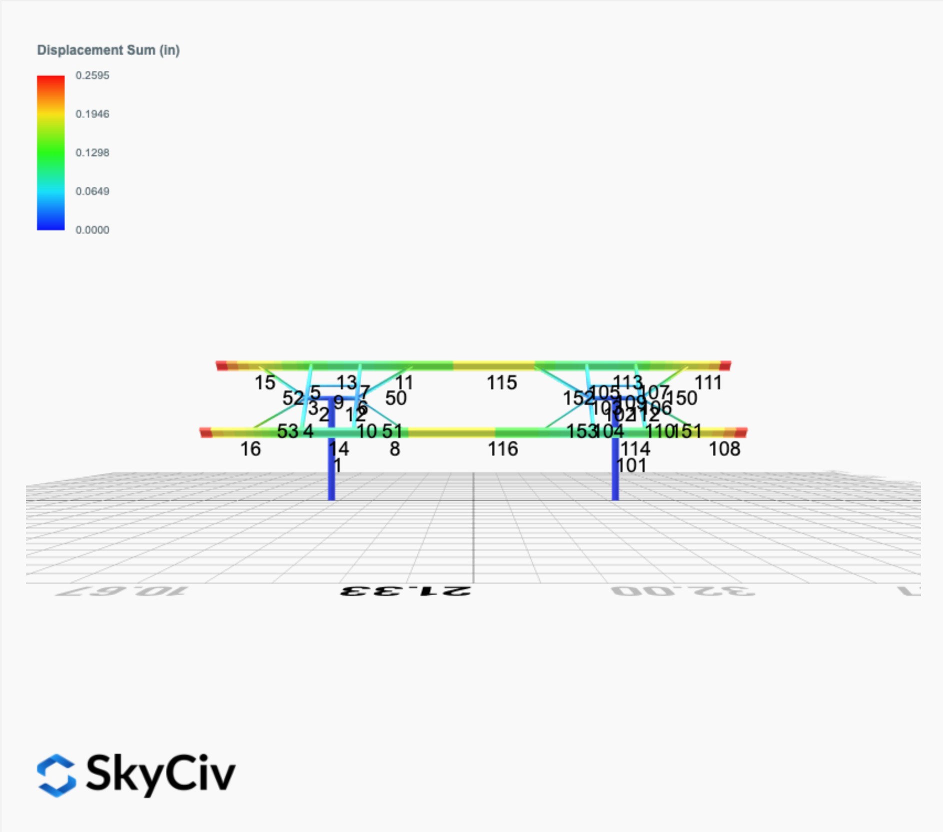
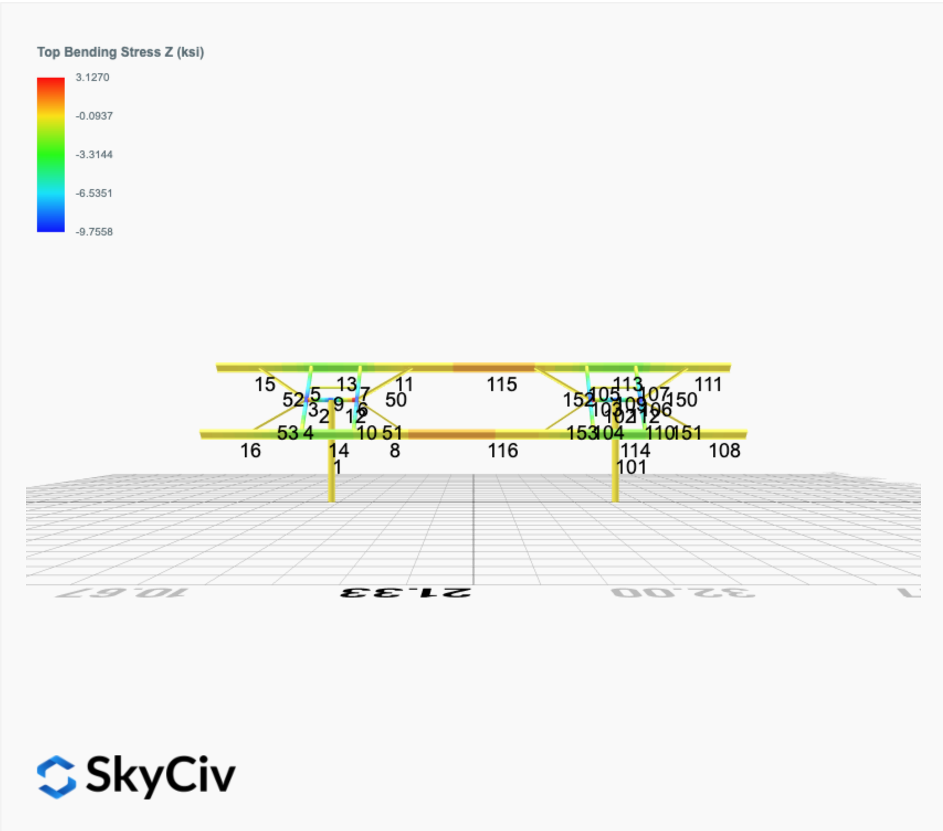
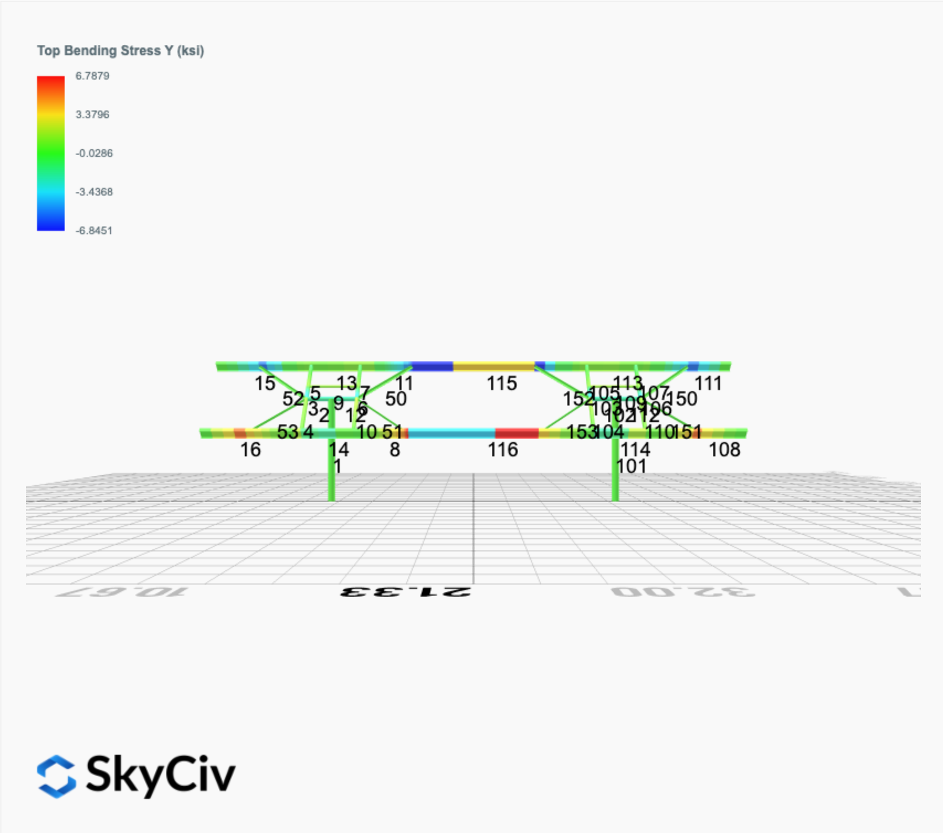


FEM Results (Envelope Worst Case for each member)






Top Bending Stress Y (ksi)
6.7879
3.3796
-0.0286
-3.4368
-6.8451
15
13
11
115
113
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150
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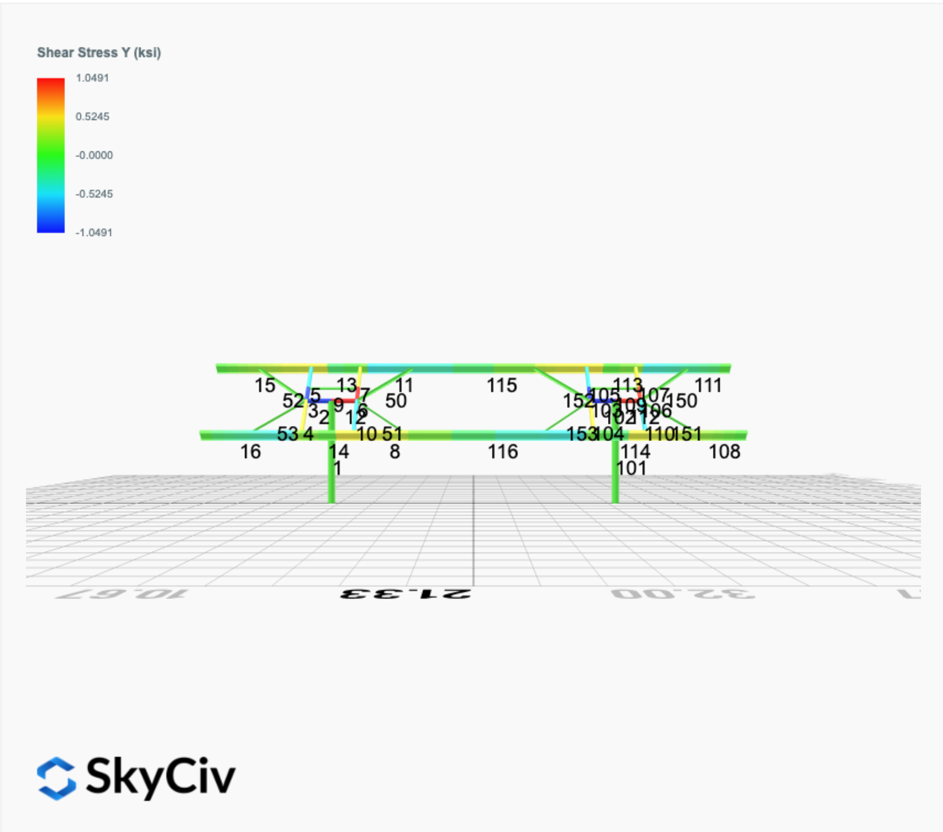
14
8
116
114
108
1
101


Shear Stress Y (ksi)
1.0491
0.5245
-0.0000
-0.5245
-1.0491
15
13
11
115
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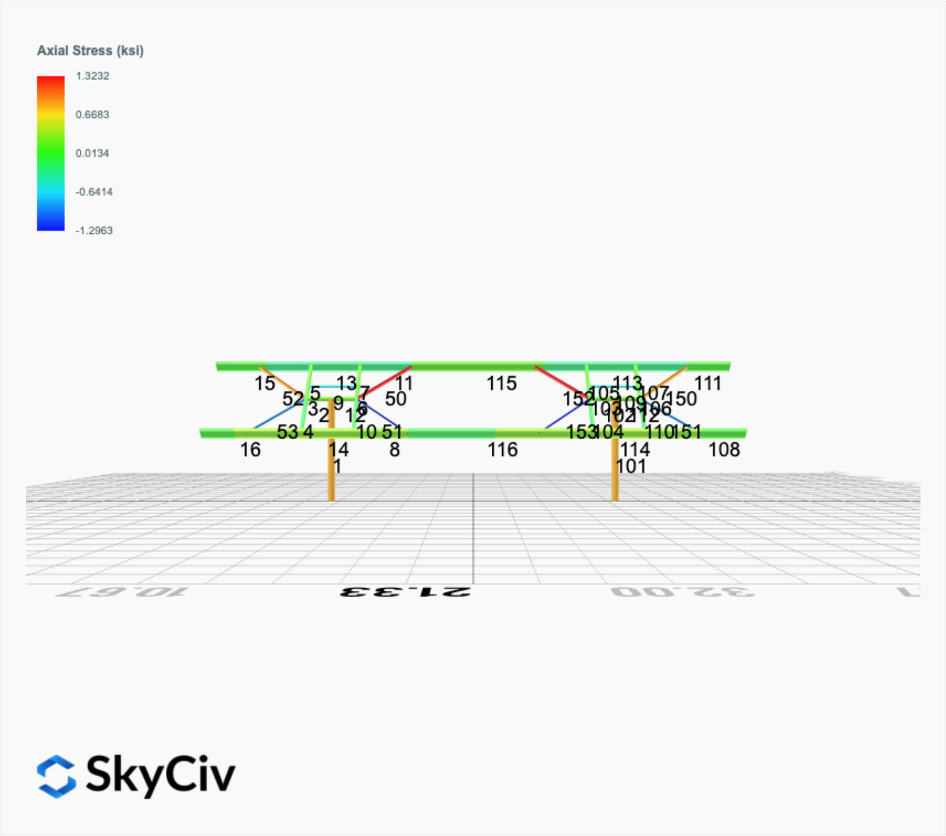
116
114
108
1
101


Axial Stress (ksi)
1.3232
0.6683
0.0134
-0.6414
-1.2963
SkyCiv

Reaction Forces for Foundation 1 (Node ID#1), (kip, kip-ft)

ASD Load Combination Results

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 1. D	0.0000	2.4720	-0.0082	-0.0172	0.0460	0.0259
ULS: 2. D + L	0.0000	2.4720	-0.0082	-0.0172	0.0460	0.0259
ULS: 3. D + (S or Lr or R)	0.0000	7.7202	-0.0268	-0.0557	0.1613	0.0638
ULS: 3. D + (S or Lr or R)	0.0000	2.4720	-0.0082	-0.0172	0.0460	0.0259
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	0.0000	6.4081	-0.0221	-0.0461	0.1325	0.0543
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	0.0000	2.4720	-0.0082	-0.0172	0.0460	0.0259
ULS: 5b. D + 0.7E	0.0000	2.4720	-0.0082	-0.0172	0.0460	0.0259
ULS: 6b. D + 0.75L + 0.75(0.7)E + 0.75S	0.0000	6.4081	-0.0221	-0.0461	0.1325	0.0543
ULS: 8. 0.6D + 0.7E	0.0000	1.4832	-0.0049	-0.0103	0.0276	0.0155
ULS: 5a. D + 0.6W_Wind downforce Case A only	-1.7295	4.1421	-0.0305	-0.0694	0.0963	14.2005
ULS: 5a. D + 0.6W_Wind downforce Case B only	0.0000	2.4720	-0.0082	-0.0172	0.0460	0.0259
ULS: 5a. D + 0.6W_Wind uplift Case A only	1.7295	0.8018	0.0141	0.0348	-0.0044	-13.8469
ULS: 5a. D + 0.6W_Wind uplift Case B only	0.0000	2.4720	-0.0082	-0.0172	0.0460	0.0259
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-1.2971	7.6607	-0.0389	-0.0852	0.1702	10.6853
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	0.0000	6.4081	-0.0221	-0.0461	0.1325	0.0543
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	1.2971	5.1555	-0.0054	-0.0071	0.0947	-10.3502
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	0.0000	6.4081	-0.0221	-0.0461	0.1325	0.0543
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-1.2971	3.7246	-0.0249	-0.0563	0.0837	10.6568
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	0.0000	2.4720	-0.0082	-0.0172	0.0460	0.0259
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	1.2971	1.2193	0.0085	0.0218	0.0082	-10.3787
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	0.0000	2.4720	-0.0082	-0.0172	0.0460	0.0259
ULS: 7. 0.6D + 0.6W_Wind downforce Case A only	-1.7295	3.1533	-0.0272	-0.0625	0.0779	14.1901
ULS: 7. 0.6D + 0.6W_Wind downforce Case B only	0.0000	1.4832	-0.0049	-0.0103	0.0276	0.0155
ULS: 7. 0.6D + 0.6W_Wind uplift Case A only	1.7295	-0.1870	0.0174	0.0417	-0.0228	-13.8572
ULS: 7. 0.6D + 0.6W_Wind uplift Case B only	0.0000	1.4832	-0.0049	-0.0103	0.0276	0.0155

Worst Case Reactions LRFD

These calculations are taken directly from the FEA via SkyCiv and are used in the Concrete Checks of the Foundatino Module. Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	12.7552
Shear X	-2.8826
Shear Z	-0.0596
Moment X	-0.1300
Moment Y (Twist)	0.2848
Moment Z	24.2638

Worst Case Reactions ASD

These results are taken from the worst case values in the above table and are used in the Soil Checks in the Foundation Module. Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	7.7202
Shear X	-1.7295
Shear Z	-0.0389
Moment X	-0.0852
Moment Y (Twist)	0.1702
Moment Z	14.2005

Reaction Forces for Foundation 2 (Node ID#101), (kip, kip-ft)

ASD Load Combination Results

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 1. D	-0.0000	2.4720	0.0082	0.0172	-0.0460	0.0259
ULS: 2. D + L	-0.0000	2.4720	0.0082	0.0172	-0.0460	0.0259
ULS: 3. D + (S or Lr or R)	-0.0000	7.7202	0.0268	0.0557	-0.1613	0.0638
ULS: 3. D + (S or Lr or R)	-0.0000	2.4720	0.0082	0.0172	-0.0460	0.0259
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	-0.0000	6.4081	0.0221	0.0461	-0.1325	0.0543
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	-0.0000	2.4720	0.0082	0.0172	-0.0460	0.0259
ULS: 5b. D + 0.7E	-0.0000	2.4720	0.0082	0.0172	-0.0460	0.0259

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 6b. D + 0.75L + 0.75(0.7)E + 0.75S	-0.0000	6.4081	0.0221	0.0461	-0.1325	0.0543
ULS: 8. 0.6D + 0.7E	-0.0000	1.4832	0.0049	0.0103	-0.0276	0.0155
ULS: 5a. D + 0.6W_Wind downforce Case A only	-1.7295	4.1421	0.0305	0.0694	-0.0963	14.2005
ULS: 5a. D + 0.6W_Wind downforce Case B only	-0.0000	2.4720	0.0082	0.0172	-0.0460	0.0259
ULS: 5a. D + 0.6W_Wind uplift Case A only	1.7295	0.8018	-0.0141	-0.0348	0.0044	-13.8469
ULS: 5a. D + 0.6W_Wind uplift Case B only	-0.0000	2.4720	0.0082	0.0172	-0.0460	0.0259
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-1.2971	7.6607	0.0389	0.0852	-0.1702	10.6853
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-0.0000	6.4081	0.0221	0.0461	-0.1325	0.0543
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	1.2971	5.1555	0.0054	0.0071	-0.0947	-10.3502
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	-0.0000	6.4081	0.0221	0.0461	-0.1325	0.0543
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-1.2971	3.7246	0.0249	0.0563	-0.0837	10.6568
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-0.0000	2.4720	0.0082	0.0172	-0.0460	0.0259
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	1.2971	1.2193	-0.0085	-0.0218	-0.0082	-10.3787
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	-0.0000	2.4720	0.0082	0.0172	-0.0460	0.0259
ULS: 7. 0.6D + 0.6W_Wind downforce Case A only	-1.7295	3.1534	0.0272	0.0625	-0.0779	14.1901
ULS: 7. 0.6D + 0.6W_Wind downforce Case B only	-0.0000	1.4832	0.0049	0.0103	-0.0276	0.0155
ULS: 7. 0.6D + 0.6W_Wind uplift Case A only	1.7295	-0.1870	-0.0174	-0.0417	0.0228	-13.8572
ULS: 7. 0.6D + 0.6W_Wind uplift Case B only	-0.0000	1.4832	0.0049	0.0103	-0.0276	0.0155

Worst Case Reactions LRFD

These calculations are taken directly from the FEA via SkyCiv and are used in the Concrete Checks of the Foundatino Module. Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	12.7553
Shear X	-2.8825
Shear Z	0.0596
Moment X	0.1296
Moment Y (Twist)	0.2846
Moment Z	24.2645

Worst Case Reactions ASD

These results are taken from the worst case values in the above table and are used in the Soil Checks in the Foundation Module. Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	7.7202
Shear X	-1.7295
Shear Z	0.0389
Moment X	0.0852
Moment Y (Twist)	0.1702
Moment Z	14.2005

Project Details

Design Code:	AISC 360-16 LRFD
Provision:	LRFD
Country:	United States
User Name:	sales@mtsolar.us
Project Name:	Wentworth Plymouth Ener - 32 mods - Jb
Unit System:	imperial

Design Input Information

Design Factors

Φ_t	Φ_c	Φ_b	Φ_v
0.9	0.9	0.9	0.9

Design Materials

ID	E (ksi)	F_y (ksi)	F_u (ksi)
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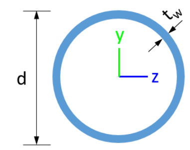
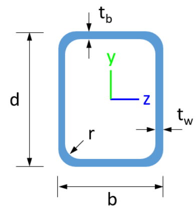
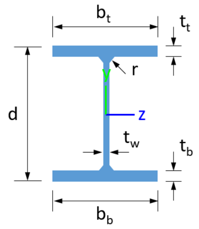
Section Dimensions



ID	Name	d (in)	t_w (in)					
3	2in Pipe Sch 120	2.38	0.25					
6	4in Pipe Sch 120	4.50	0.44					
7	6in Pipe Sch 40	6.63	0.28					



ID	Name	d (in)	b (in)	t_w (in)	t_b (in)	r (in)		
17	HSS5x3x1/4	5.00	3.00	0.23	0.23	0.23		



ID	Name	d (in)	t_w (in)	b_t (in)	b_b (in)	t_t (in)	t_b (in)	r (in)
20	W10x12	9.87	0.19	3.96	3.96	0.21	0.21	0.30

Section Properties

ID	Name	A (in^2)	J (in^4)	I_{yp} (in^4)	I_{zp} (in^4)	I_w (in^6)	S_{yp} (in^3)	S_{zp} (in^3)
3	2in Pipe Sch 120	1.67	1.91	0.96	0.96	0.00	1.13	1.13
6	4in Pipe Sch 120	5.58	23.29	11.64	11.64	0.00	7.24	7.24

7	6in Pipe Sch 40	5.58	56.28	28.14	28.14	0.00	11.28	11.28
17	HSS5x3x1/4	3.37	11.00	4.81	10.70	0.93	3.77	5.38
20	W10x12	3.54	0.05	2.18	53.80	50.90	1.74	12.60

Member Properties

Member ID	Section ID	K_zL (ft)	K_yL (ft)	L_b(ft)	C_b	LST	LSC	LD
1	7	17.03	17.03	8.11	-	300	200	1
2	6	1.30	1.30	2.00	-	300	200	1
3	17	0.92	0.92	1.42	1.19,1.18,1.19,1.18,1.19,1.19,1.18,1.18,1.15,1.18,1.18,1.19,1.17,1.19,1.18,1.18,1.18,1.18,1.18,1.19,1.12,1.19,1.18,1.19,1.17,1.19	300	200	1
4	17	2.44	2.44	3.75	1.69,1.68,1.69,1.67,1.68,1.69,1.67,1.68,1.60,1.68,1.67,1.69,1.65,1.69,1.67,1.67,1.67,1.67,1.68,1.69,1.55,1.69,1.67,1.69,1.66,1.69	300	200	1
5	17	1.52	1.52	2.33	1.68,1.67,1.68,1.67,1.68,1.68,1.67,1.67,1.63,1.67,1.67,1.68,1.66,1.68,1.67,1.67,1.67,1.67,1.67,1.68,1.60,1.68,1.67,1.68,1.66,1.68	300	200	1
6	17	0.92	0.92	1.42	1.19,1.18,1.19,1.18,1.18,1.19,1.18,1.18,1.14,1.18,1.18,1.19,1.17,1.19,1.18,1.18,1.18,1.18,1.18,1.19,1.11,1.19,1.18,1.19,1.17,1.19	300	200	1
7	17	1.52	1.52	2.33	1.68,1.67,1.68,1.67,1.68,1.68,1.67,1.67,1.63,1.67,1.67,1.68,1.66,1.68,1.67,1.67,1.67,1.67,1.67,1.68,1.60,1.68,1.67,1.68,1.66,1.68	300	200	1
8	20	1.33	1.33	2.05	1.97,1.97,1.97,1.98,1.97,1.97,1.99,1.97,2.22,1.97,2.00,1.97,2.04,1.97,1.98,1.98,1.96,1.98,1.99,1.97,2.22,1.97,2.00,1.97,2.03,1.97	300	200	1
9	3	2.60	2.60	4.00	-	300	200	1
10	17	2.44	2.44	3.75	1.69,1.68,1.69,1.67,1.68,1.69,1.67,1.68,1.60,1.68,1.67,1.69,1.65,1.69,1.67,1.67,1.67,1.67,1.68,1.69,1.55,1.69,1.67,1.69,1.66,1.69	300	200	1
11	20	1.33	1.33	2.05	1.97,1.98,1.97,1.98,1.97,1.97,2.00,1.98,2.22,1.98,2.00,1.97,2.05,1.97,1.99,1.98,1.96,1.98,1.99,1.97,2.23,1.97,2.00,1.97,2.03,1.97	300	200	1
12	6	2.00	1.30	2.00	-	300	200	1
13	20	4.88	4.00	7.50	1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.09,1.07,1.07,1.07,1.08,1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.09,1.07,1.07,1.07,1.07,1.07	300	200	1
14	20	4.88	4.00	7.50	1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.09,1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.09,1.07,1.07,1.07,1.07,1.07	300	200	1
15	20	12.60	12.60	6.00	2.33,2.32,2.33,2.32,2.32,2.33,2.32,2.32,2.33,2.32,2.32,2.33,2.30,2.33,2.32,2.32,2.31,2.32,2.32,2.33,2.25,2.33,2.32,2.33,2.31,2.33	300	200	1
16	20	12.60	12.60	6.00	2.33,2.32,2.33,2.31,2.32,2.33,2.32,2.32,2.33,2.32,2.32,2.33,2.30,2.33,2.31,2.31,2.30,2.31,2.32,2.33,2.24,2.33,2.31,2.33,2.30,2.33	300	200	1
101	7	17.03	17.03	8.11	-	300	200	1
102	6	2.00	1.30	2.00	-	300	200	1
103	17	0.92	0.92	1.42	1.19,1.18,1.19,1.18,1.18,1.19,1.18,1.18,1.14,1.18,1.18,1.19,1.17,1.19,1.18,1.18,1.18,1.18,1.18,1.19,1.11,1.19,1.18,1.19,1.17,1.19	300	200	1
104	17	2.44	2.44	3.75	1.69,1.68,1.69,1.67,1.68,1.69,1.67,1.68,1.60,1.68,1.67,1.69,1.65,1.69,1.67,1.67,1.67,1.67,1.68,1.69,1.55,1.69,1.67,1.69,1.66,1.69	300	200	1
105	17	1.52	1.52	2.33	1.68,1.67,1.68,1.67,1.68,1.68,1.67,1.67,1.63,1.67,1.67,1.68,1.66,1.68,1.67,1.67,1.67,1.67,1.67,1.68,1.60,1.68,1.67,1.68,1.66,1.68	300	200	1
106	17	0.92	0.92	1.42	1.19,1.18,1.19,1.18,1.19,1.19,1.18,1.18,1.15,1.18,1.18,1.19,1.17,1.19,1.18,1.18,1.18,1.18,1.18,1.19,1.12,1.19,1.18,1.19,1.17,1.19	300	200	1
107	17	1.52	1.52	2.33	1.68,1.67,1.68,1.67,1.68,1.68,1.67,1.67,1.63,1.67,1.67,1.68,1.66,1.68,1.67,1.67,1.67,1.67,1.67,1.68,1.60,1.68,1.67,1.68,1.66,1.68	30	20	1

				[illegible]	[illegible]	0	0	
108	20	12.60	12.60	6.00	2.33,2.32,2.33,2.31,2.32,2.33,2.32,2.32,2.33,2.32,2.32,2.33,2.30,2.33,2.31,2.31,2.30,2.31,2.32,2.33,2.24,2.33,2.31,2.33,2.30,2.33	300	200	1
109	3	2.60	2.60	4.00	-	300	200	1
110	17	2.44	2.44	3.75	1.69,1.68,1.69,1.67,1.68,1.69,1.67,1.68,1.60,1.68,1.67,1.69,1.65,1.69,1.67,1.67,1.67,1.67,1.68,1.69,1.55,1.69,1.67,1.69,1.66,1.69	300	200	1
111	20	12.60	12.60	6.00	2.33,2.32,2.33,2.32,2.32,2.33,2.32,2.32,2.33,2.32,2.32,2.33,2.30,2.33,2.32,2.32,2.31,2.32,2.32,2.33,2.25,2.33,2.32,2.33,2.31,2.33	300	200	1
112	6	1.30	1.30	2.00	-	300	200	1
113	20	4.88	4.00	7.50	1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.08,1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.09,1.07,1.07,1.07,1.07,1.07	300	200	1
114	20	4.88	4.00	7.50	1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.09,1.07,1.07,1.07,1.08,1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.09,1.07,1.07,1.07,1.07,1.07	300	200	1
115	20	8.42	8.42	12.95	1.20,1.20,1.20,1.20,1.20,1.20,1.19,1.20,1.17,1.20,1.19,1.20,1.19,1.20,1.19,1.20,1.20,1.20,1.19,1.20,1.18,1.20,1.19,1.20,1.19,1.20	300	200	1
116	20	8.42	8.42	12.95	1.20,1.20,1.20,1.20,1.20,1.19,1.20,1.17,1.20,1.19,1.20,1.19,1.20,1.20,1.20,1.20,1.19,1.20,1.18,1.20,1.19,1.20,1.19,1.20	300	200	1

Member Design Capacity

Member ID	$\Phi_t P_n$ (kip)	$\Phi_c P_n$ (kip)	$\Phi_b M_{zn}$ (k-ft)	$\Phi_b M_{yn}$ (k-ft)	$\Phi_v V_{yn}$ (kip)	$\Phi_v V_{zn}$ (kip)
1	251.16	137.11	42.30	42.30	75.35	75.35
2	251.01	248.88	27.16	27.16	75.30	75.30
3	151.65	150.70	20.17	14.14	54.12	28.95
4	151.65	145.15	20.17	14.14	54.12	28.95
5	151.65	149.10	20.17	14.14	54.12	28.95
6	151.65	150.70	20.17	14.14	54.12	28.95
7	151.65	149.10	20.17	14.14	54.12	28.95
8	159.30	142.47	46.90	6.46	56.26	44.91
9	75.10	66.32	4.25	4.25	22.53	22.53
10	151.65	145.15	20.17	14.14	54.12	28.95
11	159.30	142.47	46.90	6.46	56.26	44.91
12	251.01	246.00	27.16	27.16	75.30	75.30
13	159.30	116.35	32.69	6.46	56.26	44.91
14	159.30	116.35	32.68	6.46	56.26	44.91
15	159.30	21.54	46.90	6.46	56.26	44.91
16	159.30	21.54	46.90	6.46	56.26	44.91
101	251.16	137.11	42.30	42.30	75.35	75.35
102	251.01	246.00	27.16	27.16	75.30	75.30
103	151.65	150.70	20.17	14.14	54.12	28.95
104	151.65	145.15	20.17	14.14	54.12	28.95
105	151.65	149.10	20.17	14.14	54.12	28.95
106	151.65	150.70	20.17	14.14	54.12	28.95
107	151.65	149.10	20.17	14.14	54.12	28.95
108	159.30	21.54	46.90	6.46	56.26	44.91
109	75.10	66.32	4.25	4.25	22.53	22.53
110	151.65	145.15	20.17	14.14	54.12	28.95
111	159.30	21.54	46.90	6.46	56.26	44.91
112	251.01	248.88	27.16	27.16	75.30	75.30
113	159.30	116.35	32.69	6.46	56.26	44.91
114	159.30	116.35	32.67	6.46	56.26	44.91

115	159.30	48.27	15.54	6.46	56.26	44.91
116	159.30	48.27	15.54	6.46	56.26	44.91

Design Ratio

Member ID	P	M_z	M_y	V_y	V_z	(P,M_z,M_y)	Worst LC	KL/r	δ	Status
1	0.093	0.574	0.008	0.038	0.001	0.607	#13	0.455	Not Required	Pass
2	0.007	0.365	0.122	0.083	0.020	0.471	#21	0.054	Not Required	Pass
3	0.004	0.459	0.142	0.046	0.060	0.604	#21	0.046	Not Required	Pass
4	0.004	0.454	0.041	0.045	0.004	0.494	#21	0.082	Not Required	Pass
5	0.004	0.284	0.033	0.046	0.013	0.319	#21	0.076	Not Required	Pass
6	0.003	0.445	0.156	0.044	0.063	0.601	#21	0.046	Not Required	Pass
7	0.003	0.276	0.025	0.044	0.009	0.302	#21	0.076	Not Required	Pass
8	0.009	0.056	0.153	0.030	0.014	0.166	#21	0.102	Not Required	Pass
9	0.028	0.044	0.045	0.002	0.001	0.094	#21	0.137	Not Required	Pass
10	0.003	0.439	0.066	0.044	0.009	0.506	#21	0.082	Not Required	Pass
11	0.009	0.056	0.152	0.031	0.014	0.165	#21	0.068	Not Required	Pass
12	0.007	0.352	0.115	0.084	0.020	0.447	#21	0.083	Not Required	Pass
13	0.009	0.222	0.050	0.038	0.009	0.275	#21	0.204	Not Required	Pass
14	0.014	0.223	0.043	0.037	0.010	0.272	#21	0.306	Not Required	Pass
15	0.009	0.088	0.175	0.025	0.015	0.209	#21	0.642	Not Required	Pass
16	0.068	0.087	0.175	0.024	0.014	0.238	#21	0.642	Not Required	Pass
101	0.093	0.574	0.008	0.038	0.001	0.607	#13	0.455	Not Required	Pass
102	0.007	0.352	0.115	0.084	0.020	0.447	#21	0.083	Not Required	Pass
103	0.003	0.445	0.156	0.044	0.063	0.601	#21	0.046	Not Required	Pass
104	0.003	0.439	0.066	0.044	0.009	0.506	#21	0.082	Not Required	Pass
105	0.003	0.276	0.025	0.044	0.009	0.302	#21	0.076	Not Required	Pass
106	0.004	0.459	0.142	0.046	0.060	0.604	#21	0.046	Not Required	Pass
107	0.004	0.284	0.033	0.046	0.013	0.319	#21	0.076	Not Required	Pass
108	0.068	0.087	0.175	0.024	0.014	0.238	#21	0.963	Not Required	Warn
109	0.028	0.044	0.045	0.002	0.001	0.094	#21	0.137	Not Required	Pass
110	0.004	0.454	0.041	0.045	0.004	0.494	#21	0.082	Not Required	Pass
111	0.009	0.088	0.175	0.025	0.015	0.209	#21	0.642	Not Required	Pass
112	0.007	0.365	0.122	0.083	0.020	0.471	#21	0.054	Not Required	Pass
113	0.009	0.222	0.050	0.038	0.009	0.275	#21	0.204	Not Required	Pass
114	0.014	0.223	0.043	0.037	0.010	0.272	#21	0.306	Not Required	Pass
115	0.010	0.220	0.200	0.031	0.021	0.344	#21	0.429	Not Required	Pass
116	0.028	0.218	0.198	0.030	0.021	0.338	#21	0.644	Not Required	Pass

Definitions

Φ_t	Safety factor for tensile
Φ_c	Safety factor for compression
Φ_b	Safety factor for flexure
Φ_v	Safety factor for shear
E	Modulus of elasticity
F_y	Specified minimum yield stress
F_u	Specified minimum tensile strength
A	Cross-sectional area
J	Torsional constant
I_{yp}	Moment of inertia about the Y axes
I_{zp}	Moment of inertia about the Z axes
I_w	Warping constant
S_{yp}	Plastic section modulus about the Y axis
S_{zp}	Plastic section modulus about the Z axis
KL	Effective length
C_b	Buckling modification factor (from all load combinations)

L_b	Length between braced points
LST	Limited slenderness for tension
LSC	Limited slenderness for compression
LD	Limited deflection
P_n	Nominal axial strength (tension/comression)
M_n	Nominal flexural strength (about Z/Y axis)
V_n	Nominal shear strength (along Z/Y axis)
P	Design ratio in case of axial force
M_z	Design ratio in case of bending about Z axis
M_y	Design ratio in case of bending about Y axis
V_y	Design ratio in case of shear along Y axis
V_z	Design ratio in case of shear along Z axis
(P,M_z,M_y)	Design ratio in case of axial force and bending action
KL/r	Design ratio in case of section slenderness
δ	Design ratio in case of member deflection
OK	Capacity is provided
NG	Capacity is not provided

REFERENCES	CALCULATIONS	RESULTS

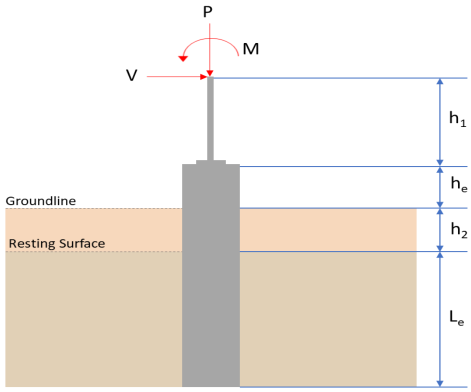
SkyCiv Foundation Design

Pile Foundation

Design Information :

Design code : IBC 2021 (International Building Code)
Unit System : Imperial

Pile Input



Geometry

Pile shape: rectangular
$b = 48\ \mathrm{in}$ - Pile width
$D = 48\ \mathrm{in}$ - Pile depth
$L = 5.25\ \mathrm{ft}$ - Total pile length
$h_1 = 0\ \mathrm{ft}$ - Lateral load height from the top of the pile,
$h_2 = 0\ \mathrm{ft}$ - Depth to resisting surface
$h_e = 0\ \mathrm{ft}$ - Length of pile above the ground

Tabulation of Soil Parameters

Layer	Label	Allowable Bearing Pressure (q_a) (psf)	Allowable Lateral Pressure (R) (psf/ft)
1	Sand, silty sand, clayey sand, silty gravel & clayey gravel	2000.000	150.000

Tabulation of Loads

Load Component	ASD	LRFD
P (kip)	7.720	12.755
V_x (kip)	-1.730	-2.883
V_z (kip)	-0.039	-0.060
M_x (kipft)	-0.085	-0.130
M_z (kipft)	14.200	24.264

Material Properties

$f'_{ck} = 2.5\ \mathrm{ksi}$ - Concrete strength,

Required depth to resist lateral loads (ASD)

H - Point of application of the lateral load

$$H = h_1 + h_2 + h_e$$

$$H = (0\ \mathrm{ft}) + (0\ \mathrm{ft}) + (0\ \mathrm{ft})$$

$$H = 0\ \mathrm{ft}$$

Considering x-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\ D}$$

$$H_o = \frac{(-1.73\ \mathrm{kip})}{1.57 \times (48\ \mathrm{in})}$$

$$H_o = -0.27548\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\ H)}{1.57\ D}$$

$$M_o = \frac{(14.2\ \text{kipft}) + ((-1.73\ \text{kip}) \times (0\ \text{ft}))}{1.57 \times (48\ \text{in})}$$

$$M_o = 2.2611\ \text{kipft/ft}$$

Required depth of embedment in earth:

$$L_x^3 - \left(14.14 \times \frac{H_o \times L_x}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,x} = 4.6925\ \text{ft}$ - Required depth in x-direction,

Considering z-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\,b}$$

$$H_o = \frac{(-0.039\ \text{kip})}{1.57 \times (48\ \text{in})}$$

$$H_o = -0.0062102\ \text{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\,H)}{1.57\,b}$$

$$M_o = \frac{(0.085\ \text{kipft}) + ((-0.039\ \text{kip}) \times (0\ \text{ft}))}{1.57 \times (48\ \text{in})}$$

$$M_o = 0.013535\ \text{kipft/ft}$$

Required depth of embedment in earth:

$$L_z^3 - \left(14.14 \times \frac{H_o \times L_z}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,z} = 0.90669\ \text{ft}$ - Required depth in z-direction,

Minimum embedded depth required:

$L_{e,req}$ - Depth of pile required,

$$L_{e,req} = MAX[L_{e,x}, L_{e,z}]$$

$$L_{e,req} = MAX[(4.6925\ \text{ft}), (0.90669\ \text{ft})]$$

$$L_{e,req} = 4.693\ \text{ft}$$

L_e - Actual embedded length of pile,

$$L_e = L - h_e - h_2$$

$$L_e = (5.25\ \text{ft}) - (0\ \text{ft}) - (0\ \text{ft})$$

$$L_e = 5.25\ \text{ft}$$

$Ratio$ - Embedded depth

$$Ratio = \frac{L_{e,req}}{L_e}$$

$$Ratio = \frac{(4.693\ \text{ft})}{(5.25\ \text{ft})}$$

$$Ratio = 0.8939$$

Status: **PASS**
Ratio: **0.890**

End-bearing Capacity (ASD)

A - Pile cross-section area

$$A = b\,D$$

$$A = (48\ \text{in}) \times (48\ \text{in})$$

$$A = 16\ \text{ft}^2$$

q - End-bearing pressure

$$q = \frac{P_v}{A}$$

$$q = \frac{(7.72\ \text{kip})}{(16\ \text{ft}^2)}$$

$$q = 0.4825\ \text{kip/ft}^2$$

$$q = 0.4825 \text{ kip/ft}^2$$

Check bearing capacity ratio:

$Ratio$ - Capacity

$$Ratio = \frac{q}{q_a}$$

$$Ratio = \frac{\left(0.4825 \text{ kip/ft}^2\right)}{(2000 \text{ psf})}$$

$$Ratio = 0.24125$$

Status: **PASS**
Ratio: **0.240**

Czerniak

Lateral Soil Pressure (ASD):

L/D - Length to least lateral dimension ratio,

$$L/D = \frac{L}{D}$$

$$L/D = \frac{(5.25 \text{ ft})}{(48 \text{ in})}$$

$$L/D = 1.3125$$

Since L/D $\leq$ 10,

Pile is short.

Considering x-direction:

$H_o = -0.27548 \text{ kip/ft}$ - Lateral force per length of pile,
$M_o = 2.2611 \text{ kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\, M_o\, L_e) + \left(3\, H_o\, L_e^2\right)}{(6\, M_o) + (4\, H_o\, L_e)}$$

$$a = \frac{(4 \times (2.2611 \text{ kipft/ft}) \times (5.25 \text{ ft})) + \left(3 \times (-0.27548 \text{ kip/ft}) \times (5.25 \text{ ft})^2\right)}{(6 \times (2.2611 \text{ kipft/ft})) + (4 \times (-0.27548 \text{ kip/ft}) \times (5.25 \text{ ft}))}$$

$$a = 3.6308 \text{ ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\, [(4\, M_o) + (3\, H_o\, L_e)]^2}{L_e^2\, [(3\, M_o) + (2\, H_o\, L_e)]}$$

$$p = \frac{0.75 \times [(4 \times (2.2611 \text{ kipft/ft})) + (3 \times (-0.27548 \text{ kip/ft}) \times (5.25 \text{ ft}))]^2}{(5.25 \text{ ft})^2 \times [(3 \times (2.2611 \text{ kipft/ft})) + (2 \times (-0.27548 \text{ kip/ft}) \times (5.25 \text{ ft}))]}$$

$$p = 0.15487 \text{ kip/ft}^2$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\, [(2\, M_o) + (H_o\, L_e)]}{L_e^2}$$

$$s = \frac{6 \times [(2 \times (2.2611 \text{ kipft/ft})) + ((-0.27548 \text{ kip/ft}) \times (5.25 \text{ ft}))]}{(5.25 \text{ ft})^2}$$

$$s = 0.66961 \text{ kip/ft}^2$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\, \frac{a}{2}$$

$$p_a = (150 \text{ psf/ft}) \times \frac{(3.6308 \text{ ft})}{2}$$

$$p_a = 0.27231 \text{ kip/ft}^2$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{\left(0.15487 \text{ kip/ft}^2\right)}{\left(0.27231 \text{ kip/ft}^2\right)}$$

$$Ratio = 0.56872$$

Status: **PASS**
Ratio: **0.570**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\,L_e$$

$$p_s = (150\ \mathrm{psf/ft}) \times (5.25\ \mathrm{ft})$$

$$p_s = 0.7875\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{\left(0.66961\ \mathrm{kip/ft^2}\right)}{\left(0.7875\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = 0.8503$$

Status: **PASS**
Ratio: **0.850**

Considering z-direction:

$H_o = -0.0062102\ \mathrm{kip/ft}$ - Lateral force per length of pile,
$M_o = 0.013535\ \mathrm{kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (0.013535\ \mathrm{kipft/ft}) \times (5.25\ \mathrm{ft})) + \left(3 \times (-0.0062102\ \mathrm{kip/ft}) \times (5.25\ \mathrm{ft})^2\right)}{(6 \times (0.013535\ \mathrm{kipft/ft})) + (4 \times (-0.0062102\ \mathrm{kip/ft}) \times (5.25\ \mathrm{ft}))}$$

$$a = 3.7696\ \mathrm{ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\,[(4\,M_o) + (3\,H_o\,L_e)]^2}{L_e^2\,[(3\,M_o) + (2\,H_o\,L_e)]}$$

$$p = \frac{0.75 \times [(4 \times (0.013535\ \mathrm{kipft/ft})) + (3 \times (-0.0062102\ \mathrm{kip/ft}) \times (5.25\ \mathrm{ft}))]^{\ 2}}{(5.25\ \mathrm{ft})^2 \times [(3 \times (0.013535\ \mathrm{kipft/ft})) + (2 \times (-0.0062102\ \mathrm{kip/ft}) \times (5.25\ \mathrm{ft}))]}$$

$$p = -0.0021093\ \mathrm{kip/ft^2}$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\,[(2\,M_o) + (H_o\,L_e)]}{L_e^2}$$

$$s = \frac{6 \times [(2 \times (0.013535\ \mathrm{kipft/ft})) + ((-0.0062102\ \mathrm{kip/ft}) \times (5.25\ \mathrm{ft}))]}{(5.25\ \mathrm{ft})^2}$$

$$s = -0.0012046\ \mathrm{kip/ft^2}$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\,\frac{a}{2}$$

$$p_a = (150\ \mathrm{psf/ft}) \times \frac{(3.7696\ \mathrm{ft})}{2}$$

$$p_a = 0.28272\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{\left(-0.0021093\ \mathrm{kip/ft^2}\right)}{\left(0.28272\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = -0.0074609$$

Status: **PASS**
Ratio: **-0.010**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\,L_e$$

$$p_s = (150\ \mathrm{psf/ft}) \times (5.25\ \mathrm{ft})$$

$$p_s = 0.7875\ \mathrm{kip/ft^2}$$

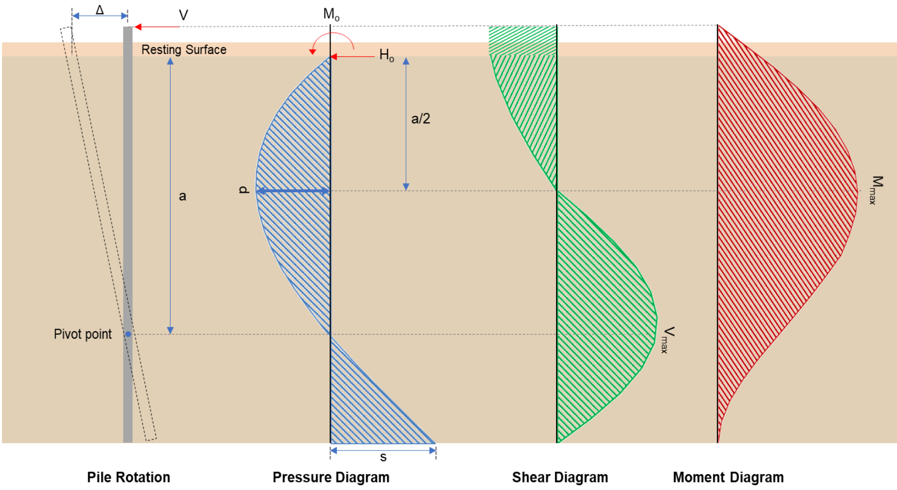
$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{(-0.0012046\ \mathrm{kip/ft^2})}{(0.7875\ \mathrm{kip/ft^2})}$$

$$Ratio = -0.0015296$$

Status: **PASS**
Ratio: **0.000**



Shear force and Bending moment (x-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\,D}$$

$$H_o = \frac{(-2.883\ \mathrm{kip})}{1.57 \times (48\ \mathrm{in})}$$

$$H_o = -0.45908\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\,H)}{1.57\,D}$$

$$M_o = \frac{(24.264\ \mathrm{kipft}) + ((-2.883\ \mathrm{kip}) \times (0\ \mathrm{ft}))}{1.57 \times (48\ \mathrm{in})}$$

$$M_o = 3.8637\ \mathrm{kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(3.8637\ \mathrm{kipft/ft})}{(-0.45908\ \mathrm{kip/ft})}$$

$$E = 8.4162\ \mathrm{ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + (3\,H_o\,L_e^2)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (3.8637\ \mathrm{kipft/ft}) \times (5.25\ \mathrm{ft})) + \left(3 \times (-0.45908\ \mathrm{kip/ft}) \times (5.25\ \mathrm{ft})^2\right)}{(6 \times (3.8637\ \mathrm{kipft/ft})) + (4 \times (-0.45908\ \mathrm{kip/ft}) \times (5.25\ \mathrm{ft}))}$$

$$a = 3.6285\ \mathrm{ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\,D)\left[1 - \left[3\left(\frac{4\,E}{L_e} + 3\right)\left(\frac{a}{L_e}\right)^2\right] + \left[4\left(\frac{3\,E}{L_e} + 2\right)\left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((-0.45908\ \mathrm{kip/ft}) \times (48\ \mathrm{in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (8.4162\ \mathrm{ft})}{(5.25\ \mathrm{ft})} + 3\right) \times \left(\frac{(3.6285\ \mathrm{ft})}{(5.25\ \mathrm{ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (8.4162\ \mathrm{ft})}{(5.25\ \mathrm{ft})} + 2\right) \times \left(\frac{(3.6285\ \mathrm{ft})}{(5.25\ \mathrm{ft})}\right)^3\right]\right]$$

$$V_{max} = 6.4199\ \mathrm{kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, D\, L_e) \left[\left(\frac{E}{L_e} + \frac{a}{2\, L_e} \right) - \left[\left(\frac{4\, E}{L_e} + 3 \right) \left(\frac{a}{2\, L_e} \right)^3 \right] + \left[\left(\frac{3\, E}{L_e} + 2 \right) \left(\frac{a}{2\, L_e} \right)^4 \right] \right]$$

$$M_{max} = ((-0.45908 \text{ kip/ft}) \times (48 \text{ in}) \times (5.25 \text{ ft})) \times \left[\left(\frac{(8.4162 \text{ ft})}{(5.25 \text{ ft})} + \frac{(3.6285 \text{ ft})}{2 \times (5.25 \text{ ft})} \right) - \left[\left(\frac{4 \times (8.4162 \text{ ft})}{(5.25 \text{ ft})} + 3 \right) \times \left(\frac{(3.6285 \text{ ft})}{2 \times (5.25 \text{ ft})} \right)^3 \right] + \left[\left(\frac{3 \times (8.4162 \text{ ft})}{(5.25 \text{ ft})} + 2 \right) \times \left(\frac{(3.6285 \text{ ft})}{2 \times (5.25 \text{ ft})} \right)^4 \right] \right]$$

$$M_{max} = 15.978 \text{ kipft}$$

Shear force and Bending moment (z-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\, b}$$

$$H_o = \frac{(-0.06 \text{ kip})}{1.57 \times (48 \text{ in})}$$

$$H_o = -0.0095541 \text{ kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\, H)}{1.57\, b}$$

$$M_o = \frac{(0.13 \text{ kipft}) + ((-0.06 \text{ kip}) \times (0 \text{ ft}))}{1.57 \times (48 \text{ in})}$$

$$M_o = 0.020701 \text{ kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(0.020701 \text{ kipft/ft})}{(-0.0095541 \text{ kip/ft})}$$

$$E = 2.1667 \text{ ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\, M_o\, L_e) + \left(3\, H_o\, L_e^2\right)}{(6\, M_o) + (4\, H_o\, L_e)}$$

$$a = \frac{(4 \times (0.020701 \text{ kipft/ft}) \times (5.25 \text{ ft})) + \left(3 \times (-0.0095541 \text{ kip/ft}) \times (5.25 \text{ ft})^2\right)}{(6 \times (0.020701 \text{ kipft/ft})) + (4 \times (-0.0095541 \text{ kip/ft}) \times (5.25 \text{ ft}))}$$

$$a = 3.7702 \text{ ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\, b) \left[1 - \left[3 \left(\frac{4\, E}{L_e} + 3 \right) \left(\frac{a}{L_e} \right)^2 \right] + \left[4 \left(\frac{3\, E}{L_e} + 2 \right) \left(\frac{a}{L_e} \right)^3 \right] \right]$$

$$V_{max} = ((-0.0095541 \text{ kip/ft}) \times (48 \text{ in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (2.1667 \text{ ft})}{(5.25 \text{ ft})} + 3 \right) \times \left(\frac{(3.7702 \text{ ft})}{(5.25 \text{ ft})} \right)^2 \right] + \left[4 \times \left(\frac{3 \times (2.1667 \text{ ft})}{(5.25 \text{ ft})} + 2 \right) \times \left(\frac{(3.7702 \text{ ft})}{(5.25 \text{ ft})} \right)^3 \right] \right]$$

$$V_{max} = 0.053446 \text{ kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, b\, L_e) \left[\left(\frac{E}{L_e} + \frac{a}{2\, L_e} \right) - \left[\left(\frac{4\, E}{L_e} + 3 \right) \left(\frac{a}{2\, L_e} \right)^3 \right] + \left[\left(\frac{3\, E}{L_e} + 2 \right) \left(\frac{a}{2\, L_e} \right)^4 \right] \right]$$

$$M_{max} = ((-0.0095541 \text{ kip/ft}) \times (48 \text{ in}) \times (5.25 \text{ ft})) \times \left[\left(\frac{(2.1667 \text{ ft})}{(5.25 \text{ ft})} + \frac{(3.7702 \text{ ft})}{2 \times (5.25 \text{ ft})} \right) - \left[\left(\frac{4 \times (2.1667 \text{ ft})}{(5.25 \text{ ft})} + 3 \right) \times \left(\frac{(3.7702 \text{ ft})}{2 \times (5.25 \text{ ft})} \right)^3 \right] + \left[\left(\frac{3 \times (2.1667 \text{ ft})}{(5.25 \text{ ft})} + 2 \right) \times \left(\frac{(3.7702 \text{ ft})}{2 \times (5.25 \text{ ft})} \right)^4 \right] \right]$$

$M_{max} = 0.12245 \text{ kipft}$

Minimum Reinforcement Check (LRFD)

Parameters:

$f'_{ck} = 2.5 \text{ ksi}$ - Concrete strength,
$f_{yk} = 60 \text{ ksi}$ - Longitudinal reinforcement strength,
$\phi = 0.65$ - Reduction factor for axial strength,
Table 22.4.2.1 $\alpha = 0.8$ - Alpha factor for axial strength,
$A_g = 2304 \text{ in}^2$ - Gross area of concrete,

Longitudinal reinforcement:

Required reinforcement due to axial load, $A_{st,required}$
22.4.2.2, 10.6.1.1 $A_{st,required}$

$$A_{st,required} = Min\left[\frac{\frac{P}{\phi\,\alpha} - (0.85\,f'_{ck}\,A_g)}{f_{yk} - (0.85\,f'_{ck})}, (0.08\,A_g)\right]$$

$$A_{st,required} = Min\left[\frac{\frac{(12.755 \text{ kip})}{(0.65)\times(0.8)} - \left(0.85 \times (2.5 \text{ ksi}) \times \left(2304 \text{ in}^2\right)\right)}{(60 \text{ ksi}) - (0.85 \times (2.5 \text{ ksi}))}, \left(0.08 \times \left(2304 \text{ in}^2\right)\right)\right]$$

$$A_{st,required} = -84.172 \text{ in}^2$$

A_{min} - Governing minimum reinforcement area,

$$A_{min} = Max\left[A_{st,required}, (0.0018\,A_g)\right]$$

$$A_{min} = Max\left[\left(-84.172 \text{ in}^2\right), \left(0.0018 \times \left(2304 \text{ in}^2\right)\right)\right]$$

$$A_{min} = 4.1472 \text{ in}^2$$

n_{rebar} - Required number of reinforcement,

$$n_{rebar} = \frac{A_{min}}{A_{rebar}}$$

$$n_{rebar} = \frac{\left(4.1472 \text{ in}^2\right)}{\left(0.3068 \text{ in}^2\right)}$$

$$n_{rebar} = 14$$

A_{st} - Actual total reinforcement area,

$$A_{st} = n_{rebar}\,\frac{\pi\,d_{bar}^2}{4}$$

$$A_{st} = (14) \times \frac{\pi \times (0.625 \text{ in})^2}{4}$$

$$A_{st} = 4.2951 \text{ in}^2$$

$Ratio$ - Capacity

$$Ratio = \frac{A_{min}}{A_{st}}$$

$$Ratio = \frac{\left(4.1472 \text{ in}^2\right)}{\left(4.2951 \text{ in}^2\right)}$$

$$Ratio = 0.96556$$

Status: **PASS**
Ratio: **0.970**

25.2.3 s_{rebar} - Minimum spacing of reinforcement,

$$s_{rebar} = Max\left[1.5, (1.5\,d_{bar})\right]$$

$$s_{rebar} = Max\left[1.5, (1.5 \times (0.625 \text{ in}))\right]$$

$$s_{rebar} = 1.5 \text{ in}$$

Ties:

25.7.2.2 Since longitudinal reinforcement is $\leq$ No. 10ø: Use #3(0.375 in)
25.7.2.1 s_{ties} - Maximum spacing of ties,

$$s_{ties} = Min\left[(16\,d_{bar}), (48\,d_{ties}), Min(D, b)\right]$$

$$s_{ties} = Min\left[(16 \times (0.625 \text{ in})), (48 \times (0.375 \text{ in})), Min((48 \text{ in}), (48 \text{ in}))\right]$$

$$s_{ties} = 10 \text{ in}$$

Summary:

Main reinforcement: **14 - #5 (0.625 in)**

Ties: **#3(0.375 in) - 10 in**

Axial Compression Strength (ACI 318-19, LRFD)

22.4.2.2 ϕP_N - Allowable axial compressive strength

$$\phi P_N = \phi \, 0.80 \, \left[\left(0.85 \, f'_{ck} \, \left[A_g - A_{st}\right]\right) + \left(f_{yk} \, A_{st}\right)\right]$$

$$\phi P_N = (0.65) \times 0.80 \times \left[\left(0.85 \times (2.5 \text{ ksi}) \times \left[\left(2304 \text{ in}^2\right) - \left(4.2951 \text{ in}^2\right)\right]\right) + \left((60 \text{ ksi}) \times \left(4.2951 \text{ in}^2\right)\right)\right]$$

$$\phi P_N = 2675.2 \text{ kip}$$

$Ratio$ - Capacity

$$Ratio = \frac{P}{\phi P_N}$$

$$Ratio = \frac{(12.755 \text{ kip})}{(2675.2 \text{ kip})}$$

$$Ratio = 0.0047679$$

Status: **PASS**
Ratio: **0.000**

Shear Strength (ACI 318-19, LRFD)

Parameters:

$b_w = 48 \text{ in}$ - Effective width,

22.5.2.2 d - Effective depth

$$d = 0.80 \, D$$

$$d = 0.80 \times (48 \text{ in})$$

$$d = 38.4 \text{ in}$$

22.5.5.1.3 λ_s - size effect modification factor

$$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{d}{10}}}, 1\right]$$

$$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{(38.4 \text{ in})}{10}}}, 1\right]$$

$$\lambda_s = 0.64282$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$,

22.5.5.1.1 $V_{c,max}$ - Max shear strength of concrete

$$V_{c,max} = 5 \, \lambda_s \, \sqrt{f'_{ck}} \, b_w \, d$$

$$V_{c,max} = 5 \times (0.64282) \times \sqrt{(2500 \text{ psi})} \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{c,max} = 296.21 \text{ kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$, $P = 12.755 \text{ kip} \rightarrow 12755 \text{ lbf}$,

22.5.5.1.1(a) $V_{c,a}$ - Shear strength of concrete (a)

$$V_{c,a} = \left[2 \, \lambda_s \, \sqrt{f'_{ck}} + \frac{P}{6 \, A_g}\right] \, b_w \, d$$

$$V_{c,a} = \left[2 \times (0.64282) \times \sqrt{(2500 \text{ psi})} + \frac{(12755 \text{ lbf})}{6 \times \left(2304 \text{ in}^2\right)}\right] \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{c,a} = 120.19 \text{ kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$,

22.5.5.1.2 $V_{c,b}$ - Shear strength of concrete (b)

$$V_{c,b} = \left[2 \, \lambda_s \, \sqrt{f'_{ck}} + \left(0.05 \, f'_{ck}\right)\right] \, b_w \, d$$

$$V_{c,b} = \left[2 \times (0.64282) \times \sqrt{(2500 \text{ psi})} + (0.05 \times (2500 \text{ psi}))\right] \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{c,b} = 348.89 \text{ kip}$$

V_c - Governing shear strength of concrete

$$V_c = Min\left[V_{c,max}, V_{c,a}, V_{c,b}\right]$$

$$V_c = Min\left[(296.21 \text{ kip}), (120.19 \text{ kip}), (348.89 \text{ kip})\right]$$

$$V_c = 120.19 \text{ kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$,

22.5.1.2 $V_{s,a}$ - Shear strength of steel (a)

$$V_{s,a} = 8 \sqrt{f'_{ck}}\, b_w\, d$$

$$V_{s,a} = 8 \times \sqrt{(2500 \text{ psi})} \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{s,a} = 737.28 \text{ kip}$$

A_v - Ties rebar area,

$$A_v = \frac{\pi\, d_{ties}^2}{4}$$

$$A_v = \frac{\pi \times (0.375 \text{ in})^2}{4}$$

$$A_v = 0.11045 \text{ in}^2$$

22.5.8.5.3 $V_{s,b}$ - Shear strength of steel (b)

$$V_{s,b} = \frac{2\, A_v\, f_{ywk}\, d}{s_{ties}}$$

$$V_{s,b} = \frac{2 \times \left(0.11045 \text{ in}^2\right) \times (60 \text{ ksi}) \times (38.4 \text{ in})}{(10 \text{ in})}$$

$$V_{s,b} = 50.894 \text{ kip}$$

V_s - Governing shear strength of steel

$$V_s = MIN\left[V_{s,a}, V_{s,b}\right]$$

$$V_s = MIN\left[(737.28 \text{ kip}), (50.894 \text{ kip})\right]$$

$$V_s = 50.894 \text{ kip}$$

22.5.1.1 ϕV_n - Allowable shear strength

$$\phi V_n = \phi\ (V_c + V_s)$$

$$\phi V_n = (0.65) \times ((120.19 \text{ kip}) + (50.894 \text{ kip}))$$

$$\phi V_n = 111.2 \text{ kip}$$

Considering x-direction:

$V_{max} = 6.4199 \text{ kip}$ - Maximum shear force in the x-direction,
$Ratio$ - Capacity

$$Ratio = \frac{V_{max}}{\phi V_n}$$

$$Ratio = \frac{(6.4199 \text{ kip})}{(111.2 \text{ kip})}$$

$$Ratio = 0.057732$$

Status: **PASS**
Ratio: **0.060**

Considering z-direction:

$V_{max} = 0.053446 \text{ kip}$ - Maximum shear force in the z-direction,
$Ratio$ - Capacity

$$Ratio = \frac{V_{max}}{\phi V_n}$$

$$Ratio = \frac{(0.053446 \text{ kip})}{(111.2 \text{ kip})}$$

$$Ratio = 0.00048062$$

Status: **PASS**
Ratio: **0.000**

Flexural Strength (ACI 318-19, LRFD)

S_m - Section modulus

$$S_m = \frac{b\, D^2}{6}$$

$$S_m = \frac{(48 \text{ in}) \times (48 \text{ in})^2}{6}$$

$$S_m = 18432 \text{ in}^3$$

	$\lambda = 1$ - Concrete modification factor (Normal concrete), Allowable flexural strength: M_n shall be the lesser of: $\phi M_{n,1}$ $$\phi M_{n,1} = \phi \times 5 \times \lambda \times \sqrt{(f_c')} \times S_m$$ $$\phi M_{n,1} = 0.65 \times 5 \times 1 \times \sqrt{(2.5ksi)} \times 18432.001 in^3$$ $$\phi M_{n,1} = 249.600 kipft$$	
14.5.2.1b	$\phi M_{n,2}$ $$\phi M_{n,2} = \phi\, 0.85\, f_{ck}'\, S_m$$ $$\phi M_{n,2} = (0.65) \times 0.85 \times (2.5 \text{ ksi}) \times \left(18432 \text{ in}^3\right)$$ $$\phi M_{n,2} = 2121.6 \text{ kipft}$$ Therefore, ϕM_n - Allowable flexural strength, $$\phi M_n = MIN[\phi M_{n,1}, \phi M_{n,2}]$$ $$\phi M_n = MIN[(249.6 \text{ kipft}), (2121.6 \text{ kipft})]$$ $$\phi M_n = 249.6 \text{ kipft}$$ **Considering x-direction:** $M_{max} = 15.978 \text{ kipft}$ - Maximum moment in the x-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\phi M_n}$$ $$Ratio = \frac{(15.978 \text{ kipft})}{(249.6 \text{ kipft})}$$ $$Ratio = 0.064014$$	Status: **PASS** Ratio: **0.060**
	Considering z-direction: $M_{max} = 0.12245 \text{ kipft}$ - Maximum moment in the z-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\phi M_n}$$ $$Ratio = \frac{(0.12245 \text{ kipft})}{(249.6 \text{ kipft})}$$ $$Ratio = 0.00049057$$	Status: **PASS** Ratio: **0.000**

REFERENCES	CALCULATIONS	RESULTS

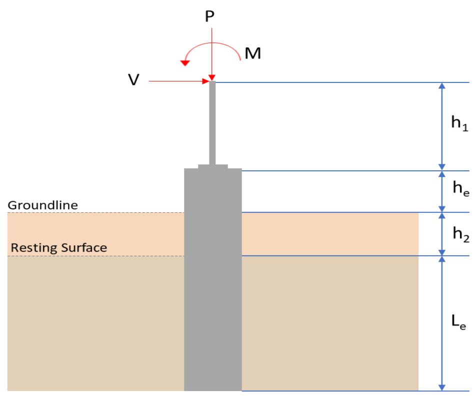
SkyCiv Foundation Design

Pile Foundation

Design Information :

Design code : IBC 2021 (International Building Code)
Unit System : Imperial

Pile Input



Geometry

Pile shape: rectangular
$b = 48\ \text{in}$ - Pile width
$D = 48\ \text{in}$ - Pile depth
$L = 5.25\ \text{ft}$ - Total pile length
$h_1 = 0\ \text{ft}$ - Lateral load height from the top of the pile,
$h_2 = 0\ \text{ft}$ - Depth to resisting surface
$h_e = 0\ \text{ft}$ - Length of pile above the ground

Tabulation of Soil Parameters

Layer	Label	Allowable Bearing Pressure (q_a) (psf)	Allowable Lateral Pressure (R) (psf/ft)
1	Sand, silty sand, clayey sand, silty gravel & clayey gravel	2000.000	150.000

Tabulation of Loads

Load Component	ASD	LRFD
P (kip)	7.720	12.755
V_x (kip)	-1.730	-2.883
V_z (kip)	0.039	0.060
M_x (kipft)	0.085	0.130
M_z (kipft)	14.200	24.264

Material Properties

$f'_{ck} = 2.5\ \text{ksi}$ - Concrete strength,

Required depth to resist lateral loads (ASD)

H - Point of application of the lateral load

$$H = h_1 + h_2 + h_e$$

$$H = (0\ \text{ft}) + (0\ \text{ft}) + (0\ \text{ft})$$

$$H = 0\ \text{ft}$$

Considering x-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\ D}$$

$$H_o = \frac{(-1.73\ \text{kip})}{1.57 \times (48\ \text{in})}$$

$$H_o = -0.27548\ \text{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\ H)}{1.57\ D}$$

$$M_o = \frac{(14.2\ \text{kipft}) + ((-1.73\ \text{kip}) \times (0\ \text{ft}))}{1.57 \times (48\ \text{in})}$$

$$M_o = 2.2611\ \text{kipft/ft}$$

Required depth of embedment in earth:

$$L_x^3 - \left(14.14 \times \frac{H_o \times L_x}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,x} = 4.6925\ \text{ft}$ - Required depth in x-direction,

Considering z-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\,b}$$

$$H_o = \frac{(0.039\ \text{kip})}{1.57 \times (48\ \text{in})}$$

$$H_o = 0.0062102\ \text{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\,H)}{1.57\,b}$$

$$M_o = \frac{(0.085\ \text{kipft}) + ((0.039\ \text{kip}) \times (0\ \text{ft}))}{1.57 \times (48\ \text{in})}$$

$$M_o = 0.013535\ \text{kipft/ft}$$

Required depth of embedment in earth:

$$L_z^3 - \left(14.14 \times \frac{H_o \times L_z}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,z} = 1.1474\ \text{ft}$ - Required depth in z-direction,

Minimum embedded depth required:

$L_{e,req}$ - Depth of pile required,

$$L_{e,req} = MAX[L_{e,x}, L_{e,z}]$$

$$L_{e,req} = MAX[(4.6925\ \text{ft}), (1.1474\ \text{ft})]$$

$$L_{e,req} = 4.693\ \text{ft}$$

L_e - Actual embedded length of pile,

$$L_e = L - h_e - h_2$$

$$L_e = (5.25\ \text{ft}) - (0\ \text{ft}) - (0\ \text{ft})$$

$$L_e = 5.25\ \text{ft}$$

$Ratio$ - Embedded depth

$$Ratio = \frac{L_{e,req}}{L_e}$$

$$Ratio = \frac{(4.693\ \text{ft})}{(5.25\ \text{ft})}$$

$$Ratio = 0.8939$$

Status: **PASS**
Ratio: **0.890**

End-bearing Capacity (ASD)

A - Pile cross-section area

$$A = b\,D$$

$$A = (48\ \text{in}) \times (48\ \text{in})$$

$$A = 16\ \text{ft}^2$$

q - End-bearing pressure

$$q = \frac{P_v}{A}$$

$$q = \frac{(7.72\ \text{kip})}{(16\ \text{ft}^2)}$$

$$q = 0.4825\ \text{kip/ft}^2$$

$q = 0.4825\ \text{kip/ft}^2$

Check bearing capacity ratio:

$Ratio$ - Capacity

$$Ratio = \frac{q}{q_a}$$

$$Ratio = \frac{\left(0.4825\ \text{kip/ft}^2\right)}{(2000\ \text{psf})}$$

$$Ratio = 0.24125$$

Status: **PASS**
Ratio: **0.240**

Czerniak

Lateral Soil Pressure (ASD):

L/D - Length to least lateral dimension ratio,

$$L/D = \frac{L}{D}$$

$$L/D = \frac{(5.25\ \text{ft})}{(48\ \text{in})}$$

$$L/D = 1.3125$$

Since L/D $\leq$ 10,

Pile is short.

Considering x-direction:

$H_o = -0.27548\ \text{kip/ft}$ - Lateral force per length of pile,
$M_o = 2.2611\ \text{kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\, M_o\, L_e) + \left(3\, H_o\, L_e^2\right)}{(6\, M_o) + (4\, H_o\, L_e)}$$

$$a = \frac{(4 \times (2.2611\ \text{kipft/ft}) \times (5.25\ \text{ft})) + \left(3 \times (-0.27548\ \text{kip/ft}) \times (5.25\ \text{ft})^2\right)}{(6 \times (2.2611\ \text{kipft/ft})) + (4 \times (-0.27548\ \text{kip/ft}) \times (5.25\ \text{ft}))}$$

$$a = 3.6308\ \text{ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\,[(4\, M_o) + (3\, H_o\, L_e)]^2}{L_e^2\,[(3\, M_o) + (2\, H_o\, L_e)]}$$

$$p = \frac{0.75 \times [(4 \times (2.2611\ \text{kipft/ft})) + (3 \times (-0.27548\ \text{kip/ft}) \times (5.25\ \text{ft}))]^2}{(5.25\ \text{ft})^2 \times [(3 \times (2.2611\ \text{kipft/ft})) + (2 \times (-0.27548\ \text{kip/ft}) \times (5.25\ \text{ft}))]}$$

$$p = 0.15487\ \text{kip/ft}^2$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\,[(2\, M_o) + (H_o\, L_e)]}{L_e^2}$$

$$s = \frac{6 \times [(2 \times (2.2611\ \text{kipft/ft})) + ((-0.27548\ \text{kip/ft}) \times (5.25\ \text{ft}))]}{(5.25\ \text{ft})^2}$$

$$s = 0.66961\ \text{kip/ft}^2$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\,\frac{a}{2}$$

$$p_a = (150\ \text{psf/ft}) \times \frac{(3.6308\ \text{ft})}{2}$$

$$p_a = 0.27231\ \text{kip/ft}^2$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{\left(0.15487\ \text{kip/ft}^2\right)}{\left(0.27231\ \text{kip/ft}^2\right)}$$

$$Ratio = 0.56872$$

Status: **PASS**
Ratio: **0.570**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\,L_e$$

$$p_s = (150\ \mathrm{psf/ft}) \times (5.25\ \mathrm{ft})$$

$$p_s = 0.7875\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{\left(0.66961\ \mathrm{kip/ft^2}\right)}{\left(0.7875\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = 0.8503$$

Status: **PASS**
Ratio: **0.850**

Considering z-direction:

$H_o = 0.0062102\ \mathrm{kip/ft}$ - Lateral force per length of pile,
$M_o = 0.013535\ \mathrm{kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (0.013535\ \mathrm{kipft/ft}) \times (5.25\ \mathrm{ft})) + \left(3 \times (0.0062102\ \mathrm{kip/ft}) \times (5.25\ \mathrm{ft})^2\right)}{(6 \times (0.013535\ \mathrm{kipft/ft})) + (4 \times (0.0062102\ \mathrm{kip/ft}) \times (5.25\ \mathrm{ft}))}$$

$$a = 3.7696\ \mathrm{ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\,[(4\,M_o) + (3\,H_o\,L_e)]^2}{L_e^2\,[(3\,M_o) + (2\,H_o\,L_e)]}$$

$$p = \frac{0.75 \times [(4 \times (0.013535\ \mathrm{kipft/ft})) + (3 \times (0.0062102\ \mathrm{kip/ft}) \times (5.25\ \mathrm{ft}))]^2}{(5.25\ \mathrm{ft})^2 \times [(3 \times (0.013535\ \mathrm{kipft/ft})) + (2 \times (0.0062102\ \mathrm{kip/ft}) \times (5.25\ \mathrm{ft}))]}$$

$$p = 0.0059376\ \mathrm{kip/ft^2}$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\,[(2\,M_o) + (H_o\,L_e)]}{L_e^2}$$

$$s = \frac{6 \times [(2 \times (0.013535\ \mathrm{kipft/ft})) + ((0.0062102\ \mathrm{kip/ft}) \times (5.25\ \mathrm{ft}))]}{(5.25\ \mathrm{ft})^2}$$

$$s = 0.01299\ \mathrm{kip/ft^2}$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\,\frac{a}{2}$$

$$p_a = (150\ \mathrm{psf/ft}) \times \frac{(3.7696\ \mathrm{ft})}{2}$$

$$p_a = 0.28272\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{\left(0.0059376\ \mathrm{kip/ft^2}\right)}{\left(0.28272\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = 0.021002$$

Status: **PASS**
Ratio: **0.020**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\,L_e$$

$$p_s = (150\ \mathrm{psf/ft}) \times (5.25\ \mathrm{ft})$$

$$p_s = 0.7875\ \mathrm{kip/ft^2}$$

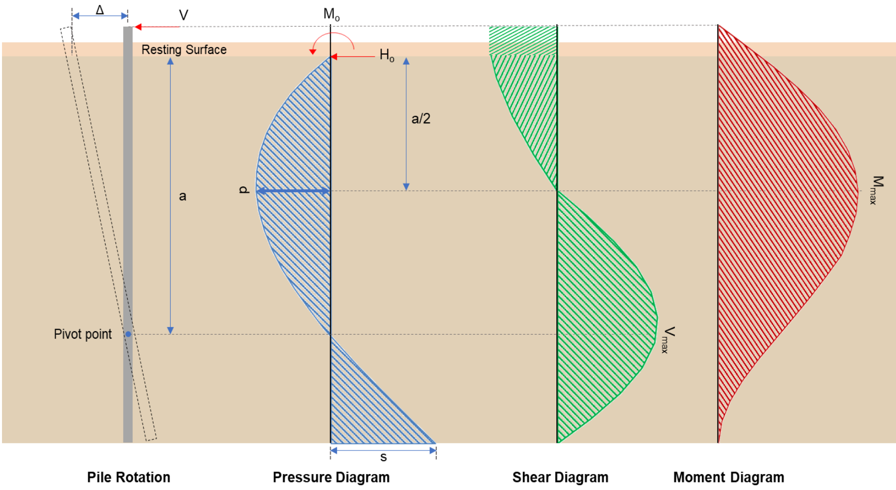
$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{(0.01299\ \text{kip/ft}^2)}{(0.7875\ \text{kip/ft}^2)}$$

$$Ratio = 0.016495$$

Status: **PASS**
Ratio: **0.020**



Shear force and Bending moment (x-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\,D}$$

$$H_o = \frac{(-2.883\ \text{kip})}{1.57 \times (48\ \text{in})}$$

$$H_o = -0.45908\ \text{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\,H)}{1.57\,D}$$

$$M_o = \frac{(24.264\ \text{kipft}) + ((-2.883\ \text{kip}) \times (0\ \text{ft}))}{1.57 \times (48\ \text{in})}$$

$$M_o = 3.8637\ \text{kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(3.8637\ \text{kipft/ft})}{(-0.45908\ \text{kip/ft})}$$

$$E = 8.4162\ \text{ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (3.8637\ \text{kipft/ft}) \times (5.25\ \text{ft})) + \left(3 \times (-0.45908\ \text{kip/ft}) \times (5.25\ \text{ft})^2\right)}{(6 \times (3.8637\ \text{kipft/ft})) + (4 \times (-0.45908\ \text{kip/ft}) \times (5.25\ \text{ft}))}$$

$$a = 3.6285\ \text{ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\,D)\left[1 - \left[3\left(\frac{4\,E}{L_e} + 3\right)\left(\frac{a}{L_e}\right)^2\right] + \left[4\left(\frac{3\,E}{L_e} + 2\right)\left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((-0.45908\ \text{kip/ft}) \times (48\ \text{in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (8.4162\ \text{ft})}{(5.25\ \text{ft})} + 3\right) \times \left(\frac{(3.6285\ \text{ft})}{(5.25\ \text{ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (8.4162\ \text{ft})}{(5.25\ \text{ft})} + 2\right) \times \left(\frac{(3.6285\ \text{ft})}{(5.25\ \text{ft})}\right)^3\right]\right]$$

$$V_{max} = 6.4199\ \text{kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, D\, L_e) \left[\left(\frac{E}{L_e} + \frac{a}{2\, L_e} \right) - \left[\left(\frac{4\, E}{L_e} + 3 \right) \left(\frac{a}{2\, L_e} \right)^3 \right] + \left[\left(\frac{3\, E}{L_e} + 2 \right) \left(\frac{a}{2\, L_e} \right)^4 \right] \right]$$

$$M_{max} = ((-0.45908 \text{ kip/ft}) \times (48 \text{ in}) \times (5.25 \text{ ft})) \times \left[\left(\frac{(8.4162 \text{ ft})}{(5.25 \text{ ft})} + \frac{(3.6285 \text{ ft})}{2 \times (5.25 \text{ ft})} \right) - \left[\left(\frac{4 \times (8.4162 \text{ ft})}{(5.25 \text{ ft})} + 3 \right) \times \left(\frac{(3.6285 \text{ ft})}{2 \times (5.25 \text{ ft})} \right)^3 \right] + \left[\left(\frac{3 \times (8.4162 \text{ ft})}{(5.25 \text{ ft})} + 2 \right) \times \left(\frac{(3.6285 \text{ ft})}{2 \times (5.25 \text{ ft})} \right)^4 \right] \right]$$

$$M_{max} = 15.978 \text{ kipft}$$

Shear force and Bending moment (z-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\, b}$$

$$H_o = \frac{(0.06 \text{ kip})}{1.57 \times (48 \text{ in})}$$

$$H_o = 0.0095541 \text{ kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\, H)}{1.57\, b}$$

$$M_o = \frac{(0.13 \text{ kipft}) + ((0.06 \text{ kip}) \times (0 \text{ ft}))}{1.57 \times (48 \text{ in})}$$

$$M_o = 0.020701 \text{ kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(0.020701 \text{ kipft/ft})}{(0.0095541 \text{ kip/ft})}$$

$$E = 2.1667 \text{ ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\, M_o\, L_e) + \left(3\, H_o\, L_e^2\right)}{(6\, M_o) + (4\, H_o\, L_e)}$$

$$a = \frac{(4 \times (0.020701 \text{ kipft/ft}) \times (5.25 \text{ ft})) + \left(3 \times (0.0095541 \text{ kip/ft}) \times (5.25 \text{ ft})^2\right)}{(6 \times (0.020701 \text{ kipft/ft})) + (4 \times (0.0095541 \text{ kip/ft}) \times (5.25 \text{ ft}))}$$

$$a = 3.7702 \text{ ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\, b) \left[1 - \left[3 \left(\frac{4\, E}{L_e} + 3 \right) \left(\frac{a}{L_e} \right)^2 \right] + \left[4 \left(\frac{3\, E}{L_e} + 2 \right) \left(\frac{a}{L_e} \right)^3 \right] \right]$$

$$V_{max} = ((0.0095541 \text{ kip/ft}) \times (48 \text{ in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (2.1667 \text{ ft})}{(5.25 \text{ ft})} + 3 \right) \times \left(\frac{(3.7702 \text{ ft})}{(5.25 \text{ ft})} \right)^2 \right] + \left[4 \times \left(\frac{3 \times (2.1667 \text{ ft})}{(5.25 \text{ ft})} + 2 \right) \times \left(\frac{(3.7702 \text{ ft})}{(5.25 \text{ ft})} \right)^3 \right] \right]$$

$$V_{max} = 0.053446 \text{ kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, b\, L_e) \left[\left(\frac{E}{L_e} + \frac{a}{2\, L_e} \right) - \left[\left(\frac{4\, E}{L_e} + 3 \right) \left(\frac{a}{2\, L_e} \right)^3 \right] + \left[\left(\frac{3\, E}{L_e} + 2 \right) \left(\frac{a}{2\, L_e} \right)^4 \right] \right]$$

$$M_{max} = ((0.0095541 \text{ kip/ft}) \times (48 \text{ in}) \times (5.25 \text{ ft})) \times \left[\left(\frac{(2.1667 \text{ ft})}{(5.25 \text{ ft})} + \frac{(3.7702 \text{ ft})}{2 \times (5.25 \text{ ft})} \right) - \left[\left(\frac{4 \times (2.1667 \text{ ft})}{(5.25 \text{ ft})} + 3 \right) \times \left(\frac{(3.7702 \text{ ft})}{2 \times (5.25 \text{ ft})} \right)^3 \right] + \left[\left(\frac{3 \times (2.1667 \text{ ft})}{(5.25 \text{ ft})} + 2 \right) \times \left(\frac{(3.7702 \text{ ft})}{2 \times (5.25 \text{ ft})} \right)^4 \right] \right]$$

$$M_{max} = 0.12245 \text{ kipft}$$

Minimum Reinforcement Check (LRFD)

Parameters:

$f'_{ck} = 2.5 \text{ ksi}$ - Concrete strength,
$f_{yk} = 60 \text{ ksi}$ - Longitudinal reinforcement strength,
$\phi = 0.65$ - Reduction factor for axial strength,
Table 22.4.2.1 $\alpha = 0.8$ - Alpha factor for axial strength,
$A_g = 2304 \text{ in}^2$ - Gross area of concrete,

Longitudinal reinforcement:

Required reinforcement due to axial load, $A_{st,required}$
22.4.2.2, 10.6.1.1 $A_{st,required}$

$$A_{st,required} = Min\left[\frac{\frac{P}{\phi\ \alpha} - (0.85\ f'_{ck}\ A_g)}{f_{yk} - (0.85\ f'_{ck})}, (0.08\ A_g)\right]$$

$$A_{st,required} = Min\left[\frac{\frac{(12.755 \text{ kip})}{(0.65)\times(0.8)} - \left(0.85 \times (2.5 \text{ ksi}) \times \left(2304 \text{ in}^2\right)\right)}{(60 \text{ ksi}) - (0.85 \times (2.5 \text{ ksi}))}, \left(0.08 \times \left(2304 \text{ in}^2\right)\right)\right]$$

$$A_{st,required} = -84.172 \text{ in}^2$$

A_{min} - Governing minimum reinforcement area,

$$A_{min} = Max\left[A_{st,required}, (0.0018\ A_g)\right]$$

$$A_{min} = Max\left[\left(-84.172 \text{ in}^2\right), \left(0.0018 \times \left(2304 \text{ in}^2\right)\right)\right]$$

$$A_{min} = 4.1472 \text{ in}^2$$

n_{rebar} - Required number of reinforcement,

$$n_{rebar} = \frac{A_{min}}{A_{rebar}}$$

$$n_{rebar} = \frac{\left(4.1472 \text{ in}^2\right)}{\left(0.3068 \text{ in}^2\right)}$$

$$n_{rebar} = 14$$

A_{st} - Actual total reinforcement area,

$$A_{st} = n_{rebar}\ \frac{\pi\ d_{bar}^2}{4}$$

$$A_{st} = (14) \times \frac{\pi \times (0.625 \text{ in})^2}{4}$$

$$A_{st} = 4.2951 \text{ in}^2$$

$Ratio$ - Capacity

$$Ratio = \frac{A_{min}}{A_{st}}$$

$$Ratio = \frac{\left(4.1472 \text{ in}^2\right)}{\left(4.2951 \text{ in}^2\right)}$$

$$Ratio = 0.96556$$

Status: **PASS**
Ratio: **0.970**

25.2.3 s_{rebar} - Minimum spacing of reinforcement,

$$s_{rebar} = Max\left[1.5, (1.5\ d_{bar})\right]$$

$$s_{rebar} = Max\left[1.5, (1.5 \times (0.625 \text{ in}))\right]$$

$$s_{rebar} = 1.5 \text{ in}$$

Ties:

25.7.2.2 Since longitudinal reinforcement is $\leq$ No. 10ø: Use #3(0.375 in)
25.7.2.1 s_{ties} - Maximum spacing of ties,

$$s_{ties} = Min\left[(16\ d_{bar}), (48\ d_{ties}), Min(D, b)\right]$$

$$s_{ties} = Min\left[(16 \times (0.625 \text{ in})), (48 \times (0.375 \text{ in})), Min((48 \text{ in}), (48 \text{ in}))\right]$$

$$s_{ties} = 10 \text{ in}$$

Summary:

Main reinforcement: **14 - #5 (0.625 in)**

Ties: **#3(0.375 in) - 10 in**

Axial Compression Strength (ACI 318-19, LRFD)

22.4.2.2 ϕP_N - Allowable axial compressive strength

$$\phi P_N = \phi\, 0.80\, \left[\left(0.85\, f'_{ck}\, \left[A_g - A_{st}\right]\right) + \left(f_{yk}\, A_{st}\right)\right]$$

$$\phi P_N = (0.65) \times 0.80 \times \left[\left(0.85 \times (2.5\ \text{ksi}) \times \left[\left(2304\ \text{in}^2\right) - \left(4.2951\ \text{in}^2\right)\right]\right) + \left((60\ \text{ksi}) \times \left(4.2951\ \text{in}^2\right)\right)\right]$$

$$\phi P_N = 2675.2\ \text{kip}$$

$Ratio$ - Capacity

$$Ratio = \frac{P}{\phi P_N}$$

$$Ratio = \frac{(12.755\ \text{kip})}{(2675.2\ \text{kip})}$$

$$Ratio = 0.0047679$$

Status: **PASS**
Ratio: **0.000**

Shear Strength (ACI 318-19, LRFD)

Parameters:

$b_w = 48\ \text{in}$ - Effective width,

22.5.2.2 d - Effective depth

$$d = 0.80\, D$$

$$d = 0.80 \times (48\ \text{in})$$

$$d = 38.4\ \text{in}$$

22.5.5.1.3 λ_s - size effect modification factor

$$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{d}{10}}}, 1\right]$$

$$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{(38.4\ \text{in})}{10}}}, 1\right]$$

$$\lambda_s = 0.64282$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5\ \text{ksi} \to 2500\ \text{psi}$,

22.5.5.1.1 $V_{c,max}$ - Max shear strength of concrete

$$V_{c,max} = 5\, \lambda_s\, \sqrt{f'_{ck}}\, b_w\, d$$

$$V_{c,max} = 5 \times (0.64282) \times \sqrt{(2500\ \text{psi})} \times (48\ \text{in}) \times (38.4\ \text{in})$$

$$V_{c,max} = 296.21\ \text{kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5\ \text{ksi} \to 2500\ \text{psi}$, $P = 12.755\ \text{kip} \to 12755\ \text{lbf}$,

22.5.5.1.1(a) $V_{c,a}$ - Shear strength of concrete (a)

$$V_{c,a} = \left[2\, \lambda_s\, \sqrt{f'_{ck}} + \frac{P}{6\, A_g}\right]\, b_w\, d$$

$$V_{c,a} = \left[2 \times (0.64282) \times \sqrt{(2500\ \text{psi})} + \frac{(12755\ \text{lbf})}{6 \times \left(2304\ \text{in}^2\right)}\right] \times (48\ \text{in}) \times (38.4\ \text{in})$$

$$V_{c,a} = 120.19\ \text{kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5\ \text{ksi} \to 2500\ \text{psi}$,

22.5.5.1.2 $V_{c,b}$ - Shear strength of concrete (b)

$$V_{c,b} = \left[2\, \lambda_s\, \sqrt{f'_{ck}} + \left(0.05\, f'_{ck}\right)\right]\, b_w\, d$$

$$V_{c,b} = \left[2 \times (0.64282) \times \sqrt{(2500\ \text{psi})} + (0.05 \times (2500\ \text{psi}))\right] \times (48\ \text{in}) \times (38.4\ \text{in})$$

$$V_{c,b} = 348.89\ \text{kip}$$

V_c - Governing shear strength of concrete

$$V_c = Min\left[V_{c,max}, V_{c,a}, V_{c,b}\right]$$

$$V_c = Min\left[(296.21\ \text{kip}), (120.19\ \text{kip}), (348.89\ \text{kip})\right]$$

$$V_c = 120.19\ \text{kip}$$

	The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5\ \text{ksi} \rightarrow 2500\ \text{psi}$,	
22.5.1.2	$V_{s,a}$ - Shear strength of steel (a) $$V_{s,a} = 8\sqrt{f'_{ck}}\, b_w\, d$$ $$V_{s,a} = 8 \times \sqrt{(2500\ \text{psi})} \times (48\ \text{in}) \times (38.4\ \text{in})$$ $$V_{s,a} = 737.28\ \text{kip}$$	
	A_v - Ties rebar area, $$A_v = \frac{\pi\, d_{ties}^2}{4}$$ $$A_v = \frac{\pi \times (0.375\ \text{in})^2}{4}$$ $$A_v = 0.11045\ \text{in}^2$$	
22.5.8.5.3	$V_{s,b}$ - Shear strength of steel (b) $$V_{s,b} = \frac{2\, A_v\, f_{ywk}\, d}{s_{ties}}$$ $$V_{s,b} = \frac{2 \times \left(0.11045\ \text{in}^2\right) \times (60\ \text{ksi}) \times (38.4\ \text{in})}{(10\ \text{in})}$$ $$V_{s,b} = 50.894\ \text{kip}$$	
	V_s - Governing shear strength of steel $$V_s = MIN\left[V_{s,a}, V_{s,b}\right]$$ $$V_s = MIN\left[(737.28\ \text{kip}), (50.894\ \text{kip})\right]$$ $$V_s = 50.894\ \text{kip}$$	
22.5.1.1	ϕV_n - Allowable shear strength $$\phi V_n = \phi\ (V_c + V_s)$$ $$\phi V_n = (0.65) \times ((120.19\ \text{kip}) + (50.894\ \text{kip}))$$ $$\phi V_n = 111.2\ \text{kip}$$	
	Considering x-direction: $V_{max} = 6.4199\ \text{kip}$ - Maximum shear force in the x-direction, $Ratio$ - Capacity $$Ratio = \frac{V_{max}}{\phi V_n}$$ $$Ratio = \frac{(6.4199\ \text{kip})}{(111.2\ \text{kip})}$$ $$Ratio = 0.057732$$	Status: **PASS** Ratio: **0.060**
	Considering z-direction: $V_{max} = 0.053446\ \text{kip}$ - Maximum shear force in the z-direction, $Ratio$ - Capacity $$Ratio = \frac{V_{max}}{\phi V_n}$$ $$Ratio = \frac{(0.053446\ \text{kip})}{(111.2\ \text{kip})}$$ $$Ratio = 0.00048062$$	Status: **PASS** Ratio: **0.000**
	Flexural Strength (ACI 318-19, LRFD) S_m - Section modulus $$S_m = \frac{b\, D^2}{6}$$ $$S_m = \frac{(48\ \text{in}) \times (48\ \text{in})^2}{6}$$ $$S_m = 18432\ \text{in}^3$$	

	$\lambda = 1$ - Concrete modification factor (Normal concrete), Allowable flexural strength: M_n shall be the lesser of: $\phi M_{n,1}$ $$\phi M_{n,1} = \phi \times 5 \times \lambda \times \sqrt{(f_c')} \times S_m$$ $$\phi M_{n,1} = 0.65 \times 5 \times 1 \times \sqrt{(2.5ksi)} \times 18432.001in^3$$ $$\phi M_{n,1} = 249.600kipft$$	
14.5.2.1b	$\phi M_{n,2}$ $$\phi M_{n,2} = \phi \, 0.85 \, f_{ck}' \, S_m$$ $$\phi M_{n,2} = (0.65) \times 0.85 \times (2.5 \text{ ksi}) \times \left(18432 \text{ in}^3\right)$$ $$\phi M_{n,2} = 2121.6 \text{ kipft}$$ Therefore, ϕM_n - Allowable flexural strength, $$\phi M_n = MIN[\phi M_{n,1}, \phi M_{n,2}]$$ $$\phi M_n = MIN[(249.6 \text{ kipft}), (2121.6 \text{ kipft})]$$ $$\phi M_n = 249.6 \text{ kipft}$$ **Considering x-direction:** $M_{max} = 15.978 \text{ kipft}$ - Maximum moment in the x-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\phi M_n}$$ $$Ratio = \frac{(15.978 \text{ kipft})}{(249.6 \text{ kipft})}$$ $$Ratio = 0.064014$$	Status: **PASS** Ratio: **0.060**
	Considering z-direction: $M_{max} = 0.12245 \text{ kipft}$ - Maximum moment in the z-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\phi M_n}$$ $$Ratio = \frac{(0.12245 \text{ kipft})}{(249.6 \text{ kipft})}$$ $$Ratio = 0.00049057$$	Status: **PASS** Ratio: **0.000**