Project Details

Project Name: Keystone State Park 5x10 1975ft Carport - V1Jb

Location: 1150 Keystone Park Rd, Derry, PA 15627, USA

Unique ID: 4P-19.75-8TOP-XD-84-L-5Hx10W-STRUTS-AADF

Dealer: ______________________________

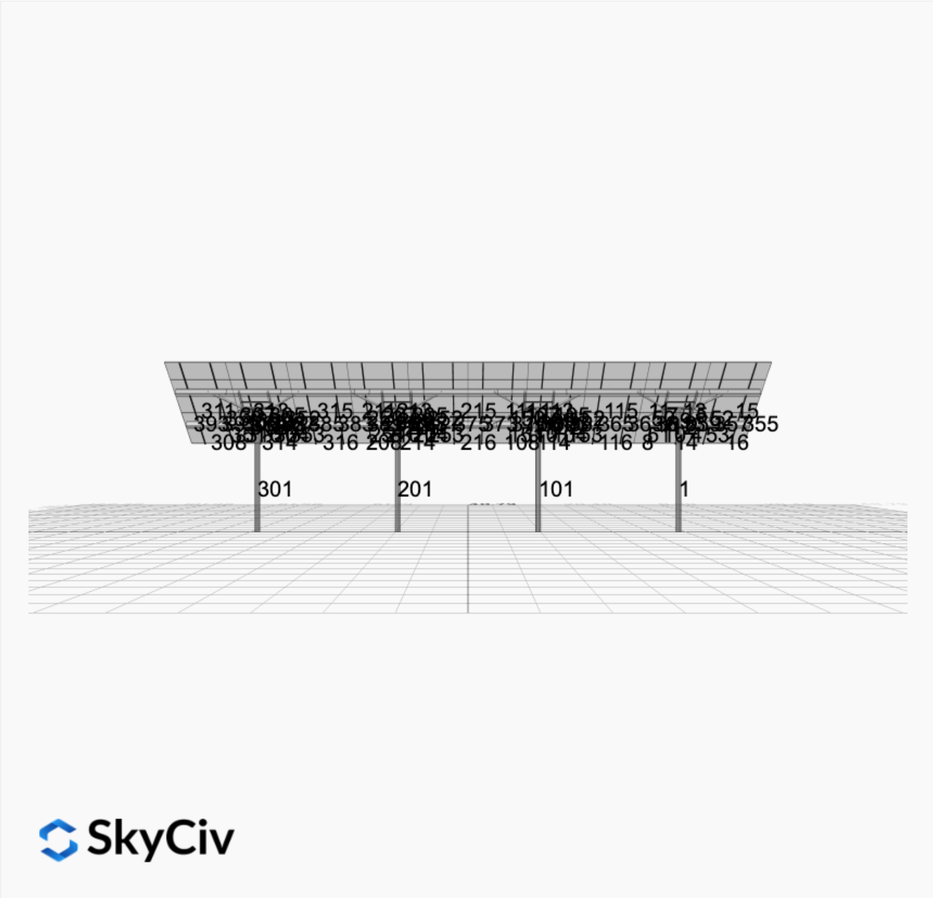
Date: Wed Apr 02 2025

Number of Modules: 50

Number of Poles: 4

Date Sold:

____



Array Dimensions N/S	18.79 ft
Array Dimensions E/W	81.58 ft
Winter Tilt Angle	35
Front Edge Clearance	12 ft

MT Solar Bill of Materials (4P-19.75-8TOP-XD-84-L-5Hx10W-STRUTS-AADF)

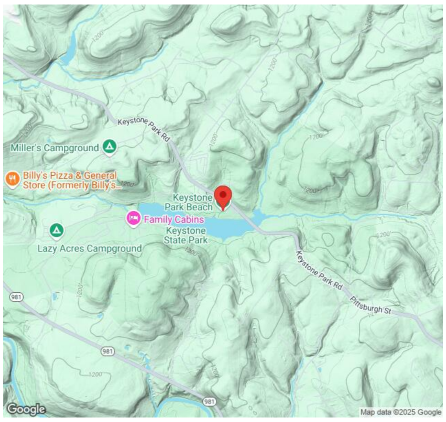
Part	Short Description	BOM Qty
MTS-PC-8	8IN Pole Cap Assembly	4
MTS-HF-XD	H-Frame Assembly-XD	4
MTS-XD-Wing-84	84IN XD Wing	4
MTS-XD-Splice-90	90IN XD Splice	6
MTS-XD-Splice-57	57IN XD Splice	6
MTS-CLAMP-ANGLE-4PK	Angle Clamp	10

Rail Bill of Materials

Part	Qty
Rails (226in)	20

Part	Qty
Rail Attachment	80
Module Mid Clamp	80
Module End Clamp	40
Ground Lug	10

Site Details:



Site Address: 1150 Keystone Park Rd, Derry, PA 15627, USA

Array Specification

Duty Classification:	XD
Module Width:	44.60 in
Module Length:	96.90in
Number of Rows:	5
Number of Columns:	10
Total Number of Modules:	50
Winter Tilt Angle:	35
Front Edge Clearance:	12
Total Array Height at Tilt:	22.78 ft
Total Frame Length:	80.75 ft
Module Info/Notes:	
Array Dimensions N/S:	18.79 ft
Array Dimensions E/W:	81.58 ft
Rail Length:	225.50 in
Rail Spacing:	4.08 ft

Support Specifications

Pole Size:	8in Pipe Sch 80
Pole Length above Grade:	17.39 ft
Number of Poles:	4
Pole Spacing:	19.75 ft

Foundation Specifications

Foundation Type:	Square
Foundation Dimensions:	48 x 48 in
Foundation Depth (below grade):	Pile 1: 8.50 ft Pile 2: 8.25 ft Pile 3: 8.25 ft Pile 4: 8.50 ft
Foundation Volume:	19.852 y^3

Site Info

Risk Category:	I
Exposure:	B
Soil Classification:	sand
Site Location:	1150 Keystone Park Rd, Derry, PA 15627, USA
Wind Speed:	103 mph

Snow Load:	44 psf

Design Disclaimer

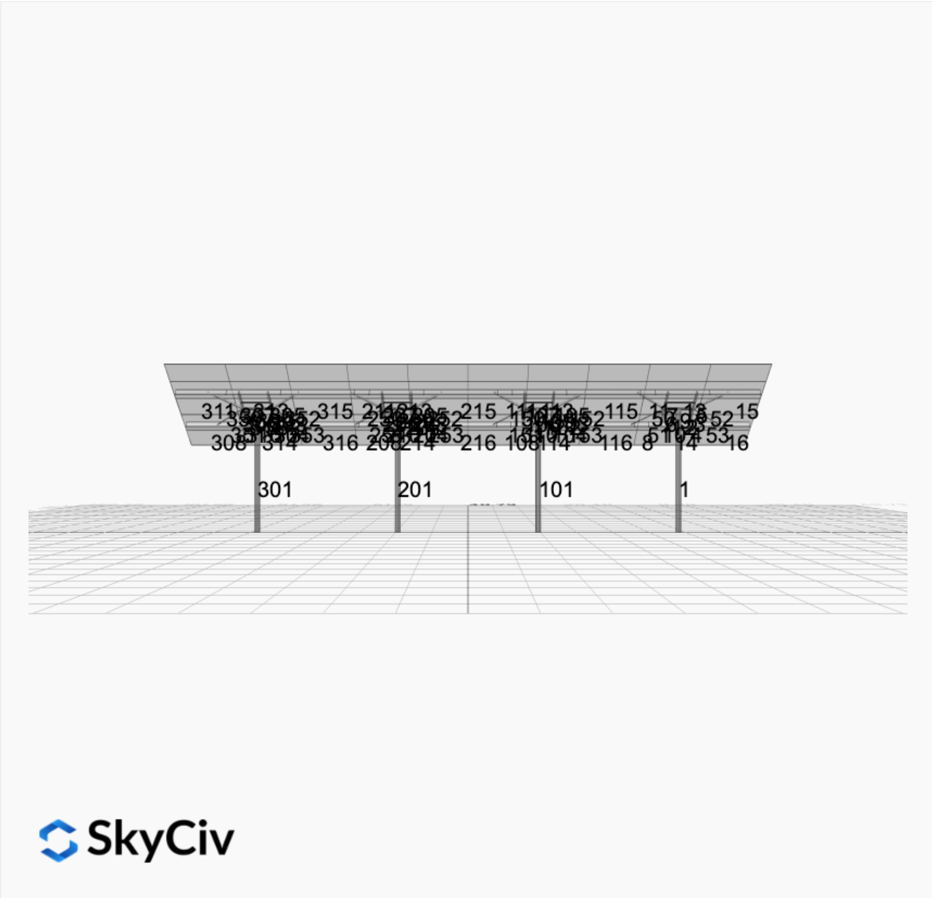
This software should be used for preliminary designs and should not be used as a final design unless reviewed, verified and designed by a qualified structural engineer.

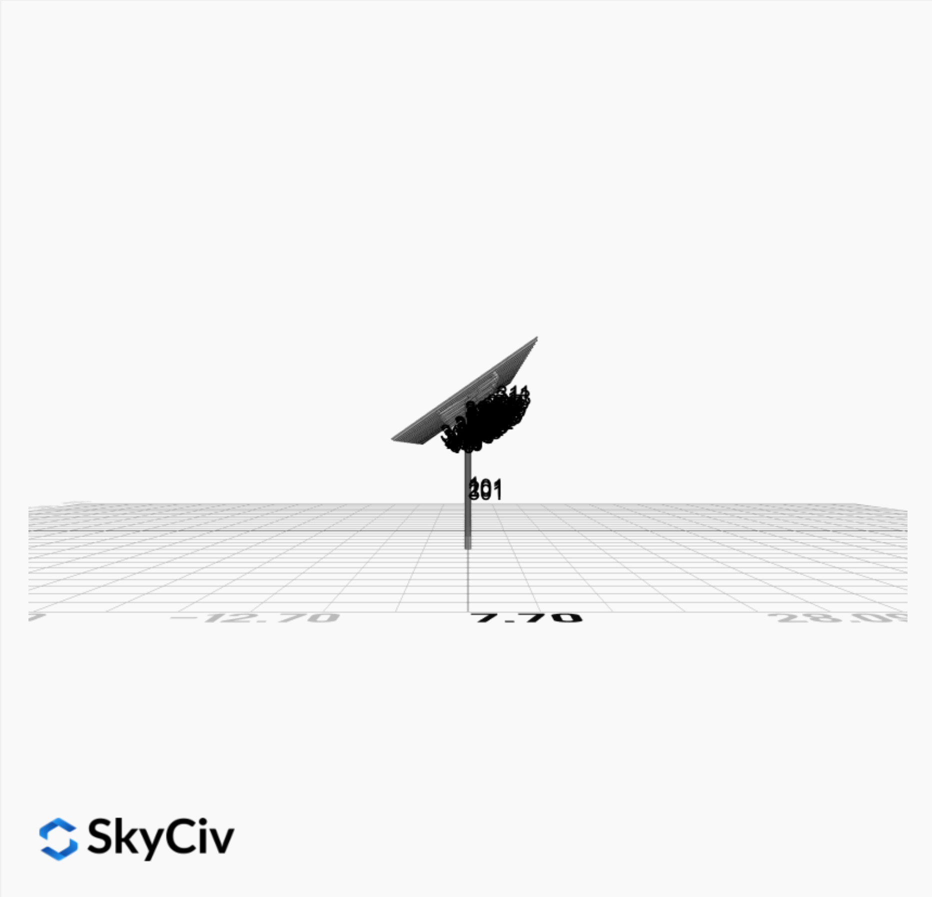
AutoDesigner Input

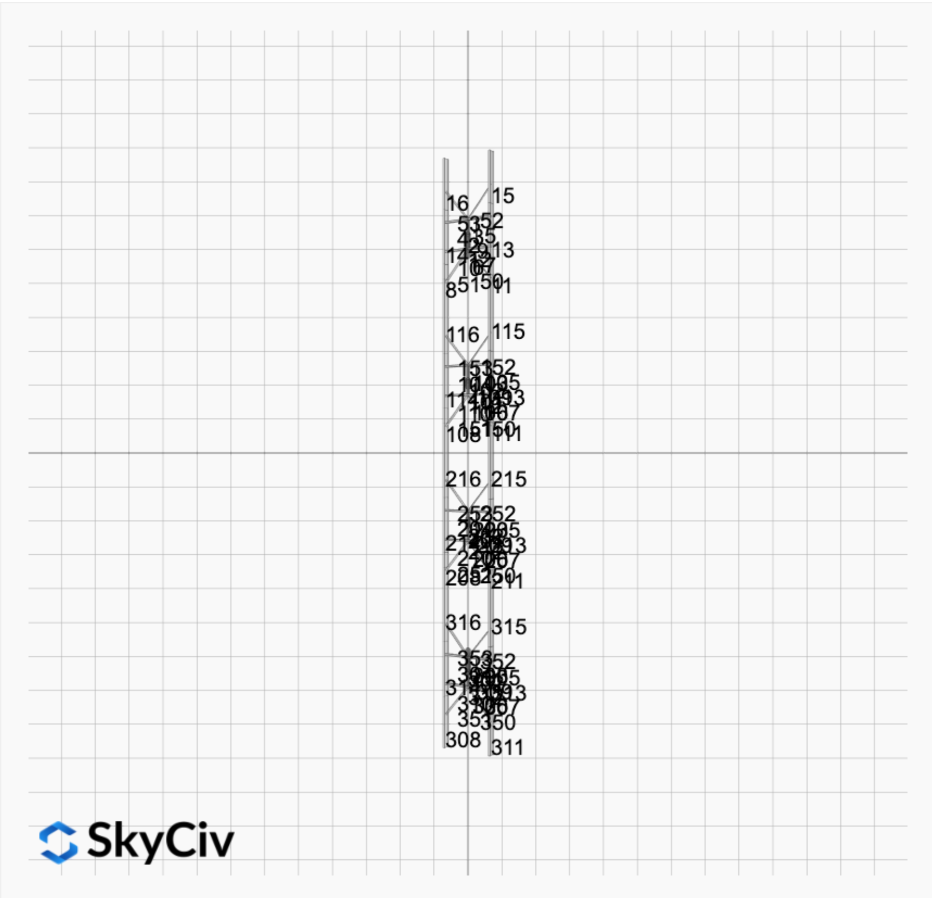

```
{"wind_speed_override":103,"snow_load_override":44,"direct_snow_load":false,"add_angle_brace":true,"product_type":"Beam","designer_name":"","designer_email":"","designer_phone":"","project_id":"Keystone State Park 5x10 1975ft Carport - V1Jb","site_address":"1150 Keystone Park Rd,
Derry, PA 15627,
USA","module_info":"","module_width":44.6,"module_length":96.9,"number_rows":"5","number_columns":10,"pole_mount_section":"4_40","core_pipe_width":65,"core_pipe_section":"2_40","adjuster_section":"2_40","core_beam_height":65,"core_beam_section":"HSS3x2x1/8","main_pipe_section":"2_12GA","pole_spacing_override":19.75,"pole_spacing":"15","tilt_angle":35,"ground_clearance":12,"risk_category":"I","exposure_category":"B","frame_duty_override":"auto","pole_override":"auto","soil_type":"sand","customer_foundation_override":"48_Square","foundation_type":"Square","foundation_size":48,"check_rails":true}
```

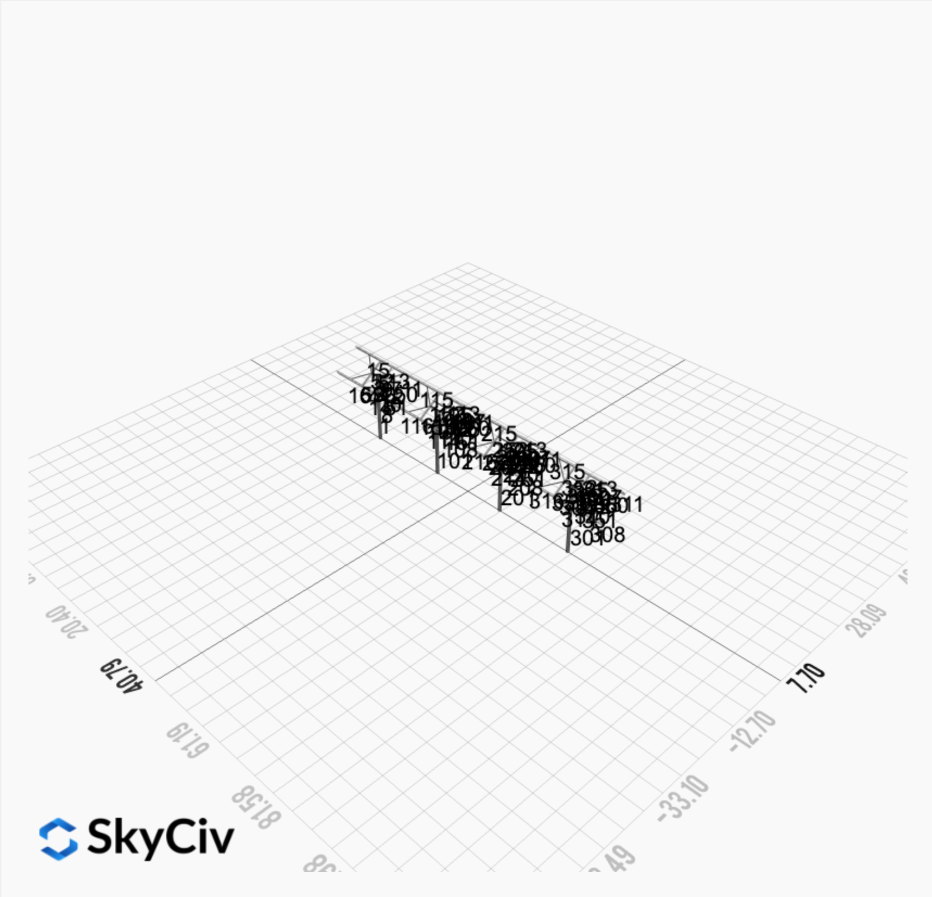

Design Notes:

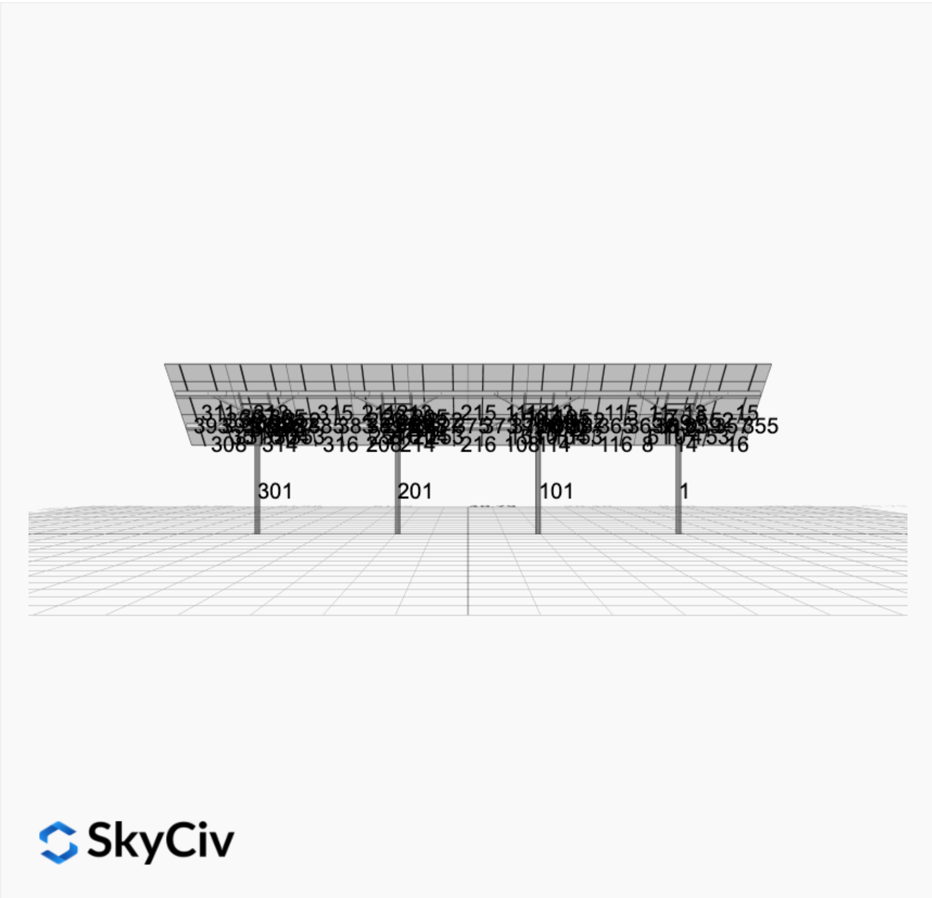
- AISC Deflection checks are set to L/1 due to structure design intent
- Foundation Soil Parameters used in this Autodesign are all estimates, proper geotechnical reports are required to confirm soil profiles
- Wind speeds, snow loads and other site specific results are based on ASCE 7 2016
- Steel frame design checks are based on AISC 360 2016 (LRFD)





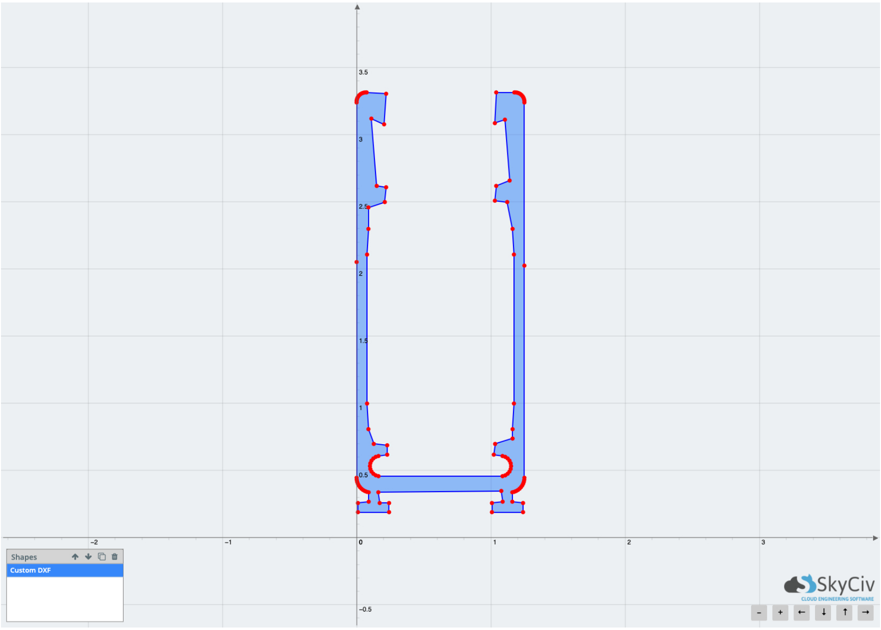



40.79
7.70
28.09
-12.70
-33.10

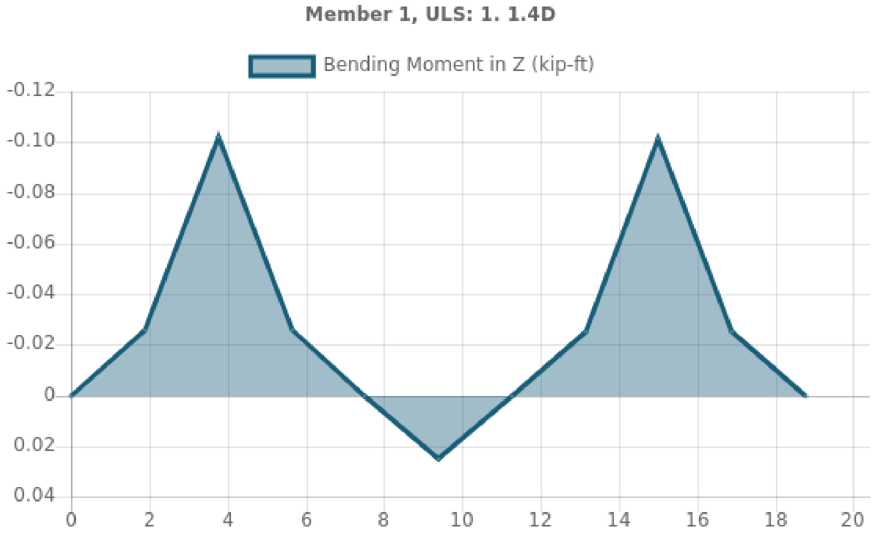

301
201
101
1

Rail Design Check

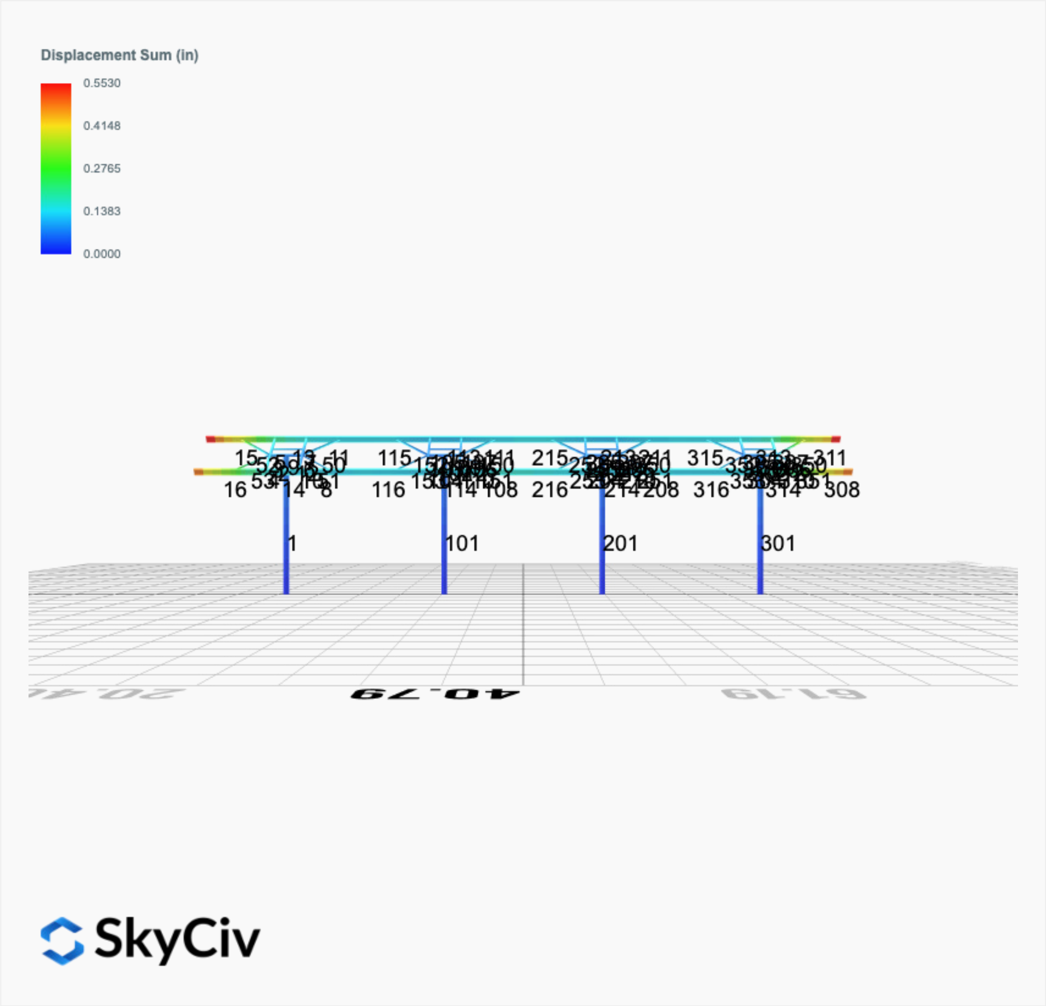
Rail Length: 18.791666666666668 ft
Additional Restraints Required: 4ft Spread Clamps
Tributary Width: 4.079166666666667 ft
Material: Aluminium
Density: 169 lb/ft^3
Elasticity Modulus: 10000 ksi
Fy: 34.5 ksi
Fu: 37 ksi
Snow (X): 0.0566 kip/ft
Snow (Y): -0.0396 kip/ft
Wind uplift Case A: 0.0831 kip/ft
Wind uplift Case A: 0.0831 kip/ft
Wind uplift Case B (X): 0.0000 kip/ft
Wind uplift Case B (Y): 0.1124 kip/ft

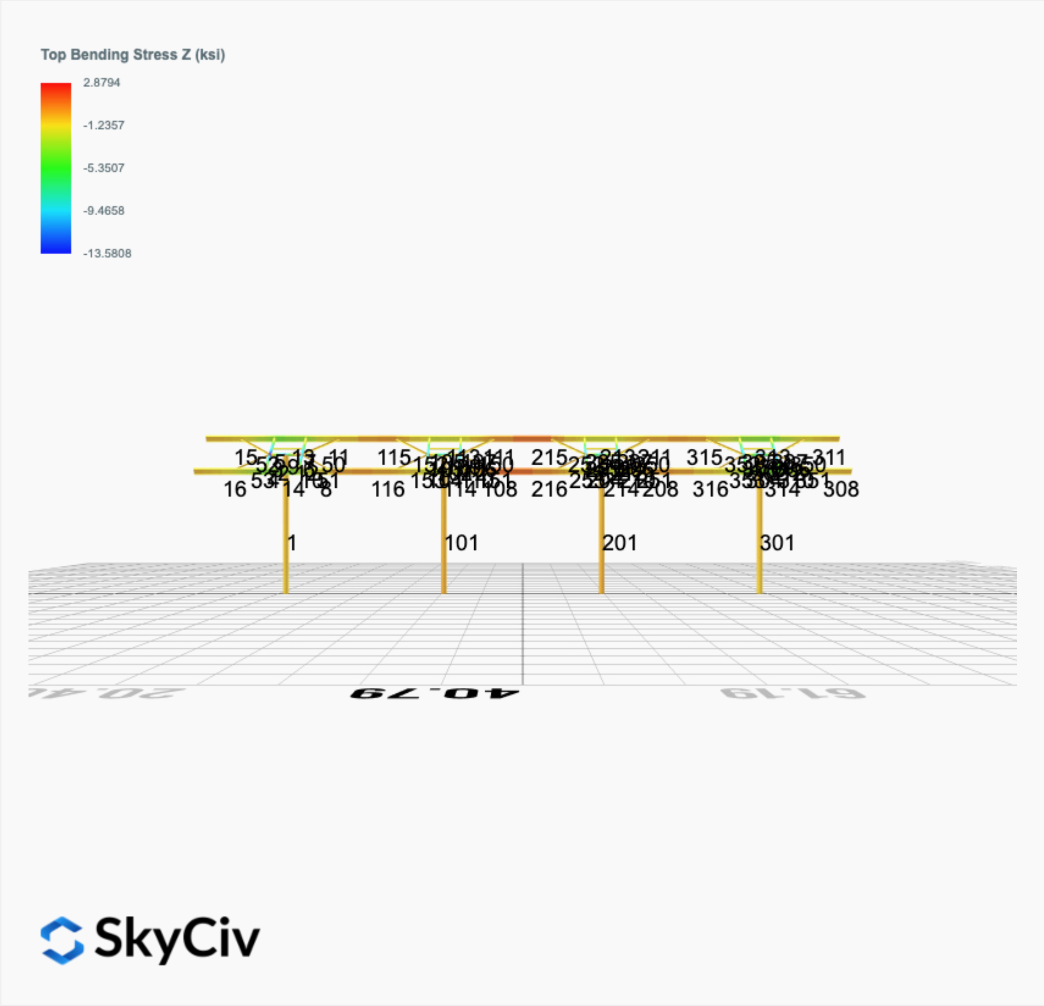


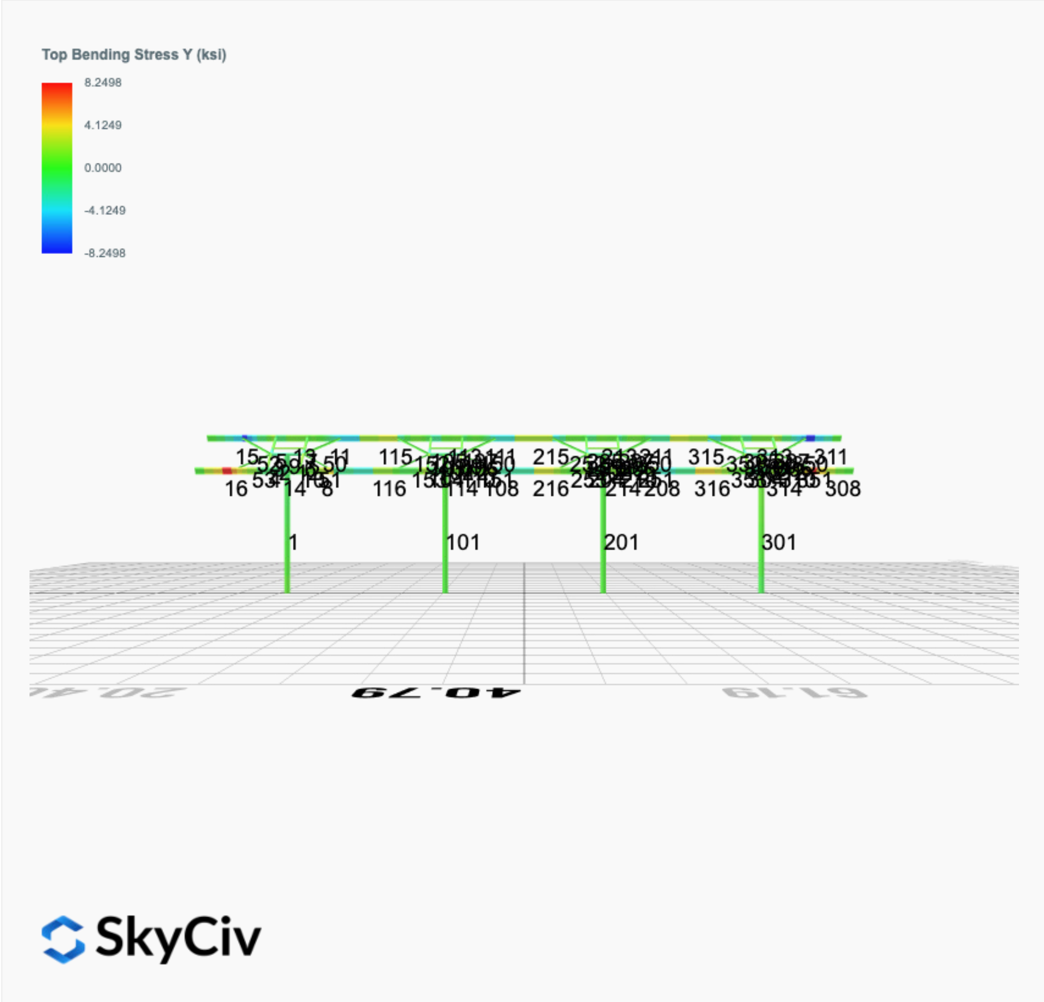
Result Check	Max Limit	Max Value	Utility	Status
Custom Stress Limit	34.5	20.37026815	0.590	PASS
Material Yield	34.5	20.37026815	0.590	PASS
Material Strength	37	20.37026815	0.551	PASS

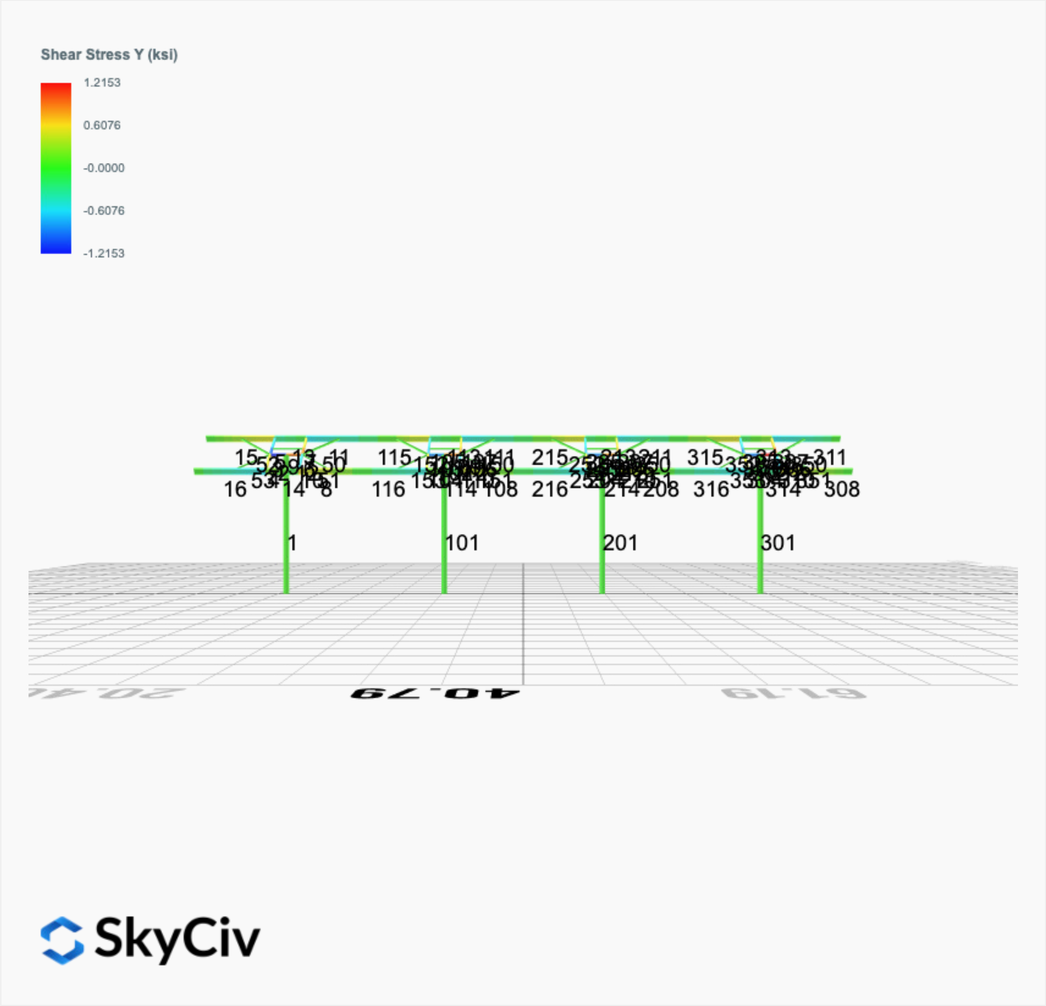


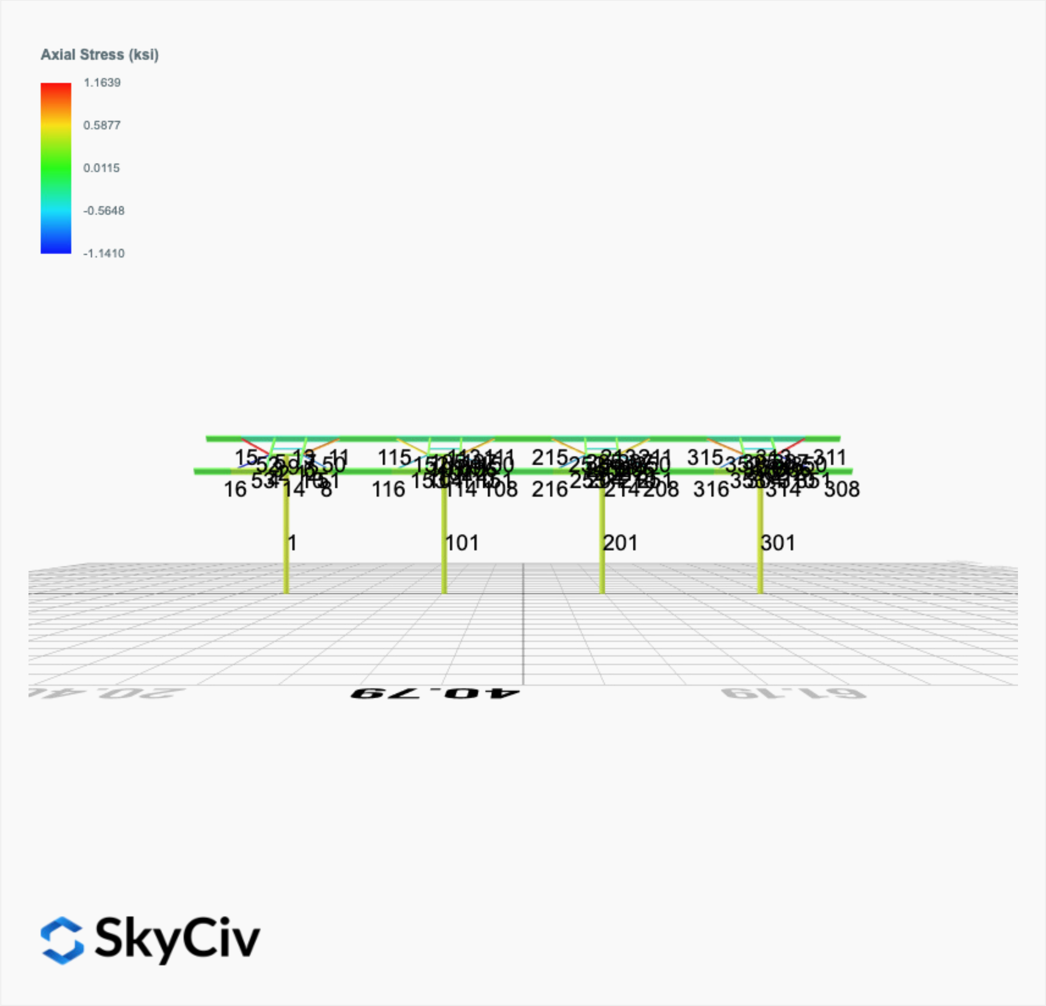
FEM Results (Envelope Worst Case for each member)




Top Bending Stress Z (ksi)
2.8794
-1.2357
-5.3507
-9.4658
-13.5808
1
101
201
301
16
116
216
316
308
15
115
215
315
40.79
SkyCiv


Top Bending Stress Y (ksi)
8.2498
4.1249
0.0000
-4.1249
-8.2498
15
11
115
215
315
311
16
14
8
116
114
108
216
208
316
314
308
1
101
201
301
40.79
SkyCiv


Shear Stress Y (ksi)
1.2153
0.6076
-0.0000
-0.6076
-1.2153
1
101
201
301
40.79


Axial Stress (ksi)
1.1639
0.5877
0.0115
-0.5648
-1.1410
15
115
215
315
16
116
216
316
108
208
308
1
101
201
301
SkyCiv

Reaction Forces for Foundation 1 (Node ID#1), (kip, kip-ft)

ASD Load Combination Results

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 1. D	-0.0207	3.4028	-0.0536	-0.2945	0.1808	0.3656
ULS: 2. D + L	-0.0207	3.4028	-0.0536	-0.2945	0.1808	0.3656
ULS: 3. D + (S or Lr or R)	-0.0693	8.9941	-0.1820	-1.0018	0.6139	1.2064
ULS: 3. D + (S or Lr or R)	-0.0207	3.4028	-0.0536	-0.2945	0.1808	0.3656
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	-0.0571	7.5963	-0.1499	-0.8250	0.5056	0.9962
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	-0.0207	3.4028	-0.0536	-0.2945	0.1808	0.3656
ULS: 5b. D + 0.7E	-0.0207	3.4028	-0.0536	-0.2945	0.1808	0.3656
ULS: 6b. D + 0.75L + 0.75(0.7)E + 0.75S	-0.0571	7.5963	-0.1499	-0.8250	0.5056	0.9962
ULS: 8. 0.6D + 0.7E	-0.0124	2.0417	-0.0321	-0.1767	0.1085	0.2194
ULS: 5a. D + 0.6W_Wind downforce Case A only	-3.2873	8.3319	-0.2239	-1.2337	0.8622	59.7087
ULS: 5a. D + 0.6W_Wind downforce Case B only	-3.2873	8.3319	-0.2239	-1.2337	0.8622	59.7087
ULS: 5a. D + 0.6W_Wind uplift Case A only	2.7530	-0.7592	0.0872	0.4763	-0.3804	-46.2457
ULS: 5a. D + 0.6W_Wind uplift Case B only	2.2617	-0.0743	0.0710	0.3878	-0.3254	-47.4573
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-2.5071	11.2931	-0.2776	-1.5294	1.0166	45.5036
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-2.5071	11.2931	-0.2776	-1.5294	1.0166	45.5036
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	2.0231	4.4748	-0.0443	-0.2469	0.0847	-33.9622
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	1.6547	4.9884	-0.0564	-0.3133	0.1260	-34.8710
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-2.4706	7.0996	-0.1813	-0.9989	0.6918	44.8729
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-2.4706	7.0996	-0.1813	-0.9989	0.6918	44.8729
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	2.0596	0.2813	0.0520	0.2836	-0.2401	-34.5928
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	1.6911	0.7950	0.0399	0.2172	-0.1988	-35.5016
ULS: 7. 0.6D + 0.6W_Wind downforce Case A only	-3.2790	6.9708	-0.2025	-1.1158	0.7898	59.5625
ULS: 7. 0.6D + 0.6W_Wind downforce Case B only	-3.2790	6.9708	-0.2025	-1.1158	0.7898	59.5625
ULS: 7. 0.6D + 0.6W_Wind uplift Case A only	2.7613	-2.1203	0.1087	0.5941	-0.4528	-46.3919
ULS: 7. 0.6D + 0.6W_Wind uplift Case B only	2.2700	-1.4354	0.0925	0.5056	-0.3977	-47.6036

Worst Case Reactions LRFD

These calculations are taken directly from the FEA via SkyCiv and are used in the Concrete Checks of the Foundatino Module.
Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	17.1459
Shear X	-5.4800
Shear Z	-0.4192
Moment X	-2.3194
Moment Y (Twist)	1.5977
Moment Z	103.2372

Worst Case Reactions ASD

These results are taken from the worst case values in the above table and are used in the Soil Checks in the Foundation Module.
Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	11.2931
Shear X	-3.2873
Shear Z	-0.2776
Moment X	-1.5294
Moment Y (Twist)	1.0166
Moment Z	59.7087

Reaction Forces for Foundation 2 (Node ID#101), (kip, kip-ft)

ASD Load Combination Results

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 1. D	0.0207	3.1292	0.0059	0.0318	-0.0142	-0.3038
ULS: 2. D + L	0.0207	3.1292	0.0059	0.0318	-0.0142	-0.3038
ULS: 3. D + (S or Lr or R)	0.0693	8.0627	0.0199	0.1075	-0.0474	-1.0682
ULS: 3. D + (S or Lr or R)	0.0207	3.1292	0.0059	0.0318	-0.0142	-0.3038
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	0.0571	6.8293	0.0164	0.0886	-0.0391	-0.8771

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	0.0207	3.1292	0.0059	0.0318	-0.0142	-0.3038
ULS: 5b. D + 0.7E	0.0207	3.1292	0.0059	0.0318	-0.0142	-0.3038
ULS: 6b. D + 0.75L + 0.75(0.7)E + 0.75S	0.0571	6.8293	0.0164	0.0886	-0.0391	-0.8771
ULS: 8. 0.6D + 0.7E	0.0124	1.8775	0.0035	0.0191	-0.0085	-0.1823
ULS: 5a. D + 0.6W_Wind downforce Case A only	-3.0829	7.2977	0.0185	0.0992	-0.0056	55.9749
ULS: 5a. D + 0.6W_Wind downforce Case B only	-3.0829	7.2977	0.0185	0.0992	-0.0056	55.9749
ULS: 5a. D + 0.6W_Wind uplift Case A only	2.6218	-0.3848	-0.0064	-0.0346	-0.0110	-44.4205
ULS: 5a. D + 0.6W_Wind uplift Case B only	2.2173	0.2097	-0.0009	-0.0047	-0.0345	-46.0079
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-2.2706	9.9557	0.0259	0.1392	-0.0327	41.3319
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-2.2706	9.9557	0.0259	0.1392	-0.0327	41.3319
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	2.0080	4.1938	0.0071	0.0388	-0.0367	-33.9647
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	1.7046	4.6396	0.0113	0.0612	-0.0543	-35.1552
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-2.3070	6.2555	0.0154	0.0824	-0.0078	41.9053
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-2.3070	6.2555	0.0154	0.0824	-0.0078	41.9053
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	1.9715	0.4937	-0.0034	-0.0180	-0.0118	-33.3913
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	1.6681	0.9395	0.0008	0.0045	-0.0294	-34.5819
ULS: 7. 0.6D + 0.6W_Wind downforce Case A only	-3.0912	6.0460	0.0162	0.0865	0.0000	56.0965
ULS: 7. 0.6D + 0.6W_Wind downforce Case B only	-3.0912	6.0460	0.0162	0.0865	0.0000	56.0965
ULS: 7. 0.6D + 0.6W_Wind uplift Case A only	2.6135	-1.6365	-0.0088	-0.0473	-0.0054	-44.2990
ULS: 7. 0.6D + 0.6W_Wind uplift Case B only	2.2090	-1.0420	-0.0033	-0.0174	-0.0288	-45.8864

Worst Case Reactions LRFD

These calculations are taken directly from the FEA via SkyCiv and are used in the Concrete Checks of the Foundatino Module.
Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	15.1134
Shear X	-5.1726
Shear Z	0.0392
Moment X	0.2124
Moment Y (Twist)	0.0924
Moment Z	96.3751

Worst Case Reactions ASD

These results are taken from the worst case values in the above table and are used in the Soil Checks in the Foundation Module.
Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	9.9557
Shear X	-3.0912
Shear Z	0.0259
Moment X	0.1392
Moment Y (Twist)	0.0543
Moment Z	56.0965

Reaction Forces for Foundation 3 (Node ID#201), (kip, kip-ft)

ASD Load Combination Results

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 1. D	0.0207	3.1292	-0.0059	-0.0318	0.0142	-0.3038
ULS: 2. D + L	0.0207	3.1292	-0.0059	-0.0318	0.0142	-0.3038
ULS: 3. D + (S or Lr or R)	0.0693	8.0627	-0.0199	-0.1075	0.0474	-1.0682
ULS: 3. D + (S or Lr or R)	0.0207	3.1292	-0.0059	-0.0318	0.0142	-0.3038
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	0.0571	6.8293	-0.0164	-0.0886	0.0391	-0.8771
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	0.0207	3.1292	-0.0059	-0.0318	0.0142	-0.3038
ULS: 5b. D + 0.7E	0.0207	3.1292	-0.0059	-0.0318	0.0142	-0.3038
ULS: 6b. D + 0.75L + 0.75(0.7)E + 0.75S	0.0571	6.8293	-0.0164	-0.0886	0.0391	-0.8771
ULS: 8. 0.6D + 0.7E	0.0124	1.8775	-0.0035	-0.0191	0.0085	-0.1823
ULS: 5a. D + 0.6W_Wind downforce Case A only	-3.0829	7.2977	-0.0185	-0.0992	0.0056	55.9750
ULS: 5a. D + 0.6W_Wind downforce Case B only	-3.0829	7.2977	-0.0185	-0.0992	0.0056	55.9750
ULS: 5a. D + 0.6W_Wind uplift Case A only	2.6218	-0.3848	0.0064	0.0346	0.0110	-44.4205
ULS: 5a. D + 0.6W_Wind uplift Case B only	2.2173	0.2097	0.0009	0.0047	0.0345	-46.0079

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-2.2706	9.9557	-0.0259	-0.1392	0.0327	41.3319
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-2.2706	9.9557	-0.0259	-0.1392	0.0327	41.3319
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	2.0080	4.1938	-0.0071	-0.0388	0.0367	-33.9647
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	1.7046	4.6396	-0.0113	-0.0612	0.0543	-35.1552
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-2.3070	6.2555	-0.0154	-0.0824	0.0078	41.9053
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-2.3070	6.2555	-0.0154	-0.0824	0.0078	41.9053
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	1.9715	0.4937	0.0034	0.0180	0.0118	-33.3913
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	1.6681	0.9395	-0.0008	-0.0045	0.0294	-34.5818
ULS: 7. 0.6D + 0.6W_Wind downforce Case A only	-3.0912	6.0460	-0.0162	-0.0865	-0.0000	56.0965
ULS: 7. 0.6D + 0.6W_Wind downforce Case B only	-3.0912	6.0460	-0.0162	-0.0865	-0.0000	56.0965
ULS: 7. 0.6D + 0.6W_Wind uplift Case A only	2.6135	-1.6365	0.0088	0.0473	0.0054	-44.2990
ULS: 7. 0.6D + 0.6W_Wind uplift Case B only	2.2090	-1.0420	0.0033	0.0174	0.0289	-45.8864

Worst Case Reactions LRFD

These calculations are taken directly from the FEA via SkyCiv and are used in the Concrete Checks of the Foundatino Module.
Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	15.1133
Shear X	-5.1726
Shear Z	-0.0392
Moment X	-0.2125
Moment Y (Twist)	0.0924
Moment Z	96.3755

Worst Case Reactions ASD

These results are taken from the worst case values in the above table and are used in the Soil Checks in the Foundation Module.
Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	9.9557
Shear X	-3.0912
Shear Z	-0.0259
Moment X	-0.1392
Moment Y (Twist)	0.0543
Moment Z	56.0965

Reaction Forces for Foundation 4 (Node ID#301), (kip, kip-ft)

ASD Load Combination Results

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 1. D	-0.0207	3.4029	0.0536	0.2946	-0.1808	0.3656
ULS: 2. D + L	-0.0207	3.4029	0.0536	0.2946	-0.1808	0.3656
ULS: 3. D + (S or Lr or R)	-0.0693	8.9941	0.1820	1.0019	-0.6139	1.2065
ULS: 3. D + (S or Lr or R)	-0.0207	3.4029	0.0536	0.2946	-0.1808	0.3656
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	-0.0571	7.5963	0.1499	0.8250	-0.5056	0.9963
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	-0.0207	3.4029	0.0536	0.2946	-0.1808	0.3656
ULS: 5b. D + 0.7E	-0.0207	3.4029	0.0536	0.2946	-0.1808	0.3656
ULS: 6b. D + 0.75L + 0.75(0.7)E + 0.75S	-0.0571	7.5963	0.1499	0.8250	-0.5056	0.9963
ULS: 8. 0.6D + 0.7E	-0.0124	2.0417	0.0321	0.1767	-0.1085	0.2194
ULS: 5a. D + 0.6W_Wind downforce Case A only	-3.2873	8.3319	0.2239	1.2337	-0.8622	59.7087
ULS: 5a. D + 0.6W_Wind downforce Case B only	-3.2873	8.3319	0.2239	1.2337	-0.8622	59.7087
ULS: 5a. D + 0.6W_Wind uplift Case A only	2.7530	-0.7592	-0.0872	-0.4763	0.3805	-46.2456
ULS: 5a. D + 0.6W_Wind uplift Case B only	2.2617	-0.0743	-0.0710	-0.3878	0.3254	-47.4573
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-2.5071	11.2931	0.2776	1.5294	-1.0166	45.5036
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-2.5071	11.2931	0.2776	1.5294	-1.0166	45.5036
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	2.0231	4.4748	0.0443	0.2469	-0.0847	-33.9622
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	1.6547	4.9884	0.0564	0.3133	-0.1260	-34.8710
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-2.4706	7.0996	0.1813	0.9989	-0.6918	44.8730
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-2.4706	7.0996	0.1813	0.9989	-0.6918	44.8730
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	2.0596	0.2813	-0.0520	-0.2836	0.2401	-34.5928
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	1.6911	0.7950	-0.0399	-0.2172	0.1988	-35.5016

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 7. 0.6D + 0.6W_Wind downforce Case A only	-3.2790	6.9708	0.2025	1.1158	-0.7898	59.5625
ULS: 7. 0.6D + 0.6W_Wind downforce Case B only	-3.2790	6.9708	0.2025	1.1158	-0.7898	59.5625
ULS: 7. 0.6D + 0.6W_Wind uplift Case A only	2.7613	-2.1203	-0.1087	-0.5941	0.4528	-46.3919
ULS: 7. 0.6D + 0.6W_Wind uplift Case B only	2.2700	-1.4354	-0.0925	-0.5056	0.3977	-47.6036

Worst Case Reactions LRFD

These calculations are taken directly from the FEA via SkyCiv and are used in the Concrete Checks of the Foundatino Module.
Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	17.1461
Shear X	-5.4798
Shear Z	0.4192
Moment X	2.3195
Moment Y (Twist)	1.5974
Moment Z	103.2369

Worst Case Reactions ASD

These results are taken from the worst case values in the above table and are used in the Soil Checks in the Foundation Module.
Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	11.2931
Shear X	-3.2873
Shear Z	0.2776
Moment X	1.5294
Moment Y (Twist)	1.0166
Moment Z	59.7087

Project Details

Design Code: AISC 360-16 LRFD
Provision: LRFD
Country: United States

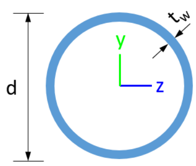
User Name: sales@mtsolar.us
Project Name: Keystone State Park 5x10 1975ft Carport - V1Jb
Unit System: imperial

Design Input Information

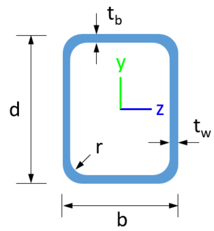
Design Factors			
Φ_t	Φ_c	Φ_b	Φ_v
0.9	0.9	0.9	0.9

Design Materials			
ID	E (ksi)	F_y (ksi)	F_u (ksi)
1	29000	50	65

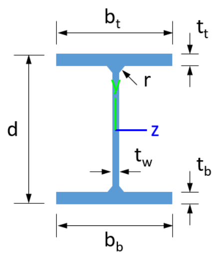
Section Dimensions



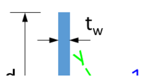
ID	Name	d (in)	t_w (in)					
3	2in Pipe Sch 120	2.38	0.25					
6	4in Pipe Sch 120	4.50	0.44					
10	8in Pipe Sch 80	8.63	0.50					

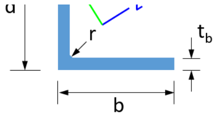


ID	Name	d (in)	b (in)	t_w (in)	t_b (in)	r (in)		
17	HSS5x3x1/4	5.00	3.00	0.23	0.23	0.23		



ID	Name	d (in)	t_w (in)	b_t (in)	b_b (in)	t_t (in)	t_b (in)	r (in)
20	W10x12	9.87	0.19	3.96	3.96	0.21	0.21	0.30





ID	Name	d (in)	t_w (in)	b (in)	t_b (in)	r (in)		
34	L3x2x3/16	3.00	0.19	2.00	0.19	0.31		

Section Properties

ID	Name	A (in^2)	J (in^4)	I_{yp} (in^4)	I_{zp} (in^4)	I_w (in^6)	S_{yp} (in^3)	S_{zp} (in^3)
3	2in Pipe Sch 120	1.67	1.91	0.96	0.96	0.00	1.13	1.13
6	4in Pipe Sch 120	5.58	23.29	11.64	11.64	0.00	7.24	7.24
10	8in Pipe Sch 80	12.76	211.43	105.72	105.72	0.00	33.05	33.05
17	HSS5x3x1/4	3.37	11.00	4.81	10.70	0.93	3.77	5.38
20	W10x12	3.54	0.05	2.18	53.80	50.90	1.74	12.60
34	L3x2x3/16	0.92	0.01	0.17	0.98	0.01	0.33	0.82

Member Properties

Member ID	Section ID	K_zL (ft)	K_yL (ft)	L_b(ft)	C_b	LST	LSC	LD
1	10	36.52	36.52	17.39	-	300	200	1
2	6	1.30	1.30	2.00	-	300	200	1
3	17	0.92	0.92	1.42	1.19,1.19,1.19,1.18,1.19,1.19,1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.19,1.18,1.18,1.17,1.18,1.18,1.18,1.18,1.18	300	200	1
4	17	2.44	2.44	3.75	1.68,1.67,1.68,1.67,1.68,1.68,1.67,1.67,1.66,1.69,1.67,1.67,1.66,1.61,1.67,1.67,1.67,1.67,1.67,1.67,1.65,1.71,1.67,1.67,1.66,1.64	300	200	1
5	17	1.52	1.52	2.33	1.68,1.67,1.68,1.67,1.67,1.68,1.67,1.67,1.66,1.66,1.67,1.67,1.66,1.66,1.67,1.67,1.67,1.67,1.67,1.67,1.66,1.66,1.67,1.67,1.66,1.67	300	200	1
6	17	0.92	0.92	1.42	1.18,1.18,1.18,1.18,1.18,1.18,1.17,1.17,1.17,1.17,1.18,1.18,1.17,1.17,1.18,1.18,1.18,1.18,1.18,1.18,1.16,1.17,1.17,1.17,1.17,1.17	300	200	1
7	17	1.52	1.52	2.33	1.68,1.67,1.68,1.67,1.67,1.68,1.67,1.67,1.66,1.66,1.67,1.67,1.66,1.66,1.67,1.67,1.67,1.67,1.67,1.67,1.65,1.66,1.67,1.67,1.66,1.66	300	200	1
8	20	1.33	1.33	2.05	1.45,1.45,1.45,1.45,1.45,1.45,1.45,1.43,1.54,1.45,1.45,1.44,1.25,1.45,1.45,1.45,1.45,1.45,1.45,1.43,1.54,1.44,1.44,1.44,1.34	300	200	1
9	3	2.60	2.60	4.00	-	300	200	1
10	17	2.44	2.44	3.75	1.69,1.68,1.69,1.67,1.68,1.69,1.67,1.67,1.65,1.69,1.67,1.67,1.66,1.14,1.67,1.67,1.68,1.67,1.67,1.67,1.64,1.70,1.67,1.67,1.66,1.61	300	200	1
11	20	1.33	1.33	2.05	1.46,1.46,1.46,1.46,1.46,1.46,1.46,1.46,1.45,1.46,1.46,1.46,1.45,1.46,1.46,1.46,1.45,1.44,1.46,1.46,1.45,1.46,1.46,1.46,1.45,1.46	300	200	1
12	6	1.30	1.30	2.00	-	300	200	1
13	20	4.88	4.00	7.50	1.08,1.08,1.08,1.08,1.08,1.08,1.08,1.09,1.14,1.08,1.08,1.08,1.12,1.08,1.08,1.07,1.08,1.08,1.08,1.09,1.14,1.08,1.08,1.08,1.12	300	200	1
14	20	4.88	4.00	7.50	1.07,1.07,1.07,1.07,1.07,1.07,1.08,1.08,1.08,1.28,1.08,1.08,1.08,1.22,1.08,1.08,1.07,1.07,1.08,1.08,1.08,1.26,1.08,1.08,1.08,1.16	300	200	1
15	20	7.00	7.00	7.00	2.33,2.32,2.33,2.32,2.32,2.33,2.32,2.32,2.33,2.33,2.32,2.32,2.31,2.32,2.33,2.33,2.30,2.29,2.32,2.32,2.31,2.31,2.32,2.32,2.31,2.31	300	200	1
16	20	7.00	7.00	7.00	2.32,2.32,2.32,2.31,2.32,2.33,2.32,2.32,2.32,2.26,2.32,2.32,2.31,2.40,2.32,2.32,2.30,2.30,2.32,2.32,2.30,2.31,2.32,2.32,2.31,2.34	300	200	1
50	34	5.67	5.67	5.67	1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14	300	200	250
51	34	5.67	5.67	5.67	1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14	300	200	250
52	34	5.67	5.67	5.67	1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14	300	200	250
53	34	5.67	5.67	5.67	1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14	300	200	250
101	10	36.52	36.52	17.39	-	300	200	1

102	6	1.30	1.30	2.00	-	300	200	1
103	17	0.92	0.92	1.42	1.19,1.18,1.19,1.18,1.18,1.19,1.18,1.18,1.17,1.17,1.18,1.18,1.17,1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.17,1.17,1.18,1.18,1.18,1.18	300	200	1
104	17	2.44	2.44	3.75	1.69,1.68,1.69,1.67,1.68,1.69,1.67,1.67,1.65,1.69,1.67,1.67,1.66,1.50,1.67,1.67,1.68,1.67,1.67,1.67,1.64,1.71,1.67,1.67,1.66,1.63	300	200	1
105	17	1.52	1.52	2.33	1.68,1.67,1.68,1.67,1.68,1.68,1.67,1.67,1.66,1.66,1.67,1.67,1.66,1.66,1.67,1.67,1.67,1.67,1.67,1.67,1.65,1.66,1.67,1.67,1.66,1.66	300	200	1
106	17	0.92	0.92	1.42	1.19,1.18,1.19,1.18,1.18,1.19,1.18,1.18,1.17,1.17,1.18,1.18,1.17,1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.17,1.17,1.18,1.18,1.18,1.18	300	200	1
107	17	1.52	1.52	2.33	1.68,1.67,1.68,1.67,1.68,1.68,1.67,1.67,1.66,1.66,1.67,1.67,1.66,1.66,1.67,1.67,1.67,1.67,1.67,1.67,1.65,1.66,1.67,1.67,1.66,1.66	300	200	1
108	20	1.33	1.33	2.05	1.82,1.82,1.82,1.82,1.82,1.82,1.87,1.87,1.97,2.07,1.88,1.88,1.90,1.27,1.85,1.85,1.78,1.86,1.86,1.86,1.97,2.07,1.88,1.88,1.90,1.45	300	200	1
109	3	2.60	2.60	4.00	-	300	200	1
110	17	2.44	2.44	3.75	1.69,1.68,1.69,1.67,1.68,1.69,1.67,1.67,1.65,1.69,1.67,1.67,1.66,1.51,1.67,1.67,1.68,1.67,1.67,1.67,1.64,1.71,1.67,1.67,1.66,1.63	300	200	1
111	20	1.33	1.33	2.05	2.06,2.06,2.06,2.06,2.06,2.06,2.08,2.08,2.11,2.12,2.08,2.08,2.09,2.10,2.07,2.07,1.95,1.74,2.08,2.08,2.11,2.15,2.08,2.08,2.08,2.10	300	200	1
112	6	1.30	1.30	2.00	-	300	200	1
113	20	4.88	4.00	7.50	1.07,1.07,1.07,1.07,1.07,1.07,1.06,1.06,1.06,1.07,1.06,1.06,1.07,1.07,1.07,1.07,1.07,1.07,1.06,1.06,1.06,1.08,1.06,1.06,1.07,1.07	300	200	1
114	20	4.88	4.00	7.50	1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.08,1.31,1.07,1.07,1.08,1.06,1.07,1.07,1.07,1.08,1.07,1.07,1.08,1.27,1.07,1.07,1.08,1.07	300	200	1
115	20	10.20	10.20	10.20	1.27,1.28,1.27,1.28,1.28,1.27,1.30,1.30,1.55,1.49,1.30,1.30,1.36,1.38,1.29,1.29,1.27,1.27,1.30,1.30,1.58,1.50,1.30,1.30,1.33,1.35	300	200	1
116	20	10.20	10.20	10.20	1.26,1.26,1.26,1.26,1.26,1.26,1.27,1.27,1.28,1.26,1.27,1.27,1.28,1.97,1.27,1.27,1.26,1.27,1.27,1.27,1.29,1.24,1.27,1.27,1.28,1.45	300	200	1
150	34	5.67	5.67	5.67	1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14	300	200	250
151	34	5.67	5.67	5.67	1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14	300	200	250
152	34	5.67	5.67	5.67	1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14	300	200	250
153	34	5.67	5.67	5.67	1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14	300	200	250
201	10	36.52	36.52	17.39	-	300	200	1
202	6	1.30	1.30	2.00	-	300	200	1
203	17	0.92	0.92	1.42	1.19,1.18,1.19,1.18,1.18,1.19,1.18,1.18,1.17,1.17,1.18,1.18,1.17,1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.17,1.17,1.18,1.18,1.18,1.18	300	200	1
204	17	2.44	2.44	3.75	1.69,1.68,1.69,1.67,1.68,1.69,1.67,1.67,1.65,1.69,1.67,1.67,1.66,1.52,1.67,1.67,1.68,1.67,1.67,1.67,1.64,1.71,1.67,1.67,1.66,1.63	300	200	1
205	17	1.52	1.52	2.33	1.68,1.67,1.68,1.67,1.68,1.68,1.67,1.67,1.66,1.66,1.67,1.67,1.66,1.66,1.67,1.67,1.67,1.67,1.67,1.67,1.65,1.66,1.67,1.67,1.66,1.66	300	200	1
206	17	0.92	0.92	1.42	1.19,1.18,1.19,1.18,1.18,1.19,1.18,1.18,1.17,1.17,1.18,1.18,1.17,1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.17,1.17,1.18,1.18,1.18,1.18	300	200	1
207	17	1.52	1.52	2.33	1.68,1.67,1.68,1.67,1.68,1.68,1.67,1.67,1.66,1.66,1.67,1.67,1.66,1.66,1.67,1.67,1.67,1.67,1.67,1.67,1.65,1.66,1.67,1.67,1.66,1.66	300	200	1
208	20	1.33	1.33	2.05	2.41,2.41,2.41,2.41,2.41,2.41,2.38,2.38,2.34,2.29,2.38,2.38,2.37,1.29,2.39,2.39,2.31,2.40,2.38,2.38,2.34,2.30,2.38,2.38,2.37,1.82	300	200	1
209	3	2.60	2.60	4.00	-	300	200	1
210	17	2.44	2.44	3.75	1.69,1.68,1.69,1.67,1.68,1.69,1.67,1.67,1.65,1.69,1.67,1.67,1.66,1.50,1.67,1.67,1.68,1.67,1.67,1.67,1.64,1.71,1.67,1.67,1.66,1.63	300	200	1
211	20	1.33	1.33	2.05	2.29,2.29,2.29,2.29,2.29,2.29,2.24,2.24,1.98,2.01,2.22,2.22,2.16,2.12,2.26,2.26,2.35,2.31,2.25,2.25,1.94,1.98,2.21,2.21,2.20,2.15	300	200	1
212	6	1.30	1.30	2.00	-	300	200	1
213	20	4.88	4.00	7.50	1.07,1.07,1.07,1.07,1.07,1.07,1.06,1.06,1.06,1.07,1.06,1.06,1.07,1.07,1.07,1.07,1.07,1.07,1.06,1.06,1.06,1.08,1.06,1.06,1.07,1.07	300	200	1
214	20	4.88	4.00	7.50	1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.07,1.08,1.31,1.07,1.07,1.08,1.06,1.07,1.07,1.07,1.07,1.07,1.07,1.08,1.27,1.07,1.07,1.08,1.07	300	200	1
215	20	10.20	10.20	10.20	1.11,1.12,1.11,1.12,1.12,1.11,1.12,1.12,1.14,1.14,1.12,1.12,1.13,1.13,1.12,1.12,1.11,1.11,1.12,1.12,1.14,1.14,1.13,1.13,1.13,1.13	300	200	1

216	20	10.20	10.20	10.20	1.10,1.11,1.10,1.11,1.11,1.10,1.11,1.11,1.11,1.13,1.11,1.11,1.11,1.06,1.11,1.11,1.11,1.11,1.11,1.11,1.12,1.12,1.11,1.11,1.11,1.08	300	200	1
250	34	5.67	5.67	5.67	1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14	300	200	250
251	34	5.67	5.67	5.67	1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14	300	200	250
252	34	5.67	5.67	5.67	1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14	300	200	250
253	34	5.67	5.67	5.67	1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14	300	200	250
301	10	36.52	36.52	17.39	-	300	200	1
302	6	1.30	1.30	2.00	-	300	200	1
303	17	0.92	0.92	1.42	1.18,1.18,1.18,1.18,1.18,1.18,1.17,1.17,1.17,1.17,1.18,1.18,1.17,1.17,1.18,1.18,1.18,1.18,1.18,1.18,1.16,1.17,1.17,1.17,1.17,1.17	300	200	1
304	17	2.44	2.44	3.75	1.69,1.68,1.69,1.67,1.68,1.69,1.67,1.67,1.65,1.69,1.67,1.67,1.66,1.14,1.67,1.67,1.68,1.67,1.67,1.67,1.64,1.70,1.67,1.67,1.66,1.61	300	200	1
305	17	1.52	1.52	2.33	1.68,1.67,1.68,1.67,1.67,1.68,1.67,1.67,1.66,1.66,1.67,1.67,1.66,1.66,1.67,1.67,1.67,1.67,1.67,1.67,1.65,1.66,1.67,1.67,1.66,1.66	300	200	1
306	17	0.92	0.92	1.42	1.19,1.19,1.19,1.18,1.19,1.19,1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.19,1.18,1.18,1.17,1.18,1.18,1.18,1.18,1.18	300	200	1
307	17	1.52	1.52	2.33	1.68,1.67,1.68,1.67,1.67,1.68,1.67,1.67,1.66,1.66,1.67,1.67,1.66,1.66,1.67,1.67,1.67,1.67,1.67,1.67,1.66,1.66,1.67,1.67,1.66,1.67	300	200	1
308	20	7.00	7.00	7.00	2.32,2.32,2.32,2.31,2.32,2.33,2.32,2.32,2.32,2.26,2.32,2.32,2.31,2.40,2.32,2.32,2.30,2.30,2.32,2.32,2.30,2.31,2.32,2.32,2.31,2.34	300	200	1
309	3	2.60	2.60	4.00	-	300	200	1
310	17	2.44	2.44	3.75	1.68,1.67,1.68,1.67,1.68,1.68,1.67,1.67,1.66,1.69,1.67,1.67,1.66,1.61,1.67,1.67,1.67,1.67,1.67,1.67,1.65,1.71,1.67,1.67,1.66,1.64	300	200	1
311	20	7.00	7.00	7.00	2.33,2.32,2.33,2.32,2.32,2.33,2.32,2.32,2.33,2.33,2.32,2.32,2.31,2.32,2.33,2.33,2.30,2.29,2.32,2.32,2.31,2.31,2.32,2.32,2.31,2.31	300	200	1
312	6	1.30	1.30	2.00	-	300	200	1
313	20	4.88	4.00	7.50	1.08,1.08,1.08,1.08,1.08,1.08,1.08,1.08,1.09,1.14,1.08,1.08,1.08,1.12,1.08,1.08,1.07,1.08,1.08,1.08,1.09,1.14,1.08,1.08,1.08,1.12	300	200	1
314	20	4.88	4.00	7.50	1.07,1.07,1.07,1.07,1.07,1.07,1.08,1.08,1.28,1.08,1.08,1.08,1.22,1.08,1.08,1.07,1.07,1.08,1.08,1.08,1.26,1.08,1.08,1.08,1.16	300	200	1
315	20	10.20	10.20	10.20	2.26,2.27,2.26,2.28,2.26,2.26,2.31,2.31,2.37,2.32,2.31,2.31,2.32,2.30,2.30,2.30,2.23,2.21,2.30,2.30,2.37,2.32,2.31,2.31,2.31,2.29	300	200	1
316	20	10.20	10.20	10.20	2.23,2.24,2.23,2.24,2.24,2.23,2.27,2.27,2.30,2.05,2.27,2.27,2.27,2.72,2.26,2.26,2.20,2.21,2.26,2.26,2.30,2.07,2.27,2.27,2.27,2.45	300	200	1
350	34	5.67	5.67	5.67	1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14	300	200	250
351	34	5.67	5.67	5.67	1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14	300	200	250
352	34	5.67	5.67	5.67	1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14	300	200	250
353	34	5.67	5.67	5.67	1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14	300	200	250

Member Design Capacity

Member ID	$\Phi_t P_n$ (kip)	$\Phi_c P_n$ (kip)	$\Phi_b M_{zn}$ (k-ft)	$\Phi_b M_{yn}$ (k-ft)	$\Phi_v V_{yn}$ (kip)	$\Phi_v V_{zn}$ (kip)
1	574.32	124.37	123.94	123.94	172.30	172.30
2	251.01	248.88	27.16	27.16	75.30	75.30
3	151.65	150.70	20.17	14.14	54.12	28.95
4	151.65	145.15	20.17	14.14	54.12	28.95
5	151.65	149.10	20.17	14.14	54.12	28.95
6	151.65	150.70	20.17	14.14	54.12	28.95
7	151.65	149.10	20.17	14.14	54.12	28.95
8	159.30	140.46	46.90	6.46	56.26	44.91
[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]

9	75.10	66.32	4.25	4.25	22.53	22.53
10	151.65	145.15	20.17	14.14	54.12	28.95
11	159.30	140.46	46.90	6.46	56.26	44.91
12	251.01	248.88	27.16	27.16	75.30	75.30
13	159.30	97.43	32.81	6.46	56.26	44.91
14	159.30	97.43	32.68	6.46	56.26	44.91
15	159.30	68.92	46.90	6.46	56.26	44.91
16	159.30	68.92	46.90	6.46	56.26	44.91
50	41.27	8.45	1.63	0.76	15.23	10.15
51	41.27	8.45	1.63	0.76	15.23	10.15
52	41.27	8.45	1.63	0.76	15.23	10.15
53	41.27	8.45	1.63	0.76	15.23	10.15
101	574.32	124.37	123.94	123.94	172.30	172.30
102	251.01	248.88	27.16	27.16	75.30	75.30
103	151.65	150.70	20.17	14.14	54.12	28.95
104	151.65	145.15	20.17	14.14	54.12	28.95
105	151.65	149.10	20.17	14.14	54.12	28.95
106	151.65	150.70	20.17	14.14	54.12	28.95
107	151.65	149.10	20.17	14.14	54.12	28.95
108	159.30	140.46	46.90	6.46	56.26	44.91
109	75.10	66.32	4.25	4.25	22.53	22.53
110	151.65	145.15	20.17	14.14	54.12	28.95
111	159.30	140.46	46.90	6.46	56.26	44.91
112	251.01	248.88	27.16	27.16	75.30	75.30
113	159.30	97.43	32.50	6.46	56.26	44.91
114	159.30	97.43	32.37	6.46	56.26	44.91
115	159.30	32.87	24.38	6.46	56.26	44.91
116	159.30	32.87	23.88	6.46	56.26	44.91
150	41.27	8.45	1.63	0.76	15.23	10.15
151	41.27	8.45	1.63	0.76	15.23	10.15
152	41.27	8.45	1.63	0.76	15.23	10.15
153	41.27	8.45	1.63	0.76	15.23	10.15
201	574.32	124.37	123.94	123.94	172.30	172.30
202	251.01	248.88	27.16	27.16	75.30	75.30
203	151.65	150.70	20.17	14.14	54.12	28.95
204	151.65	145.15	20.17	14.14	54.12	28.95
205	151.65	149.10	20.17	14.14	54.12	28.95
206	151.65	150.70	20.17	14.14	54.12	28.95
207	151.65	149.10	20.17	14.14	54.12	28.95
208	159.30	140.46	46.90	6.46	56.26	44.91
209	75.10	66.32	4.25	4.25	22.53	22.53
210	151.65	145.15	20.17	14.14	54.12	28.95
211	159.30	140.46	46.90	6.46	56.26	44.91
212	251.01	248.88	27.16	27.16	75.30	75.30
213	159.30	97.43	32.49	6.46	56.26	44.91
214	159.30	97.43	32.52	6.46	56.26	44.91
215	159.30	32.87	21.31	6.46	56.26	44.91
216	159.30	32.87	20.45	6.46	56.26	44.91
250	41.27	8.45	1.63	0.76	15.23	10.15
251	41.27	8.45	1.63	0.76	15.23	10.15
252	41.27	8.45	1.63	0.76	15.23	10.15
253	41.27	8.45	1.63	0.76	15.23	10.15

301	574.32	124.37	123.94	123.94	172.30	172.30
302	251.01	248.88	27.16	27.16	75.30	75.30
303	151.65	150.70	20.17	14.14	54.12	28.95
304	151.65	145.15	20.17	14.14	54.12	28.95
305	151.65	149.10	20.17	14.14	54.12	28.95
306	151.65	150.70	20.17	14.14	54.12	28.95
307	151.65	149.10	20.17	14.14	54.12	28.95
308	159.30	68.92	46.90	6.46	56.26	44.91
309	75.10	66.32	4.25	4.25	22.53	22.53
310	151.65	145.15	20.17	14.14	54.12	28.95
311	159.30	68.92	46.90	6.46	56.26	44.91
312	251.01	248.88	27.16	27.16	75.30	75.30
313	159.30	97.43	32.81	6.46	56.26	44.91
314	159.30	97.43	32.68	6.46	56.26	44.91
315	159.30	32.87	42.56	6.46	56.26	44.91
316	159.30	32.87	39.50	6.46	56.26	44.91
350	41.27	8.45	1.63	0.76	15.23	10.15
351	41.27	8.45	1.63	0.76	15.23	10.15
352	41.27	8.45	1.63	0.76	15.23	10.15
353	41.27	8.45	1.63	0.76	15.23	10.15

Design Ratio

Member ID	P	M_z	M_y	V_y	V_z	(P,M_z,M_y)	Worst LC	KL/r	δ	Status
1	0.138	0.833	0.040	0.032	0.002	0.912	#13	0.761	Not Required	Pass
2	0.007	0.590	0.238	0.120	0.042	0.779	#13	0.054	Not Required	Pass
3	0.002	0.782	0.133	0.079	0.054	0.916	#21	0.046	Not Required	Pass
4	0.003	0.765	0.038	0.076	0.006	0.776	#21	0.122	Not Required	Pass
5	0.002	0.485	0.034	0.078	0.013	0.512	#21	0.076	Not Required	Pass
6	0.004	0.607	0.131	0.059	0.057	0.740	#21	0.046	Not Required	Pass
7	0.004	0.377	0.029	0.060	0.010	0.407	#21	0.076	Not Required	Pass
8	0.010	0.127	0.141	0.043	0.012	0.181	#21	0.102	Not Required	Pass
9	0.026	0.105	0.061	0.004	0.001	0.178	#21	0.137	Not Required	Pass
10	0.004	0.607	0.075	0.061	0.011	0.683	#21	0.122	Not Required	Pass
11	0.010	0.123	0.128	0.043	0.012	0.166	#21	0.068	Not Required	Pass
12	0.005	0.407	0.179	0.096	0.032	0.537	#13	0.054	Not Required	Pass
13	0.011	0.405	0.046	0.054	0.009	0.452	#21	0.204	Not Required	Pass
14	0.017	0.409	0.061	0.054	0.010	0.476	#21	0.306	Not Required	Pass
15	0.011	0.181	0.246	0.044	0.017	0.324	#21	0.357	Not Required	Pass
16	0.024	0.177	0.246	0.042	0.017	0.330	#21	0.357	Not Required	Pass
50	0.240	0.010	0.002	0.003	0.002	0.250	#21	0.783	Not Required	Pass
51	0.050	0.006	0.016	0.002	0.002	0.066	#21	0.783	Not Required	Pass
52	0.266	0.010	0.002	0.004	0.003	0.276	#23	0.783	Not Required	Pass
53	0.054	0.006	0.016	0.003	0.003	0.069	#23	0.522	Not Required	Pass
101	0.122	0.778	0.004	0.030	0.000	0.832	#13	0.761	Not Required	Pass
102	0.005	0.418	0.185	0.093	0.035	0.559	#13	0.054	Not Required	Pass
103	0.005	0.607	0.113	0.060	0.046	0.714	#21	0.046	Not Required	Pass
104	0.005	0.584	0.047	0.058	0.008	0.634	#21	0.122	Not Required	Pass
105	0.005	0.377	0.020	0.060	0.005	0.400	#21	0.076	Not Required	Pass
106	0.004	0.618	0.116	0.062	0.047	0.732	#21	0.046	Not Required	Pass
107	0.004	0.383	0.018	0.061	0.005	0.397	#21	0.076	Not Required	Pass

108	0.009	0.061	0.059	0.037	0.008	0.121	#21	0.102	Not Required	Pass
109	0.020	0.053	0.044	0.001	0.000	0.103	#21	0.137	Not Required	Pass
110	0.004	0.604	0.044	0.060	0.007	0.650	#21	0.082	Not Required	Pass
111	0.009	0.050	0.060	0.038	0.008	0.113	#21	0.102	Not Required	Pass
112	0.005	0.435	0.186	0.096	0.034	0.574	#13	0.054	Not Required	Pass
113	0.009	0.206	0.051	0.048	0.009	0.255	#21	0.306	Not Required	Pass
114	0.015	0.184	0.068	0.047	0.010	0.228	#21	0.306	Not Required	Pass
115	0.013	0.102	0.169	0.033	0.016	0.207	#21	0.780	Not Required	Pass
116	0.044	0.112	0.182	0.031	0.016	0.238	#21	0.780	Not Required	Pass
150	0.182	0.010	0.002	0.002	0.001	0.192	#21	0.783	Not Required	Pass
151	0.036	0.006	0.016	0.002	0.002	0.053	#21	0.522	Not Required	Pass
152	0.171	0.010	0.002	0.003	0.001	0.181	#24	0.783	Not Required	Pass
153	0.034	0.006	0.016	0.002	0.002	0.051	#24	0.783	Not Required	Pass
201	0.122	0.778	0.004	0.030	0.000	0.832	#13	0.761	Not Required	Pass
202	0.005	0.435	0.186	0.096	0.034	0.574	#13	0.054	Not Required	Pass
203	0.004	0.618	0.116	0.062	0.047	0.732	#21	0.046	Not Required	Pass
204	0.004	0.604	0.044	0.060	0.007	0.650	#21	0.082	Not Required	Pass
205	0.004	0.383	0.018	0.061	0.005	0.397	#21	0.076	Not Required	Pass
206	0.005	0.607	0.113	0.060	0.046	0.714	#21	0.046	Not Required	Pass
207	0.005	0.377	0.020	0.060	0.005	0.400	#21	0.076	Not Required	Pass
208	0.009	0.036	0.062	0.031	0.007	0.084	#13	0.102	Not Required	Pass
209	0.020	0.053	0.044	0.001	0.000	0.103	#21	0.137	Not Required	Pass
210	0.005	0.584	0.047	0.058	0.008	0.634	#21	0.122	Not Required	Pass
211	0.009	0.051	0.060	0.033	0.007	0.074	#21	0.102	Not Required	Pass
212	0.005	0.418	0.185	0.093	0.035	0.559	#13	0.054	Not Required	Pass
213	0.009	0.206	0.051	0.048	0.009	0.255	#21	0.306	Not Required	Pass
214	0.015	0.184	0.068	0.047	0.010	0.228	#21	0.306	Not Required	Pass
215	0.020	0.229	0.088	0.038	0.013	0.287	#21	0.780	Not Required	Pass
216	0.040	0.253	0.086	0.037	0.013	0.311	#21	0.780	Not Required	Pass
250	0.171	0.010	0.002	0.003	0.001	0.181	#24	0.783	Not Required	Pass
251	0.034	0.006	0.016	0.002	0.002	0.051	#24	0.783	Not Required	Pass
252	0.182	0.010	0.002	0.002	0.001	0.192	#21	0.783	Not Required	Pass
253	0.036	0.006	0.016	0.002	0.002	0.053	#21	0.522	Not Required	Pass
301	0.138	0.833	0.040	0.032	0.002	0.912	#13	0.761	Not Required	Pass
302	0.005	0.407	0.179	0.096	0.032	0.537	#13	0.054	Not Required	Pass
303	0.004	0.607	0.131	0.059	0.057	0.740	#21	0.046	Not Required	Pass
304	0.004	0.607	0.075	0.061	0.011	0.683	#21	0.122	Not Required	Pass
305	0.004	0.377	0.029	0.060	0.010	0.407	#21	0.076	Not Required	Pass
306	0.002	0.783	0.133	0.079	0.054	0.916	#21	0.046	Not Required	Pass
307	0.002	0.485	0.034	0.078	0.013	0.512	#21	0.076	Not Required	Pass
308	0.024	0.177	0.246	0.042	0.017	0.330	#21	0.535	Not Required	Pass
309	0.026	0.105	0.061	0.004	0.001	0.178	#21	0.137	Not Required	Pass
310	0.003	0.765	0.038	0.076	0.006	0.776	#21	0.122	Not Required	Pass
311	0.011	0.181	0.246	0.044	0.017	0.324	#21	0.357	Not Required	Pass
312	0.007	0.590	0.238	0.120	0.042	0.779	#13	0.054	Not Required	Pass
313	0.011	0.405	0.046	0.054	0.009	0.452	#21	0.204	Not Required	Pass
314	0.017	0.409	0.061	0.054	0.010	0.476	#21	0.306	Not Required	Pass
315	0.013	0.130	0.169	0.043	0.016	0.192	#21	0.520	Not Required	Pass
316	0.044	0.137	0.182	0.043	0.016	0.221	#21	0.780	Not Required	Pass
350	0.266	0.010	0.002	0.004	0.003	0.276	#23	0.783	Not Required	Pass
351	0.054	0.006	0.016	0.003	0.003	0.069	#23	0.522	Not Required	Pass

352	0.240	0.010	0.002	0.003	0.002	0.250	#21	0.783	Not Required	Pass
353	0.050	0.006	0.016	0.002	0.002	0.066	#21	0.783	Not Required	Pass

Definitions

Φ_t	Safety factor for tensile
Φ_c	Safety factor for compression
Φ_b	Safety factor for flexure
Φ_v	Safety factor for shear
E	Modulus of elasticity
F_y	Specified minimum yield stress
F_u	Specified minimum tensile strength
A	Cross-sectional area
J	Torsional constant
I_{yp}	Moment of inertia about the Y axes
I_{zp}	Moment of inertia about the Z axes
I_w	Warping constant
S_{yp}	Plastic section modulus about the Y axis
S_{zp}	Plastic section modulus about the Z axis
KL	Effective length
C_b	Buckling modification factor (from all load combinations)
L_b	Length between braced points
LST	Limited slenderness for tension
LSC	Limited slenderness for compression
LD	Limited deflection
P_n	Nominal axial strength (tension/comression)
M_n	Nominal flexural strength (about Z/Y axis)
V_n	Nominal shear strength (along Z/Y axis)
P	Design ratio in case of axial force
M_z	Design ratio in case of bending about Z axis
M_y	Design ratio in case of bending about Y axis
V_y	Design ratio in case of shear along Y axis
V_z	Design ratio in case of shear along Z axis
(P,M_z,M_y)	Design ratio in case of axial force and bending action
KL/r	Design ratio in case of section slenderness
δ	Design ratio in case of member deflection
OK	Capacity is provided
NG	Capacity is not provided

REFERENCES	CALCULATIONS	RESULTS

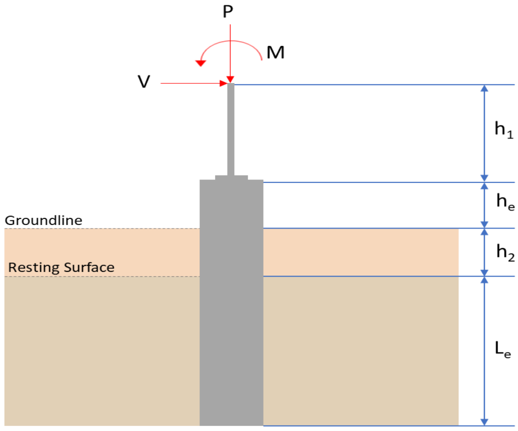
SkyCiv Foundation Design

Pile Foundation

Design Information :

Design code : IBC 2021 (International Building Code)
Unit System : Imperial

Pile Input



Geometry

Pile shape: rectangular
$b = 48\ \mathrm{in}$ - Pile width
$D = 48\ \mathrm{in}$ - Pile depth
$L = 8.25\ \mathrm{ft}$ - Total pile length
$h_1 = 0\ \mathrm{ft}$ - Lateral load height from the top of the pile,
$h_2 = 0\ \mathrm{ft}$ - Depth to resisting surface
$h_e = 0\ \mathrm{ft}$ - Length of pile above the ground

Tabulation of Soil Parameters

Layer	Label	Allowable Bearing Pressure (q_a) (psf)	Allowable Lateral Pressure (R) (psf/ft)
1	Sand, silty sand, clayey sand, silty gravel & clayey gravel	2000.000	150.000

Tabulation of Loads

Load Component	ASD	LRFD
P (kip)	9.956	15.113
V_x (kip)	-3.091	-5.173
V_z (kip)	0.026	0.039
M_x (kipft)	0.139	0.212
M_z (kipft)	56.096	96.375

Material Properties

$f'_{ck} = 2.5\ \mathrm{ksi}$ - Concrete strength,

Required depth to resist lateral loads (ASD)

H - Point of application of the lateral load

$$H = h_1 + h_2 + h_e$$

$$H = (0\ \mathrm{ft}) + (0\ \mathrm{ft}) + (0\ \mathrm{ft})$$

$$H = 0\ \mathrm{ft}$$

Considering x-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\ D}$$

$$H_o = \frac{(-3.091\ \mathrm{kip})}{1.57 \times (48\ \mathrm{in})}$$

$$H_o = -0.4922\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\ H)}{1.57\ D}$$

$$M_o = \frac{(56.096\ \text{kipft}) + ((-3.091\ \text{kip}) \times (0\ \text{ft}))}{1.57 \times (48\ \text{in})}$$

$$M_o = 8.9325\ \text{kipft/ft}$$

Required depth of embedment in earth:

$$L_x^3 - \left(14.14 \times \frac{H_o \times L_x}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,x} = 7.8455\ \text{ft}$ - Required depth in x-direction,

Considering z-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\ b}$$

$$H_o = \frac{(0.026\ \text{kip})}{1.57 \times (48\ \text{in})}$$

$$H_o = 0.0041401\ \text{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\ H)}{1.57\ b}$$

$$M_o = \frac{(0.139\ \text{kipft}) + ((0.026\ \text{kip}) \times (0\ \text{ft}))}{1.57 \times (48\ \text{in})}$$

$$M_o = 0.022134\ \text{kipft/ft}$$

Required depth of embedment in earth:

$$L_z^3 - \left(14.14 \times \frac{H_o \times L_z}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,z} = 1.2781\ \text{ft}$ - Required depth in z-direction,

Minimum embedded depth required:

$L_{e,req}$ - Depth of pile required,

$$L_{e,req} = MAX[L_{e,x}, L_{e,z}]$$

$$L_{e,req} = MAX[(7.8455\ \text{ft}), (1.2781\ \text{ft})]$$

$$L_{e,req} = 7.845\ \text{ft}$$

L_e - Actual embedded length of pile,

$$L_e = L - h_e - h_2$$

$$L_e = (8.25\ \text{ft}) - (0\ \text{ft}) - (0\ \text{ft})$$

$$L_e = 8.25\ \text{ft}$$

$Ratio$ - Embedded depth

$$Ratio = \frac{L_{e,req}}{L_e}$$

$$Ratio = \frac{(7.845\ \text{ft})}{(8.25\ \text{ft})}$$

$$Ratio = 0.95091$$

Status: **PASS**
Ratio: **0.950**

End-bearing Capacity (ASD)

A - Pile cross-section area

$$A = b\ D$$

$$A = (48\ \text{in}) \times (48\ \text{in})$$

$$A = 16\ \text{ft}^2$$

q - End-bearing pressure

$$q = \frac{P_v}{A}$$

$$q = \frac{(9.956 \text{ kip})}{(16 \text{ ft}^2)}$$

$$q = 0.62225 \text{ kip/ft}^2$$

Check bearing capacity ratio:

$Ratio$ - Capacity

$$Ratio = \frac{q}{q_a}$$

$$Ratio = \frac{\left(0.62225 \text{ kip/ft}^2\right)}{(2000 \text{ psf})}$$

$$Ratio = 0.31112$$

Status: **PASS**
Ratio: **0.310**

Czerniak

Lateral Soil Pressure (ASD):

L/D - Length to least lateral dimension ratio,

$$L/D = \frac{L}{D}$$

$$L/D = \frac{(8.25 \text{ ft})}{(48 \text{ in})}$$

$$L/D = 2.0625$$

Since L/D $\leq$ 10,

Pile is short.

Considering x-direction:

$H_o = -0.4922 \text{ kip/ft}$ - Lateral force per length of pile,
$M_o = 8.9325 \text{ kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\, M_o\, L_e) + \left(3\, H_o\, L_e^2\right)}{(6\, M_o) + (4\, H_o\, L_e)}$$

$$a = \frac{(4 \times (8.9325 \text{ kipft/ft}) \times (8.25 \text{ ft})) + \left(3 \times (-0.4922 \text{ kip/ft}) \times (8.25 \text{ ft})^2\right)}{(6 \times (8.9325 \text{ kipft/ft})) + (4 \times (-0.4922 \text{ kip/ft}) \times (8.25 \text{ ft}))}$$

$$a = 5.6599 \text{ ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\, [(4\, M_o) + (3\, H_o\, L_e)]^2}{L_e^2\, [(3\, M_o) + (2\, H_o\, L_e)]}$$

$$p = \frac{0.75 \times [(4 \times (8.9325 \text{ kipft/ft})) + (3 \times (-0.4922 \text{ kip/ft}) \times (8.25 \text{ ft}))]^2}{(8.25 \text{ ft})^2 \times [(3 \times (8.9325 \text{ kipft/ft})) + (2 \times (-0.4922 \text{ kip/ft}) \times (8.25 \text{ ft}))]}$$

$$p = 0.32717 \text{ kip/ft}^2$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\, [(2\, M_o) + (H_o\, L_e)]}{L_e^2}$$

$$s = \frac{6 \times [(2 \times (8.9325 \text{ kipft/ft})) + ((-0.4922 \text{ kip/ft}) \times (8.25 \text{ ft}))]}{(8.25 \text{ ft})^2}$$

$$s = 1.2169 \text{ kip/ft}^2$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\, \frac{a}{2}$$

$$p_a = (150 \text{ psf/ft}) \times \frac{(5.6599 \text{ ft})}{2}$$

$$p_a = 0.42449 \text{ kip/ft}^2$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{\left(0.32717\ \mathrm{kip/ft^2}\right)}{\left(0.42449\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = 0.77074$$

Status: **PASS**
Ratio: **0.770**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\, L_e$$

$$p_s = (150\ \mathrm{psf/ft}) \times (8.25\ \mathrm{ft})$$

$$p_s = 1.2375\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{\left(1.2169\ \mathrm{kip/ft^2}\right)}{\left(1.2375\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = 0.98336$$

Status: **PASS**
Ratio: **0.980**

Considering z-direction:

$H_o = 0.0041401\ \mathrm{kip/ft}$ - Lateral force per length of pile,
$M_o = 0.022134\ \mathrm{kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\, M_o\, L_e) + \left(3\, H_o\, L_e^2\right)}{(6\, M_o) + (4\, H_o\, L_e)}$$

$$a = \frac{(4 \times (0.022134\ \mathrm{kipft/ft}) \times (8.25\ \mathrm{ft})) + \left(3 \times (0.0041401\ \mathrm{kip/ft}) \times (8.25\ \mathrm{ft})^2\right)}{(6 \times (0.022134\ \mathrm{kipft/ft})) + (4 \times (0.0041401\ \mathrm{kip/ft}) \times (8.25\ \mathrm{ft}))}$$

$$a = 5.8486\ \mathrm{ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\ [(4\, M_o) + (3\, H_o\, L_e)]^2}{L_e^2\ [(3\, M_o) + (2\, H_o\, L_e)]}$$

$$p = \frac{0.75 \times [(4 \times (0.022134\ \mathrm{kipft/ft})) + (3 \times (0.0041401\ \mathrm{kip/ft}) \times (8.25\ \mathrm{ft}))]^{\ 2}}{(8.25\ \mathrm{ft})^2 \times [(3 \times (0.022134\ \mathrm{kipft/ft})) + (2 \times (0.0041401\ \mathrm{kip/ft}) \times (8.25\ \mathrm{ft}))]}$$

$$p = 0.0029842\ \mathrm{kip/ft^2}$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\ [(2\, M_o) + (H_o\, L_e)]}{L_e^2}$$

$$s = \frac{6 \times [(2 \times (0.022134\ \mathrm{kipft/ft})) + ((0.0041401\ \mathrm{kip/ft}) \times (8.25\ \mathrm{ft}))]}{(8.25\ \mathrm{ft})^2}$$

$$s = 0.0069134\ \mathrm{kip/ft^2}$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\, \frac{a}{2}$$

$$p_a = (150\ \mathrm{psf/ft}) \times \frac{(5.8486\ \mathrm{ft})}{2}$$

$$p_a = 0.43865\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{\left(0.0029842\ \mathrm{kip/ft^2}\right)}{\left(0.43865\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = 0.0068031$$

Status: **PASS**
Ratio: **0.010**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\,L_e$$

$$p_s = (150\ \mathrm{psf/ft}) \times (8.25\ \mathrm{ft})$$

$$p_s = 1.2375\ \mathrm{kip/ft^2}$$

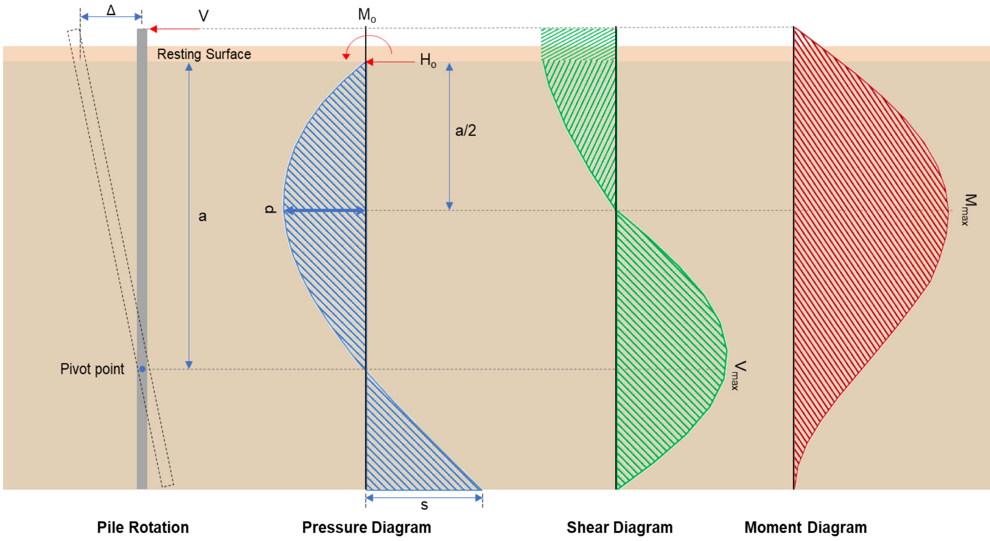
$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{\left(0.0069134\ \mathrm{kip/ft^2}\right)}{\left(1.2375\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = 0.0055866$$

Status: **PASS**
Ratio: **0.010**



Shear force and Bending moment (x-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\,D}$$

$$H_o = \frac{(-5.173\ \mathrm{kip})}{1.57 \times (48\ \mathrm{in})}$$

$$H_o = -0.82373\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\,H)}{1.57\,D}$$

$$M_o = \frac{(96.375\ \mathrm{kipft}) + ((-5.173\ \mathrm{kip}) \times (0\ \mathrm{ft}))}{1.57 \times (48\ \mathrm{in})}$$

$$M_o = 15.346\ \mathrm{kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(15.346\ \mathrm{kipft/ft})}{(-0.82373\ \mathrm{kip/ft})}$$

$$E = 18.63\ \mathrm{ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (15.346\ \mathrm{kipft/ft}) \times (8.25\ \mathrm{ft})) + \left(3 \times (-0.82373\ \mathrm{kip/ft}) \times (8.25\ \mathrm{ft})^2\right)}{\cdots}$$

$$a = \frac{}{(6 \times (15.346\ \mathrm{kipft/ft})) + (4 \times (-0.82373\ \mathrm{kip/ft}) \times (8.25\ \mathrm{ft})}$$

$$a = 5.6567\ \mathrm{ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\, D) \left[1 - \left[3 \left(\frac{4\,E}{L_e} + 3\right) \left(\frac{a}{L_e}\right)^2\right] + \left[4 \left(\frac{3\,E}{L_e} + 2\right) \left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((-0.82373\ \mathrm{kip/ft}) \times (48\ \mathrm{in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (18.63\ \mathrm{ft})}{(8.25\ \mathrm{ft})} + 3\right) \times \left(\frac{(5.6567\ \mathrm{ft})}{(8.25\ \mathrm{ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (18.63\ \mathrm{ft})}{(8.25\ \mathrm{ft})} + 2\right) \times \left(\frac{(5.6567\ \mathrm{ft})}{(8.25\ \mathrm{ft})}\right)^3\right]\right]$$

$$V_{max} = 15.345\ \mathrm{kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, D\, L_e) \left[\left(\frac{E}{L_e} + \frac{a}{2\,L_e}\right) - \left[\left(\frac{4\,E}{L_e} + 3\right) \left(\frac{a}{2\,L_e}\right)^3\right] + \left[\left(\frac{3\,E}{L_e} + 2\right) \left(\frac{a}{2\,L_e}\right)^4\right]\right]$$

$$M_{max} = ((-0.82373\ \mathrm{kip/ft}) \times (48\ \mathrm{in}) \times (8.25\ \mathrm{ft})) \times \left[\left(\frac{(18.63\ \mathrm{ft})}{(8.25\ \mathrm{ft})} + \frac{(5.6567\ \mathrm{ft})}{2 \times (8.25\ \mathrm{ft})}\right) - \left[\left(\frac{4 \times (18.63\ \mathrm{ft})}{(8.25\ \mathrm{ft})} + 3\right) \times \left(\frac{(5.6567\ \mathrm{ft})}{2 \times (8.25\ \mathrm{ft})}\right)^3\right] + \left[\left(\frac{3 \times (18.63\ \mathrm{ft})}{(8.25\ \mathrm{ft})} + 2\right) \times \left(\frac{(5.6567\ \mathrm{ft})}{2 \times (8.25\ \mathrm{ft})}\right)^4\right]\right]$$

$$M_{max} = 60.82\ \mathrm{kipft}$$

Shear force and Bending moment (z-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\,b}$$

$$H_o = \frac{(0.039\ \mathrm{kip})}{1.57 \times (48\ \mathrm{in})}$$

$$H_o = 0.0062102\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\, H)}{1.57\,b}$$

$$M_o = \frac{(0.212\ \mathrm{kipft}) + ((0.039\ \mathrm{kip}) \times (0\ \mathrm{ft}))}{1.57 \times (48\ \mathrm{in})}$$

$$M_o = 0.033758\ \mathrm{kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(0.033758\ \mathrm{kipft/ft})}{(0.0062102\ \mathrm{kip/ft})}$$

$$E = 5.4359\ \mathrm{ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (0.033758\ \mathrm{kipft/ft}) \times (8.25\ \mathrm{ft})) + \left(3 \times (0.0062102\ \mathrm{kip/ft}) \times (8.25\ \mathrm{ft})^2\right)}{(6 \times (0.033758\ \mathrm{kipft/ft})) + (4 \times (0.0062102\ \mathrm{kip/ft}) \times (8.25\ \mathrm{ft}))}$$

$$a = 5.8458\ \mathrm{ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\, b) \left[1 - \left[3 \left(\frac{4\,E}{L_e} + 3\right) \left(\frac{a}{L_e}\right)^2\right] + \left[4 \left(\frac{3\,E}{L_e} + 2\right) \left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((0.0062102\ \text{kip/ft}) \times (48\ \text{in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (5.4359\ \text{ft})}{(8.25\ \text{ft})} + 3\right) \times \left(\frac{(5.8458\ \text{ft})}{(8.25\ \text{ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (5.4359\ \text{ft})}{(8.25\ \text{ft})} + 2\right) \times \left(\frac{(5.8458\ \text{ft})}{(8.25\ \text{ft})}\right)^3\right]\right]$$

$$V_{max} = 0.045447\ \text{kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, b\, L_e)\left[\left(\frac{E}{L_e} + \frac{a}{2\,L_e}\right) - \left[\left(\frac{4\,E}{L_e} + 3\right)\left(\frac{a}{2\,L_e}\right)^3\right] + \left[\left(\frac{3\,E}{L_e} + 2\right)\left(\frac{a}{2\,L_e}\right)^4\right]\right]$$

$$M_{max} = ((0.0062102\ \text{kip/ft}) \times (48\ \text{in}) \times (8.25\ \text{ft})) \times \left[\left(\frac{(5.4359\ \text{ft})}{(8.25\ \text{ft})} + \frac{(5.8458\ \text{ft})}{2 \times (8.25\ \text{ft})}\right) - \left[\left(\frac{4 \times (5.4359\ \text{ft})}{(8.25\ \text{ft})} + 3\right) \times \left(\frac{(5.8458\ \text{ft})}{2 \times (8.25\ \text{ft})}\right)^3\right] + \left[\left(\frac{3 \times (5.4359\ \text{ft})}{(8.25\ \text{ft})} + 2\right) \times \left(\frac{(5.8458\ \text{ft})}{2 \times (8.25\ \text{ft})}\right)^4\right]\right]$$

$$M_{max} = 0.16912\ \text{kipft}$$

Minimum Reinforcement Check (LRFD)

Parameters:

$f'_{ck} = 2.5\ \text{ksi}$ - Concrete strength,
$f_{yk} = 60\ \text{ksi}$ - Longitudinal reinforcement strength,
$\phi = 0.65$ - Reduction factor for axial strength,
Table 22.4.2.1 $\alpha = 0.8$ - Alpha factor for axial strength,
$A_g = 2304\ \text{in}^2$ - Gross area of concrete,

Longitudinal reinforcement:

Required reinforcement due to axial load, $A_{st,required}$

22.4.2.2, 10.6.1.1 $A_{st,required}$

$$A_{st,required} = Min\left[\frac{\frac{P}{\phi\,\alpha} - (0.85\, f'_{ck}\, A_g)}{f_{yk} - (0.85\, f'_{ck})}, (0.08\, A_g)\right]$$

$$A_{st,required} = Min\left[\frac{\frac{(15.113\ \text{kip})}{(0.65)\times(0.8)} - (0.85 \times (2.5\ \text{ksi}) \times (2304\ \text{in}^2))}{(60\ \text{ksi}) - (0.85 \times (2.5\ \text{ksi}))}, (0.08 \times (2304\ \text{in}^2))\right]$$

$$A_{st,required} = -84.094\ \text{in}^2$$

A_{min} - Governing minimum reinforcement area,

$$A_{min} = Max[A_{st,required}, (0.0018\, A_g)]$$

$$A_{min} = Max\left[(-84.094\ \text{in}^2), (0.0018 \times (2304\ \text{in}^2))\right]$$

$$A_{min} = 4.1472\ \text{in}^2$$

n_{rebar} - Required number of reinforcement,

$$n_{rebar} = \frac{A_{min}}{A_{rebar}}$$

$$n_{rebar} = \frac{(4.1472\ \text{in}^2)}{(0.3068\ \text{in}^2)}$$

$$n_{rebar} = 14$$

A_{st} - Actual total reinforcement area,

$$A_{st} = n_{rebar}\, \frac{\pi\, d_{bar}^2}{4}$$

$$A_{st} = (14) \times \frac{\pi \times (0.625\ \text{in})^2}{4}$$

$$A_{st} = 4.2951\ \text{in}^2$$

$Ratio$ - Capacity

$$Ratio = \frac{A_{min}}{A_{st}}$$

$$Ratio = \frac{(4.1472\ \text{in}^2)}{(4.2951\ \text{in}^2)}$$

$$Ratio = 0.96556$$

Status: **PASS**
Ratio: **0.970**

25.2.3 s_{rebar} - Minimum spacing of reinforcement,

$$s_{rebar} = Max\left[1.5, (1.5\ d_{bar})\right]$$

$$s_{rebar} = Max\left[1.5, (1.5 \times (0.625\ \text{in}))\right]$$

$$s_{rebar} = 1.5\ \text{in}$$

Ties:

25.7.2.2 Since longitudinal reinforcement is $\leq$ No. 10⌀: Use #3(0.375 in)
25.7.2.1 s_{ties} - Maximum spacing of ties,

$$s_{ties} = Min\left[(16\ d_{bar}), (48\ d_{ties}), Min\,(D, b)\right]$$

$$s_{ties} = Min\left[(16 \times (0.625\ \text{in})), (48 \times (0.375\ \text{in})), Min\,((48\ \text{in}), (48\ \text{in}))\right]$$

$$s_{ties} = 10\ \text{in}$$

Summary:

Main reinforcement: **14 - #5 (0.625 in)**
Ties: **#3(0.375 in) - 10 in**

Axial Compression Strength (ACI 318-19, LRFD)

22.4.2.2 ϕP_N - Allowable axial compressive strength

$$\phi P_N = \phi\ 0.80\ \left[\left(0.85\ f'_{ck}\ [A_g - A_{st}]\right) + (f_{yk}\ A_{st})\right]$$

$$\phi P_N = (0.65) \times 0.80 \times \left[\left(0.85 \times (2.5\ \text{ksi}) \times \left[\left(2304\ \text{in}^2\right) - \left(4.2951\ \text{in}^2\right)\right]\right) + \left((60\ \text{ksi}) \times \left(4.2951\ \text{in}^2\right)\right)\right]$$

$$\phi P_N = 2675.2\ \text{kip}$$

$Ratio$ - Capacity

$$Ratio = \frac{P}{\phi P_N}$$

$$Ratio = \frac{(15.113\ \text{kip})}{(2675.2\ \text{kip})}$$

$$Ratio = 0.0056493$$

Status: **PASS**
Ratio: **0.010**

Shear Strength (ACI 318-19, LRFD)

Parameters:

$b_w = 48\ \text{in}$ - Effective width,
22.5.2.2 d - Effective depth

$$d = 0.80\ D$$

$$d = 0.80 \times (48\ \text{in})$$

$$d = 38.4\ \text{in}$$

22.5.5.1.3 λ_s - size effect modification factor

$$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{d}{10}}}, 1\right]$$

$$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{(38.4\ \text{in})}{10}}}, 1\right]$$

$$\lambda_s = 0.64282$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5\ \text{ksi} \rightarrow 2500\ \text{psi}$,
22.5.5.1.1 $V_{c,max}$ - Max shear strength of concrete

$$V_{c,max} = 5\ \lambda_s\ \sqrt{f'_{ck}}\ b_w\ d$$

$$V_{c,max} = 5 \times (0.64282) \times \sqrt{(2500\ \text{psi})} \times (48\ \text{in}) \times (38.4\ \text{in})$$

$$V_{c,max} = 296.21 \text{ kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$,$P = 15.113 \text{ kip} \rightarrow 15113 \text{ lbf}$,

22.5.5.1.1(a) $V_{c,a}$ - Shear strength of concrete (a)

$$V_{c,a} = \left[2\,\lambda_s\,\sqrt{f'_{ck}} + \frac{P}{6\,A_g}\right]\,b_w\,d$$

$$V_{c,a} = \left[2 \times (0.64282) \times \sqrt{(2500 \text{ psi})} + \frac{(15113 \text{ lbf})}{6 \times (2304 \text{ in}^2)}\right] \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{c,a} = 120.5 \text{ kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$,

22.5.5.1.2 $V_{c,b}$ - Shear strength of concrete (b)

$$V_{c,b} = \left[2\,\lambda_s\,\sqrt{f'_{ck}} + \left(0.05\,f'_{ck}\right)\right]\,b_w\,d$$

$$V_{c,b} = \left[2 \times (0.64282) \times \sqrt{(2500 \text{ psi})} + (0.05 \times (2500 \text{ psi}))\right] \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{c,b} = 348.89 \text{ kip}$$

V_c - Governing shear strength of concrete

$$V_c = Min\left[V_{c,max}, V_{c,a}, V_{c,b}\right]$$

$$V_c = Min\left[(296.21 \text{ kip}), (120.5 \text{ kip}), (348.89 \text{ kip})\right]$$

$$V_c = 120.5 \text{ kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$,

22.5.1.2 $V_{s,a}$ - Shear strength of steel (a)

$$V_{s,a} = 8\,\sqrt{f'_{ck}}\,b_w\,d$$

$$V_{s,a} = 8 \times \sqrt{(2500 \text{ psi})} \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{s,a} = 737.28 \text{ kip}$$

A_v - Ties rebar area,

$$A_v = \frac{\pi\,d_{ties}^2}{4}$$

$$A_v = \frac{\pi \times (0.375 \text{ in})^2}{4}$$

$$A_v = 0.11045 \text{ in}^2$$

22.5.8.5.3 $V_{s,b}$ - Shear strength of steel (b)

$$V_{s,b} = \frac{2\,A_v\,f_{ywk}\,d}{s_{ties}}$$

$$V_{s,b} = \frac{2 \times \left(0.11045 \text{ in}^2\right) \times (60 \text{ ksi}) \times (38.4 \text{ in})}{(10 \text{ in})}$$

$$V_{s,b} = 50.894 \text{ kip}$$

V_s - Governing shear strength of steel

$$V_s = MIN\left[V_{s,a}, V_{s,b}\right]$$

$$V_s = MIN\left[(737.28 \text{ kip}), (50.894 \text{ kip})\right]$$

$$V_s = 50.894 \text{ kip}$$

22.5.1.1 ϕV_n - Allowable shear strength

$$\phi V_n = \phi\,\left(V_c + V_s\right)$$

$$\phi V_n = (0.65) \times ((120.5 \text{ kip}) + (50.894 \text{ kip}))$$

$$\phi V_n = 111.41 \text{ kip}$$

Considering x-direction:

$V_{max} = 15.345 \text{ kip}$ - Maximum shear force in the x-direction,
$Ratio$ - Capacity

$$Ratio = \frac{V_{max}}{\phi V_n}$$

	$$Ratio = \frac{(15.345\ \mathrm{kip})}{(111.41\ \mathrm{kip})}$$ $$Ratio = 0.13773$$	Status: **PASS** Ratio: **0.140**
	Considering z-direction: $V_{max} = 0.045447\ \mathrm{kip}$ - Maximum shear force in the z-direction, $Ratio$ - Capacity $$Ratio = \frac{V_{max}}{\phi V_n}$$ $$Ratio = \frac{(0.045447\ \mathrm{kip})}{(111.41\ \mathrm{kip})}$$ $$Ratio = 0.00040794$$	Status: **PASS** Ratio: **0.000**
14.5.2.1b	**Flexural Strength (ACI 318-19, LRFD)** S_m - Section modulus $$S_m = \frac{b\ D^2}{6}$$ $$S_m = \frac{(48\ \mathrm{in}) \times (48\ \mathrm{in})^2}{6}$$ $$S_m = 18432\ \mathrm{in}^3$$ $\lambda = 1$ - Concrete modification factor (Normal concrete), Allowable flexural strength: M_n shall be the lesser of: $\phi M_{n,1}$ $$\phi M_{n,1} = \phi \times 5 \times \lambda \times \sqrt{(f_c')} \times S_m$$ $$\phi M_{n,1} = 0.65 \times 5 \times 1 \times \sqrt{(2.5 ksi)} \times 18432.001 in^3$$ $$\phi M_{n,1} = 249.600 kipft$$ $\phi M_{n,2}$ $$\phi M_{n,2} = \phi\ 0.85\ f_{ck}'\ S_m$$ $$\phi M_{n,2} = (0.65) \times 0.85 \times (2.5\ \mathrm{ksi}) \times \left(18432\ \mathrm{in}^3\right)$$ $$\phi M_{n,2} = 2121.6\ \mathrm{kipft}$$ Therefore, ϕM_n - Allowable flexural strength, $$\phi M_n = MIN[\phi M_{n,1}, \phi M_{n,2}]$$ $$\phi M_n = MIN[(249.6\ \mathrm{kipft}), (2121.6\ \mathrm{kipft})]$$ $$\phi M_n = 249.6\ \mathrm{kipft}$$ **Considering x-direction:** $M_{max} = 60.82\ \mathrm{kipft}$ - Maximum moment in the x-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\phi M_n}$$ $$Ratio = \frac{(60.82\ \mathrm{kipft})}{(249.6\ \mathrm{kipft})}$$ $$Ratio = 0.24367$$	Status: **PASS** Ratio: **0.240**
	Considering z-direction: $M_{max} = 0.16912\ \mathrm{kipft}$ - Maximum moment in the z-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\phi M_n}$$	

	$$Ratio = \frac{(0.16912 \text{ kipft})}{(249.6 \text{ kipft})}$$ $$Ratio = 0.00067756$$	Status: **PASS** Ratio: **0.000**

REFERENCES	CALCULATIONS	RESULTS

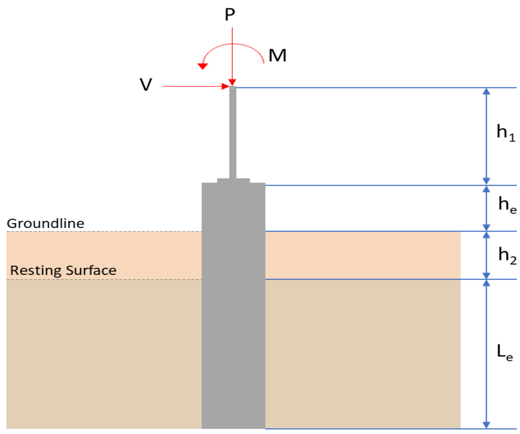
SkyCiv Foundation Design

Pile Foundation

Design Information :

Design code : IBC 2021 (International Building Code)
Unit System : Imperial

Pile Input



Geometry

Pile shape: rectangular
$b = 48 \text{ in}$ - Pile width
$D = 48 \text{ in}$ - Pile depth
$L = 8.25 \text{ ft}$ - Total pile length
$h_1 = 0 \text{ ft}$ - Lateral load height from the top of the pile,
$h_2 = 0 \text{ ft}$ - Depth to resisting surface
$h_e = 0 \text{ ft}$ - Length of pile above the ground

Tabulation of Soil Parameters

Layer	Label	Allowable Bearing Pressure (q_a) (psf)	Allowable Lateral Pressure (R) (psf/ft)
1	Sand, silty sand, clayey sand, silty gravel & clayey gravel	2000.000	150.000

Tabulation of Loads

Load Component	ASD	LRFD
P (kip)	9.956	15.113
V_x (kip)	-3.091	-5.173
V_z (kip)	-0.026	-0.039
M_x (kipft)	-0.139	-0.212
M_z (kipft)	56.096	96.375

Material Properties

$f'_{ck} = 2.5 \text{ ksi}$ - Concrete strength,

Required depth to resist lateral loads (ASD)

H - Point of application of the lateral load

$$H = h_1 + h_2 + h_e$$

$$H = (0 \text{ ft}) + (0 \text{ ft}) + (0 \text{ ft})$$

$$H = 0 \text{ ft}$$

Considering x-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\, D}$$

$$H_o = \frac{(-3.091 \text{ kip})}{1.57 \times (48 \text{ in})}$$

$$H_o = -0.4922 \text{ kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\,H)}{1.57\,D}$$

$$M_o = \frac{(56.096\ \text{kipft}) + ((-3.091\ \text{kip}) \times (0\ \text{ft}))}{1.57 \times (48\ \text{in})}$$

$$M_o = 8.9325\ \text{kipft/ft}$$

Required depth of embedment in earth:

$$L_x^3 - \left(14.14 \times \frac{H_o \times L_x}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,x} = 7.8455\ \text{ft}$ - Required depth in x-direction,

Considering z-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\,b}$$

$$H_o = \frac{(-0.026\ \text{kip})}{1.57 \times (48\ \text{in})}$$

$$H_o = -0.0041401\ \text{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\,H)}{1.57\,b}$$

$$M_o = \frac{(0.139\ \text{kipft}) + ((-0.026\ \text{kip}) \times (0\ \text{ft}))}{1.57 \times (48\ \text{in})}$$

$$M_o = 0.022134\ \text{kipft/ft}$$

Required depth of embedment in earth:

$$L_z^3 - \left(14.14 \times \frac{H_o \times L_z}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,z} = 1.1414\ \text{ft}$ - Required depth in z-direction,

Minimum embedded depth required:

$L_{e,req}$ - Depth of pile required,

$$L_{e,req} = MAX[L_{e,x}, L_{e,z}]$$

$$L_{e,req} = MAX[(7.8455\ \text{ft}), (1.1414\ \text{ft})]$$

$$L_{e,req} = 7.845\ \text{ft}$$

L_e - Actual embedded length of pile,

$$L_e = L - h_e - h_2$$

$$L_e = (8.25\ \text{ft}) - (0\ \text{ft}) - (0\ \text{ft})$$

$$L_e = 8.25\ \text{ft}$$

$Ratio$ - Embedded depth

$$Ratio = \frac{L_{e,req}}{L_e}$$

$$Ratio = \frac{(7.845\ \text{ft})}{(8.25\ \text{ft})}$$

$$Ratio = 0.95091$$

Status: **PASS**
Ratio: **0.950**

End-bearing Capacity (ASD)

A - Pile cross-section area

$$A = b\,D$$

$$A = (48\ \text{in}) \times (48\ \text{in})$$

$$A = 16\ \text{ft}^2$$

q - End-bearing pressure

$$q = \frac{P_v}{A}$$

$$q = \frac{(9.956\ \text{kip})}{(16\ \text{ft}^2)}$$

$$q = 0.62225\ \text{kip/ft}^2$$

Check bearing capacity ratio:

$Ratio$ - Capacity

$$Ratio = \frac{q}{q_a}$$

$$Ratio = \frac{\left(0.62225\ \text{kip/ft}^2\right)}{(2000\ \text{psf})}$$

$$Ratio = 0.31112$$

Status: **PASS**
Ratio: **0.310**

Czerniak

Lateral Soil Pressure (ASD):

L/D - Length to least lateral dimension ratio,

$$L/D = \frac{L}{D}$$

$$L/D = \frac{(8.25\ \text{ft})}{(48\ \text{in})}$$

$$L/D = 2.0625$$

Since L/D $\leq$ 10,

Pile is short.

Considering x-direction:

$H_o = -0.4922\ \text{kip/ft}$ - Lateral force per length of pile,
$M_o = 8.9325\ \text{kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (8.9325\ \text{kipft/ft}) \times (8.25\ \text{ft})) + \left(3 \times (-0.4922\ \text{kip/ft}) \times (8.25\ \text{ft})^2\right)}{(6 \times (8.9325\ \text{kipft/ft})) + (4 \times (-0.4922\ \text{kip/ft}) \times (8.25\ \text{ft}))}$$

$$a = 5.6599\ \text{ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\,[(4\,M_o) + (3\,H_o\,L_e)]^2}{L_e^2\,[(3\,M_o) + (2\,H_o\,L_e)]}$$

$$p = \frac{0.75 \times [(4 \times (8.9325\ \text{kipft/ft})) + (3 \times (-0.4922\ \text{kip/ft}) \times (8.25\ \text{ft}))]^2}{(8.25\ \text{ft})^2 \times [(3 \times (8.9325\ \text{kipft/ft})) + (2 \times (-0.4922\ \text{kip/ft}) \times (8.25\ \text{ft}))]}$$

$$p = 0.32717\ \text{kip/ft}^2$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\,[(2\,M_o) + (H_o\,L_e)]}{L_e^2}$$

$$s = \frac{6 \times [(2 \times (8.9325\ \text{kipft/ft})) + ((-0.4922\ \text{kip/ft}) \times (8.25\ \text{ft}))]}{(8.25\ \text{ft})^2}$$

$$s = 1.2169\ \text{kip/ft}^2$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\,\frac{a}{2}$$

$$p_a = (150\ \text{psf/ft}) \times \frac{(5.6599\ \text{ft})}{2}$$

$$p_a = 0.42449\ \text{kip/ft}^2$$

$Ratio$ - Lateral soil capacity

	$$Ratio = \frac{p}{p_a}$$ $$Ratio = \frac{\left(0.32717\ \mathrm{kip/ft^2}\right)}{\left(0.42449\ \mathrm{kip/ft^2}\right)}$$ $$Ratio = 0.77074$$	Status: **PASS** Ratio: **0.770**	
	p_s - Allowable lateral soil pressure at depth L_e, $$p_s = R\,L_e$$ $$p_s = (150\ \mathrm{psf/ft}) \times (8.25\ \mathrm{ft})$$ $$p_s = 1.2375\ \mathrm{kip/ft^2}$$ $Ratio$ - Lateral soil capacity $$Ratio = \frac{s}{p_s}$$ $$Ratio = \frac{\left(1.2169\ \mathrm{kip/ft^2}\right)}{\left(1.2375\ \mathrm{kip/ft^2}\right)}$$ $$Ratio = 0.98336$$	Status: **PASS** Ratio: **0.980**	

Considering z-direction:

$H_o = -0.0041401\ \mathrm{kip/ft}$ - Lateral force per length of pile,
$M_o = 0.022134\ \mathrm{kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (0.022134\ \mathrm{kipft/ft}) \times (8.25\ \mathrm{ft})) + \left(3 \times (-0.0041401\ \mathrm{kip/ft}) \times (8.25\ \mathrm{ft})^2\right)}{(6 \times (0.022134\ \mathrm{kipft/ft})) + (4 \times (-0.0041401\ \mathrm{kip/ft}) \times (8.25\ \mathrm{ft}))}$$

$$a = 5.8486\ \mathrm{ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\,[(4\,M_o) + (3\,H_o\,L_e)]^2}{L_e^2\,[(3\,M_o) + (2\,H_o\,L_e)]}$$

$$p = \frac{0.75 \times [(4 \times (0.022134\ \mathrm{kipft/ft})) + (3 \times (-0.0041401\ \mathrm{kip/ft}) \times (8.25\ \mathrm{ft}))]^2}{(8.25\ \mathrm{ft})^2 \times [(3 \times (0.022134\ \mathrm{kipft/ft})) + (2 \times (-0.0041401\ \mathrm{kip/ft}) \times (8.25\ \mathrm{ft}))]}$$

$$p = -0.0011195\ \mathrm{kip/ft^2}$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\,[(2\,M_o) + (H_o\,L_e)]}{L_e^2}$$

$$s = \frac{6 \times [(2 \times (0.022134\ \mathrm{kipft/ft})) + ((-0.0041401\ \mathrm{kip/ft}) \times (8.25\ \mathrm{ft}))]}{(8.25\ \mathrm{ft})^2}$$

$$s = 0.00089137\ \mathrm{kip/ft^2}$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\,\frac{a}{2}$$

$$p_a = (150\ \mathrm{psf/ft}) \times \frac{(5.8486\ \mathrm{ft})}{2}$$

$$p_a = 0.43865\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{\left(-0.0011195\ \mathrm{kip/ft^2}\right)}{\left(0.43865\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = -0.0025522$$

Status: **PASS**
Ratio: **0.000**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\,L_e$$

$$p_s = (150\ \mathrm{psf/ft}) \times (8.25\ \mathrm{ft})$$

$$p_s = 1.2375\ \mathrm{kip/ft^2}$$

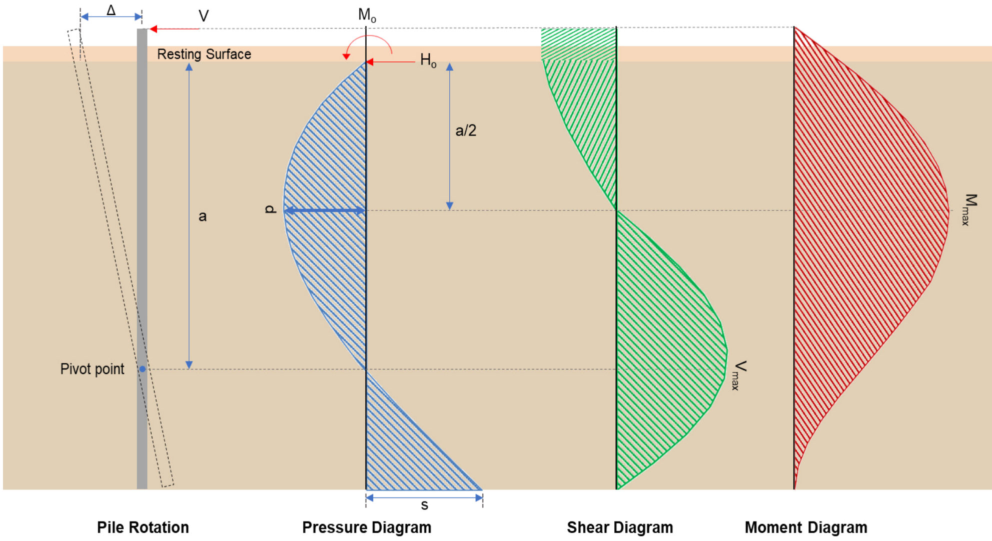
$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{(0.00089137\ \mathrm{kip/ft^2})}{(1.2375\ \mathrm{kip/ft^2})}$$

$$Ratio = 0.0007203$$

Status: **PASS**
Ratio: **0.000**



Shear force and Bending moment (x-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\,D}$$

$$H_o = \frac{(-5.173\ \mathrm{kip})}{1.57 \times (48\ \mathrm{in})}$$

$$H_o = -0.82373\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\,H)}{1.57\,D}$$

$$M_o = \frac{(96.375\ \mathrm{kipft}) + ((-5.173\ \mathrm{kip}) \times (0\ \mathrm{ft}))}{1.57 \times (48\ \mathrm{in})}$$

$$M_o = 15.346\ \mathrm{kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(15.346\ \mathrm{kipft/ft})}{(-0.82373\ \mathrm{kip/ft})}$$

$$E = 18.63\ \mathrm{ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (15.346\ \mathrm{kipft/ft}) \times (8.25\ \mathrm{ft})) + \left(3 \times (-0.82373\ \mathrm{kip/ft}) \times (8.25\ \mathrm{ft})^2\right)}{}$$

$$a = \frac{}{(6 \times (15.346\ \mathrm{kipft/ft})) + (4 \times (-0.82373\ \mathrm{kip/ft}) \times (8.25\ \mathrm{ft}))}$$

$$a = 5.6567\ \mathrm{ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\, D) \left[1 - \left[3 \left(\frac{4\,E}{L_e} + 3\right) \left(\frac{a}{L_e}\right)^2\right] + \left[4 \left(\frac{3\,E}{L_e} + 2\right) \left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((-0.82373\ \mathrm{kip/ft}) \times (48\ \mathrm{in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (18.63\ \mathrm{ft})}{(8.25\ \mathrm{ft})} + 3\right) \times \left(\frac{(5.6567\ \mathrm{ft})}{(8.25\ \mathrm{ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (18.63\ \mathrm{ft})}{(8.25\ \mathrm{ft})} + 2\right) \times \left(\frac{(5.6567\ \mathrm{ft})}{(8.25\ \mathrm{ft})}\right)^3\right]\right]$$

$$V_{max} = 15.345\ \mathrm{kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, D\, L_e) \left[\left(\frac{E}{L_e} + \frac{a}{2\,L_e}\right) - \left[\left(\frac{4\,E}{L_e} + 3\right) \left(\frac{a}{2\,L_e}\right)^3\right] + \left[\left(\frac{3\,E}{L_e} + 2\right) \left(\frac{a}{2\,L_e}\right)^4\right]\right]$$

$$M_{max} = ((-0.82373\ \mathrm{kip/ft}) \times (48\ \mathrm{in}) \times (8.25\ \mathrm{ft})) \times \left[\left(\frac{(18.63\ \mathrm{ft})}{(8.25\ \mathrm{ft})} + \frac{(5.6567\ \mathrm{ft})}{2 \times (8.25\ \mathrm{ft})}\right) - \left[\left(\frac{4 \times (18.63\ \mathrm{ft})}{(8.25\ \mathrm{ft})} + 3\right) \times \left(\frac{(5.6567\ \mathrm{ft})}{2 \times (8.25\ \mathrm{ft})}\right)^3\right] + \left[\left(\frac{3 \times (18.63\ \mathrm{ft})}{(8.25\ \mathrm{ft})} + 2\right) \times \left(\frac{(5.6567\ \mathrm{ft})}{2 \times (8.25\ \mathrm{ft})}\right)^4\right]\right]$$

$$M_{max} = 60.82\ \mathrm{kipft}$$

Shear force and Bending moment (z-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\,b}$$

$$H_o = \frac{(-0.039\ \mathrm{kip})}{1.57 \times (48\ \mathrm{in})}$$

$$H_o = -0.0062102\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\, H)}{1.57\,b}$$

$$M_o = \frac{(0.212\ \mathrm{kipft}) + ((-0.039\ \mathrm{kip}) \times (0\ \mathrm{ft}))}{1.57 \times (48\ \mathrm{in})}$$

$$M_o = 0.033758\ \mathrm{kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(0.033758\ \mathrm{kipft/ft})}{(-0.0062102\ \mathrm{kip/ft})}$$

$$E = 5.4359\ \mathrm{ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (0.033758\ \mathrm{kipft/ft}) \times (8.25\ \mathrm{ft})) + \left(3 \times (-0.0062102\ \mathrm{kip/ft}) \times (8.25\ \mathrm{ft})^2\right)}{(6 \times (0.033758\ \mathrm{kipft/ft})) + (4 \times (-0.0062102\ \mathrm{kip/ft}) \times (8.25\ \mathrm{ft}))}$$

$$a = 5.8458\ \mathrm{ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\, b) \left[1 - \left[3 \left(\frac{4\,E}{L_e} + 3\right) \left(\frac{a}{L_e}\right)^2\right] + \left[4 \left(\frac{3\,E}{L_e} + 2\right) \left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((-0.0062102\ \text{kip/ft}) \times (48\ \text{in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (5.4359\ \text{ft})}{(8.25\ \text{ft})} + 3\right) \times \left(\frac{(5.8458\ \text{ft})}{(8.25\ \text{ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (5.4359\ \text{ft})}{(8.25\ \text{ft})} + 2\right) \times \left(\frac{(5.8458\ \text{ft})}{(8.25\ \text{ft})}\right)^3\right]\right]$$

$$V_{max} = 0.045447\ \text{kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, b\, L_e)\left[\left(\frac{E}{L_e} + \frac{a}{2\, L_e}\right) - \left[\left(\frac{4\, E}{L_e} + 3\right)\left(\frac{a}{2\, L_e}\right)^3\right] + \left[\left(\frac{3\, E}{L_e} + 2\right)\left(\frac{a}{2\, L_e}\right)^4\right]\right]$$

$$M_{max} = ((-0.0062102\ \text{kip/ft}) \times (48\ \text{in}) \times (8.25\ \text{ft})) \times \left[\left(\frac{(5.4359\ \text{ft})}{(8.25\ \text{ft})} + \frac{(5.8458\ \text{ft})}{2 \times (8.25\ \text{ft})}\right) - \left[\left(\frac{4 \times (5.4359\ \text{ft})}{(8.25\ \text{ft})} + 3\right) \times \left(\frac{(5.8458\ \text{ft})}{2 \times (8.25\ \text{ft})}\right)^3\right] + \left[\left(\frac{3 \times (5.4359\ \text{ft})}{(8.25\ \text{ft})} + 2\right) \times \left(\frac{(5.8458\ \text{ft})}{2 \times (8.25\ \text{ft})}\right)^4\right]\right]$$

$$M_{max} = 0.16912\ \text{kipft}$$

Minimum Reinforcement Check (LRFD)

Parameters:

$f'_{ck} = 2.5\ \text{ksi}$ - Concrete strength,
$f_{yk} = 60\ \text{ksi}$ - Longitudinal reinforcement strength,
$\phi = 0.65$ - Reduction factor for axial strength,
Table 22.4.2.1 $\alpha = 0.8$ - Alpha factor for axial strength,
$A_g = 2304\ \text{in}^2$ - Gross area of concrete,

Longitudinal reinforcement:

Required reinforcement due to axial load, $A_{st,required}$
22.4.2.2, 10.6.1.1 $A_{st,required}$

$$A_{st,required} = Min\left[\frac{\frac{P}{\phi\, \alpha} - (0.85\, f'_{ck}\, A_g)}{f_{yk} - (0.85\, f'_{ck})}, (0.08\, A_g)\right]$$

$$A_{st,required} = Min\left[\frac{\frac{(15.113\ \text{kip})}{(0.65)\times(0.8)} - (0.85 \times (2.5\ \text{ksi}) \times (2304\ \text{in}^2))}{(60\ \text{ksi}) - (0.85 \times (2.5\ \text{ksi}))}, (0.08 \times (2304\ \text{in}^2))\right]$$

$$A_{st,required} = -84.094\ \text{in}^2$$

A_{min} - Governing minimum reinforcement area,

$$A_{min} = Max\left[A_{st,required}, (0.0018\, A_g)\right]$$

$$A_{min} = Max\left[(-84.094\ \text{in}^2), (0.0018 \times (2304\ \text{in}^2))\right]$$

$$A_{min} = 4.1472\ \text{in}^2$$

n_{rebar} - Required number of reinforcement,

$$n_{rebar} = \frac{A_{min}}{A_{rebar}}$$

$$n_{rebar} = \frac{(4.1472\ \text{in}^2)}{(0.3068\ \text{in}^2)}$$

$$n_{rebar} = 14$$

A_{st} - Actual total reinforcement area,

$$A_{st} = n_{rebar}\,\frac{\pi\, d_{bar}^2}{4}$$

$$A_{st} = (14) \times \frac{\pi \times (0.625\ \text{in})^2}{4}$$

$$A_{st} = 4.2951\ \text{in}^2$$

$Ratio$ - Capacity

$$Ratio = \frac{A_{min}}{A_{st}}$$

$$Ratio = \frac{(4.1472\ \text{in}^2)}{(4.2951\ \text{in}^2)}$$

$$Ratio = 0.96556$$

Status: PASS
Ratio: 0.970

25.2.3 s_{rebar} - Minimum spacing of reinforcement,

$$s_{rebar} = Max\left[1.5, \left(1.5\ d_{bar}\right)\right]$$

$$s_{rebar} = Max\left[1.5, \left(1.5 \times (0.625\ \text{in})\right)\right]$$

$$s_{rebar} = 1.5\ \text{in}$$

Ties:

25.7.2.2 Since longitudinal reinforcement is $\leq$ No. 10ø: Use #3(0.375 in)

25.7.2.1 s_{ties} - Maximum spacing of ties,

$$s_{ties} = Min\left[\left(16\ d_{bar}\right), \left(48\ d_{ties}\right), Min\left(D, b\right)\right]$$

$$s_{ties} = Min\left[\left(16 \times (0.625\ \text{in})\right), \left(48 \times (0.375\ \text{in})\right), Min\left((48\ \text{in}), (48\ \text{in})\right)\right]$$

$$s_{ties} = 10\ \text{in}$$

Summary:

Main reinforcement: **14 - #5 (0.625 in)**
Ties: **#3(0.375 in) - 10 in**

Axial Compression Strength (ACI 318-19, LRFD)

22.4.2.2 ϕP_N - Allowable axial compressive strength

$$\phi P_N = \phi\ 0.80\ \left[\left(0.85\ f'_{ck}\ \left[A_g - A_{st}\right]\right) + \left(f_{yk}\ A_{st}\right)\right]$$

$$\phi P_N = (0.65) \times 0.80 \times \left[\left(0.85 \times (2.5\ \text{ksi}) \times \left[\left(2304\ \text{in}^2\right) - \left(4.2951\ \text{in}^2\right)\right]\right) + \left((60\ \text{ksi}) \times \left(4.2951\ \text{in}^2\right)\right)\right]$$

$$\phi P_N = 2675.2\ \text{kip}$$

$Ratio$ - Capacity

$$Ratio = \frac{P}{\phi P_N}$$

$$Ratio = \frac{(15.113\ \text{kip})}{(2675.2\ \text{kip})}$$

$$Ratio = 0.0056493$$

Status: PASS
Ratio: 0.010

Shear Strength (ACI 318-19, LRFD)

Parameters:

$b_w = 48\ \text{in}$ - Effective width,

22.5.2.2 d - Effective depth

$$d = 0.80\ D$$

$$d = 0.80 \times (48\ \text{in})$$

$$d = 38.4\ \text{in}$$

22.5.5.1.3 λ_s - size effect modification factor

$$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{d}{10}}}, 1\right]$$

$$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{(38.4\ \text{in})}{10}}}, 1\right]$$

$$\lambda_s = 0.64282$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5\ \text{ksi} \rightarrow 2500\ \text{psi}$,

22.5.5.1.1 $V_{c,max}$ - Max shear strength of concrete

$$V_{c,max} = 5\ \lambda_s\ \sqrt{f'_{ck}}\ b_w\ d$$

$$V_{c,max} = 5 \times (0.64282) \times \sqrt{(2500\ \text{psi})} \times (48\ \text{in}) \times (38.4\ \text{in})$$

$$V_{c,max} = 296.21 \text{ kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$,$P = 15.113 \text{ kip} \rightarrow 15113 \text{ lbf}$,

22.5.5.1.1(a) $V_{c,a}$ - Shear strength of concrete (a)

$$V_{c,a} = \left[2\,\lambda_s\,\sqrt{f'_{ck}} + \frac{P}{6\,A_g}\right]\,b_w\,d$$

$$V_{c,a} = \left[2 \times (0.64282) \times \sqrt{(2500 \text{ psi})} + \frac{(15113 \text{ lbf})}{6 \times (2304 \text{ in}^2)}\right] \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{c,a} = 120.5 \text{ kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$,

22.5.5.1.2 $V_{c,b}$ - Shear strength of concrete (b)

$$V_{c,b} = \left[2\,\lambda_s\,\sqrt{f'_{ck}} + \left(0.05\,f'_{ck}\right)\right]\,b_w\,d$$

$$V_{c,b} = \left[2 \times (0.64282) \times \sqrt{(2500 \text{ psi})} + (0.05 \times (2500 \text{ psi}))\right] \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{c,b} = 348.89 \text{ kip}$$

V_c - Governing shear strength of concrete

$$V_c = Min\left[V_{c,max}, V_{c,a}, V_{c,b}\right]$$

$$V_c = Min\left[(296.21 \text{ kip}), (120.5 \text{ kip}), (348.89 \text{ kip})\right]$$

$$V_c = 120.5 \text{ kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$,

22.5.1.2 $V_{s,a}$ - Shear strength of steel (a)

$$V_{s,a} = 8\,\sqrt{f'_{ck}}\,b_w\,d$$

$$V_{s,a} = 8 \times \sqrt{(2500 \text{ psi})} \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{s,a} = 737.28 \text{ kip}$$

A_v - Ties rebar area,

$$A_v = \frac{\pi\,d_{ties}^2}{4}$$

$$A_v = \frac{\pi \times (0.375 \text{ in})^2}{4}$$

$$A_v = 0.11045 \text{ in}^2$$

22.5.8.5.3 $V_{s,b}$ - Shear strength of steel (b)

$$V_{s,b} = \frac{2\,A_v\,f_{ywk}\,d}{s_{ties}}$$

$$V_{s,b} = \frac{2 \times \left(0.11045 \text{ in}^2\right) \times (60 \text{ ksi}) \times (38.4 \text{ in})}{(10 \text{ in})}$$

$$V_{s,b} = 50.894 \text{ kip}$$

V_s - Governing shear strength of steel

$$V_s = MIN\left[V_{s,a}, V_{s,b}\right]$$

$$V_s = MIN\left[(737.28 \text{ kip}), (50.894 \text{ kip})\right]$$

$$V_s = 50.894 \text{ kip}$$

22.5.1.1 ϕV_n - Allowable shear strength

$$\phi V_n = \phi\,\left(V_c + V_s\right)$$

$$\phi V_n = (0.65) \times ((120.5 \text{ kip}) + (50.894 \text{ kip}))$$

$$\phi V_n = 111.41 \text{ kip}$$

Considering x-direction:

$V_{max} = 15.345 \text{ kip}$ - Maximum shear force in the x-direction,
$Ratio$ - Capacity

$$Ratio = \frac{V_{max}}{\phi V_n}$$

$$Ratio = \frac{(15.345 \text{ kip})}{(111.41 \text{ kip})}$$

$$Ratio = 0.13773$$

Status: **PASS**
Ratio: **0.140**

Considering z-direction:

$V_{max} = 0.045447 \text{ kip}$ - Maximum shear force in the z-direction,
$Ratio$ - Capacity

$$Ratio = \frac{V_{max}}{\phi V_n}$$

$$Ratio = \frac{(0.045447 \text{ kip})}{(111.41 \text{ kip})}$$

$$Ratio = 0.00040794$$

Status: **PASS**
Ratio: **0.000**

Flexural Strength (ACI 318-19, LRFD)

S_m - Section modulus

$$S_m = \frac{b\,D^2}{6}$$

$$S_m = \frac{(48 \text{ in}) \times (48 \text{ in})^2}{6}$$

$$S_m = 18432 \text{ in}^3$$

$\lambda = 1$ - Concrete modification factor (Normal concrete),
Allowable flexural strength:
M_n shall be the lesser of:
$\phi M_{n,1}$

$$\phi M_{n,1} = \phi \times 5 \times \lambda \times \sqrt{(f'_c)} \times S_m$$

$$\phi M_{n,1} = 0.65 \times 5 \times 1 \times \sqrt{(2.5ksi)} \times 18432.001in^3$$

$$\phi M_{n,1} = 249.600kipft$$

14.5.2.1b $\phi M_{n,2}$

$$\phi M_{n,2} = \phi\; 0.85\; f'_{ck}\; S_m$$

$$\phi M_{n,2} = (0.65) \times 0.85 \times (2.5 \text{ ksi}) \times \left(18432 \text{ in}^3\right)$$

$$\phi M_{n,2} = 2121.6 \text{ kipft}$$

Therefore,
ϕM_n - Allowable flexural strength,

$$\phi M_n = MIN[\phi M_{n,1}, \phi M_{n,2}]$$

$$\phi M_n = MIN[(249.6 \text{ kipft}), (2121.6 \text{ kipft})]$$

$$\phi M_n = 249.6 \text{ kipft}$$

Considering x-direction:

$M_{max} = 60.82 \text{ kipft}$ - Maximum moment in the x-direction,
$Ratio$ - Capacity

$$Ratio = \frac{M_{max}}{\phi M_n}$$

$$Ratio = \frac{(60.82 \text{ kipft})}{(249.6 \text{ kipft})}$$

$$Ratio = 0.24367$$

Status: **PASS**
Ratio: **0.240**

Considering z-direction:

$M_{max} = 0.16912 \text{ kipft}$ - Maximum moment in the z-direction,
$Ratio$ - Capacity

$$Ratio = \frac{M_{max}}{\phi M_n}$$

	$$Ratio = \frac{(0.16912 \text{ kipft})}{(249.6 \text{ kipft})}$$ $$Ratio = 0.00067756$$	Status: **PASS** Ratio: **0.000**

REFERENCES	CALCULATIONS	RESULTS

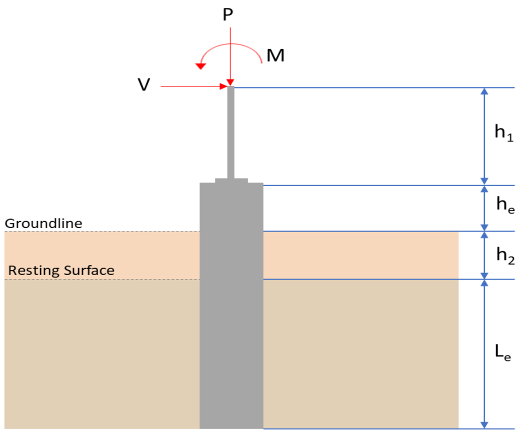
SkyCiv Foundation Design

Pile Foundation

Design Information :

Design code : IBC 2021 (International Building Code)
Unit System : Imperial

Pile Input



Geometry

Pile shape: rectangular
$b = 48\ \mathrm{in}$ - Pile width
$D = 48\ \mathrm{in}$ - Pile depth
$L = 8.5\ \mathrm{ft}$ - Total pile length
$h_1 = 0\ \mathrm{ft}$ - Lateral load height from the top of the pile,
$h_2 = 0\ \mathrm{ft}$ - Depth to resisting surface
$h_e = 0\ \mathrm{ft}$ - Length of pile above the ground

Tabulation of Soil Parameters

Layer	Label	Allowable Bearing Pressure (q_a) (psf)	Allowable Lateral Pressure (R) (psf/ft)
1	Sand, silty sand, clayey sand, silty gravel & clayey gravel	2000.000	150.000

Tabulation of Loads

Load Component	ASD	LRFD
P (kip)	11.293	17.146
V_x (kip)	-3.287	-5.480
V_z (kip)	-0.278	-0.419
M_x (kipft)	-1.529	-2.319
M_z (kipft)	59.709	103.237

Material Properties

$f'_{ck} = 2.5\ \mathrm{ksi}$ - Concrete strength,

Required depth to resist lateral loads (ASD)

H - Point of application of the lateral load

$$H = h_1 + h_2 + h_e$$

$$H = (0\ \mathrm{ft}) + (0\ \mathrm{ft}) + (0\ \mathrm{ft})$$

$$H = 0\ \mathrm{ft}$$

Considering x-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\, D}$$

$$H_o = \frac{(-3.287\ \mathrm{kip})}{1.57 \times (48\ \mathrm{in})}$$

$$H_o = -0.52341\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\ H)}{1.57\ D}$$

$$M_o = \frac{(59.709\ \text{kipft}) + ((-3.287\ \text{kip}) \times (0\ \text{ft}))}{1.57 \times (48\ \text{in})}$$

$$M_o = 9.5078\ \text{kipft/ft}$$

Required depth of embedment in earth:

$$L_x^3 - \left(14.14 \times \frac{H_o \times L_x}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,x} = 7.9883\ \text{ft}$ - Required depth in x-direction,

Considering z-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\ b}$$

$$H_o = \frac{(-0.278\ \text{kip})}{1.57 \times (48\ \text{in})}$$

$$H_o = -0.044268\ \text{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\ H)}{1.57\ b}$$

$$M_o = \frac{(1.529\ \text{kipft}) + ((-0.278\ \text{kip}) \times (0\ \text{ft}))}{1.57 \times (48\ \text{in})}$$

$$M_o = 0.24347\ \text{kipft/ft}$$

Required depth of embedment in earth:

$$L_z^3 - \left(14.14 \times \frac{H_o \times L_z}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,z} = 2.3633\ \text{ft}$ - Required depth in z-direction,

Minimum embedded depth required:

$L_{e,req}$ - Depth of pile required,

$$L_{e,req} = MAX[L_{e,x}, L_{e,z}]$$

$$L_{e,req} = MAX[(7.9883\ \text{ft}), (2.3633\ \text{ft})]$$

$$L_{e,req} = 7.988\ \text{ft}$$

L_e - Actual embedded length of pile,

$$L_e = L - h_e - h_2$$

$$L_e = (8.5\ \text{ft}) - (0\ \text{ft}) - (0\ \text{ft})$$

$$L_e = 8.5\ \text{ft}$$

$Ratio$ - Embedded depth

$$Ratio = \frac{L_{e,req}}{L_e}$$

$$Ratio = \frac{(7.988\ \text{ft})}{(8.5\ \text{ft})}$$

$$Ratio = 0.93976$$

Status: **PASS**
Ratio: **0.940**

End-bearing Capacity (ASD)

A - Pile cross-section area

$$A = b\ D$$

$$A = (48\ \text{in}) \times (48\ \text{in})$$

$$A = 16\ \text{ft}^2$$

q - End-bearing pressure

$$q = \frac{P_v}{A}$$

$$q = \frac{(11.293\ \mathrm{kip})}{(16\ \mathrm{ft}^2)}$$

$$q = 0.70581\ \mathrm{kip/ft^2}$$

Check bearing capacity ratio:

$Ratio$ - Capacity

$$Ratio = \frac{q}{q_a}$$

$$Ratio = \frac{\left(0.70581\ \mathrm{kip/ft^2}\right)}{(2000\ \mathrm{psf})}$$

$$Ratio = 0.35291$$

Status: **PASS**
Ratio: **0.350**

Czerniak

Lateral Soil Pressure (ASD):

L/D - Length to least lateral dimension ratio,

$$L/D = \frac{L}{D}$$

$$L/D = \frac{(8.5\ \mathrm{ft})}{(48\ \mathrm{in})}$$

$$L/D = 2.125$$

Since L/D $\leq$ 10,

Pile is short.

Considering x-direction:

$H_o = -0.52341\ \mathrm{kip/ft}$ - Lateral force per length of pile,
$M_o = 9.5078\ \mathrm{kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (9.5078\ \mathrm{kipft/ft}) \times (8.5\ \mathrm{ft})) + \left(3 \times (-0.52341\ \mathrm{kip/ft}) \times (8.5\ \mathrm{ft})^2\right)}{(6 \times (9.5078\ \mathrm{kipft/ft})) + (4 \times (-0.52341\ \mathrm{kip/ft}) \times (8.5\ \mathrm{ft}))}$$

$$a = 5.8351\ \mathrm{ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\,[(4\,M_o) + (3\,H_o\,L_e)]^2}{L_e^2\,[(3\,M_o) + (2\,H_o\,L_e)]}$$

$$p = \frac{0.75 \times [(4 \times (9.5078\ \mathrm{kipft/ft})) + (3 \times (-0.52341\ \mathrm{kip/ft}) \times (8.5\ \mathrm{ft}))]^{\ 2}}{(8.5\ \mathrm{ft})^2 \times [(3 \times (9.5078\ \mathrm{kipft/ft})) + (2 \times (-0.52341\ \mathrm{kip/ft}) \times (8.5\ \mathrm{ft}))]}$$

$$p = 0.32229\ \mathrm{kip/ft^2}$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\,[(2\,M_o) + (H_o\,L_e)]}{L_e^2}$$

$$s = \frac{6 \times [(2 \times (9.5078\ \mathrm{kipft/ft})) + ((-0.52341\ \mathrm{kip/ft}) \times (8.5\ \mathrm{ft}))]}{(8.5\ \mathrm{ft})^2}$$

$$s = 1.2097\ \mathrm{kip/ft^2}$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\,\frac{a}{2}$$

$$p_a = (150\ \mathrm{psf/ft}) \times \frac{(5.8351\ \mathrm{ft})}{2}$$

$$p_a = 0.43763\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{\left(0.32229\ \mathrm{kip/ft^2}\right)}{\left(0.43763\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = 0.73644$$

Status: **PASS**
Ratio: **0.740**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\,L_e$$

$$p_s = (150\ \mathrm{psf/ft}) \times (8.5\ \mathrm{ft})$$

$$p_s = 1.275\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{\left(1.2097\ \mathrm{kip/ft^2}\right)}{\left(1.275\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = 0.94877$$

Status: **PASS**
Ratio: **0.950**

Considering z-direction:

$H_o = -0.044268\ \mathrm{kip/ft}$ - Lateral force per length of pile,
$M_o = 0.24347\ \mathrm{kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (0.24347\ \mathrm{kipft/ft}) \times (8.5\ \mathrm{ft})) + \left(3 \times (-0.044268\ \mathrm{kip/ft}) \times (8.5\ \mathrm{ft})^2\right)}{(6 \times (0.24347\ \mathrm{kipft/ft})) + (4 \times (-0.044268\ \mathrm{kip/ft}) \times (8.5\ \mathrm{ft}))}$$

$$a = 6.0261\ \mathrm{ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\,[(4\,M_o) + (3\,H_o\,L_e)]^2}{L_e^2\,[(3\,M_o) + (2\,H_o\,L_e)]}$$

$$p = \frac{0.75 \times [(4 \times (0.24347\ \mathrm{kipft/ft})) + (3 \times (-0.044268\ \mathrm{kip/ft}) \times (8.5\ \mathrm{ft}))]^{2}}{(8.5\ \mathrm{ft})^2 \times [(3 \times (0.24347\ \mathrm{kipft/ft})) + (2 \times (-0.044268\ \mathrm{kip/ft}) \times (8.5\ \mathrm{ft}))]}$$

$$p = -0.011258\ \mathrm{kip/ft^2}$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\,[(2\,M_o) + (H_o\,L_e)]}{L_e^2}$$

$$s = \frac{6 \times [(2 \times (0.24347\ \mathrm{kipft/ft})) + ((-0.044268\ \mathrm{kip/ft}) \times (8.5\ \mathrm{ft}))]}{(8.5\ \mathrm{ft})^2}$$

$$s = 0.0091905\ \mathrm{kip/ft^2}$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\,\frac{a}{2}$$

$$p_a = (150\ \mathrm{psf/ft}) \times \frac{(6.0261\ \mathrm{ft})}{2}$$

$$p_a = 0.45196\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{\left(-0.011258\ \mathrm{kip/ft^2}\right)}{\left(0.45196\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = -0.02491$$

Status: **PASS**
Ratio: **-0.020**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\,L_e$$

$$p_s = (150\ \mathrm{psf/ft}) \times (8.5\ \mathrm{ft})$$

$$p_s = 1.275\ \mathrm{kip/ft^2}$$

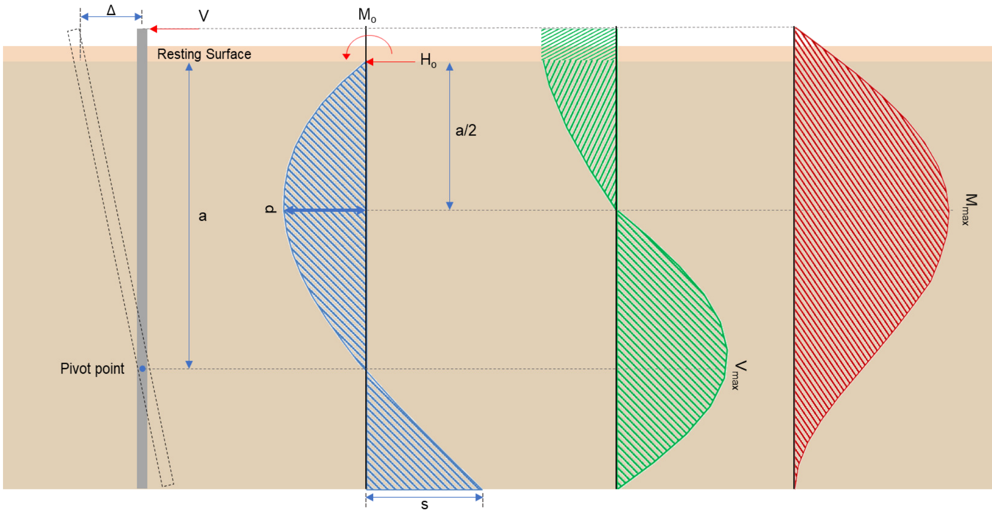
$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{(0.0091905\ \mathrm{kip/ft^2})}{(1.275\ \mathrm{kip/ft^2})}$$

$$Ratio = 0.0072082$$

Status: **PASS**
Ratio: **0.010**



Pile Rotation | Pressure Diagram | Shear Diagram | Moment Diagram

Shear force and Bending moment (x-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\,D}$$

$$H_o = \frac{(-5.48\ \mathrm{kip})}{1.57 \times (48\ \mathrm{in})}$$

$$H_o = -0.87261\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\,H)}{1.57\,D}$$

$$M_o = \frac{(103.24\ \mathrm{kipft}) + ((-5.48\ \mathrm{kip}) \times (0\ \mathrm{ft}))}{1.57 \times (48\ \mathrm{in})}$$

$$M_o = 16.439\ \mathrm{kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(16.439\ \mathrm{kipft/ft})}{(-0.87261\ \mathrm{kip/ft})}$$

$$E = 18.839\ \mathrm{ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (16.439\ \mathrm{kipft/ft}) \times (8.5\ \mathrm{ft})) + \left(3 \times (-0.87261\ \mathrm{kip/ft}) \times (8.5\ \mathrm{ft})^2\right)}{\ldots}$$

$$a = \frac{\qquad}{(6 \times (16.439\ \text{kipft/ft})) + (4 \times (-0.87261\ \text{kip/ft}) \times (8.5\ \text{ft}))}$$

$$a = 5.8305\ \text{ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\ D)\left[1 - \left[3\left(\frac{4\,E}{L_e} + 3\right)\left(\frac{a}{L_e}\right)^2\right] + \left[4\left(\frac{3\,E}{L_e} + 2\right)\left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((-0.87261\ \text{kip/ft}) \times (48\ \text{in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (18.839\ \text{ft})}{(8.5\ \text{ft})} + 3\right) \times \left(\frac{(5.8305\ \text{ft})}{(8.5\ \text{ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (18.839\ \text{ft})}{(8.5\ \text{ft})} + 2\right) \times \left(\frac{(5.8305\ \text{ft})}{(8.5\ \text{ft})}\right)^3\right]\right]$$

$$V_{max} = 15.996\ \text{kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\ D\,L_e)\left[\left(\frac{E}{L_e} + \frac{a}{2\,L_e}\right) - \left[\left(\frac{4\,E}{L_e} + 3\right)\left(\frac{a}{2\,L_e}\right)^3\right] + \left[\left(\frac{3\,E}{L_e} + 2\right)\left(\frac{a}{2\,L_e}\right)^4\right]\right]$$

$$M_{max} = ((-0.87261\ \text{kip/ft}) \times (48\ \text{in}) \times (8.5\ \text{ft})) \times \left[\left(\frac{(18.839\ \text{ft})}{(8.5\ \text{ft})} + \frac{(5.8305\ \text{ft})}{2 \times (8.5\ \text{ft})}\right) - \left[\left(\frac{4 \times (18.839\ \text{ft})}{(8.5\ \text{ft})} + 3\right) \times \left(\frac{(5.8305\ \text{ft})}{2 \times (8.5\ \text{ft})}\right)^3\right] + \left[\left(\frac{3 \times (18.839\ \text{ft})}{(8.5\ \text{ft})} + 2\right) \times \left(\frac{(5.8305\ \text{ft})}{2 \times (8.5\ \text{ft})}\right)^4\right]\right]$$

$$M_{max} = 65.28\ \text{kipft}$$

Shear force and Bending moment (z-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\,b}$$

$$H_o = \frac{(-0.419\ \text{kip})}{1.57 \times (48\ \text{in})}$$

$$H_o = -0.06672\ \text{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\ H)}{1.57\,b}$$

$$M_o = \frac{(2.319\ \text{kipft}) + ((-0.419\ \text{kip}) \times (0\ \text{ft}))}{1.57 \times (48\ \text{in})}$$

$$M_o = 0.36927\ \text{kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(0.36927\ \text{kipft/ft})}{(-0.06672\ \text{kip/ft})}$$

$$E = 5.5346\ \text{ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (0.36927\ \text{kipft/ft}) \times (8.5\ \text{ft})) + \left(3 \times (-0.06672\ \text{kip/ft}) \times (8.5\ \text{ft})^2\right)}{(6 \times (0.36927\ \text{kipft/ft})) + (4 \times (-0.06672\ \text{kip/ft}) \times (8.5\ \text{ft}))}$$

$$a = 6.025\ \text{ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\ b)\left[1 - \left[3\left(\frac{4\,E}{L_e} + 3\right)\left(\frac{a}{L_e}\right)^2\right] + \left[4\left(\frac{3\,E}{L_e} + 2\right)\left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((-0.06672\ \text{kip/ft}) \times (48\ \text{in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (5.5346\ \text{ft})}{(8.5\ \text{ft})} + 3\right) \times \left(\frac{(6.025\ \text{ft})}{(8.5\ \text{ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (5.5346\ \text{ft})}{(8.5\ \text{ft})} + 2\right) \times \left(\frac{(6.025\ \text{ft})}{(8.5\ \text{ft})}\right)^3\right]\right]$$

$$V_{max} = 0.48462\ \text{kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, b\, L_e) \left[\left(\frac{E}{L_e} + \frac{a}{2\, L_e}\right) - \left[\left(\frac{4\, E}{L_e} + 3\right) \left(\frac{a}{2\, L_e}\right)^3\right] + \left[\left(\frac{3\, E}{L_e} + 2\right) \left(\frac{a}{2\, L_e}\right)^4\right]\right]$$

$$M_{max} = ((-0.06672\ \text{kip/ft}) \times (48\ \text{in}) \times (8.5\ \text{ft})) \times \left[\left(\frac{(5.5346\ \text{ft})}{(8.5\ \text{ft})} + \frac{(6.025\ \text{ft})}{2 \times (8.5\ \text{ft})}\right) - \left[\left(\frac{4 \times (5.5346\ \text{ft})}{(8.5\ \text{ft})} + 3\right) \times \left(\frac{(6.025\ \text{ft})}{2 \times (8.5\ \text{ft})}\right)^3\right] + \left[\left(\frac{3 \times (5.5346\ \text{ft})}{(8.5\ \text{ft})} + 2\right) \times \left(\frac{(6.025\ \text{ft})}{2 \times (8.5\ \text{ft})}\right)^4\right]\right]$$

$$M_{max} = 1.8566\ \text{kipft}$$

Minimum Reinforcement Check (LRFD)

Parameters:

$f'_{ck} = 2.5\ \text{ksi}$ - Concrete strength,
$f_{yk} = 60\ \text{ksi}$ - Longitudinal reinforcement strength,
$\phi = 0.65$ - Reduction factor for axial strength,
Table 22.4.2.1 $\alpha = 0.8$ - Alpha factor for axial strength,
$A_g = 2304\ \text{in}^2$ - Gross area of concrete,

Longitudinal reinforcement:

Required reinforcement due to axial load, $A_{st,required}$

22.4.2.2, 10.6.1.1 $A_{st,required}$

$$A_{st,required} = Min\left[\frac{\frac{P}{\phi\, \alpha} - (0.85\, f'_{ck}\, A_g)}{f_{yk} - (0.85\, f'_{ck})}, (0.08\, A_g)\right]$$

$$A_{st,required} = Min\left[\frac{\frac{(17.146\ \text{kip})}{(0.65) \times (0.8)} - \left(0.85 \times (2.5\ \text{ksi}) \times \left(2304\ \text{in}^2\right)\right)}{(60\ \text{ksi}) - (0.85 \times (2.5\ \text{ksi}))}, \left(0.08 \times \left(2304\ \text{in}^2\right)\right)\right]$$

$$A_{st,required} = -84.026\ \text{in}^2$$

A_{min} - Governing minimum reinforcement area,

$$A_{min} = Max\left[A_{st,required}, (0.0018\, A_g)\right]$$

$$A_{min} = Max\left[\left(-84.026\ \text{in}^2\right), \left(0.0018 \times \left(2304\ \text{in}^2\right)\right)\right]$$

$$A_{min} = 4.1472\ \text{in}^2$$

n_{rebar} - Required number of reinforcement,

$$n_{rebar} = \frac{A_{min}}{A_{rebar}}$$

$$n_{rebar} = \frac{\left(4.1472\ \text{in}^2\right)}{\left(0.3068\ \text{in}^2\right)}$$

$$n_{rebar} = 14$$

A_{st} - Actual total reinforcement area,

$$A_{st} = n_{rebar}\, \frac{\pi\, d_{bar}^2}{4}$$

$$A_{st} = (14) \times \frac{\pi \times (0.625\ \text{in})^2}{4}$$

$$A_{st} = 4.2951\ \text{in}^2$$

$Ratio$ - Capacity

$$Ratio = \frac{A_{min}}{A_{st}}$$

$$Ratio = \frac{\left(4.1472\ \text{in}^2\right)}{\left(4.2951\ \text{in}^2\right)}$$

$$Ratio = 0.96556$$

Status: PASS
Ratio: 0.970

25.2.3 s_{rebar} - Minimum spacing of reinforcement,

$$s_{rebar} = Max\left[1.5, \left(1.5\, d_{bar}\right)\right]$$

$$s_{rebar} = Max\left[1.5, \left(1.5 \times (0.625\text{ in})\right)\right]$$

$$s_{rebar} = 1.5\text{ in}$$

Ties:

25.7.2.2 Since longitudinal reinforcement is $\leq$ No. 10⌀: Use #3(0.375 in)
25.7.2.1 s_{ties} - Maximum spacing of ties,

$$s_{ties} = Min\left[\left(16\, d_{bar}\right), \left(48\, d_{ties}\right), Min\left(D, b\right)\right]$$

$$s_{ties} = Min\left[\left(16 \times (0.625\text{ in})\right), \left(48 \times (0.375\text{ in})\right), Min\left((48\text{ in}), (48\text{ in})\right)\right]$$

$$s_{ties} = 10\text{ in}$$

Summary:

Main reinforcement: **14 - #5 (0.625 in)**
Ties: **#3(0.375 in) - 10 in**

Axial Compression Strength (ACI 318-19, LRFD)

22.4.2.2 ϕP_N - Allowable axial compressive strength

$$\phi P_N = \phi\, 0.80\, \left[\left(0.85\, f'_{ck}\, \left[A_g - A_{st}\right]\right) + \left(f_{yk}\, A_{st}\right)\right]$$

$$\phi P_N = (0.65) \times 0.80 \times \left[\left(0.85 \times (2.5\text{ ksi}) \times \left[\left(2304\text{ in}^2\right) - \left(4.2951\text{ in}^2\right)\right]\right) + \left((60\text{ ksi}) \times \left(4.2951\text{ in}^2\right)\right)\right]$$

$$\phi P_N = 2675.2\text{ kip}$$

$Ratio$ - Capacity

$$Ratio = \frac{P}{\phi P_N}$$

$$Ratio = \frac{(17.146\text{ kip})}{(2675.2\text{ kip})}$$

$$Ratio = 0.0064093$$

Status: PASS
Ratio: 0.010

Shear Strength (ACI 318-19, LRFD)

Parameters:

$b_w = 48\text{ in}$ - Effective width,
22.5.2.2 d - Effective depth

$$d = 0.80\, D$$

$$d = 0.80 \times (48\text{ in})$$

$$d = 38.4\text{ in}$$

22.5.5.1.3 λ_s - size effect modification factor

$$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{d}{10}}}, 1\right]$$

$$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{(38.4\text{ in})}{10}}}, 1\right]$$

$$\lambda_s = 0.64282$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5\text{ ksi} \rightarrow 2500\text{ psi}$,
22.5.5.1.1 $V_{c,max}$ - Max shear strength of concrete

$$V_{c,max} = 5\, \lambda_s\, \sqrt{f'_{ck}}\, b_w\, d$$

$$V_{c,max} = 5 \times (0.64282) \times \sqrt{(2500\text{ psi})} \times (48\text{ in}) \times (38.4\text{ in})$$

$$V_{c,max} = 296.21 \text{ kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$, $P = 17.146 \text{ kip} \rightarrow 17146 \text{ lbf}$,

22.5.5.1.1(a) $V_{c,a}$ - Shear strength of concrete (a)

$$V_{c,a} = \left[2\,\lambda_s\,\sqrt{f'_{ck}} + \frac{P}{6\,A_g}\right]\,b_w\,d$$

$$V_{c,a} = \left[2 \times (0.64282) \times \sqrt{(2500 \text{ psi})} + \frac{(17146 \text{ lbf})}{6 \times (2304 \text{ in}^2)}\right] \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{c,a} = 120.77 \text{ kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$,

22.5.5.1.2 $V_{c,b}$ - Shear strength of concrete (b)

$$V_{c,b} = \left[2\,\lambda_s\,\sqrt{f'_{ck}} + \left(0.05\,f'_{ck}\right)\right]\,b_w\,d$$

$$V_{c,b} = \left[2 \times (0.64282) \times \sqrt{(2500 \text{ psi})} + (0.05 \times (2500 \text{ psi}))\right] \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{c,b} = 348.89 \text{ kip}$$

V_c - Governing shear strength of concrete

$$V_c = Min\left[V_{c,max}, V_{c,a}, V_{c,b}\right]$$

$$V_c = Min\left[(296.21 \text{ kip}), (120.77 \text{ kip}), (348.89 \text{ kip})\right]$$

$$V_c = 120.77 \text{ kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$,

22.5.1.2 $V_{s,a}$ - Shear strength of steel (a)

$$V_{s,a} = 8\,\sqrt{f'_{ck}}\,b_w\,d$$

$$V_{s,a} = 8 \times \sqrt{(2500 \text{ psi})} \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{s,a} = 737.28 \text{ kip}$$

A_v - Ties rebar area,

$$A_v = \frac{\pi\,d_{ties}^2}{4}$$

$$A_v = \frac{\pi \times (0.375 \text{ in})^2}{4}$$

$$A_v = 0.11045 \text{ in}^2$$

22.5.8.5.3 $V_{s,b}$ - Shear strength of steel (b)

$$V_{s,b} = \frac{2\,A_v\,f_{ywk}\,d}{s_{ties}}$$

$$V_{s,b} = \frac{2 \times \left(0.11045 \text{ in}^2\right) \times (60 \text{ ksi}) \times (38.4 \text{ in})}{(10 \text{ in})}$$

$$V_{s,b} = 50.894 \text{ kip}$$

V_s - Governing shear strength of steel

$$V_s = MIN\left[V_{s,a}, V_{s,b}\right]$$

$$V_s = MIN\left[(737.28 \text{ kip}), (50.894 \text{ kip})\right]$$

$$V_s = 50.894 \text{ kip}$$

22.5.1.1 ϕV_n - Allowable shear strength

$$\phi V_n = \phi\,\left(V_c + V_s\right)$$

$$\phi V_n = (0.65) \times ((120.77 \text{ kip}) + (50.894 \text{ kip}))$$

$$\phi V_n = 111.58 \text{ kip}$$

Considering x-direction:

$V_{max} = 15.996 \text{ kip}$ - Maximum shear force in the x-direction,
$Ratio$ - Capacity

$$Ratio = \frac{V_{max}}{\phi V_n}$$

	$$Ratio = \frac{(15.996 \text{ kip})}{(111.58 \text{ kip})}$$ $$Ratio = 0.14335$$	Status: **PASS** Ratio: **0.140**
	Considering z-direction: $V_{max} = 0.48462 \text{ kip}$ - Maximum shear force in the z-direction, $Ratio$ - Capacity $$Ratio = \frac{V_{max}}{\phi V_n}$$ $$Ratio = \frac{(0.48462 \text{ kip})}{(111.58 \text{ kip})}$$ $$Ratio = 0.0043432$$	Status: **PASS** Ratio: **0.000**
14.5.2.1b	**Flexural Strength (ACI 318-19, LRFD)** S_m - Section modulus $$S_m = \frac{b\, D^2}{6}$$ $$S_m = \frac{(48 \text{ in}) \times (48 \text{ in})^2}{6}$$ $$S_m = 18432 \text{ in}^3$$ $\lambda = 1$ - Concrete modification factor (Normal concrete), Allowable flexural strength: M_n shall be the lesser of: $\phi M_{n,1}$ $$\phi M_{n,1} = \phi \times 5 \times \lambda \times \sqrt{(f_c')} \times S_m$$ $$\phi M_{n,1} = 0.65 \times 5 \times 1 \times \sqrt{(2.5ksi)} \times 18432.001in^3$$ $$\phi M_{n,1} = 249.600kipft$$ $\phi M_{n,2}$ $$\phi M_{n,2} = \phi\, 0.85\, f_{ck}'\, S_m$$ $$\phi M_{n,2} = (0.65) \times 0.85 \times (2.5 \text{ ksi}) \times \left(18432 \text{ in}^3\right)$$ $$\phi M_{n,2} = 2121.6 \text{ kipft}$$ Therefore, ϕM_n - Allowable flexural strength, $$\phi M_n = MIN[\phi M_{n,1}, \phi M_{n,2}]$$ $$\phi M_n = MIN[(249.6 \text{ kipft}), (2121.6 \text{ kipft})]$$ $$\phi M_n = 249.6 \text{ kipft}$$ **Considering x-direction:** $M_{max} = 65.28 \text{ kipft}$ - Maximum moment in the x-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\phi M_n}$$ $$Ratio = \frac{(65.28 \text{ kipft})}{(249.6 \text{ kipft})}$$ $$Ratio = 0.26154$$	Status: **PASS** Ratio: **0.260**
	Considering z-direction: $M_{max} = 1.8566 \text{ kipft}$ - Maximum moment in the z-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\phi M_n}$$	

	$$Ratio = \frac{(1.8566\ \text{kipft})}{(249.6\ \text{kipft})}$$ $$Ratio = 0.0074381$$	Status: **PASS** Ratio: **0.010**

REFERENCES	CALCULATIONS	RESULTS

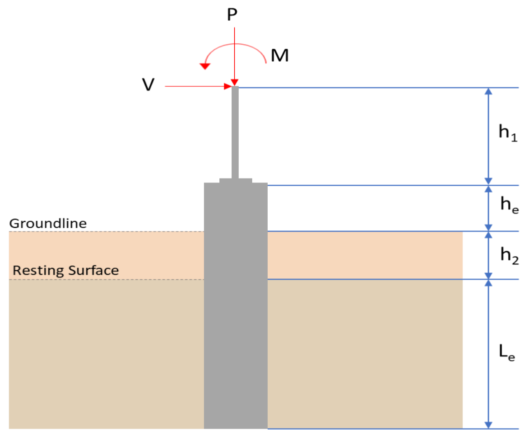
SkyCiv Foundation Design

Pile Foundation

Design Information :

Design code : IBC 2021 (International Building Code)
Unit System : Imperial

Pile Input



Geometry

Pile shape: rectangular
$b = 48\ \mathrm{in}$ - Pile width
$D = 48\ \mathrm{in}$ - Pile depth
$L = 8.5\ \mathrm{ft}$ - Total pile length
$h_1 = 0\ \mathrm{ft}$ - Lateral load height from the top of the pile,
$h_2 = 0\ \mathrm{ft}$ - Depth to resisting surface
$h_e = 0\ \mathrm{ft}$ - Length of pile above the ground

Tabulation of Soil Parameters

Layer	Label	Allowable Bearing Pressure (q_a) (psf)	Allowable Lateral Pressure (R) (psf/ft)
1	Sand, silty sand, clayey sand, silty gravel & clayey gravel	2000.000	150.000

Tabulation of Loads

Load Component	ASD	LRFD
P (kip)	11.293	17.146
V_x (kip)	-3.287	-5.480
V_z (kip)	0.278	0.419
M_x (kipft)	1.529	2.319
M_z (kipft)	59.709	103.237

Material Properties

$f'_{ck} = 2.5\ \mathrm{ksi}$ - Concrete strength,

Required depth to resist lateral loads (ASD)

H - Point of application of the lateral load

$$H = h_1 + h_2 + h_e$$

$$H = (0\ \mathrm{ft}) + (0\ \mathrm{ft}) + (0\ \mathrm{ft})$$

$$H = 0\ \mathrm{ft}$$

Considering x-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\ D}$$

$$H_o = \frac{(-3.287\ \mathrm{kip})}{1.57 \times (48\ \mathrm{in})}$$

$$H_o = -0.52341\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\ H)}{1.57\ D}$$

$$M_o = \frac{(59.709\ \text{kipft}) + ((-3.287\ \text{kip}) \times (0\ \text{ft}))}{1.57 \times (48\ \text{in})}$$

$$M_o = 9.5078\ \text{kipft/ft}$$

Required depth of embedment in earth:

$$L_x^3 - \left(14.14 \times \frac{H_o \times L_x}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,x} = 7.9883\ \text{ft}$ - Required depth in x-direction,

Considering z-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\ b}$$

$$H_o = \frac{(0.278\ \text{kip})}{1.57 \times (48\ \text{in})}$$

$$H_o = 0.044268\ \text{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\ H)}{1.57\ b}$$

$$M_o = \frac{(1.529\ \text{kipft}) + ((0.278\ \text{kip}) \times (0\ \text{ft}))}{1.57 \times (48\ \text{in})}$$

$$M_o = 0.24347\ \text{kipft/ft}$$

Required depth of embedment in earth:

$$L_z^3 - \left(14.14 \times \frac{H_o \times L_z}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,z} = 3.0182\ \text{ft}$ - Required depth in z-direction,

Minimum embedded depth required:

$L_{e,req}$ - Depth of pile required,

$$L_{e,req} = MAX[L_{e,x}, L_{e,z}]$$

$$L_{e,req} = MAX[(7.9883\ \text{ft}), (3.0182\ \text{ft})]$$

$$L_{e,req} = 7.988\ \text{ft}$$

L_e - Actual embedded length of pile,

$$L_e = L - h_e - h_2$$

$$L_e = (8.5\ \text{ft}) - (0\ \text{ft}) - (0\ \text{ft})$$

$$L_e = 8.5\ \text{ft}$$

$Ratio$ - Embedded depth

$$Ratio = \frac{L_{e,req}}{L_e}$$

$$Ratio = \frac{(7.988\ \text{ft})}{(8.5\ \text{ft})}$$

$$Ratio = 0.93976$$

Status: **PASS**
Ratio: **0.940**

End-bearing Capacity (ASD)

A - Pile cross-section area

$$A = b\ D$$

$$A = (48\ \text{in}) \times (48\ \text{in})$$

$$A = 16\ \text{ft}^2$$

q - End-bearing pressure

$$q = \frac{P_v}{A}$$

$$q = \frac{(11.293 \text{ kip})}{(16 \text{ ft}^2)}$$

$$q = 0.70581 \text{ kip/ft}^2$$

Check bearing capacity ratio:

$Ratio$ - Capacity

$$Ratio = \frac{q}{q_a}$$

$$Ratio = \frac{\left(0.70581 \text{ kip/ft}^2\right)}{(2000 \text{ psf})}$$

$$Ratio = 0.35291$$

Status: **PASS**
Ratio: **0.350**

Czerniak

Lateral Soil Pressure (ASD):

L/D - Length to least lateral dimension ratio,

$$L/D = \frac{L}{D}$$

$$L/D = \frac{(8.5 \text{ ft})}{(48 \text{ in})}$$

$$L/D = 2.125$$

Since L/D $\leq$ 10,

Pile is short.

Considering x-direction:

$H_o = -0.52341 \text{ kip/ft}$ - Lateral force per length of pile,
$M_o = 9.5078 \text{ kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\, M_o\, L_e) + \left(3\, H_o\, L_e^2\right)}{(6\, M_o) + (4\, H_o\, L_e)}$$

$$a = \frac{(4 \times (9.5078 \text{ kipft/ft}) \times (8.5 \text{ ft})) + \left(3 \times (-0.52341 \text{ kip/ft}) \times (8.5 \text{ ft})^2\right)}{(6 \times (9.5078 \text{ kipft/ft})) + (4 \times (-0.52341 \text{ kip/ft}) \times (8.5 \text{ ft}))}$$

$$a = 5.8351 \text{ ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\, [(4\, M_o) + (3\, H_o\, L_e)]^2}{L_e^2\, [(3\, M_o) + (2\, H_o\, L_e)]}$$

$$p = \frac{0.75 \times [(4 \times (9.5078 \text{ kipft/ft})) + (3 \times (-0.52341 \text{ kip/ft}) \times (8.5 \text{ ft}))]^2}{(8.5 \text{ ft})^2 \times [(3 \times (9.5078 \text{ kipft/ft})) + (2 \times (-0.52341 \text{ kip/ft}) \times (8.5 \text{ ft}))]}$$

$$p = 0.32229 \text{ kip/ft}^2$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\, [(2\, M_o) + (H_o\, L_e)]}{L_e^2}$$

$$s = \frac{6 \times [(2 \times (9.5078 \text{ kipft/ft})) + ((-0.52341 \text{ kip/ft}) \times (8.5 \text{ ft}))]}{(8.5 \text{ ft})^2}$$

$$s = 1.2097 \text{ kip/ft}^2$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\, \frac{a}{2}$$

$$p_a = (150 \text{ psf/ft}) \times \frac{(5.8351 \text{ ft})}{2}$$

$$p_a = 0.43763 \text{ kip/ft}^2$$

$Ratio$ - Lateral soil capacity

	$$Ratio = \frac{p}{p_a}$$ $$Ratio = \frac{\left(0.32229 \text{ kip/ft}^2\right)}{\left(0.43763 \text{ kip/ft}^2\right)}$$ $$Ratio = 0.73644$$	Status: **PASS** Ratio: **0.740**
	p_s - Allowable lateral soil pressure at depth L_e, $$p_s = R\, L_e$$ $$p_s = (150 \text{ psf/ft}) \times (8.5 \text{ ft})$$ $$p_s = 1.275 \text{ kip/ft}^2$$ $Ratio$ - Lateral soil capacity $$Ratio = \frac{s}{p_s}$$ $$Ratio = \frac{\left(1.2097 \text{ kip/ft}^2\right)}{\left(1.275 \text{ kip/ft}^2\right)}$$ $$Ratio = 0.94877$$	Status: **PASS** Ratio: **0.950**
	Considering z-direction: $H_o = 0.044268 \text{ kip/ft}$ - Lateral force per length of pile, $M_o = 0.24347 \text{ kipft/ft}$ - Overturning moment per length of pile, a - Distance from resting surface to pivot point, $$a = \frac{(4\, M_o\, L_e) + \left(3\, H_o\, L_e^2\right)}{(6\, M_o) + (4\, H_o\, L_e)}$$ $$a = \frac{(4 \times (0.24347 \text{ kipft/ft}) \times (8.5 \text{ ft})) + \left(3 \times (0.044268 \text{ kip/ft}) \times (8.5 \text{ ft})^2\right)}{(6 \times (0.24347 \text{ kipft/ft})) + (4 \times (0.044268 \text{ kip/ft}) \times (8.5 \text{ ft}))}$$ $$a = 6.0261 \text{ ft}$$ p - Earth pressure against the pile at distance a/2 from resting surface, $$p = \frac{0.75\, [(4\, M_o) + (3\, H_o\, L_e)]^2}{L_e^2\, [(3\, M_o) + (2\, H_o\, L_e)]}$$ $$p = \frac{0.75 \times [(4 \times (0.24347 \text{ kipft/ft})) + (3 \times (0.044268 \text{ kip/ft}) \times (8.5 \text{ ft}))]^2}{(8.5 \text{ ft})^2 \times [(3 \times (0.24347 \text{ kipft/ft})) + (2 \times (0.044268 \text{ kip/ft}) \times (8.5 \text{ ft}))]}$$ $$p = 0.030949 \text{ kip/ft}^2$$ s - Earth pressure against the pile at distance L_e, $$s = \frac{6\, [(2\, M_o) + (H_o\, L_e)]}{L_e^2}$$ $$s = \frac{6 \times [(2 \times (0.24347 \text{ kipft/ft})) + ((0.044268 \text{ kip/ft}) \times (8.5 \text{ ft}))]}{(8.5 \text{ ft})^2}$$ $$s = 0.071686 \text{ kip/ft}^2$$ **Check lateral soil pressure capacity:** p_a - Allowable lateral soil pressure at depth a/2, $$p_a = R\, \frac{a}{2}$$ $$p_a = (150 \text{ psf/ft}) \times \frac{(6.0261 \text{ ft})}{2}$$ $$p_a = 0.45196 \text{ kip/ft}^2$$ $Ratio$ - Lateral soil capacity $$Ratio = \frac{p}{p_a}$$ $$Ratio = \frac{\left(0.030949 \text{ kip/ft}^2\right)}{\left(0.45196 \text{ kip/ft}^2\right)}$$	

$$Ratio = 0.068478$$

Status: **PASS**
Ratio: **0.070**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\,L_e$$

$$p_s = (150\ \mathrm{psf/ft}) \times (8.5\ \mathrm{ft})$$

$$p_s = 1.275\ \mathrm{kip/ft^2}$$

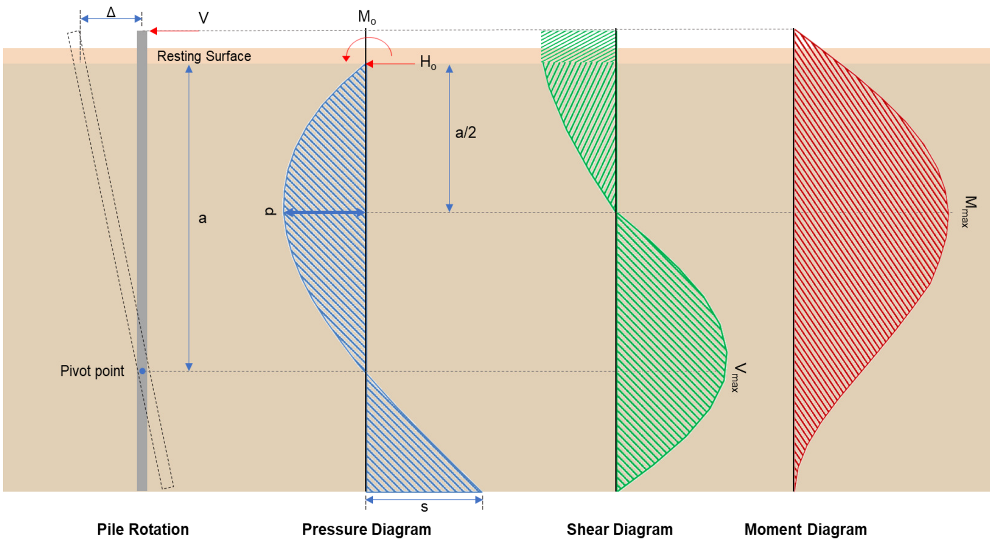
$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{\left(0.071686\ \mathrm{kip/ft^2}\right)}{\left(1.275\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = 0.056224$$

Status: **PASS**
Ratio: **0.060**



Shear force and Bending moment (x-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\,D}$$

$$H_o = \frac{(-5.48\ \mathrm{kip})}{1.57 \times (48\ \mathrm{in})}$$

$$H_o = -0.87261\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\,H)}{1.57\,D}$$

$$M_o = \frac{(103.24\ \mathrm{kipft}) + ((-5.48\ \mathrm{kip}) \times (0\ \mathrm{ft}))}{1.57 \times (48\ \mathrm{in})}$$

$$M_o = 16.439\ \mathrm{kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(16.439\ \mathrm{kipft/ft})}{(-0.87261\ \mathrm{kip/ft})}$$

$$E = 18.839\ \mathrm{ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (16.439\ \mathrm{kipft/ft}) \times (8.5\ \mathrm{ft})) + \left(3 \times (-0.87261\ \mathrm{kip/ft}) \times (8.5\ \mathrm{ft})^2\right)}{}$$

$$a = \frac{}{(6 \times (16.439\ \text{kipft/ft})) + (4 \times (-0.87261\ \text{kip/ft}) \times (8.5\ \text{ft}))}$$

$$a = 5.8305\ \text{ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\, D) \left[1 - \left[3 \left(\frac{4\, E}{L_e} + 3\right) \left(\frac{a}{L_e}\right)^2\right] + \left[4 \left(\frac{3\, E}{L_e} + 2\right) \left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((-0.87261\ \text{kip/ft}) \times (48\ \text{in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (18.839\ \text{ft})}{(8.5\ \text{ft})} + 3\right) \times \left(\frac{(5.8305\ \text{ft})}{(8.5\ \text{ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (18.839\ \text{ft})}{(8.5\ \text{ft})} + 2\right) \times \left(\frac{(5.8305\ \text{ft})}{(8.5\ \text{ft})}\right)^3\right]\right]$$

$$V_{max} = 15.996\ \text{kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, D\, L_e) \left[\left(\frac{E}{L_e} + \frac{a}{2\, L_e}\right) - \left[\left(\frac{4\, E}{L_e} + 3\right) \left(\frac{a}{2\, L_e}\right)^3\right] + \left[\left(\frac{3\, E}{L_e} + 2\right) \left(\frac{a}{2\, L_e}\right)^4\right]\right]$$

$$M_{max} = ((-0.87261\ \text{kip/ft}) \times (48\ \text{in}) \times (8.5\ \text{ft})) \times \left[\left(\frac{(18.839\ \text{ft})}{(8.5\ \text{ft})} + \frac{(5.8305\ \text{ft})}{2 \times (8.5\ \text{ft})}\right) - \left[\left(\frac{4 \times (18.839\ \text{ft})}{(8.5\ \text{ft})} + 3\right) \times \left(\frac{(5.8305\ \text{ft})}{2 \times (8.5\ \text{ft})}\right)^3\right] + \left[\left(\frac{3 \times (18.839\ \text{ft})}{(8.5\ \text{ft})} + 2\right) \times \left(\frac{(5.8305\ \text{ft})}{2 \times (8.5\ \text{ft})}\right)^4\right]\right]$$

$$M_{max} = 65.28\ \text{kipft}$$

Shear force and Bending moment (z-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\, b}$$

$$H_o = \frac{(0.419\ \text{kip})}{1.57 \times (48\ \text{in})}$$

$$H_o = 0.06672\ \text{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\, H)}{1.57\, b}$$

$$M_o = \frac{(2.319\ \text{kipft}) + ((0.419\ \text{kip}) \times (0\ \text{ft}))}{1.57 \times (48\ \text{in})}$$

$$M_o = 0.36927\ \text{kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(0.36927\ \text{kipft/ft})}{(0.06672\ \text{kip/ft})}$$

$$E = 5.5346\ \text{ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\, M_o\, L_e) + \left(3\, H_o\, L_e^2\right)}{(6\, M_o) + (4\, H_o\, L_e)}$$

$$a = \frac{(4 \times (0.36927\ \text{kipft/ft}) \times (8.5\ \text{ft})) + \left(3 \times (0.06672\ \text{kip/ft}) \times (8.5\ \text{ft})^2\right)}{(6 \times (0.36927\ \text{kipft/ft})) + (4 \times (0.06672\ \text{kip/ft}) \times (8.5\ \text{ft}))}$$

$$a = 6.025\ \text{ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\, b) \left[1 - \left[3 \left(\frac{4\, E}{L_e} + 3\right) \left(\frac{a}{L_e}\right)^2\right] + \left[4 \left(\frac{3\, E}{L_e} + 2\right) \left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((0.06672\ \text{kip/ft}) \times (48\ \text{in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (5.5346\ \text{ft})}{(8.5\ \text{ft})} + 3\right) \times \left(\frac{(6.025\ \text{ft})}{(8.5\ \text{ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (5.5346\ \text{ft})}{(8.5\ \text{ft})} + 2\right) \times \left(\frac{(6.025\ \text{ft})}{(8.5\ \text{ft})}\right)^3\right]\right]$$

$$V_{max} = 0.48462\ \text{kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, b\, L_e) \left[\left(\frac{E}{L_e} + \frac{a}{2\, L_e}\right) - \left[\left(\frac{4\, E}{L_e} + 3\right) \left(\frac{a}{2\, L_e}\right)^3\right] + \left[\left(\frac{3\, E}{L_e} + 2\right) \left(\frac{a}{2\, L_e}\right)^4\right]\right]$$

$$M_{max} = ((0.06672\ \text{kip/ft}) \times (48\ \text{in}) \times (8.5\ \text{ft})) \times \left[\left(\frac{(5.5346\ \text{ft})}{(8.5\ \text{ft})} + \frac{(6.025\ \text{ft})}{2 \times (8.5\ \text{ft})}\right) - \left[\left(\frac{4 \times (5.5346\ \text{ft})}{(8.5\ \text{ft})} + 3\right) \times \left(\frac{(6.025\ \text{ft})}{2 \times (8.5\ \text{ft})}\right)^3\right] + \left[\left(\frac{3 \times (5.5346\ \text{ft})}{(8.5\ \text{ft})} + 2\right) \times \left(\frac{(6.025\ \text{ft})}{2 \times (8.5\ \text{ft})}\right)^4\right]\right]$$

$$M_{max} = 1.8566\ \text{kipft}$$

Minimum Reinforcement Check (LRFD)

Parameters:

$f'_{ck} = 2.5\ \text{ksi}$ - Concrete strength,
$f_{yk} = 60\ \text{ksi}$ - Longitudinal reinforcement strength,
$\phi = 0.65$ - Reduction factor for axial strength,
Table 22.4.2.1 $\alpha = 0.8$ - Alpha factor for axial strength,
$A_g = 2304\ \text{in}^2$ - Gross area of concrete,

Longitudinal reinforcement:

Required reinforcement due to axial load, $A_{st,required}$

22.4.2.2, 10.6.1.1 $A_{st,required}$

$$A_{st,required} = Min\left[\frac{\frac{P}{\phi\ \alpha} - (0.85\ f'_{ck}\ A_g)}{f_{yk} - (0.85\ f'_{ck})}, (0.08\ A_g)\right]$$

$$A_{st,required} = Min\left[\frac{\frac{(17.146\ \text{kip})}{(0.65) \times (0.8)} - (0.85 \times (2.5\ \text{ksi}) \times (2304\ \text{in}^2))}{(60\ \text{ksi}) - (0.85 \times (2.5\ \text{ksi}))}, (0.08 \times (2304\ \text{in}^2))\right]$$

$$A_{st,required} = -84.026\ \text{in}^2$$

A_{min} - Governing minimum reinforcement area,

$$A_{min} = Max[A_{st,required}, (0.0018\ A_g)]$$

$$A_{min} = Max[(-84.026\ \text{in}^2), (0.0018 \times (2304\ \text{in}^2))]$$

$$A_{min} = 4.1472\ \text{in}^2$$

n_{rebar} - Required number of reinforcement,

$$n_{rebar} = \frac{A_{min}}{A_{rebar}}$$

$$n_{rebar} = \frac{(4.1472\ \text{in}^2)}{(0.3068\ \text{in}^2)}$$

$$n_{rebar} = 14$$

A_{st} - Actual total reinforcement area,

$$A_{st} = n_{rebar}\ \frac{\pi\ d_{bar}^2}{4}$$

$$A_{st} = (14) \times \frac{\pi \times (0.625\ \text{in})^2}{4}$$

$$A_{st} = 4.2951\ \text{in}^2$$

$Ratio$ - Capacity

$$Ratio = \frac{A_{min}}{A_{st}}$$

$$Ratio = \frac{(4.1472\ \text{in}^2)}{(4.2951\ \text{in}^2)}$$

$$Ratio = 0.96556$$

Status: **PASS**
Ratio: **0.970**

25.2.3 s_{rebar} - Minimum spacing of reinforcement,

$$s_{rebar} = Max\left[1.5, \left(1.5\, d_{bar}\right)\right]$$

$$s_{rebar} = Max\left[1.5, \left(1.5 \times (0.625\text{ in})\right)\right]$$

$$s_{rebar} = 1.5\text{ in}$$

Ties:

25.7.2.2 Since longitudinal reinforcement is $\leq$ No. 10⌀: Use #3(0.375 in)
25.7.2.1 s_{ties} - Maximum spacing of ties,

$$s_{ties} = Min\left[\left(16\, d_{bar}\right), \left(48\, d_{ties}\right), Min\left(D, b\right)\right]$$

$$s_{ties} = Min\left[\left(16 \times (0.625\text{ in})\right), \left(48 \times (0.375\text{ in})\right), Min\left((48\text{ in}), (48\text{ in})\right)\right]$$

$$s_{ties} = 10\text{ in}$$

Summary:

Main reinforcement: **14 - #5 (0.625 in)**
Ties: **#3(0.375 in) - 10 in**

Axial Compression Strength (ACI 318-19, LRFD)

22.4.2.2 ϕP_N - Allowable axial compressive strength

$$\phi P_N = \phi\, 0.80\, \left[\left(0.85\, f'_{ck}\, \left[A_g - A_{st}\right]\right) + \left(f_{yk}\, A_{st}\right)\right]$$

$$\phi P_N = (0.65) \times 0.80 \times \left[\left(0.85 \times (2.5\text{ ksi}) \times \left[\left(2304\text{ in}^2\right) - \left(4.2951\text{ in}^2\right)\right]\right) + \left((60\text{ ksi}) \times \left(4.2951\text{ in}^2\right)\right)\right]$$

$$\phi P_N = 2675.2\text{ kip}$$

$Ratio$ - Capacity

$$Ratio = \frac{P}{\phi P_N}$$

$$Ratio = \frac{(17.146\text{ kip})}{(2675.2\text{ kip})}$$

$$Ratio = 0.0064093$$

Status: **PASS**
Ratio: **0.010**

Shear Strength (ACI 318-19, LRFD)

Parameters:

$b_w = 48\text{ in}$ - Effective width,
22.5.2.2 d - Effective depth

$$d = 0.80\, D$$

$$d = 0.80 \times (48\text{ in})$$

$$d = 38.4\text{ in}$$

22.5.5.1.3 λ_s - size effect modification factor

$$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{d}{10}}}, 1\right]$$

$$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{(38.4\text{ in})}{10}}}, 1\right]$$

$$\lambda_s = 0.64282$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5\text{ ksi} \rightarrow 2500\text{ psi}$,
22.5.5.1.1 $V_{c,max}$ - Max shear strength of concrete

$$V_{c,max} = 5\, \lambda_s\, \sqrt{f'_{ck}}\, b_w\, d$$

$$V_{c,max} = 5 \times (0.64282) \times \sqrt{(2500\text{ psi})} \times (48\text{ in}) \times (38.4\text{ in})$$

$$V_{c,max} = 296.21 \text{ kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$,$P = 17.146 \text{ kip} \rightarrow 17146 \text{ lbf}$,

22.5.5.1.1(a) $V_{c,a}$ - Shear strength of concrete (a)

$$V_{c,a} = \left[2\,\lambda_s\,\sqrt{f'_{ck}} + \frac{P}{6\,A_g}\right]\,b_w\,d$$

$$V_{c,a} = \left[2 \times (0.64282) \times \sqrt{(2500 \text{ psi})} + \frac{(17146 \text{ lbf})}{6 \times (2304 \text{ in}^2)}\right] \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{c,a} = 120.77 \text{ kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$,

22.5.5.1.2 $V_{c,b}$ - Shear strength of concrete (b)

$$V_{c,b} = \left[2\,\lambda_s\,\sqrt{f'_{ck}} + \left(0.05\,f'_{ck}\right)\right]\,b_w\,d$$

$$V_{c,b} = \left[2 \times (0.64282) \times \sqrt{(2500 \text{ psi})} + (0.05 \times (2500 \text{ psi}))\right] \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{c,b} = 348.89 \text{ kip}$$

V_c - Governing shear strength of concrete

$$V_c = Min\left[V_{c,max}, V_{c,a}, V_{c,b}\right]$$

$$V_c = Min\left[(296.21 \text{ kip}), (120.77 \text{ kip}), (348.89 \text{ kip})\right]$$

$$V_c = 120.77 \text{ kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$,

22.5.1.2 $V_{s,a}$ - Shear strength of steel (a)

$$V_{s,a} = 8\,\sqrt{f'_{ck}}\,b_w\,d$$

$$V_{s,a} = 8 \times \sqrt{(2500 \text{ psi})} \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{s,a} = 737.28 \text{ kip}$$

A_v - Ties rebar area,

$$A_v = \frac{\pi\,d_{ties}^2}{4}$$

$$A_v = \frac{\pi \times (0.375 \text{ in})^2}{4}$$

$$A_v = 0.11045 \text{ in}^2$$

22.5.8.5.3 $V_{s,b}$ - Shear strength of steel (b)

$$V_{s,b} = \frac{2\,A_v\,f_{ywk}\,d}{s_{ties}}$$

$$V_{s,b} = \frac{2 \times \left(0.11045 \text{ in}^2\right) \times (60 \text{ ksi}) \times (38.4 \text{ in})}{(10 \text{ in})}$$

$$V_{s,b} = 50.894 \text{ kip}$$

V_s - Governing shear strength of steel

$$V_s = MIN\left[V_{s,a}, V_{s,b}\right]$$

$$V_s = MIN\left[(737.28 \text{ kip}), (50.894 \text{ kip})\right]$$

$$V_s = 50.894 \text{ kip}$$

22.5.1.1 ϕV_n - Allowable shear strength

$$\phi V_n = \phi\,\left(V_c + V_s\right)$$

$$\phi V_n = (0.65) \times ((120.77 \text{ kip}) + (50.894 \text{ kip}))$$

$$\phi V_n = 111.58 \text{ kip}$$

Considering x-direction:

$V_{max} = 15.996 \text{ kip}$ - Maximum shear force in the x-direction,
$Ratio$ - Capacity

$$Ratio = \frac{V_{max}}{\phi V_n}$$

	$$Ratio = \frac{(15.996\ \text{kip})}{(111.58\ \text{kip})}$$ $$Ratio = 0.14335$$	Status: **PASS** Ratio: **0.140**
	Considering z-direction: $V_{max} = 0.48462\ \text{kip}$ - Maximum shear force in the z-direction, $Ratio$ - Capacity $$Ratio = \frac{V_{max}}{\phi V_n}$$ $$Ratio = \frac{(0.48462\ \text{kip})}{(111.58\ \text{kip})}$$ $$Ratio = 0.0043432$$	Status: **PASS** Ratio: **0.000**
14.5.2.1b	**Flexural Strength (ACI 318-19, LRFD)** S_m - Section modulus $$S_m = \frac{b\,D^2}{6}$$ $$S_m = \frac{(48\ \text{in}) \times (48\ \text{in})^2}{6}$$ $$S_m = 18432\ \text{in}^3$$ $\lambda = 1$ - Concrete modification factor (Normal concrete), Allowable flexural strength: M_n shall be the lesser of: $\phi M_{n,1}$ $$\phi M_{n,1} = \phi \times 5 \times \lambda \times \sqrt{(f_c')} \times S_m$$ $$\phi M_{n,1} = 0.65 \times 5 \times 1 \times \sqrt{(2.5ksi)} \times 18432.001in^3$$ $$\phi M_{n,1} = 249.600kipft$$ $\phi M_{n,2}$ $$\phi M_{n,2} = \phi\ 0.85\ f'_{ck}\ S_m$$ $$\phi M_{n,2} = (0.65) \times 0.85 \times (2.5\ \text{ksi}) \times \left(18432\ \text{in}^3\right)$$ $$\phi M_{n,2} = 2121.6\ \text{kipft}$$ Therefore, ϕM_n - Allowable flexural strength, $$\phi M_n = MIN[\phi M_{n,1}, \phi M_{n,2}]$$ $$\phi M_n = MIN[(249.6\ \text{kipft}), (2121.6\ \text{kipft})]$$ $$\phi M_n = 249.6\ \text{kipft}$$ **Considering x-direction:** $M_{max} = 65.28\ \text{kipft}$ - Maximum moment in the x-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\phi M_n}$$ $$Ratio = \frac{(65.28\ \text{kipft})}{(249.6\ \text{kipft})}$$ $$Ratio = 0.26154$$	Status: **PASS** Ratio: **0.260**
	Considering z-direction: $M_{max} = 1.8566\ \text{kipft}$ - Maximum moment in the z-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\phi M_n}$$	

	$$Ratio = \frac{(1.8566\ \text{kipft})}{(249.6\ \text{kipft})}$$ $$Ratio = 0.0074381$$	Status: **PASS** Ratio: **0.010**