Project Details

Project Name: Domes Church - 4x9 - V1Jb

Date: Wed Mar 05 2025

Location: 804 N 16th Ave, Caldwell, ID 83605, USA

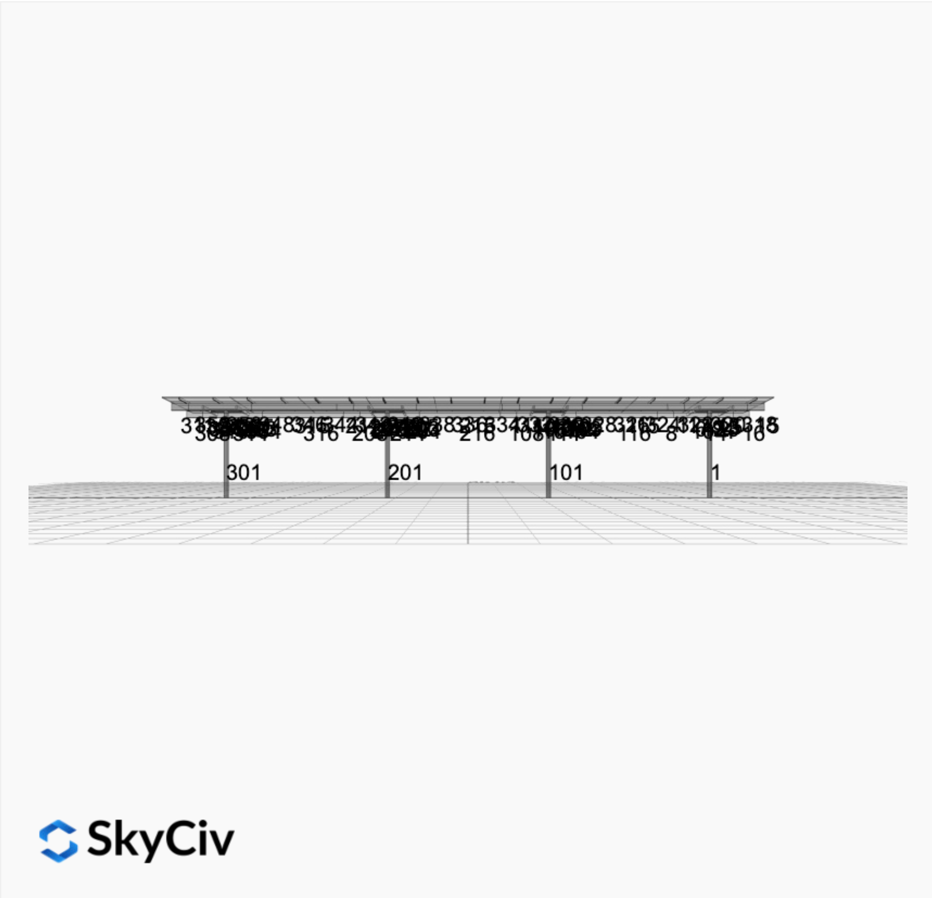
Number of Modules: 36

Unique ID: 4P-19.75-6TOP-XD-24-L-4Hx9W-EI8A

Number of Poles: 4

Dealer: ______________________________

Date Sold: ______________________________



Array Dimensions N/S	15.05 ft
Array Dimensions E/W	71.08 ft
Winter Tilt Angle	5
Front Edge Clearance	10 ft

MT Solar Bill of Materials (4P-19.75-6TOP-XD-24-L-4Hx9W-EI8A)

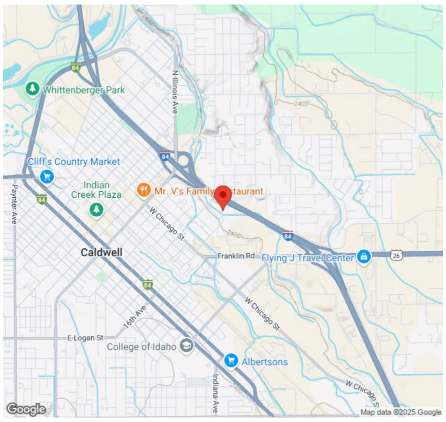
Part	Short Description	BOM Qty
MTS-PC-6	6IN Pole Cap Assembly	4
MTS-HF-XD	H-Frame Assembly-XD	4
MTS-XD-Wing-24	24IN XD Wing	4
MTS-XD-Splice-90	90IN XD Splice	6
MTS-XD-Splice-57	57IN XD Splice	6
MTS-CLAMP-HOOK-4PK	Hook Clamp	9

Rail Bill of Materials

Part	Qty
Rails (179in)	18
Rail Attachment	36
Module Mid Clamp	54

Part	Qty
Module End Clamp	36
Ground Lug	9

Site Details:



Site Address: 804 N 16th Ave, Caldwell, ID 83605, USA

Array Specification

Duty Classification:	XD
Module Width:	44.65 in
Module Length:	93.78in
Number of Rows:	4
Number of Columns:	9
Total Number of Modules:	36
Winter Tilt Angle:	5
Front Edge Clearance:	10
Total Array Height at Tilt:	11.31 ft
Total Frame Length:	70.75 ft
Frame Weight:	4359 lbs
Array Dimensions N/S:	15.05 ft
Array Dimensions E/W:	71.08 ft
Rail Length:	180.60 in
Rail Spacing:	3.95 ft

Support Specifications

Pole Size:	6in Pipe Sch 40
Pole Length above Grade:	10.66 ft
Number of Poles:	4
Pole Spacing:	19.75 ft

Foundation Specifications

Foundation Type:	Round
Foundation Dimensions:	Ø36 in
Foundation Depth (below grade):	Pile 1: 5.50 ft Pile 2: 5.75 ft Pile 3: 5.75 ft Pile 4: 5.50 ft
Foundation Volume:	5.890 y^3

Site Info

Risk Category:	I
Exposure:	B
Soil Classification:	sand
Site Location:	804 N 16th Ave, Caldwell, ID 83605, USA
Wind Speed:	96 mph

Snow Load:	50.64 psf

Design Disclaimer

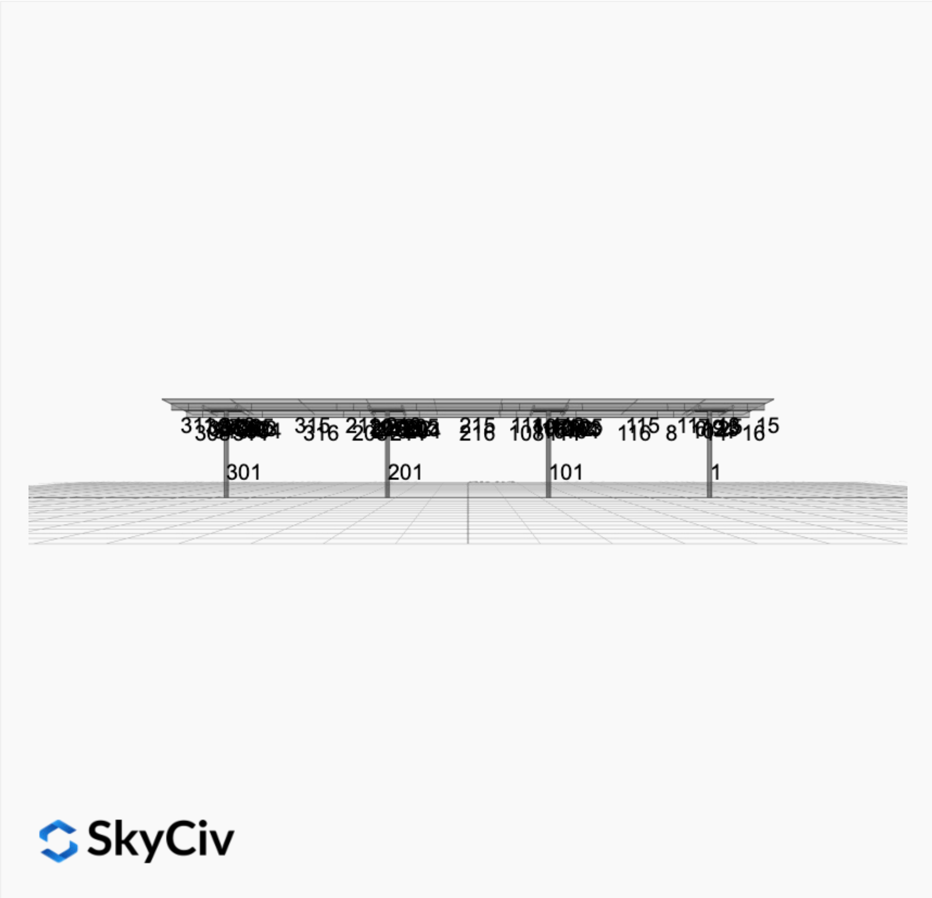
This software should be used for preliminary designs and should not be used as a final design unless reviewed, verified and designed by a qualified structural engineer.

AutoDesigner Input

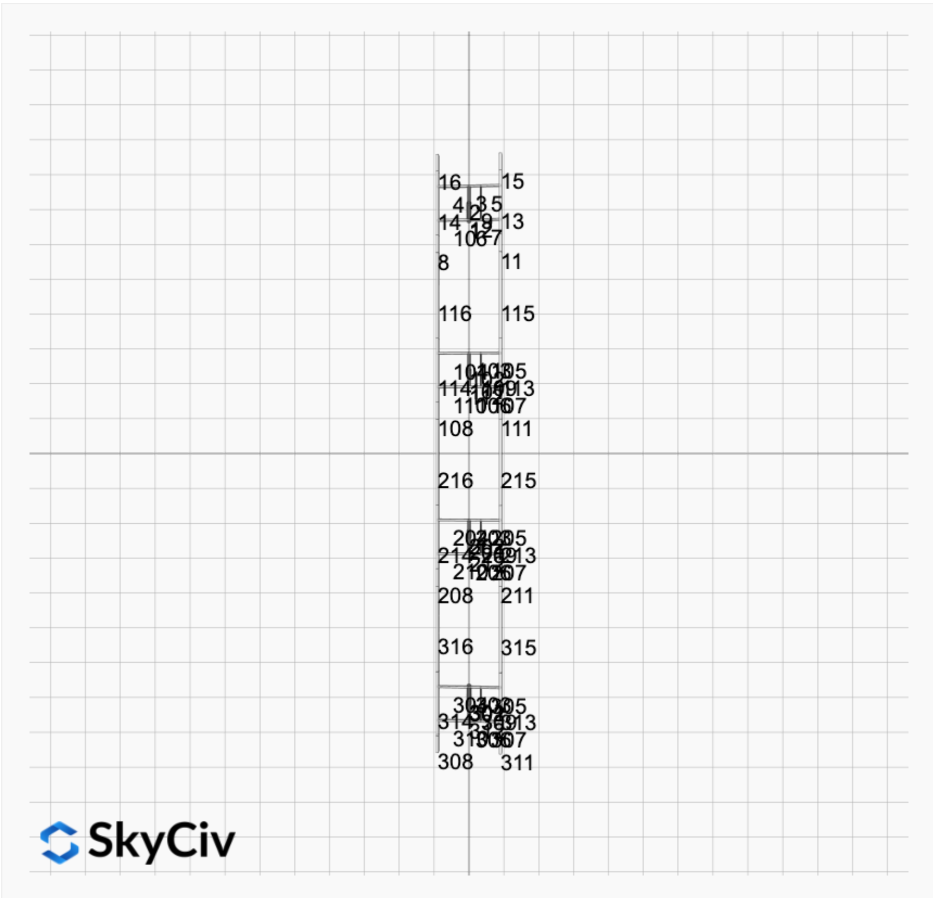
{"wind_speed_override":null,"snow_load_override":null,"direct_snow_load":false,"add_angle_brace":false,"product_type":"Beam","designer_name":"","designer_email":"","designer_phone":"","project_id":"Domes Church - 4x9 - V1Jb","site_address":"804 N 16th Ave, Caldwell, ID 83605, USA","module_width":44.65,"module_length":93.78,"number_rows":"4","number_columns":9,"pole_mount_section":"4_40","core_pipe_width":65,"core_pipe_section":"2_40","adjuster_section":"2_40","core_beam_height":65,"core_beam_section":"HSS3x2x1/8","main_pipe_section":"2_12GA","pole_spacing":"15","tilt_angle":5,"ground_clearance":10,"risk_category":"I","exposure_category":"B","frame_duty_override":"auto","pole_override":"auto","soil_type":"sand","customer_foundation_override":"48_Square","foundation_type":"Round","foundation_size":36,"check_rails":true}

Design Notes:

- AISC Deflection checks are set to L/1 due to structure design intent
- Foundation Soil Parameters used in this Autodesign are all estimates, proper geotechnical reports are required to confirm soil profiles
- Wind speeds, snow loads and other site specific results are based on ASCE 7 2016
- Steel frame design checks are based on AISC 360 2016 (LRFD)



SkyCiv



16
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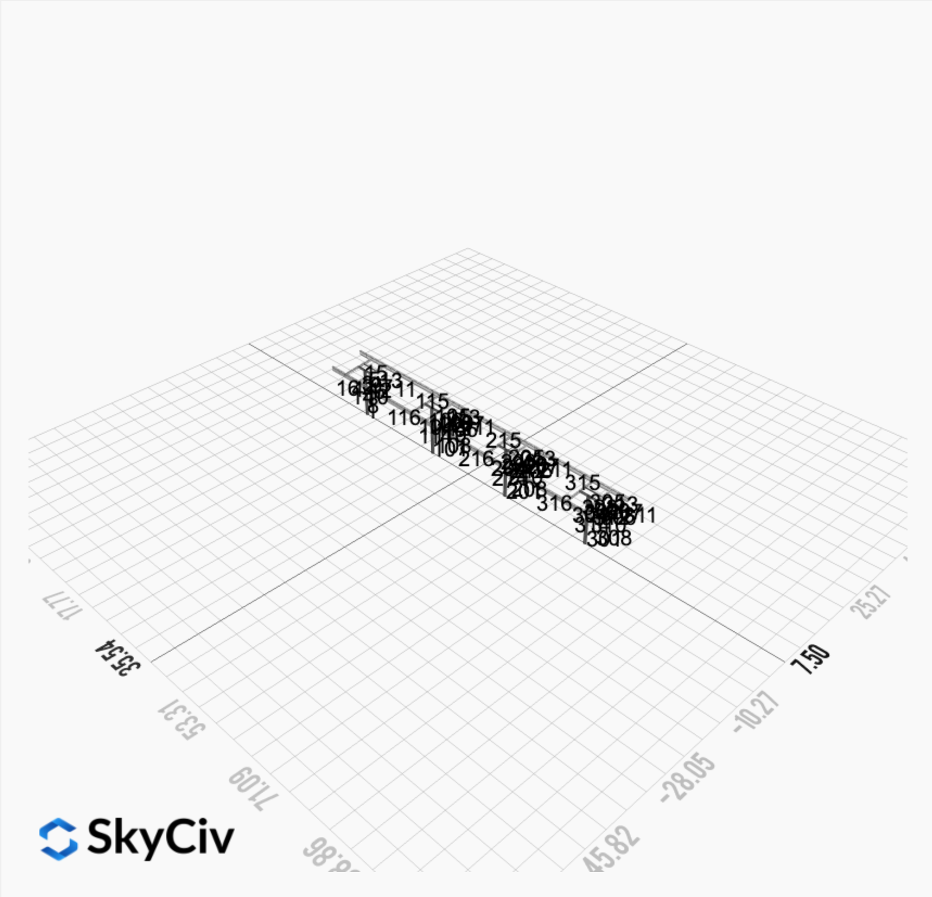
SkyCiv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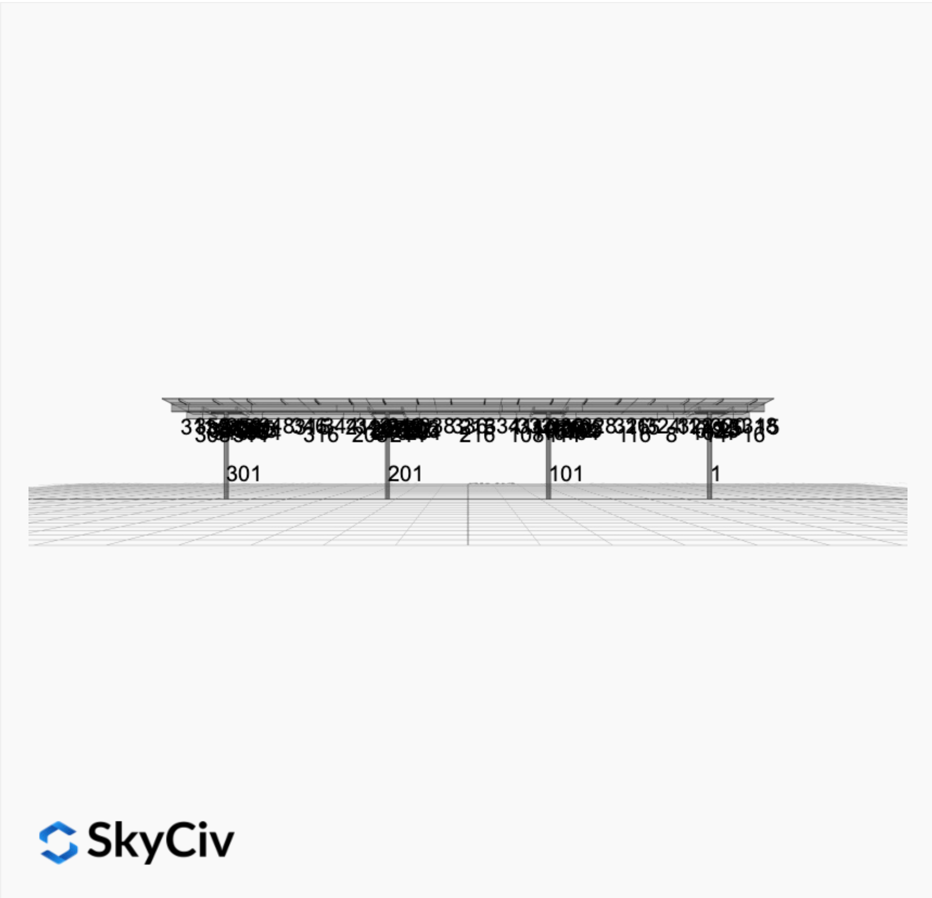
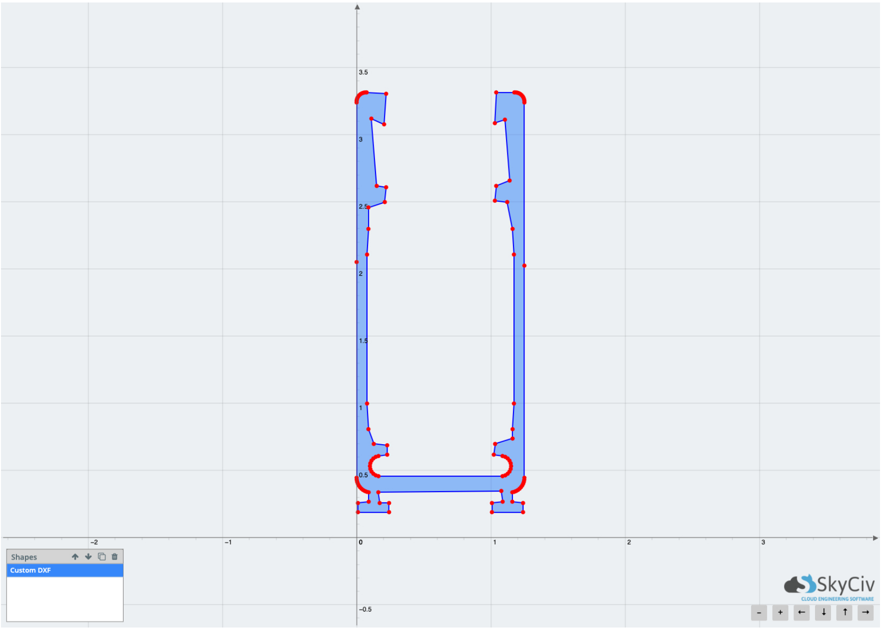
35.54
53.31
71.09
7.50
25.27
-10.27
-28.05

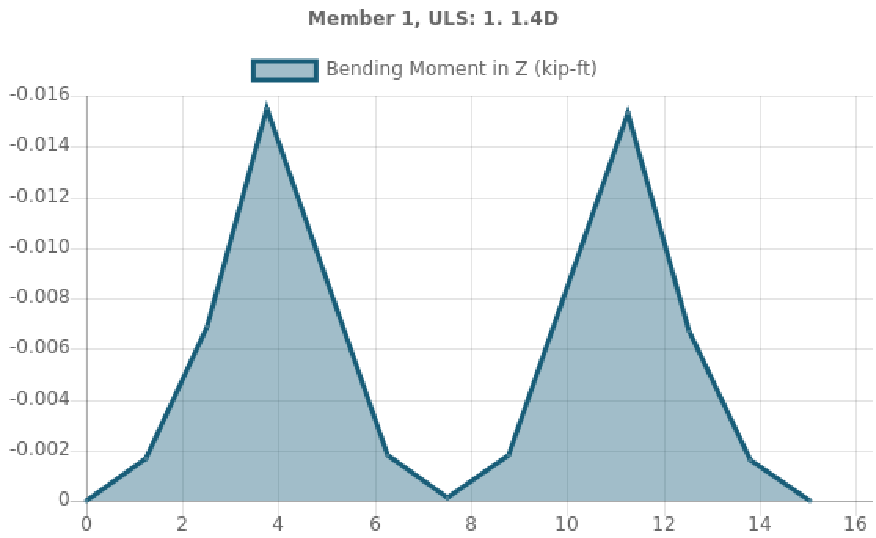

301
201
101
1

Rail Design Check

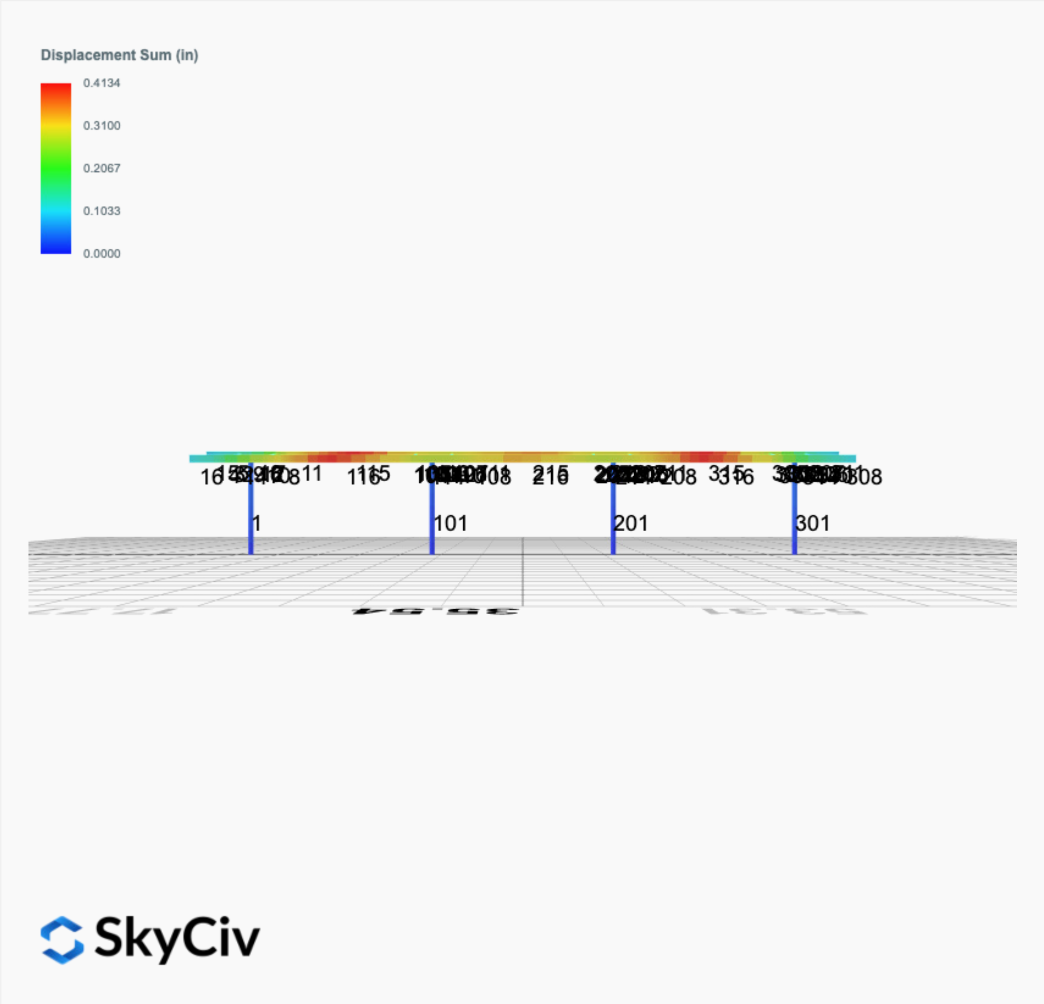
Rail Length: 15.049999999999999 ft
Additional Restraints Required: None
Tributary Width: 3.9491666666666667 ft
Material: Aluminium
Density: 169 lb/ft^3
Elasticity Modulus: 10000 ksi
Fy: 34.5 ksi
Fu: 37 ksi
Snow (X): 0.1205 kip/ft
Snow (Y): -0.0105 kip/ft
Wind uplift Case A: 0.0201 kip/ft
Wind uplift Case A: 0.0201 kip/ft
Wind uplift Case B (X): 0.0000 kip/ft
Wind uplift Case B (Y): 0.0461 kip/ft

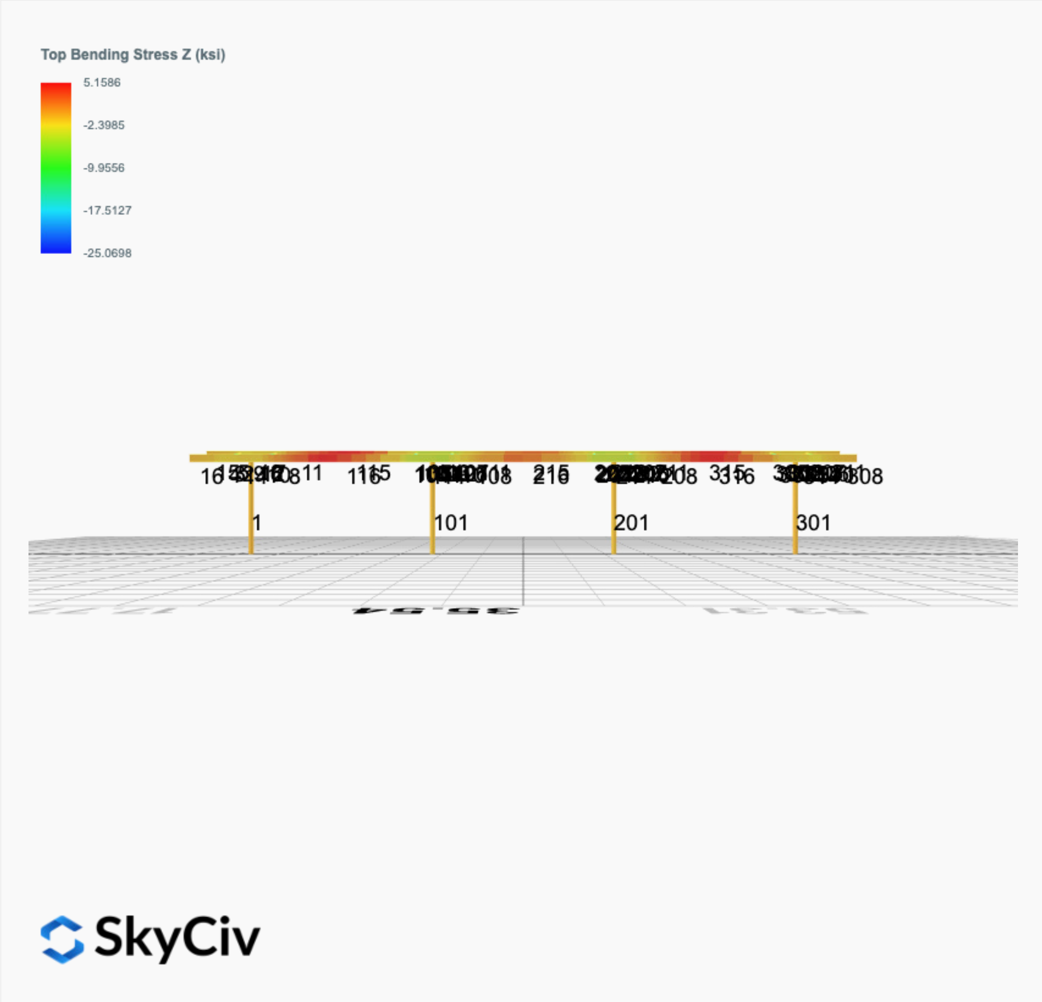


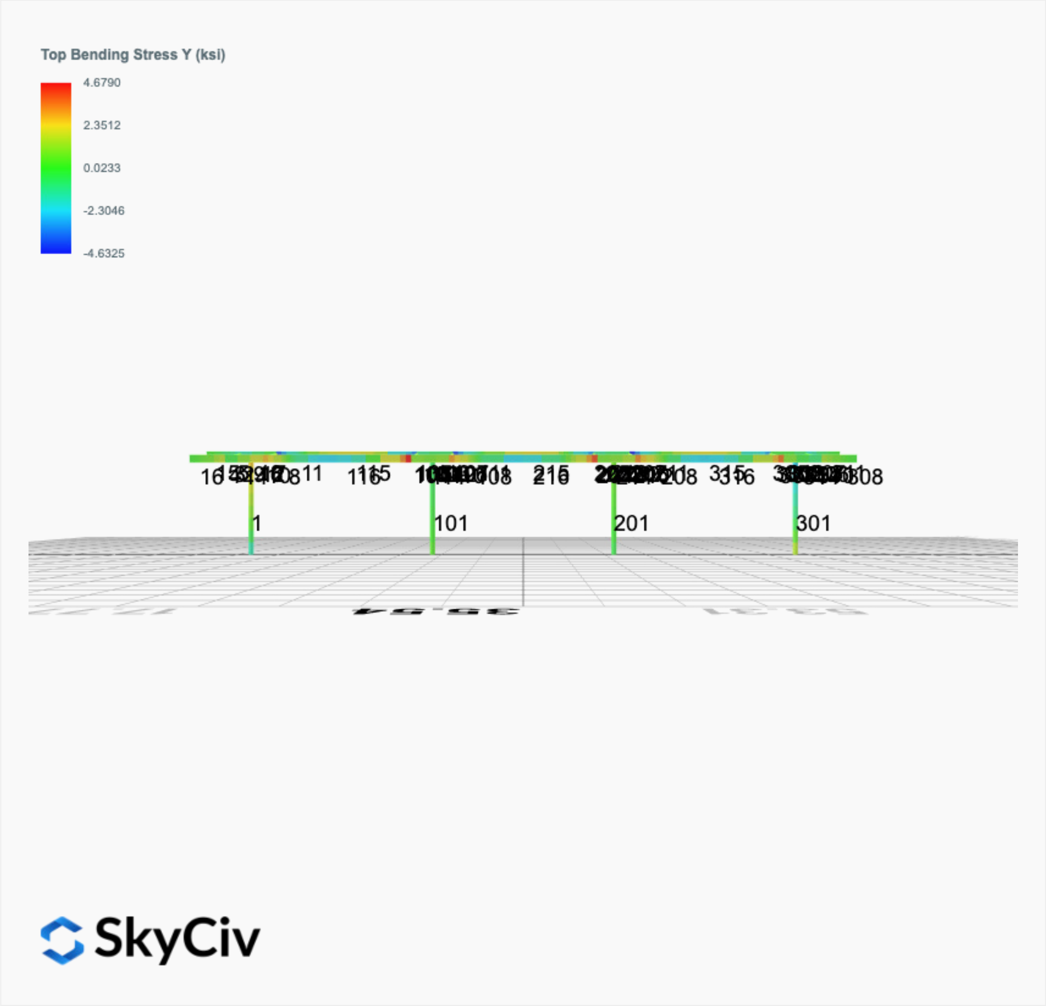
Result Check	Max Limit	Max Value	Utility	Status
Custom Stress Limit	34.5	7.46082917	0.216	PASS
Material Yield	34.5	7.46082917	0.216	PASS
Material Strength	37	7.46082917	0.202	PASS



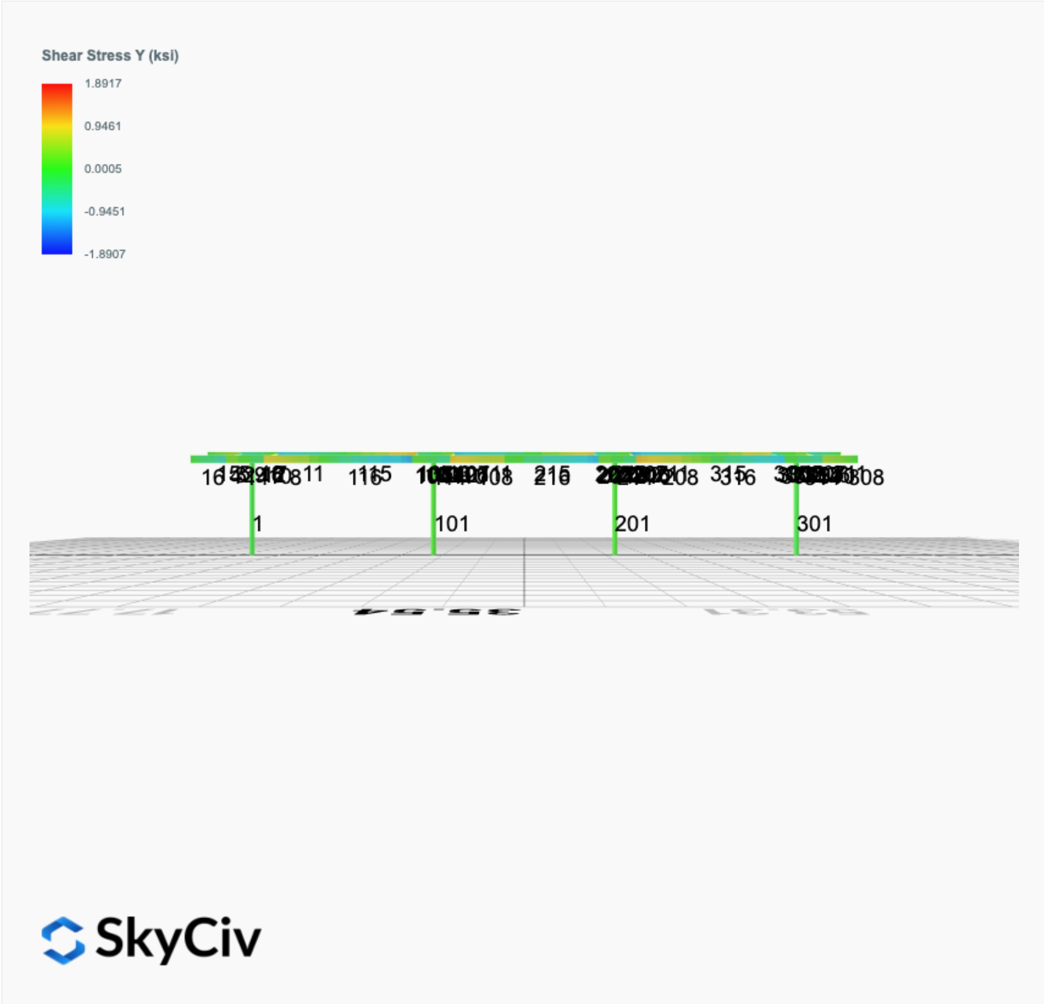
FEM Results (Envelope Worst Case for each member)



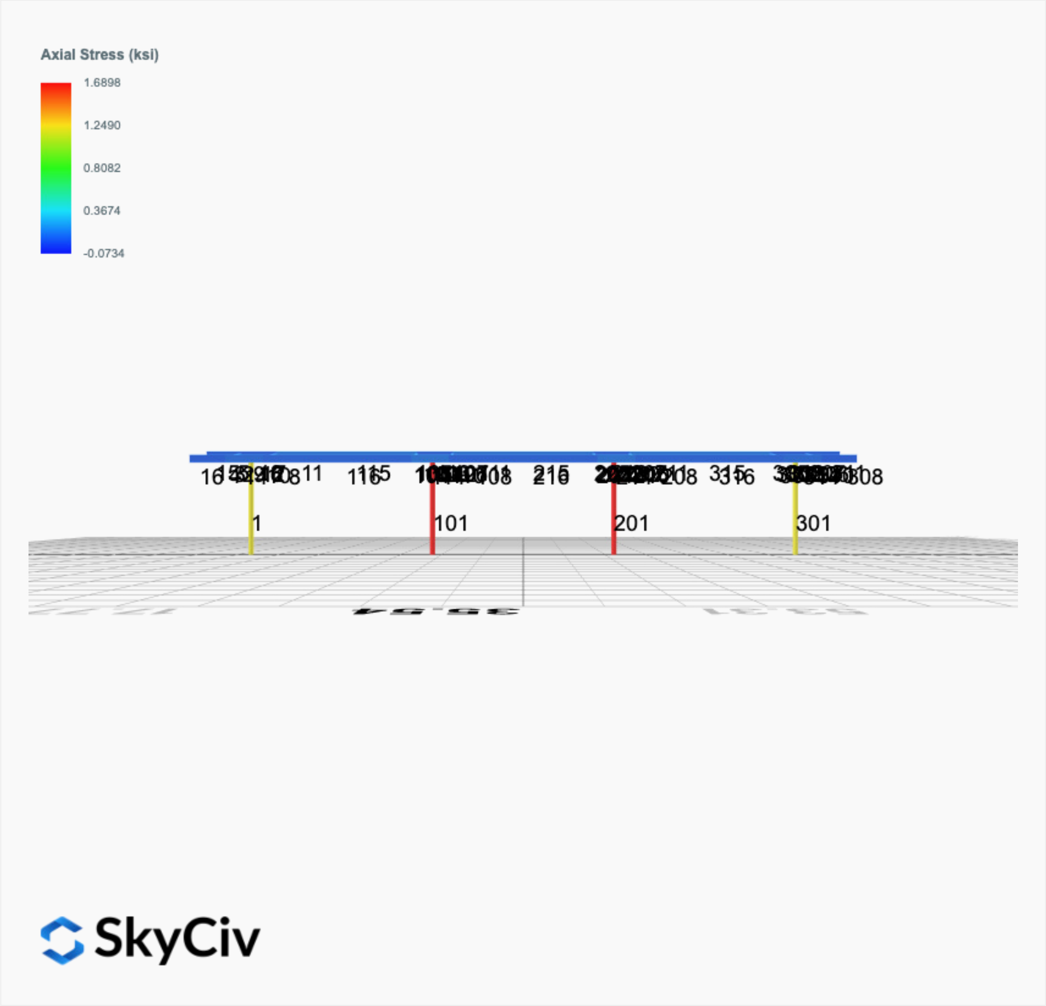

Top Bending Stress Z (ksi)
5.1586
-2.3985
-9.9556
-17.5127
-25.0698
1
101
201
301
SkyCiv



Top Bending Stress Y (ksi)
4.6790
2.3512
0.0233
-2.3046
-4.6325
1
101
201
301



Shear Stress Y (ksi)
1.8917
0.9461
0.0005
-0.9451
-1.8907
1
101
201
301
SkyCiv


Axial Stress (ksi)
1.6898
1.2490
0.8082
0.3674
-0.0734
1
101
201
301
SkyCiv

Reaction Forces for Foundation 1 (Node ID#1), (kip, kip-ft)

ASD Load Combination Results

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 1. D	0.0050	1.8414	0.0531	0.1783	-0.0059	-0.0133
ULS: 2. D + L	0.0050	1.8414	0.0531	0.1783	-0.0059	-0.0133
ULS: 3. D + (S or Lr or R)	0.0299	8.6542	0.3191	1.0742	-0.0356	-0.2359
ULS: 3. D + (S or Lr or R)	0.0050	1.8414	0.0531	0.1783	-0.0059	-0.0133
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	0.0237	6.9510	0.2526	0.8502	-0.0282	-0.1803
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	0.0050	1.8414	0.0531	0.1783	-0.0059	-0.0133
ULS: 5b. D + 0.7E	0.0050	1.8414	0.0531	0.1783	-0.0059	-0.0133
ULS: 6b. D + 0.75L + 0.75(0.7)E + 0.75S	0.0237	6.9510	0.2526	0.8502	-0.0282	-0.1803
ULS: 8. 0.6D + 0.7E	0.0030	1.1048	0.0319	0.1070	-0.0035	-0.0080
ULS: 5a. D + 0.6W_Wind downforce Case A only	-0.1112	3.1055	0.1035	0.3468	-0.0197	1.5170
ULS: 5a. D + 0.6W_Wind downforce Case B only	-0.1112	3.1055	0.1035	0.3468	-0.0197	1.5170
ULS: 5a. D + 0.6W_Wind uplift Case A only	0.0302	1.5015	0.0413	0.1386	-0.0048	1.0723
ULS: 5a. D + 0.6W_Wind uplift Case B only	0.0909	1.0365	0.0178	0.0607	0.0080	-3.9508
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-0.0634	7.8990	0.2904	0.9766	-0.0386	0.9675
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-0.0634	7.8990	0.2904	0.9766	-0.0386	0.9675
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	0.0426	6.6961	0.2437	0.8205	-0.0274	0.6339
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	0.0881	6.3473	0.2261	0.7620	-0.0178	-3.1334
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-0.0821	2.7895	0.0909	0.3046	-0.0163	1.1345
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-0.0821	2.7895	0.0909	0.3046	-0.0163	1.1345
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	0.0239	1.5865	0.0443	0.1485	-0.0051	0.8009
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	0.0694	1.2377	0.0266	0.0901	0.0045	-2.9665
ULS: 7. 0.6D + 0.6W_Wind downforce Case A only	-0.1132	2.3689	0.0822	0.2754	-0.0174	1.5224
ULS: 7. 0.6D + 0.6W_Wind downforce Case B only	-0.1132	2.3689	0.0822	0.2754	-0.0174	1.5224
ULS: 7. 0.6D + 0.6W_Wind uplift Case A only	0.0282	0.7649	0.0201	0.0673	-0.0024	1.0776
ULS: 7. 0.6D + 0.6W_Wind uplift Case B only	0.0889	0.2999	-0.0035	-0.0106	0.0103	-3.9455

Worst Case Reactions LRFD

These calculations are taken directly from the FEA via SkyCiv and are used in the Concrete Checks of the Foundatino Module.
Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	14.1631
Shear X	-0.1937
Shear Z	0.5334
Moment X	1.8039
Moment Y (Twist)	0.0672
Moment Z	7.0869

Worst Case Reactions ASD

These results are taken from the worst case values in the above table and are used in the Soil Checks in the Foundation Module.
Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	8.6542
Shear X	-0.1132
Shear Z	0.3191
Moment X	1.0742
Moment Y (Twist)	0.0386
Moment Z	3.9508

Reaction Forces for Foundation 2 (Node ID#101), (kip, kip-ft)

ASD Load Combination Results

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 1. D	-0.0050	2.3665	-0.0085	-0.0288	0.0015	0.0786
ULS: 2. D + L	-0.0050	2.3665	-0.0085	-0.0288	0.0015	0.0786
ULS: 3. D + (S or Lr or R)	-0.0299	11.7972	-0.0510	-0.1742	0.0088	0.3312
ULS: 3. D + (S or Lr or R)	-0.0050	2.3665	-0.0085	-0.0288	0.0015	0.0786
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	-0.0237	9.4396	-0.0404	-0.1378	0.0070	0.2681

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	-0.0050	2.3665	-0.0085	-0.0288	0.0015	0.0786
ULS: 5b. D + 0.7E	-0.0050	2.3665	-0.0085	-0.0288	0.0015	0.0786
ULS: 6b. D + 0.75L + 0.75(0.7)E + 0.75S	-0.0237	9.4396	-0.0404	-0.1378	0.0070	0.2681
ULS: 8. 0.6D + 0.7E	-0.0030	1.4199	-0.0051	-0.0173	0.0009	0.0472
ULS: 5a. D + 0.6W_Wind downforce Case A only	-0.1525	4.1158	-0.0158	-0.0539	0.0012	1.9610
ULS: 5a. D + 0.6W_Wind downforce Case B only	-0.1525	4.1158	-0.0158	-0.0539	0.0012	1.9610
ULS: 5a. D + 0.6W_Wind uplift Case A only	0.0410	1.8933	-0.0056	-0.0192	-0.0007	1.2803
ULS: 5a. D + 0.6W_Wind uplift Case B only	0.0765	1.2582	-0.0055	-0.0183	0.0061	-4.4853
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-0.1343	10.7515	-0.0459	-0.1566	0.0068	1.6798
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-0.1343	10.7515	-0.0459	-0.1566	0.0068	1.6798
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	0.0108	9.0846	-0.0382	-0.1306	0.0053	1.1693
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	0.0375	8.6083	-0.0382	-0.1299	0.0105	-3.1549
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-0.1156	3.6785	-0.0140	-0.0476	0.0013	1.4904
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-0.1156	3.6785	-0.0140	-0.0476	0.0013	1.4904
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	0.0295	2.0116	-0.0063	-0.0216	-0.0002	0.9799
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	0.0561	1.5353	-0.0063	-0.0209	0.0050	-3.3443
ULS: 7. 0.6D + 0.6W_Wind downforce Case A only	-0.1505	3.1692	-0.0124	-0.0424	0.0006	1.9295
ULS: 7. 0.6D + 0.6W_Wind downforce Case B only	-0.1505	3.1692	-0.0124	-0.0424	0.0006	1.9295
ULS: 7. 0.6D + 0.6W_Wind uplift Case A only	0.0430	0.9467	-0.0022	-0.0077	-0.0013	1.2488
ULS: 7. 0.6D + 0.6W_Wind uplift Case B only	0.0785	0.3116	-0.0021	-0.0068	0.0055	-4.5168

Worst Case Reactions LRFD

These calculations are taken directly from the FEA via SkyCiv and are used in the Concrete Checks of the Foundatino Module.
Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	19.3871
Shear X	-0.2633
Shear Z	-0.0845
Moment X	-0.2907
Moment Y (Twist)	0.0185
Moment Z	7.9106

Worst Case Reactions ASD

These results are taken from the worst case values in the above table and are used in the Soil Checks in the Foundation Module.
Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	11.7972
Shear X	-0.1525
Shear Z	-0.0510
Moment X	-0.1742
Moment Y (Twist)	0.0105
Moment Z	4.5168

Reaction Forces for Foundation 3 (Node ID#201), (kip, kip-ft)

ASD Load Combination Results

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 1. D	-0.0050	2.3667	0.0085	0.0287	-0.0015	0.0788
ULS: 2. D + L	-0.0050	2.3667	0.0085	0.0287	-0.0015	0.0788
ULS: 3. D + (S or Lr or R)	-0.0300	11.7980	0.0513	0.1734	-0.0086	0.3323
ULS: 3. D + (S or Lr or R)	-0.0050	2.3667	0.0085	0.0287	-0.0015	0.0788
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	-0.0238	9.4401	0.0406	0.1372	-0.0068	0.2689
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	-0.0050	2.3667	0.0085	0.0287	-0.0015	0.0788
ULS: 5b. D + 0.7E	-0.0050	2.3667	0.0085	0.0287	-0.0015	0.0788
ULS: 6b. D + 0.75L + 0.75(0.7)E + 0.75S	-0.0238	9.4401	0.0406	0.1372	-0.0068	0.2689
ULS: 8. 0.6D + 0.7E	-0.0030	1.4200	0.0051	0.0172	-0.0009	0.0473
ULS: 5a. D + 0.6W_Wind downforce Case A only	-0.1525	4.1160	0.0159	0.0537	-0.0013	1.9613
ULS: 5a. D + 0.6W_Wind downforce Case B only	-0.1525	4.1160	0.0159	0.0537	-0.0013	1.9613
ULS: 5a. D + 0.6W_Wind uplift Case A only	0.0410	1.8934	0.0056	0.0192	0.0007	1.2808
ULS: 5a. D + 0.6W_Wind uplift Case B only	0.0764	1.2582	0.0055	0.0182	-0.0058	-4.4859

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-0.1343	10.7522	0.0461	0.1560	-0.0067	1.6808
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-0.1343	10.7522	0.0461	0.1560	-0.0067	1.6808
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	0.0108	9.0852	0.0384	0.1301	-0.0052	1.1704
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	0.0373	8.6088	0.0384	0.1294	-0.0101	-3.1547
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-0.1156	3.6787	0.0141	0.0474	-0.0014	1.4907
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-0.1156	3.6787	0.0141	0.0474	-0.0014	1.4907
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	0.0295	2.0117	0.0064	0.0215	0.0002	0.9803
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	0.0560	1.5353	0.0063	0.0208	-0.0047	-3.3448
ULS: 7. 0.6D + 0.6W_Wind downforce Case A only	-0.1505	3.1694	0.0125	0.0422	-0.0007	1.9297
ULS: 7. 0.6D + 0.6W_Wind downforce Case B only	-0.1505	3.1694	0.0125	0.0422	-0.0007	1.9297
ULS: 7. 0.6D + 0.6W_Wind uplift Case A only	0.0430	0.9467	0.0022	0.0077	0.0013	1.2493
ULS: 7. 0.6D + 0.6W_Wind uplift Case B only	0.0784	0.3116	0.0021	0.0067	-0.0052	-4.5175

Worst Case Reactions LRFD

These calculations are taken directly from the FEA via SkyCiv and are used in the Concrete Checks of the Foundatino Module.
Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	19.3883
Shear X	-0.2633
Shear Z	0.0850
Moment X	0.2894
Moment Y (Twist)	0.0179
Moment Z	7.9113

Worst Case Reactions ASD

These results are taken from the worst case values in the above table and are used in the Soil Checks in the Foundation Module.
Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	11.7980
Shear X	-0.1525
Shear Z	0.0513
Moment X	0.1734
Moment Y (Twist)	0.0101
Moment Z	4.5175

Reaction Forces for Foundation 4 (Node ID#301), (kip, kip-ft)

ASD Load Combination Results

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 1. D	0.0050	1.8413	-0.0532	-0.1787	0.0060	-0.0133
ULS: 2. D + L	0.0050	1.8413	-0.0532	-0.1787	0.0060	-0.0133
ULS: 3. D + (S or Lr or R)	0.0299	8.6538	-0.3194	-1.0770	0.0360	-0.2360
ULS: 3. D + (S or Lr or R)	0.0050	1.8413	-0.0532	-0.1787	0.0060	-0.0133
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	0.0237	6.9507	-0.2528	-0.8524	0.0285	-0.1803
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	0.0050	1.8413	-0.0532	-0.1787	0.0060	-0.0133
ULS: 5b. D + 0.7E	0.0050	1.8413	-0.0532	-0.1787	0.0060	-0.0133
ULS: 6b. D + 0.75L + 0.75(0.7)E + 0.75S	0.0237	6.9507	-0.2528	-0.8524	0.0285	-0.1803
ULS: 8. 0.6D + 0.7E	0.0030	1.1048	-0.0319	-0.1072	0.0036	-0.0080
ULS: 5a. D + 0.6W_Wind downforce Case A only	-0.1112	3.1054	-0.1036	-0.3476	0.0198	1.5171
ULS: 5a. D + 0.6W_Wind downforce Case B only	-0.1112	3.1054	-0.1036	-0.3476	0.0198	1.5171
ULS: 5a. D + 0.6W_Wind uplift Case A only	0.0302	1.5015	-0.0413	-0.1390	0.0048	1.0727
ULS: 5a. D + 0.6W_Wind uplift Case B only	0.0909	1.0365	-0.0178	-0.0610	-0.0078	-3.9517
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-0.0634	7.8987	-0.2907	-0.9791	0.0389	0.9675
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-0.0634	7.8987	-0.2907	-0.9791	0.0389	0.9675
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	0.0426	6.6958	-0.2440	-0.8226	0.0277	0.6342
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	0.0881	6.3471	-0.2263	-0.7641	0.0182	-3.1341
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-0.0821	2.7894	-0.0910	-0.3054	0.0163	1.1345
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-0.0821	2.7894	-0.0910	-0.3054	0.0163	1.1345
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	0.0239	1.5864	-0.0443	-0.1489	0.0051	0.8012
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	0.0695	1.2377	-0.0267	-0.0904	-0.0043	-2.9671

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 7. 0.6D + 0.6W_Wind downforce Case A only	-0.1132	2.3688	-0.0823	-0.2761	0.0174	1.5224
ULS: 7. 0.6D + 0.6W_Wind downforce Case B only	-0.1132	2.3688	-0.0823	-0.2761	0.0174	1.5224
ULS: 7. 0.6D + 0.6W_Wind uplift Case A only	0.0282	0.7649	-0.0201	-0.0675	0.0024	1.0780
ULS: 7. 0.6D + 0.6W_Wind uplift Case B only	0.0889	0.2999	0.0034	0.0105	-0.0102	-3.9464

Worst Case Reactions LRFD

These calculations are taken directly from the FEA via SkyCiv and are used in the Concrete Checks of the Foundatino Module.
Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	14.1625
Shear X	-0.1937
Shear Z	-0.5339
Moment X	-1.8086
Moment Y (Twist)	0.0677
Moment Z	7.0886

Worst Case Reactions ASD

These results are taken from the worst case values in the above table and are used in the Soil Checks in the Foundation Module.
Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	8.6538
Shear X	-0.1132
Shear Z	-0.3194
Moment X	-1.0770
Moment Y (Twist)	0.0389
Moment Z	3.9517

Project Details

Design Code:	AISC 360-16 LRFD
Provision:	LRFD
Country:	United States
User Name:	sales@mtsolar.us
Unit System:	imperial

Design Input Information

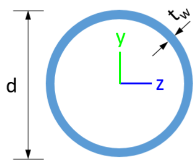
Design Factors

Φ_t	Φ_c	Φ_b	Φ_v
0.9	0.9	0.9	0.9

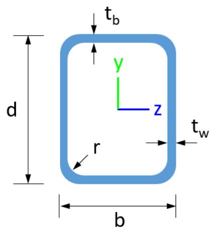
Design Materials

ID	E (ksi)	F_y (ksi)	F_u (ksi)
1	29000	50	65

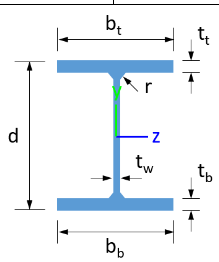
Section Dimensions



ID	Name	d (in)	t_w (in)
3	2in Pipe Sch 120	2.38	0.25
6	4in Pipe Sch 120	4.50	0.44
7	6in Pipe Sch 40	6.63	0.28



ID	Name	d (in)	b (in)	t_w (in)	t_b (in)	r (in)
17	HSS5x3x1/4	5.00	3.00	0.23	0.23	0.23



ID	Name	d (in)	t_w (in)	b_t (in)	b_b (in)	t_t (in)	t_b (in)	r (in)
20	W10x12	9.87	0.19	3.96	3.96	0.21	0.21	0.30

Section Properties

ID	Name	A (in^2)	J (in^4)	I_{yp} (in^4)	I_{zp} (in^4)	I_w (in^6)	S_{yp} (in^3)	S_{zp} (in^3)
[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]

3	2in Pipe Sch 120	1.67	1.91	0.96	0.96	0.00	1.13	1.13
6	4in Pipe Sch 120	5.58	23.29	11.64	11.64	0.00	7.24	7.24
7	6in Pipe Sch 40	5.58	56.28	28.14	28.14	0.00	11.28	11.28
17	HSS5x3x1/4	3.37	11.00	4.81	10.70	0.93	3.77	5.38
20	W10x12	3.54	0.05	2.18	53.80	50.90	1.74	12.60

Member Properties

Member ID	Section ID	K_zL (ft)	K_yL (ft)	L_b(ft)	C_b	LST	LSC	LD
1	7	22.38	22.38	10.66	-	300	200	1
2	6	1.30	1.30	2.00	-	300	200	1
3	17	0.92	0.92	1.42	1.19,1.18,1.19,1.18,1.18,1.19,1.18,1.18,1.18,1.19,1.18,1.18,1.19,1.14,1.18,1.18,1.18,1.18,1.18,1.18,1.19,1.25,1.18,1.18,1.19,1.16	300	200	1
4	17	2.44	2.44	3.75	1.70,1.68,1.70,1.67,1.68,1.70,1.67,1.67,1.68,1.68,1.68,1.68,1.77,1.70,1.67,1.67,1.67,1.67,1.69,1.69,1.71,1.70,1.68,1.68,2.00,1.70	300	200	1
5	17	1.52	1.52	2.33	1.69,1.67,1.69,1.67,1.68,1.69,1.67,1.67,1.67,1.68,1.67,1.67,1.69,1.63,1.67,1.67,1.67,1.67,1.68,1.68,1.69,1.76,1.67,1.67,1.69,1.65	300	200	1
6	17	0.92	0.92	1.42	1.19,1.18,1.19,1.18,1.19,1.19,1.18,1.18,1.19,1.19,1.19,1.19,1.19,1.16,1.18,1.18,1.18,1.18,1.19,1.19,1.19,1.23,1.19,1.19,1.19,1.17	300	200	1
7	17	1.52	1.52	2.33	1.68,1.67,1.68,1.67,1.67,1.68,1.67,1.67,1.67,1.68,1.67,1.67,1.68,1.64,1.67,1.67,1.67,1.67,1.68,1.68,1.68,1.76,1.67,1.67,1.68,1.65	300	200	1
8	20	1.33	1.33	2.05	1.26,1.26,1.26,1.26,1.26,1.26,1.26,1.26,1.24,1.30,1.25,1.25,1.14,1.42,1.26,1.26,1.26,1.27,1.26,1.26,1.23,1.33,1.25,1.25,1.02,1.51	300	200	1
9	3	2.60	2.60	4.00	-	300	200	1
10	17	2.44	2.44	3.75	1.69,1.67,1.69,1.67,1.68,1.69,1.67,1.67,1.68,1.67,1.68,1.68,1.75,1.69,1.67,1.67,1.67,1.67,1.68,1.68,1.71,1.69,1.68,1.68,2.01,1.69	300	200	1
11	20	1.33	1.33	2.05	1.27,1.27,1.27,1.27,1.27,1.27,1.28,1.28,1.29,1.19,1.28,1.28,1.33,1.65,1.27,1.27,1.27,1.26,1.28,1.28,1.30,1.07,1.28,1.28,1.35,1.49	300	200	1
12	6	1.30	1.30	2.00	-	300	200	1
13	20	4.88	4.00	7.50	1.28,1.28,1.28,1.29,1.28,1.28,1.28,1.28,1.28,1.37,1.28,1.28,1.27,1.47,1.28,1.28,1.28,1.29,1.28,1.28,1.28,2.34,1.28,1.28,1.27,1.31	300	200	1
14	20	4.88	4.00	7.50	1.27,1.28,1.27,1.28,1.28,1.27,1.28,1.28,1.28,1.28,1.28,1.28,1.52,1.28,1.28,1.28,1.28,1.28,1.28,1.28,1.26,1.28,1.28,1.28,3.18,1.30	300	200	1
15	20	4.20	4.20	2.00	2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33	300	200	1
16	20	4.20	4.20	2.00	2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33	300	200	1
101	7	22.38	22.38	10.66	-	300	200	1
102	6	1.30	1.30	2.00	-	300	200	1
103	17	0.92	0.92	1.42	1.19,1.18,1.19,1.18,1.18,1.19,1.18,1.18,1.18,1.19,1.18,1.18,1.19,1.15,1.18,1.18,1.18,1.18,1.18,1.18,1.19,1.22,1.18,1.18,1.19,1.17	300	200	1
104	17	2.44	2.44	3.75	1.69,1.67,1.69,1.67,1.68,1.69,1.67,1.67,1.67,1.67,1.67,1.67,1.72,1.69,1.67,1.67,1.67,1.67,1.68,1.68,1.70,1.69,1.67,1.67,1.82,1.69	300	200	1
105	17	1.52	1.52	2.33	1.68,1.67,1.68,1.67,1.67,1.68,1.67,1.67,1.67,1.67,1.67,1.67,1.68,1.64,1.67,1.67,1.67,1.67,1.67,1.67,1.68,1.72,1.67,1.67,1.68,1.65	300	200	1
106	17	0.92	0.92	1.42	1.19,1.18,1.19,1.18,1.18,1.19,1.18,1.18,1.18,1.18,1.18,1.18,1.19,1.16,1.18,1.18,1.18,1.18,1.18,1.18,1.19,1.22,1.18,1.18,1.19,1.17	300	200	1
107	17	1.52	1.52	2.33	1.68,1.67,1.68,1.67,1.67,1.68,1.67,1.67,1.67,1.68,1.67,1.67,1.68,1.64,1.67,1.67,1.67,1.67,1.67,1.67,1.68,1.73,1.67,1.67,1.68,1.65	300	200	1
108	20	1.33	1.33	2.05	2.39,2.38,2.39,2.38,2.38,2.39,2.36,2.36,2.31,2.08,2.35,2.35,1.60,1.65,2.37,2.37,2.36,2.27,2.36,2.36,2.28,2.06,2.34,2.34,1.12,1.50	300	200	1
109	3	2.60	2.60	4.00	-	300	200	1
110	17	2.44	2.44	3.75	1.69,1.67,1.69,1.67,1.68,1.69,1.67,1.67,1.68,1.67,1.68,1.68,1.73,1.69,1.67,1.67,1.67,1.67,1.68,1.68,1.70,1.69,1.67,1.67,1.87,1.69	300	200	1
111	20	1.33	1.33	2.05	2.28,2.30,2.28,2.29,2.29,2.28,2.16,2.16,2.10,1.90,2.11,2.11,2.06,1.37,2.25,2.25,2.21,2.36,2.12,2.12,2.09,1.30,2.11,2.11,1.95,1.50	300	200	1
112	6	1.30	1.30	2.00	-	300	200	1

113	20	4.88	4.00	7.50	1.06,1.06,1.06,1.06,1.06,1.06,1.06,1.06,1.06,1.05,1.06,1.06,1.07,1.71,1.06,1.06,1.06,1.06,1.06,1.06,1.03,1.06,1.06,1.07,1.25	300	200	1
114	20	4.88	4.00	7.50	1.06,1.06,1.06,1.06,1.06,1.06,1.06,1.06,1.05,1.06,1.06,1.06,1.04,1.12,1.06,1.06,1.06,1.06,1.06,1.05,1.07,1.06,1.06,1.02,1.25	300	200	1
115	20	10.20	10.20	10.20	1.17,1.17,1.17,1.17,1.17,1.17,1.17,1.17,1.16,1.24,1.16,1.16,1.14,1.08,1.17,1.17,1.17,1.18,1.17,1.17,1.15,1.56,1.16,1.16,1.13,1.10	300	200	1
116	20	10.20	10.20	10.20	1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.19,1.15,1.18,1.18,1.31,1.12,1.18,1.18,1.18,1.17,1.18,1.18,1.20,1.14,1.18,1.18,2.11,1.10	300	200	1
201	7	22.38	22.38	10.66	-	300	200	1
202	6	1.30	1.30	2.00	-	300	200	1
203	17	0.92	0.92	1.42	1.19,1.18,1.19,1.18,1.18,1.19,1.18,1.18,1.18,1.18,1.18,1.18,1.19,1.16,1.18,1.18,1.18,1.18,1.18,1.18,1.19,1.22,1.18,1.18,1.19,1.17	300	200	1
204	17	2.44	2.44	3.75	1.69,1.67,1.69,1.67,1.68,1.69,1.67,1.67,1.67,1.67,1.67,1.67,1.73,1.69,1.67,1.67,1.67,1.67,1.68,1.68,1.70,1.69,1.67,1.67,1.87,1.69	300	200	1
205	17	1.52	1.52	2.33	1.68,1.67,1.68,1.67,1.67,1.68,1.67,1.67,1.67,1.67,1.67,1.67,1.68,1.64,1.67,1.67,1.67,1.67,1.67,1.67,1.68,1.72,1.67,1.67,1.68,1.65	300	200	1
206	17	0.92	0.92	1.42	1.19,1.18,1.19,1.18,1.18,1.19,1.18,1.18,1.18,1.19,1.18,1.18,1.19,1.15,1.18,1.18,1.18,1.18,1.18,1.18,1.19,1.22,1.18,1.18,1.19,1.17	300	200	1
207	17	1.52	1.52	2.33	1.68,1.67,1.68,1.67,1.67,1.68,1.67,1.67,1.67,1.67,1.67,1.67,1.68,1.64,1.67,1.67,1.67,1.67,1.67,1.67,1.68,1.72,1.67,1.67,1.68,1.65	300	200	1
208	20	1.33	1.33	2.05	2.07,2.07,2.07,2.07,2.07,2.07,2.08,2.08,2.09,1.77,2.08,2.08,2.22,1.44,2.07,2.07,2.07,2.02,2.08,2.08,2.11,1.65,2.08,2.08,1.30,1.35	300	200	1
209	3	2.60	2.60	4.00	-	300	200	1
210	17	2.44	2.44	3.75	1.69,1.67,1.69,1.67,1.68,1.69,1.67,1.67,1.67,1.67,1.68,1.68,1.72,1.69,1.67,1.67,1.67,1.67,1.68,1.68,1.70,1.69,1.67,1.67,1.82,1.69	300	200	1
211	20	1.33	1.33	2.05	2.03,2.03,2.03,2.03,2.03,2.03,1.97,1.97,1.88,2.33,1.92,1.92,1.66,1.26,2.01,2.01,1.99,2.07,1.95,1.95,1.81,1.58,1.91,1.91,1.59,1.35	300	200	1
212	6	1.30	1.30	2.00	-	300	200	1
213	20	4.88	4.00	7.50	1.06,1.06,1.06,1.06,1.06,1.06,1.06,1.06,1.05,1.06,1.06,1.07,1.71,1.06,1.06,1.06,1.06,1.06,1.06,1.06,1.03,1.06,1.06,1.07,1.25	300	200	1
214	20	4.88	4.00	7.50	1.05,1.06,1.05,1.06,1.06,1.05,1.06,1.06,1.05,1.06,1.05,1.05,1.04,1.12,1.06,1.06,1.06,1.06,1.05,1.05,1.05,1.06,1.05,1.05,1.02,1.25	300	200	1
215	20	6.63	6.63	10.20	1.21,1.21,1.21,1.21,1.21,1.21,1.21,1.21,1.20,1.38,1.20,1.20,1.18,1.11,1.21,1.21,1.21,1.22,1.21,1.21,1.19,2.80,1.20,1.20,1.17,1.13	300	200	1
216	20	10.20	10.20	10.20	1.22,1.22,1.22,1.22,1.22,1.22,1.22,1.24,1.19,1.23,1.23,2.22,1.15,1.22,1.22,1.22,1.21,1.22,1.22,1.25,1.18,1.23,1.23,1.43,1.13	300	200	1
301	7	22.38	22.38	10.66	-	300	200	1
302	6	1.30	1.30	2.00	-	300	200	1
303	17	0.92	0.92	1.42	1.19,1.18,1.19,1.18,1.19,1.19,1.18,1.18,1.19,1.19,1.19,1.19,1.19,1.16,1.18,1.18,1.18,1.18,1.19,1.19,1.19,1.23,1.19,1.19,1.19,1.17	300	200	1
304	17	2.44	2.44	3.75	1.69,1.67,1.69,1.67,1.68,1.69,1.67,1.67,1.68,1.67,1.68,1.68,1.75,1.69,1.67,1.67,1.67,1.67,1.68,1.68,1.71,1.69,1.68,1.68,2.01,1.69	300	200	1
305	17	1.52	1.52	2.33	1.68,1.67,1.68,1.67,1.67,1.68,1.67,1.67,1.67,1.68,1.67,1.67,1.68,1.64,1.67,1.67,1.67,1.67,1.67,1.67,1.68,1.76,1.67,1.67,1.68,1.65	300	200	1
306	17	0.92	0.92	1.42	1.19,1.18,1.19,1.18,1.18,1.19,1.18,1.18,1.18,1.19,1.18,1.18,1.19,1.14,1.18,1.18,1.18,1.18,1.18,1.18,1.19,1.25,1.18,1.18,1.19,1.16	300	200	1
307	17	1.52	1.52	2.33	1.69,1.67,1.69,1.67,1.68,1.69,1.67,1.67,1.67,1.68,1.67,1.67,1.69,1.63,1.67,1.67,1.67,1.67,1.68,1.68,1.69,1.76,1.67,1.67,1.69,1.65	300	200	1
308	20	4.20	4.20	2.00	2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33	300	200	1
309	3	2.60	2.60	4.00	-	300	200	1
310	17	2.44	2.44	3.75	1.70,1.68,1.70,1.67,1.68,1.70,1.67,1.67,1.68,1.68,1.68,1.68,1.77,1.70,1.67,1.67,1.67,1.67,1.69,1.69,1.71,1.70,1.68,1.68,2.00,1.70	300	200	1
311	20	4.20	4.20	2.00	2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33	300	200	1
312	6	1.30	1.30	2.00	-	300	200	1
313	20	4.88	4.00	7.50	1.28,1.28,1.28,1.29,1.28,1.28,1.28,1.28,1.28,1.37,1.28,1.28,1.27,1.47,1.28,1.28,1.28,1.29,1.28,1.28,1.28,2.34,1.28,1.28,1.27,1.31	300	200	1
314	20	4.88	4.00	7.50	1.27,1.28,1.27,1.28,1.28,1.27,1.28,1.28,1.28,1.28,1.28,1.28,1.52,1.28,1.28,1.28,1.28,1.28,1.28,1.28,1.26,1.28,1.28,1.28,3.18,1.30	300	200	1

315	20	10.20	10.20	10.20	1.08,1.08,1.08,1.08,1.08,1.08,1.08,1.08,1.09,1.10,1.08,1.08,1.10,1.14,1.08,1.08,1.08,1.08,1.08,1.08,1.09,1.17,1.08,1.08,1.10,1.12	300	200	1
316	20	10.20	10.20	10.20	1.08,1.08,1.08,1.08,1.08,1.08,1.08,1.08,1.09,1.09,1.08,1.08,1.10,1.11,1.08,1.08,1.08,1.08,1.08,1.08,1.09,1.10,1.08,1.08,1.60,1.13	300	200	1

Member Design Capacity

Member ID	$\Phi_t P_n$ (kip)	$\Phi_c P_n$ (kip)	$\Phi_b M_{zn}$ (k-ft)	$\Phi_b M_{yn}$ (k-ft)	$\Phi_v V_{yn}$ (kip)	$\Phi_v V_{zn}$ (kip)
1	251.16	88.17	42.30	42.30	75.35	75.35
2	251.01	248.88	27.16	27.16	75.30	75.30
3	151.65	150.70	20.17	14.14	54.12	28.95
4	151.65	145.15	20.17	14.14	54.12	28.95
5	151.65	149.10	20.17	14.14	54.12	28.95
6	151.65	150.70	20.17	14.14	54.12	28.95
7	151.65	149.10	20.17	14.14	54.12	28.95
8	159.30	140.46	46.90	6.46	56.26	44.91
9	75.10	66.32	4.25	4.25	22.53	22.53
10	151.65	145.15	20.17	14.14	54.12	28.95
11	159.30	140.46	46.90	6.46	56.26	44.91
12	251.01	248.88	27.16	27.16	75.30	75.30
13	159.30	97.43	38.86	6.46	56.26	44.91
14	159.30	97.43	38.51	6.46	56.26	44.91
15	159.30	113.66	46.90	6.46	56.26	44.91
16	159.30	113.66	46.90	6.46	56.26	44.91
101	251.16	88.17	42.30	42.30	75.35	75.35
102	251.01	248.88	27.16	27.16	75.30	75.30
103	151.65	150.70	20.17	14.14	54.12	28.95
104	151.65	145.15	20.17	14.14	54.12	28.95
105	151.65	149.10	20.17	14.14	54.12	28.95
106	151.65	150.70	20.17	14.14	54.12	28.95
107	151.65	149.10	20.17	14.14	54.12	28.95
108	159.30	140.46	46.90	6.46	56.26	44.91
109	75.10	66.32	4.25	4.25	22.53	22.53
110	151.65	145.15	20.17	14.14	54.12	28.95
111	159.30	140.46	46.90	6.46	56.26	44.91
112	251.01	248.88	27.16	27.16	75.30	75.30
113	159.30	97.43	31.49	6.46	56.26	44.91
114	159.30	97.43	31.27	6.46	56.26	44.91
115	159.30	32.87	20.82	6.46	56.26	44.91
116	159.30	32.87	21.21	6.46	56.26	44.91
201	251.16	88.17	42.30	42.30	75.35	75.35
202	251.01	248.88	27.16	27.16	75.30	75.30
203	151.65	150.70	20.17	14.14	54.12	28.95
204	151.65	145.15	20.17	14.14	54.12	28.95
205	151.65	149.10	20.17	14.14	54.12	28.95
206	151.65	150.70	20.17	14.14	54.12	28.95
207	151.65	149.10	20.17	14.14	54.12	28.95
208	159.30	140.46	46.90	6.46	56.26	44.91
209	75.10	66.32	4.25	4.25	22.53	22.53
210	151.65	145.15	20.17	14.14	54.12	28.95
211	159.30	140.46	46.90	6.46	56.26	44.91
212	251.01	248.88	27.16	27.16	75.30	75.30

212	251.01	248.88	27.16	27.16	75.30	75.30
213	159.30	97.43	31.49	6.46	56.26	44.91
214	159.30	97.43	31.27	6.46	56.26	44.91
215	159.30	75.13	21.45	6.46	56.26	44.91
216	159.30	32.87	21.80	6.46	56.26	44.91
301	251.16	88.17	42.30	42.30	75.35	75.35
302	251.01	248.88	27.16	27.16	75.30	75.30
303	151.65	150.70	20.17	14.14	54.12	28.95
304	151.65	145.15	20.17	14.14	54.12	28.95
305	151.65	149.10	20.17	14.14	54.12	28.95
306	151.65	150.70	20.17	14.14	54.12	28.95
307	151.65	149.10	20.17	14.14	54.12	28.95
308	159.30	113.66	46.90	6.46	56.26	44.91
309	75.10	66.32	4.25	4.25	22.53	22.53
310	151.65	145.15	20.17	14.14	54.12	28.95
311	159.30	113.66	46.90	6.46	56.26	44.91
312	251.01	248.88	27.16	27.16	75.30	75.30
313	159.30	97.43	38.86	6.46	56.26	44.91
314	159.30	97.43	38.51	6.46	56.26	44.91
315	159.30	32.87	20.84	6.46	56.26	44.91
316	159.30	32.87	20.84	6.46	56.26	44.91

Design Ratio

Member ID	P	M_z	M_y	V_y	V_z	(P,M_z,M_y)	Worst LC	KL/r	δ	Status
1	0.161	0.168	0.091	0.003	0.007	0.206	#24	0.598	Not Required	Pass
2	0.001	0.369	0.011	0.082	0.001	0.380	#21	0.036	Not Required	Pass
3	0.002	0.572	0.012	0.056	0.004	0.584	#21	0.046	Not Required	Pass
4	0.001	0.566	0.032	0.057	0.008	0.598	#21	0.082	Not Required	Pass
5	0.002	0.355	0.006	0.057	0.001	0.362	#21	0.076	Not Required	Pass
6	0.002	0.708	0.039	0.072	0.010	0.749	#21	0.046	Not Required	Pass
7	0.002	0.439	0.052	0.070	0.014	0.457	#21	0.076	Not Required	Pass
8	0.001	0.148	0.043	0.043	0.005	0.172	#21	0.102	Not Required	Pass
9	0.002	0.085	0.032	0.003	0.002	0.118	#21	0.206	Not Required	Pass
10	0.003	0.695	0.030	0.070	0.005	0.700	#21	0.082	Not Required	Pass
11	0.003	0.147	0.049	0.044	0.005	0.167	#21	0.102	Not Required	Pass
12	0.001	0.511	0.020	0.103	0.002	0.525	#21	0.054	Not Required	Pass
13	0.004	0.110	0.119	0.058	0.007	0.165	#21	0.306	Not Required	Pass
14	0.002	0.109	0.113	0.057	0.006	0.164	#24	0.204	Not Required	Pass
15	0.000	0.020	0.011	0.016	0.002	0.031	#21	Not Required	Not Required	Pass
16	0.000	0.019	0.011	0.016	0.002	0.031	#21	Not Required	Not Required	Pass
101	0.220	0.187	0.014	0.003	0.001	0.275	#21	0.598	Not Required	Pass
102	0.001	0.626	0.021	0.129	0.002	0.647	#21	0.054	Not Required	Pass
103	0.002	0.891	0.018	0.089	0.003	0.910	#21	0.046	Not Required	Pass
104	0.003	0.887	0.043	0.089	0.009	0.914	#21	0.082	Not Required	Pass
105	0.002	0.553	0.048	0.088	0.013	0.566	#21	0.076	Not Required	Pass
106	0.003	0.871	0.012	0.087	0.002	0.883	#21	0.046	Not Required	Pass
107	0.003	0.541	0.041	0.087	0.010	0.551	#21	0.076	Not Required	Pass
108	0.001	0.058	0.044	0.050	0.005	0.102	#21	0.102	Not Required	Pass
109	0.004	0.095	0.014	0.001	0.000	0.111	#21	0.206	Not Required	Pass
110	0.002	0.863	0.043	0.086	0.010	0.895	#21	0.082	Not Required	Pass

111	0.003	0.054	0.044	0.050	0.005	0.099	#21	0.102	Not Required	Pass
112	0.001	0.603	0.020	0.126	0.002	0.624	#21	0.054	Not Required	Pass
113	0.004	0.280	0.125	0.070	0.007	0.368	#21	0.306	Not Required	Pass
114	0.004	0.291	0.127	0.070	0.007	0.372	#21	0.306	Not Required	Pass
115	0.013	0.417	0.061	0.056	0.005	0.483	#21	0.780	Not Required	Pass
116	0.004	0.411	0.065	0.056	0.005	0.476	#21	0.780	Not Required	Pass
201	0.220	0.187	0.014	0.003	0.001	0.275	#21	0.598	Not Required	Pass
202	0.001	0.603	0.020	0.125	0.002	0.624	#21	0.054	Not Required	Pass
203	0.003	0.872	0.012	0.087	0.002	0.884	#21	0.046	Not Required	Pass
204	0.002	0.864	0.043	0.086	0.010	0.896	#21	0.082	Not Required	Pass
205	0.003	0.542	0.041	0.087	0.010	0.552	#21	0.076	Not Required	Pass
206	0.002	0.891	0.018	0.089	0.003	0.910	#21	0.046	Not Required	Pass
207	0.002	0.553	0.048	0.088	0.013	0.566	#21	0.076	Not Required	Pass
208	0.001	0.082	0.054	0.056	0.005	0.096	#24	0.102	Not Required	Pass
209	0.004	0.095	0.014	0.001	0.000	0.111	#21	0.206	Not Required	Pass
210	0.003	0.887	0.043	0.089	0.009	0.914	#21	0.082	Not Required	Pass
211	0.003	0.086	0.053	0.056	0.005	0.097	#21	0.102	Not Required	Pass
212	0.001	0.626	0.021	0.129	0.002	0.647	#21	0.054	Not Required	Pass
213	0.004	0.281	0.125	0.070	0.007	0.368	#21	0.306	Not Required	Pass
214	0.004	0.291	0.127	0.070	0.007	0.372	#21	0.306	Not Required	Pass
215	0.005	0.261	0.063	0.050	0.005	0.327	#21	0.507	Not Required	Pass
216	0.003	0.248	0.063	0.050	0.005	0.312	#21	0.780	Not Required	Pass
301	0.161	0.168	0.091	0.003	0.007	0.206	#24	0.598	Not Required	Pass
302	0.001	0.511	0.020	0.103	0.002	0.525	#21	0.054	Not Required	Pass
303	0.002	0.709	0.040	0.072	0.010	0.749	#21	0.046	Not Required	Pass
304	0.003	0.695	0.030	0.070	0.005	0.700	#21	0.082	Not Required	Pass
305	0.002	0.439	0.052	0.070	0.014	0.457	#21	0.076	Not Required	Pass
306	0.002	0.572	0.012	0.056	0.004	0.584	#21	0.046	Not Required	Pass
307	0.002	0.355	0.006	0.057	0.001	0.362	#21	0.076	Not Required	Pass
308	0.000	0.019	0.011	0.016	0.002	0.031	#21	Not Required	Not Required	Pass
309	0.002	0.085	0.032	0.003	0.002	0.118	#21	0.206	Not Required	Pass
310	0.001	0.566	0.032	0.057	0.008	0.599	#21	0.122	Not Required	Pass
311	0.000	0.020	0.011	0.016	0.002	0.031	#21	Not Required	Not Required	Pass
312	0.001	0.369	0.011	0.082	0.001	0.380	#21	0.036	Not Required	Pass
313	0.004	0.110	0.119	0.058	0.007	0.165	#21	0.204	Not Required	Pass
314	0.002	0.109	0.113	0.057	0.006	0.164	#24	0.306	Not Required	Pass
315	0.013	0.445	0.061	0.044	0.005	0.513	#21	0.780	Not Required	Pass
316	0.004	0.442	0.065	0.043	0.005	0.506	#21	0.780	Not Required	Pass

Definitions

Φ_t	Safety factor for tensile
Φ_c	Safety factor for compression
Φ_b	Safety factor for flexure
Φ_v	Safety factor for shear
E	Modulus of elasticity
F_y	Specified minimum yield stress
F_u	Specified minimum tensile strength
A	Cross-sectional area
J	Torsional constant
I_{yp}	Moment of inertia about the Y axes
I_{zp}	Moment of inertia about the Z axes
I_w	Warping constant
S_{yp}	Plastic section modulus about the Y axis
S_{zp}	Plastic section modulus about the Z axis

KL	Effective length
C_b	Buckling modification factor (from all load combinations)
L_b	Length between braced points
LST	Limited slenderness for tension
LSC	Limited slenderness for compression
LD	Limited deflection
P_n	Nominal axial strength (tension/comression)
M_n	Nominal flexural strength (about Z/Y axis)
V_n	Nominal shear strength (along Z/Y axis)
P	Design ratio in case of axial force
M_z	Design ratio in case of bending about Z axis
M_y	Design ratio in case of bending about Y axis
V_y	Design ratio in case of shear along Y axis
V_z	Design ratio in case of shear along Z axis
(P,M_z,M_y)	Design ratio in case of axial force and bending action
KL/r	Design ratio in case of section slenderness
δ	Design ratio in case of member deflection
OK	Capacity is provided
NG	Capacity is not provided

REFERENCES	CALCULATIONS	RESULTS

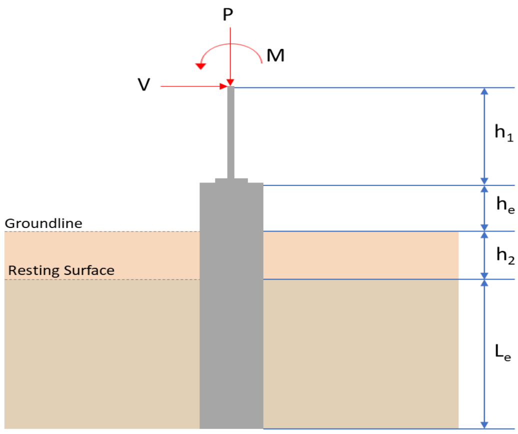
SkyCiv Foundation Design

Pile Foundation

Design Information :

Design code : IBC 2021 (International Building Code)
Unit System : Imperial

Pile Input



Geometry

Pile shape: round
$D = 36\ \mathrm{in}$ - Pile diameter
$L = 5.5\ \mathrm{ft}$ - Total pile length
$h_1 = 0\ \mathrm{ft}$ - Lateral load height from the top of the pile,
$h_2 = 0\ \mathrm{ft}$ - Depth to resisting surface
$h_e = 0\ \mathrm{ft}$ - Length of pile above the ground

Tabulation of Soil Parameters

Layer	Label	Allowable Bearing Pressure (q_a) (psf)	Allowable Lateral Pressure (R) (psf/ft)
1	Sand, silty sand, clayey sand, silty gravel & clayey gravel	2000.000	150.000

Tabulation of Loads

Load Component	ASD	LRFD
P (kip)	8.654	14.163
V_x (kip)	-0.113	-0.194
V_z (kip)	0.319	0.533
M_x (kipft)	1.074	1.804
M_z (kipft)	3.951	7.087

Material Properties

$f'_{ck} = 2.5\ \mathrm{ksi}$ - Concrete strength,

Required depth to resist lateral loads (ASD)

H - Point of application of the lateral load

$$H = h_1 + h_2 + h_e$$

$$H = (0\ \mathrm{ft}) + (0\ \mathrm{ft}) + (0\ \mathrm{ft})$$

$$H = 0\ \mathrm{ft}$$

Considering x-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{D}$$

$$H_o = \frac{(-0.113\ \mathrm{kip})}{(36\ \mathrm{in})}$$

$$H_o = -0.037667\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\ H)}{D}$$

$$M_o = \frac{(3.951\ \text{kipft}) + ((-0.113\ \text{kip}) \times (0\ \text{ft}))}{(36\ \text{in})}$$

$$M_o = 1.317\ \text{kipft/ft}$$

Required depth of embedment in earth:

$$L_x^3 - \left(14.14 \times \frac{H_o \times L_x}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,x} = 5.2749\ \text{ft}$ - Required depth in x-direction,

Considering z-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{D}$$

$$H_o = \frac{(0.319\ \text{kip})}{(36\ \text{in})}$$

$$H_o = 0.10633\ \text{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\ H)}{D}$$

$$M_o = \frac{(1.074\ \text{kipft}) + ((0.319\ \text{kip}) \times (0\ \text{ft}))}{(36\ \text{in})}$$

$$M_o = 0.358\ \text{kipft/ft}$$

Required depth of embedment in earth:

$$L_z^3 - \left(14.14 \times \frac{H_o \times L_z}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,z} = 4.4796\ \text{ft}$ - Required depth in z-direction,

Minimum embedded depth required:

$L_{e,req}$ - Depth of pile required,

$$L_{e,req} = MAX[L_{e,x}, L_{e,z}]$$

$$L_{e,req} = MAX[(5.2749\ \text{ft}), (4.4796\ \text{ft})]$$

$$L_{e,req} = 5.275\ \text{ft}$$

L_e - Actual embedded length of pile,

$$L_e = L - h_e - h_2$$

$$L_e = (5.5\ \text{ft}) - (0\ \text{ft}) - (0\ \text{ft})$$

$$L_e = 5.5\ \text{ft}$$

$Ratio$ - Embedded depth

$$Ratio = \frac{L_{e,req}}{L_e}$$

$$Ratio = \frac{(5.275\ \text{ft})}{(5.5\ \text{ft})}$$

$$Ratio = 0.9591$$

Status: **PASS**
Ratio: **0.960**

End-bearing Capacity (ASD)

A - Pile cros-section area

$$A = \pi \left(\frac{D}{2}\right)^2$$

$$A = \pi \times \left(\frac{(36\ \text{in})}{2}\right)^2$$

$$A = 7.0686\ \text{ft}^2$$

q - End-bearing pressure

$$q = \frac{P_v}{A}$$

$$q = \frac{(8.654 \text{ kip})}{(7.0686 \text{ ft}^2)}$$

$$q = 1.2243 \text{ kip/ft}^2$$

Check bearing capacity ratio:

$Ratio$ - Capacity

$$Ratio = \frac{q}{q_a}$$

$$Ratio = \frac{\left(1.2243 \text{ kip/ft}^2\right)}{(2000 \text{ psf})}$$

$$Ratio = 0.61215$$

Status: **PASS**
Ratio: **0.610**

Czerniak

Lateral Soil Pressure (ASD):

L/D - Length to least lateral dimension ratio,

$$L/D = \frac{L}{D}$$

$$L/D = \frac{(5.5 \text{ ft})}{(36 \text{ in})}$$

$$L/D = 1.8333$$

Since L/D $\leq$ 10,

Pile is short.

Considering x-direction:

$H_o = -0.037667 \text{ kip/ft}$ - Lateral force per length of pile,
$M_o = 1.317 \text{ kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\, M_o\, L_e) + \left(3\, H_o\, L_e^2\right)}{(6\, M_o) + (4\, H_o\, L_e)}$$

$$a = \frac{(4 \times (1.317 \text{ kipft/ft}) \times (5.5 \text{ ft})) + \left(3 \times (-0.037667 \text{ kip/ft}) \times (5.5 \text{ ft})^2\right)}{(6 \times (1.317 \text{ kipft/ft})) + (4 \times (-0.037667 \text{ kip/ft}) \times (5.5 \text{ ft}))}$$

$$a = 3.7102 \text{ ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{1.178\, [(4\, M_o) + (3\, H_o\, L_e)]^2}{L_e^2\, [(3\, M_o) + (2\, H_o\, L_e)]}$$

$$p = \frac{1.178 \times [(4 \times (1.317 \text{ kipft/ft})) + (3 \times (-0.037667 \text{ kip/ft}) \times (5.5 \text{ ft}))]^{\,2}}{(5.5 \text{ ft})^2 \times [(3 \times (1.317 \text{ kipft/ft})) + (2 \times (-0.037667 \text{ kip/ft}) \times (5.5 \text{ ft}))]}$$

$$p = 0.23773 \text{ kip/ft}^2$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{9.425\, [(2\, M_o) + (H_o\, L_e)]}{L_e^2}$$

$$s = \frac{9.425 \times [(2 \times (1.317 \text{ kipft/ft})) + ((-0.037667 \text{ kip/ft}) \times (5.5 \text{ ft}))]}{(5.5 \text{ ft})^2}$$

$$s = 0.75613 \text{ kip/ft}^2$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\, \frac{a}{2}$$

$$p_a = (150 \text{ psf/ft}) \times \frac{(3.7102 \text{ ft})}{2}$$

$$p_a = 0.27826 \text{ kip/ft}^2$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{\left(0.23773\ \mathrm{kip/ft^2}\right)}{\left(0.27826\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = 0.85432$$

Status: **PASS**
Ratio: **0.850**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\,L_e$$

$$p_s = (150\ \mathrm{psf/ft}) \times (5.5\ \mathrm{ft})$$

$$p_s = 0.825\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{\left(0.75613\ \mathrm{kip/ft^2}\right)}{\left(0.825\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = 0.91652$$

Status: **PASS**
Ratio: **0.920**

Considering z-direction:

$H_o = 0.10633\ \mathrm{kip/ft}$ - Lateral force per length of pile,
$M_o = 0.358\ \mathrm{kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (0.358\ \mathrm{kipft/ft}) \times (5.5\ \mathrm{ft})) + \left(3 \times (0.10633\ \mathrm{kip/ft}) \times (5.5\ \mathrm{ft})^2\right)}{(6 \times (0.358\ \mathrm{kipft/ft})) + (4 \times (0.10633\ \mathrm{kip/ft}) \times (5.5\ \mathrm{ft}))}$$

$$a = 3.9056\ \mathrm{ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{1.178\ [(4\,M_o) + (3\,H_o\,L_e)]^2}{L_e^2\ [(3\,M_o) + (2\,H_o\,L_e)]}$$

$$p = \frac{1.178 \times [(4 \times (0.358\ \mathrm{kipft/ft})) + (3 \times (0.10633\ \mathrm{kip/ft}) \times (5.5\ \mathrm{ft}))]^{\ 2}}{(5.5\ \mathrm{ft})^2 \times [(3 \times (0.358\ \mathrm{kipft/ft})) + (2 \times (0.10633\ \mathrm{kip/ft}) \times (5.5\ \mathrm{ft}))]}$$

$$p = 0.17623\ \mathrm{kip/ft^2}$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{9.425\ [(2\,M_o) + (H_o\,L_e)]}{L_e^2}$$

$$s = \frac{9.425 \times [(2 \times (0.358\ \mathrm{kipft/ft})) + ((0.10633\ \mathrm{kip/ft}) \times (5.5\ \mathrm{ft}))]}{(5.5\ \mathrm{ft})^2}$$

$$s = 0.4053\ \mathrm{kip/ft^2}$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\,\frac{a}{2}$$

$$p_a = (150\ \mathrm{psf/ft}) \times \frac{(3.9056\ \mathrm{ft})}{2}$$

$$p_a = 0.29292\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{\left(0.17623\ \mathrm{kip/ft^2}\right)}{\left(0.29292\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = 0.60164$$

Status: **PASS**
Ratio: **0.600**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\,L_e$$

$$p_s = (150\ \mathrm{psf/ft}) \times (5.5\ \mathrm{ft})$$

$$p_s = 0.825\ \mathrm{kip/ft^2}$$

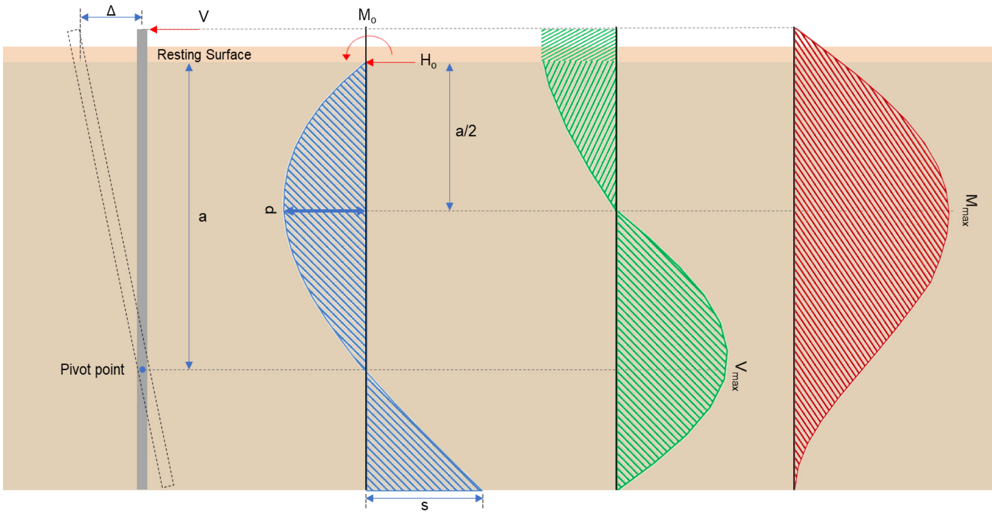
$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{(0.4053\ \mathrm{kip/ft^2})}{(0.825\ \mathrm{kip/ft^2})}$$

$$Ratio = 0.49127$$

Status: **PASS**
Ratio: **0.490**



Pile Rotation | Pressure Diagram | Shear Diagram | Moment Diagram

Shear force and Bending moment (x-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{D}$$

$$H_o = \frac{(-0.194\ \mathrm{kip})}{(36\ \mathrm{in})}$$

$$H_o = -0.064667\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\,H)}{D}$$

$$M_o = \frac{(7.087\ \mathrm{kipft}) + ((-0.194\ \mathrm{kip}) \times (0\ \mathrm{ft}))}{(36\ \mathrm{in})}$$

$$M_o = 2.3623\ \mathrm{kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(2.3623\ \mathrm{kipft/ft})}{(-0.064667\ \mathrm{kip/ft})}$$

$$E = 36.531\ \mathrm{ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (2.3623\ \text{kipft/ft}) \times (5.5\ \text{ft})) + \left(3 \times (-0.064667\ \text{kip/ft}) \times (5.5\ \text{ft})^2\right)}{(6 \times (2.3623\ \text{kipft/ft})) + (4 \times (-0.064667\ \text{kip/ft}) \times (5.5\ \text{ft}))}$$

$$a = 3.7085\ \text{ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\, D) \left[1 - \left[3 \left(\frac{4\,E}{L_e} + 3\right) \left(\frac{a}{L_e}\right)^2\right] + \left[4 \left(\frac{3\,E}{L_e} + 2\right) \left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((-0.064667\ \text{kip/ft}) \times (36\ \text{in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (36.531\ \text{ft})}{(5.5\ \text{ft})} + 3\right) \times \left(\frac{(3.7085\ \text{ft})}{(5.5\ \text{ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (36.531\ \text{ft})}{(5.5\ \text{ft})} + 2\right) \times \left(\frac{(3.7085\ \text{ft})}{(5.5\ \text{ft})}\right)^3\right]\right]$$

$$V_{max} = 2.4139\ \text{kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, D\, L_e) \left[\left(\frac{E}{L_e} + \frac{a}{2\,L_e}\right) - \left[\left(\frac{4\,E}{L_e} + 3\right) \left(\frac{a}{2\,L_e}\right)^3\right] + \left[\left(\frac{3\,E}{L_e} + 2\right) \left(\frac{a}{2\,L_e}\right)^4\right]\right]$$

$$M_{max} = ((-0.064667\ \text{kip/ft}) \times (36\ \text{in}) \times (5.5\ \text{ft})) \times \left[\left(\frac{(36.531\ \text{ft})}{(5.5\ \text{ft})} + \frac{(3.7085\ \text{ft})}{2 \times (5.5\ \text{ft})}\right) - \left[\left(\frac{4 \times (36.531\ \text{ft})}{(5.5\ \text{ft})} + 3\right) \times \left(\frac{(3.7085\ \text{ft})}{2 \times (5.5\ \text{ft})}\right)^3\right] + \left[\left(\frac{3 \times (36.531\ \text{ft})}{(5.5\ \text{ft})} + 2\right) \times \left(\frac{(3.7085\ \text{ft})}{2 \times (5.5\ \text{ft})}\right)^4\right]\right]$$

$$M_{max} = 6.54\ \text{kipft}$$

Shear force and Bending moment (z-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{D}$$

$$H_o = \frac{(0.533\ \text{kip})}{(36\ \text{in})}$$

$$H_o = 0.17767\ \text{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\, H)}{D}$$

$$M_o = \frac{(1.804\ \text{kipft}) + ((0.533\ \text{kip}) \times (0\ \text{ft}))}{(36\ \text{in})}$$

$$M_o = 0.60133\ \text{kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(0.60133\ \text{kipft/ft})}{(0.17767\ \text{kip/ft})}$$

$$E = 3.3846\ \text{ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\, M_o\, L_e) + \left(3\, H_o\, L_e^2\right)}{(6\, M_o) + (4\, H_o\, L_e)}$$

$$a = \frac{(4 \times (0.60133\ \text{kipft/ft}) \times (5.5\ \text{ft})) + \left(3 \times (0.17767\ \text{kip/ft}) \times (5.5\ \text{ft})^2\right)}{(6 \times (0.60133\ \text{kipft/ft})) + (4 \times (0.17767\ \text{kip/ft}) \times (5.5\ \text{ft}))}$$

$$a = 3.905\ \text{ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\, b) \left[1 - \left[3 \left(\frac{4\,E}{L_e} + 3\right) \left(\frac{a}{L_e}\right)^2\right] + \left[4 \left(\frac{3\,E}{L_e} + 2\right) \left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((0.17767\ \text{kip/ft}) \times (36\ \text{in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (3.3846\ \text{ft})}{(5.5\ \text{ft})} + 3\right) \times \left(\frac{(3.905\ \text{ft})}{(5.5\ \text{ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (3.3846\ \text{ft})}{(5.5\ \text{ft})} + 2\right) \times \left(\frac{(3.905\ \text{ft})}{(5.5\ \text{ft})}\right)^3\right]\right]$$

$$V_{max} = 0.93444\ \text{kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, b\, L_e) \left[\left(\frac{E}{L_e} + \frac{a}{2\, L_e}\right) - \left[\left(\frac{4\, E}{L_e} + 3\right) \left(\frac{a}{2\, L_e}\right)^3\right] + \left[\left(\frac{3\, E}{L_e} + 2\right) \left(\frac{a}{2\, L_e}\right)^4\right]\right]$$

$$M_{max} = ((0.17767\ \text{kip/ft}) \times (36\ \text{in}) \times (5.5\ \text{ft})) \times \left[\left(\frac{(3.3846\ \text{ft})}{(5.5\ \text{ft})} + \frac{(3.905\ \text{ft})}{2 \times (5.5\ \text{ft})}\right) - \left[\left(\frac{4 \times (3.3846\ \text{ft})}{(5.5\ \text{ft})} + 3\right) \times \left(\frac{(3.905\ \text{ft})}{2 \times (5.5\ \text{ft})}\right)^3\right] + \left[\left(\frac{3 \times (3.3846\ \text{ft})}{(5.5\ \text{ft})} + 2\right) \times \left(\frac{(3.905\ \text{ft})}{2 \times (5.5\ \text{ft})}\right)^4\right]\right]$$

$$M_{max} = 2.3075\ \text{kipft}$$

Minimum Reinforcement Check (LRFD)

Parameters:

$f'_{ck} = 2.5\ \text{ksi}$ - Concrete strength,
$f_{yk} = 60\ \text{ksi}$ - Longitudinal reinforcement strength,
$\phi = 0.65$ - Reduction factor for axial strength,
Table 22.4.2.1 $\alpha = 0.85$ - Alpha factor for axial strength,
$A_g = 1017.9\ \text{in}^2$ - Gross area of concrete,

Longitudinal reinforcement:

Required reinforcement due to axial load, $A_{st,required}$
22.4.2.2, 10.6.1.1 $A_{st,required}$

$$A_{st,required} = Min\left[\frac{\frac{P}{\phi\, \alpha} - (0.85\, f'_{ck}\, A_g)}{f_{yk} - (0.85\, f'_{ck})}, (0.08\, A_g)\right]$$

$$A_{st,required} = Min\left[\frac{\frac{(14.163\ \text{kip})}{(0.65) \times (0.85)} - (0.85 \times (2.5\ \text{ksi}) \times (1017.9\ \text{in}^2))}{(60\ \text{ksi}) - (0.85 \times (2.5\ \text{ksi}))}, (0.08 \times (1017.9\ \text{in}^2))\right]$$

$$A_{st,required} = -36.93\ \text{in}^2$$

A_{min} - Governing minimum reinforcement area,

$$A_{min} = Max\left[A_{st,required}, (0.0018\, A_g)\right]$$

$$A_{min} = Max\left[(-36.93\ \text{in}^2), (0.0018 \times (1017.9\ \text{in}^2))\right]$$

$$A_{min} = 1.8322\ \text{in}^2$$

n_{rebar} - Required number of reinforcement,

$$n_{rebar} = \frac{A_{min}}{A_{rebar}}$$

$$n_{rebar} = \frac{(1.8322\ \text{in}^2)}{(0.3068\ \text{in}^2)}$$

$$n_{rebar} = 6$$

A_{st} - Actual total reinforcement area,

$$A_{st} = n_{rebar}\, \frac{\pi\, d_{bar}^2}{4}$$

$$A_{st} = (6) \times \frac{\pi \times (0.625\ \text{in})^2}{4}$$

$$A_{st} = 1.8408\ \text{in}^2$$

$Ratio$ - Capacity

$$Ratio = \frac{A_{min}}{A_{st}}$$

$$(1.8322\ \text{in}^2)$$

	$$Ratio = \frac{}{(1.8408\ \mathrm{in}^2)}$$ $$Ratio = 0.99533$$	Status: **PASS** Ratio: **1.000**
25.2.3	s_{rebar} - Minimum spacing of reinforcement, $$s_{rebar} = Max\left[1.5, \left(1.5\ d_{bar}\right)\right]$$ $$s_{rebar} = Max\left[1.5, \left(1.5 \times (0.625\ \mathrm{in})\right)\right]$$ $$s_{rebar} = 1.5\ \mathrm{in}$$	
	Ties:	
25.7.2.2 *25.7.2.1*	Since longitudinal reinforcement is $\leq$ No. 10ø: Use #3(0.375 in) s_{ties} - Maximum center-to-center spacing of ties, $$s_{ties} = Min\left[\left(16\ d_{bar}\right), \left(48\ d_{ties}\right), D\right]$$ $$s_{ties} = Min\left[\left(16 \times (0.625\ \mathrm{in})\right), \left(48 \times (0.375\ \mathrm{in})\right), (36\ \mathrm{in})\right]$$ $$s_{ties} = 10\ \mathrm{in}$$	
	Summary: Main reinforcement: **6 - #5 (0.625 in)** Ties: **#3(0.375 in) - 10 in**	
	Axial Compression Strength (ACI 318-19, LRFD)	
22.4.2.2	ϕP_N - Allowable axial compressive strength $$\phi P_N = \phi\ 0.85\ \left[\left(0.85\ f'_{ck}\ \left[A_g - A_{st}\right]\right) + \left(f_{yk}\ A_{st}\right)\right]$$ $$\phi P_N = (0.65) \times 0.85 \times \left[\left(0.85 \times (2.5\ \mathrm{ksi}) \times \left[\left(1017.9\ \mathrm{in}^2\right) - \left(1.8408\ \mathrm{in}^2\right)\right]\right) + \left((60\ \mathrm{ksi}) \times \left(1.8408\ \mathrm{in}^2\right)\right)\right]$$ $$\phi P_N = 1253.9\ \mathrm{kip}$$ $Ratio$ - Capacity $$Ratio = \frac{P}{\phi P_N}$$ $$Ratio = \frac{(14.163\ \mathrm{kip})}{(1253.9\ \mathrm{kip})}$$ $$Ratio = 0.011295$$	Status: **PASS** Ratio: **0.010**
	Shear Strength (ACI 318-19, LRFD)	
	Parameters:	
22.5.2.2	$b_w = 36\ \mathrm{in}$ - Effective width, d - Effective depth $$d = 0.80\ D$$ $$d = 0.80 \times (36\ \mathrm{in})$$ $$d = 28.8\ \mathrm{in}$$	
22.5.5.1.3	λ_s - size effect modification factor $$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{d}{10}}}, 1\right]$$ $$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{(28.8\ \mathrm{in})}{10}}}, 1\right]$$ $$\lambda_s = 0.71796$$	
22.5.5.1.1	The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5\ \mathrm{ksi} \rightarrow 2500\ \mathrm{psi}$, $V_{c,max}$ - Max shear strength of concrete $$V_{c,max} = 5\ \lambda_s\ \sqrt{f'_{ck}}\ b_w\ d$$ $$V_{c,max} = 5 \times (0.71796) \times \sqrt{(2500\ \mathrm{psi})} \times (36\ \mathrm{in}) \times (28.8\ \mathrm{in})$$	

$$V_{c,max} = 186.09\ \text{kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5\ \text{ksi} \rightarrow 2500\ \text{psi}$, $P = 14.163\ \text{kip} \rightarrow 14163\ \text{lbf}$,

22.5.5.1.1(a) $V_{c,a}$ - Shear strength of concrete (a)

$$V_{c,a} = \left[2\ \lambda_s\ \sqrt{f'_{ck}} + \frac{P}{6\ A_g}\right]\ b_w\ d$$

$$V_{c,a} = \left[2 \times (0.71796) \times \sqrt{(2500\ \text{psi})} + \frac{(14163\ \text{lbf})}{6 \times (1017.9\ \text{in}^2)}\right] \times (36\ \text{in}) \times (28.8\ \text{in})$$

$$V_{c,a} = 76.842\ \text{kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5\ \text{ksi} \rightarrow 2500\ \text{psi}$,

22.5.5.1.2 $V_{c,b}$ - Shear strength of concrete (b)

$$V_{c,b} = \left[2\ \lambda_s\ \sqrt{f'_{ck}} + \left(0.05\ f'_{ck}\right)\right]\ b_w\ d$$

$$V_{c,b} = \left[2 \times (0.71796) \times \sqrt{(2500\ \text{psi})} + (0.05 \times (2500\ \text{psi}))\right] \times (36\ \text{in}) \times (28.8\ \text{in})$$

$$V_{c,b} = 204.04\ \text{kip}$$

V_c - Governing shear strength of concrete

$$V_c = Min\left[V_{c,max}, V_{c,a}, V_{c,b}\right]$$

$$V_c = Min\left[(186.09\ \text{kip}), (76.842\ \text{kip}), (204.04\ \text{kip})\right]$$

$$V_c = 76.842\ \text{kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5\ \text{ksi} \rightarrow 2500\ \text{psi}$,

22.5.1.2 $V_{s,a}$ - Shear strength of steel (a)

$$V_{s,a} = 8\ \sqrt{f'_{ck}}\ b_w\ d$$

$$V_{s,a} = 8 \times \sqrt{(2500\ \text{psi})} \times (36\ \text{in}) \times (28.8\ \text{in})$$

$$V_{s,a} = 414.72\ \text{kip}$$

A_v - Ties rebar area,

$$A_v = \frac{\pi\ d_{ties}^2}{4}$$

$$A_v = \frac{\pi \times (0.375\ \text{in})^2}{4}$$

$$A_v = 0.11045\ \text{in}^2$$

22.5.8.5.3 $V_{s,b}$ - Shear strength of steel (b)

$$V_{s,b} = \frac{2\ A_v\ f_{ywk}\ d}{s_{ties}}$$

$$V_{s,b} = \frac{2 \times \left(0.11045\ \text{in}^2\right) \times (60\ \text{ksi}) \times (28.8\ \text{in})}{(10\ \text{in})}$$

$$V_{s,b} = 38.17\ \text{kip}$$

V_s - Governing shear strength of steel

$$V_s = MIN\left[V_{s,a}, V_{s,b}\right]$$

$$V_s = MIN\left[(414.72\ \text{kip}), (38.17\ \text{kip})\right]$$

$$V_s = 38.17\ \text{kip}$$

22.5.1.1 ϕV_n - Allowable shear strength

$$\phi V_n = \phi\ (V_c + V_s)$$

$$\phi V_n = (0.65) \times ((76.842\ \text{kip}) + (38.17\ \text{kip}))$$

$$\phi V_n = 74.758\ \text{kip}$$

Considering x-direction:

$V_{max} = 2.4139\ \text{kip}$ - Maximum shear force in the x-direction,
$Ratio$ - Capacity

$$Ratio = \frac{V_{max}}{\phi V_n}$$

	$$Ratio = \frac{(2.4139\ \text{kip})}{(74.758\ \text{kip})}$$ $$Ratio = 0.032289$$	Status: **PASS** Ratio: **0.030**
	Considering z-direction: $V_{max} = 0.93444\ \text{kip}$ - Maximum shear force in the z-direction, $Ratio$ - Capacity $$Ratio = \frac{V_{max}}{\phi V_n}$$ $$Ratio = \frac{(0.93444\ \text{kip})}{(74.758\ \text{kip})}$$ $$Ratio = 0.012499$$	Status: **PASS** Ratio: **0.010**
14.5.2.1b	**Flexural Strength (ACI 318-19, LRFD)** S_m - Section modulus $$S_m = \frac{\pi\, D^3}{32}$$ $$S_m = \frac{\pi \times (36\ \text{in})^3}{32}$$ $$S_m = 4580.4\ \text{in}^3$$ $\lambda = 1$ - Concrete modification factor (Normal concrete), Allowable flexural strength: M_n shall be the lesser of: $\phi M_{n,1}$ $$\phi M_{n,1} = \phi \times 5 \times \lambda \times \sqrt{(f'_c)} \times S_m$$ $$\phi M_{n,1} = 0.65 \times 5 \times 1 \times \sqrt{(2.5ksi)} \times 4580.442in^3$$ $$\phi M_{n,1} = 62.027kipft$$ $\phi M_{n,2}$ $$\phi M_{n,2} = \phi\, 0.85\, f'_{ck}\, S_m$$ $$\phi M_{n,2} = (0.65) \times 0.85 \times (2.5\ \text{ksi}) \times \left(4580.4\ \text{in}^3\right)$$ $$\phi M_{n,2} = 527.23\ \text{kipft}$$ Therefore, ϕM_n - Allowable flexural strength, $$\phi M_n = MIN[\phi M_{n,1}, \phi M_{n,2}]$$ $$\phi M_n = MIN[(62.027\ \text{kipft}), (527.23\ \text{kipft})]$$ $$\phi M_n = 62.027\ \text{kipft}$$ **Considering x-direction:** $M_{max} = 6.54\ \text{kipft}$ - Maximum moment in the x-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\phi M_n}$$ $$Ratio = \frac{(6.54\ \text{kipft})}{(62.027\ \text{kipft})}$$ $$Ratio = 0.10544$$	Status: **PASS** Ratio: **0.110**
	Considering z-direction: $M_{max} = 2.3075\ \text{kipft}$ - Maximum moment in the z-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\ }$$	

	$$Ratio = \frac{}{\phi M_n}$$ $$Ratio = \frac{(2.3075 \text{ kipft})}{(62.027 \text{ kipft})}$$ $$Ratio = 0.037201$$	Status: **PASS** Ratio: **0.040**

REFERENCES	CALCULATIONS	RESULTS

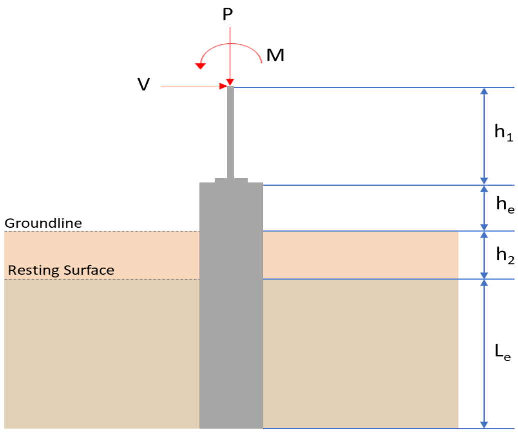
SkyCiv Foundation Design

Pile Foundation

Design Information :

Design code : IBC 2021 (International Building Code)
Unit System : Imperial

Pile Input



Geometry

Pile shape: round
$D = 36\ \mathrm{in}$ - Pile diameter
$L = 5.5\ \mathrm{ft}$ - Total pile length
$h_1 = 0\ \mathrm{ft}$ - Lateral load height from the top of the pile,
$h_2 = 0\ \mathrm{ft}$ - Depth to resisting surface
$h_e = 0\ \mathrm{ft}$ - Length of pile above the ground

Tabulation of Soil Parameters

Layer	Label	Allowable Bearing Pressure (q_a) (psf)	Allowable Lateral Pressure (R) (psf/ft)
1	Sand, silty sand, clayey sand, silty gravel & clayey gravel	2000.000	150.000

Tabulation of Loads

Load Component	ASD	LRFD
P (kip)	8.654	14.162
V_x (kip)	-0.113	-0.194
V_z (kip)	-0.319	-0.534
M_x (kipft)	-1.077	-1.809
M_z (kipft)	3.952	7.089

Material Properties

$f'_{ck} = 2.5\ \mathrm{ksi}$ - Concrete strength,

Required depth to resist lateral loads (ASD)

H - Point of application of the lateral load

$$H = h_1 + h_2 + h_e$$

$$H = (0\ \mathrm{ft}) + (0\ \mathrm{ft}) + (0\ \mathrm{ft})$$

$$H = 0\ \mathrm{ft}$$

Considering x-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{D}$$

$$H_o = \frac{(-0.113\ \mathrm{kip})}{(36\ \mathrm{in})}$$

$$H_o = -0.037667\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\ H)}{D}$$

$$M_o = \frac{(3.952\ \text{kipft}) + ((-0.113\ \text{kip}) \times (0\ \text{ft}))}{(36\ \text{in})}$$

$$M_o = 1.3173\ \text{kipft/ft}$$

Required depth of embedment in earth:

$$L_x^3 - \left(14.14 \times \frac{H_o \times L_x}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,x} = 5.2754\ \text{ft}$ - Required depth in x-direction,

Considering z-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{D}$$

$$H_o = \frac{(-0.319\ \text{kip})}{(36\ \text{in})}$$

$$H_o = -0.10633\ \text{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\ H)}{D}$$

$$M_o = \frac{(1.077\ \text{kipft}) + ((-0.319\ \text{kip}) \times (0\ \text{ft}))}{(36\ \text{in})}$$

$$M_o = 0.359\ \text{kipft/ft}$$

Required depth of embedment in earth:

$$L_z^3 - \left(14.14 \times \frac{H_o \times L_z}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,z} = 2.6482\ \text{ft}$ - Required depth in z-direction,

Minimum embedded depth required:

$L_{e,req}$ - Depth of pile required,

$$L_{e,req} = MAX[L_{e,x}, L_{e,z}]$$

$$L_{e,req} = MAX[(5.2754\ \text{ft}), (2.6482\ \text{ft})]$$

$$L_{e,req} = 5.275\ \text{ft}$$

L_e - Actual embedded length of pile,

$$L_e = L - h_e - h_2$$

$$L_e = (5.5\ \text{ft}) - (0\ \text{ft}) - (0\ \text{ft})$$

$$L_e = 5.5\ \text{ft}$$

$Ratio$ - Embedded depth

$$Ratio = \frac{L_{e,req}}{L_e}$$

$$Ratio = \frac{(5.275\ \text{ft})}{(5.5\ \text{ft})}$$

$$Ratio = 0.9591$$

Status: **PASS**
Ratio: **0.960**

End-bearing Capacity (ASD)

A - Pile cros-section area

$$A = \pi \left(\frac{D}{2}\right)^2$$

$$A = \pi \times \left(\frac{(36\ \text{in})}{2}\right)^2$$

$$A = 7.0686\ \text{ft}^2$$

q - End-bearing pressure

$$q = \frac{P_v}{A}$$

$$q = \frac{(8.654\ \mathrm{kip})}{(7.0686\ \mathrm{ft}^2)}$$

$$q = 1.2243\ \mathrm{kip/ft}^2$$

Check bearing capacity ratio:

$Ratio$ - Capacity

$$Ratio = \frac{q}{q_a}$$

$$Ratio = \frac{\left(1.2243\ \mathrm{kip/ft}^2\right)}{(2000\ \mathrm{psf})}$$

$$Ratio = 0.61215$$

Status: **PASS**
Ratio: **0.610**

Czerniak

Lateral Soil Pressure (ASD):

L/D - Length to least lateral dimension ratio,

$$L/D = \frac{L}{D}$$

$$L/D = \frac{(5.5\ \mathrm{ft})}{(36\ \mathrm{in})}$$

$$L/D = 1.8333$$

Since L/D $\leq$ 10,

Pile is short.

Considering x-direction:

$H_o = -0.037667\ \mathrm{kip/ft}$ - Lateral force per length of pile,
$M_o = 1.3173\ \mathrm{kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (1.3173\ \mathrm{kipft/ft}) \times (5.5\ \mathrm{ft})) + \left(3 \times (-0.037667\ \mathrm{kip/ft}) \times (5.5\ \mathrm{ft})^2\right)}{(6 \times (1.3173\ \mathrm{kipft/ft})) + (4 \times (-0.037667\ \mathrm{kip/ft}) \times (5.5\ \mathrm{ft}))}$$

$$a = 3.7102\ \mathrm{ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{1.178\ [(4\,M_o) + (3\,H_o\,L_e)]^2}{L_e^2\ [(3\,M_o) + (2\,H_o\,L_e)]}$$

$$p = \frac{1.178 \times [(4 \times (1.3173\ \mathrm{kipft/ft})) + (3 \times (-0.037667\ \mathrm{kip/ft}) \times (5.5\ \mathrm{ft}))]^{\,2}}{(5.5\ \mathrm{ft})^2 \times [(3 \times (1.3173\ \mathrm{kipft/ft})) + (2 \times (-0.037667\ \mathrm{kip/ft}) \times (5.5\ \mathrm{ft}))]}$$

$$p = 0.2378\ \mathrm{kip/ft}^2$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{9.425\ [(2\,M_o) + (H_o\,L_e)]}{L_e^2}$$

$$s = \frac{9.425 \times [(2 \times (1.3173\ \mathrm{kipft/ft})) + ((-0.037667\ \mathrm{kip/ft}) \times (5.5\ \mathrm{ft}))]}{(5.5\ \mathrm{ft})^2}$$

$$s = 0.75634\ \mathrm{kip/ft}^2$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\,\frac{a}{2}$$

$$p_a = (150\ \mathrm{psf/ft}) \times \frac{(3.7102\ \mathrm{ft})}{2}$$

$$p_a = 0.27826\ \mathrm{kip/ft}^2$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{\left(0.2378\ \mathrm{kip/ft^2}\right)}{\left(0.27826\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = 0.85458$$

Status: **PASS**
Ratio: **0.850**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\, L_e$$

$$p_s = (150\ \mathrm{psf/ft}) \times (5.5\ \mathrm{ft})$$

$$p_s = 0.825\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{\left(0.75634\ \mathrm{kip/ft^2}\right)}{\left(0.825\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = 0.91677$$

Status: **PASS**
Ratio: **0.920**

Considering z-direction:

$H_o = -0.10633\ \mathrm{kip/ft}$ - Lateral force per length of pile,
$M_o = 0.359\ \mathrm{kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\, M_o\, L_e) + \left(3\, H_o\, L_e^2\right)}{(6\, M_o) + (4\, H_o\, L_e)}$$

$$a = \frac{(4 \times (0.359\ \mathrm{kipft/ft}) \times (5.5\ \mathrm{ft})) + \left(3 \times (-0.10633\ \mathrm{kip/ft}) \times (5.5\ \mathrm{ft})^2\right)}{(6 \times (0.359\ \mathrm{kipft/ft})) + (4 \times (-0.10633\ \mathrm{kip/ft}) \times (5.5\ \mathrm{ft}))}$$

$$a = 3.9053\ \mathrm{ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{1.178\ [(4\, M_o) + (3\, H_o\, L_e)]^2}{L_e^2\ [(3\, M_o) + (2\, H_o\, L_e)]}$$

$$p = \frac{1.178 \times [(4 \times (0.359\ \mathrm{kipft/ft})) + (3 \times (-0.10633\ \mathrm{kip/ft}) \times (5.5\ \mathrm{ft}))]^{\ 2}}{(5.5\ \mathrm{ft})^2 \times [(3 \times (0.359\ \mathrm{kipft/ft})) + (2 \times (-0.10633\ \mathrm{kip/ft}) \times (5.5\ \mathrm{ft}))]}$$

$$p = -0.04263\ \mathrm{kip/ft^2}$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{9.425\ [(2\, M_o) + (H_o\, L_e)]}{L_e^2}$$

$$s = \frac{9.425 \times [(2 \times (0.359\ \mathrm{kipft/ft})) + ((-0.10633\ \mathrm{kip/ft}) \times (5.5\ \mathrm{ft}))]}{(5.5\ \mathrm{ft})^2}$$

$$s = 0.041491\ \mathrm{kip/ft^2}$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\, \frac{a}{2}$$

$$p_a = (150\ \mathrm{psf/ft}) \times \frac{(3.9053\ \mathrm{ft})}{2}$$

$$p_a = 0.2929\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{\left(-0.04263\ \mathrm{kip/ft^2}\right)}{\left(0.2929\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = -0.14555$$

Status: **PASS**
Ratio: **-0.150**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\, L_e$$

$$p_s = (150\ \mathrm{psf/ft}) \times (5.5\ \mathrm{ft})$$

$$p_s = 0.825\ \mathrm{kip/ft^2}$$

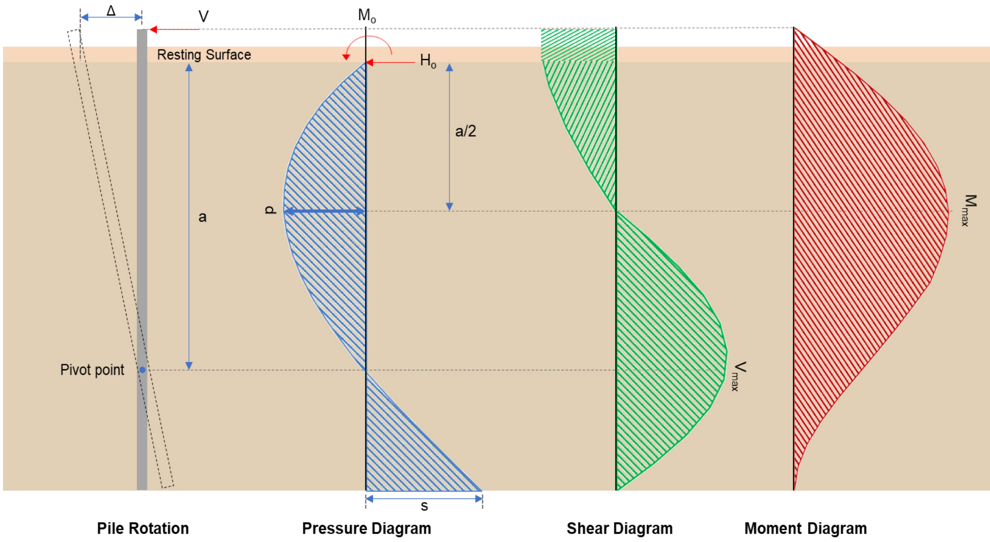
$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{(0.041491\ \mathrm{kip/ft^2})}{(0.825\ \mathrm{kip/ft^2})}$$

$$Ratio = 0.050292$$

Status: **PASS**
Ratio: **0.050**



Shear force and Bending moment (x-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{D}$$

$$H_o = \frac{(-0.194\ \mathrm{kip})}{(36\ \mathrm{in})}$$

$$H_o = -0.064667\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\, H)}{D}$$

$$M_o = \frac{(7.089\ \mathrm{kipft}) + ((-0.194\ \mathrm{kip}) \times (0\ \mathrm{ft}))}{(36\ \mathrm{in})}$$

$$M_o = 2.363\ \mathrm{kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(2.363\ \mathrm{kipft/ft})}{(-0.064667\ \mathrm{kip/ft})}$$

$$E = 36.541\ \mathrm{ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\, M_o\, L_e) + \left(3\, H_o\, L_e^2\right)}{(6\, M_o) + (4\, H_o\, L_e)}$$

$$a = \frac{(4 \times (2.363\ \text{kipft/ft}) \times (5.5\ \text{ft})) + \left(3 \times (-0.064667\ \text{kip/ft}) \times (5.5\ \text{ft})^{2}\right)}{(6 \times (2.363\ \text{kipft/ft})) + (4 \times (-0.064667\ \text{kip/ft}) \times (5.5\ \text{ft}))}$$

$$a = 3.7085\ \text{ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\, D) \left[1 - \left[3 \left(\frac{4\,E}{L_e} + 3\right) \left(\frac{a}{L_e}\right)^2\right] + \left[4 \left(\frac{3\,E}{L_e} + 2\right) \left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((-0.064667\ \text{kip/ft}) \times (36\ \text{in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (36.541\ \text{ft})}{(5.5\ \text{ft})} + 3\right) \times \left(\frac{(3.7085\ \text{ft})}{(5.5\ \text{ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (36.541\ \text{ft})}{(5.5\ \text{ft})} + 2\right) \times \left(\frac{(3.7085\ \text{ft})}{(5.5\ \text{ft})}\right)^3\right]\right]$$

$$V_{max} = 2.4145\ \text{kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, D\, L_e) \left[\left(\frac{E}{L_e} + \frac{a}{2\,L_e}\right) - \left[\left(\frac{4\,E}{L_e} + 3\right) \left(\frac{a}{2\,L_e}\right)^3\right] + \left[\left(\frac{3\,E}{L_e} + 2\right) \left(\frac{a}{2\,L_e}\right)^4\right]\right]$$

$$M_{max} = ((-0.064667\ \text{kip/ft}) \times (36\ \text{in}) \times (5.5\ \text{ft})) \times \left[\left(\frac{(36.541\ \text{ft})}{(5.5\ \text{ft})} + \frac{(3.7085\ \text{ft})}{2 \times (5.5\ \text{ft})}\right) - \left[\left(\frac{4 \times (36.541\ \text{ft})}{(5.5\ \text{ft})} + 3\right) \times \left(\frac{(3.7085\ \text{ft})}{2 \times (5.5\ \text{ft})}\right)^3\right] + \left[\left(\frac{3 \times (36.541\ \text{ft})}{(5.5\ \text{ft})} + 2\right) \times \left(\frac{(3.7085\ \text{ft})}{2 \times (5.5\ \text{ft})}\right)^4\right]\right]$$

$$M_{max} = 6.5418\ \text{kipft}$$

Shear force and Bending moment (z-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{D}$$

$$H_o = \frac{(-0.534\ \text{kip})}{(36\ \text{in})}$$

$$H_o = -0.178\ \text{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\, H)}{D}$$

$$M_o = \frac{(1.809\ \text{kipft}) + ((-0.534\ \text{kip}) \times (0\ \text{ft}))}{(36\ \text{in})}$$

$$M_o = 0.603\ \text{kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(0.603\ \text{kipft/ft})}{(-0.178\ \text{kip/ft})}$$

$$E = 3.3876\ \text{ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (0.603\ \text{kipft/ft}) \times (5.5\ \text{ft})) + \left(3 \times (-0.178\ \text{kip/ft}) \times (5.5\ \text{ft})^2\right)}{(6 \times (0.603\ \text{kipft/ft})) + (4 \times (-0.178\ \text{kip/ft}) \times (5.5\ \text{ft}))}$$

$$a = 3.9049\ \text{ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\, b) \left[1 - \left[3 \left(\frac{4\,E}{L_e} + 3\right) \left(\frac{a}{L_e}\right)^2\right] + \left[4 \left(\frac{3\,E}{L_e} + 2\right) \left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((-0.178\ \text{kip/ft}) \times (36\ \text{in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (3.3876\ \text{ft})}{(5.5\ \text{ft})} + 3\right) \times \left(\frac{(3.9049\ \text{ft})}{(5.5\ \text{ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (3.3876\ \text{ft})}{(5.5\ \text{ft})} + 2\right) \times \left(\frac{(3.9049\ \text{ft})}{(5.5\ \text{ft})}\right)^3\right]\right]$$

$$V_{max} = 0.9367\ \text{kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, b\, L_e)\left[\left(\frac{E}{L_e} + \frac{a}{2\,L_e}\right) - \left[\left(\frac{4\,E}{L_e} + 3\right)\left(\frac{a}{2\,L_e}\right)^3\right] + \left[\left(\frac{3\,E}{L_e} + 2\right)\left(\frac{a}{2\,L_e}\right)^4\right]\right]$$

$$M_{max} = ((-0.178\ \text{kip/ft}) \times (36\ \text{in}) \times (5.5\ \text{ft})) \times \left[\left(\frac{(3.3876\ \text{ft})}{(5.5\ \text{ft})} + \frac{(3.9049\ \text{ft})}{2 \times (5.5\ \text{ft})}\right) - \left[\left(\frac{4 \times (3.3876\ \text{ft})}{(5.5\ \text{ft})} + 3\right) \times \left(\frac{(3.9049\ \text{ft})}{2 \times (5.5\ \text{ft})}\right)^3\right] + \left[\left(\frac{3 \times (3.3876\ \text{ft})}{(5.5\ \text{ft})} + 2\right) \times \left(\frac{(3.9049\ \text{ft})}{2 \times (5.5\ \text{ft})}\right)^4\right]\right]$$

$$M_{max} = 2.3132\ \text{kipft}$$

Minimum Reinforcement Check (LRFD)

Parameters:

$f'_{ck} = 2.5\ \text{ksi}$ - Concrete strength,
$f_{yk} = 60\ \text{ksi}$ - Longitudinal reinforcement strength,
$\phi = 0.65$ - Reduction factor for axial strength,
Table 22.4.2.1 $\alpha = 0.85$ - Alpha factor for axial strength,
$A_g = 1017.9\ \text{in}^2$ - Gross area of concrete,

Longitudinal reinforcement:

Required reinforcement due to axial load, $A_{st,required}$
22.4.2.2, 10.6.1.1 $A_{st,required}$

$$A_{st,required} = Min\left[\frac{\frac{P}{\phi\,\alpha} - (0.85\, f'_{ck}\, A_g)}{f_{yk} - (0.85\, f'_{ck})}, (0.08\, A_g)\right]$$

$$A_{st,required} = Min\left[\frac{\frac{(14.162\ \text{kip})}{(0.65)\times(0.85)} - (0.85 \times (2.5\ \text{ksi}) \times (1017.9\ \text{in}^2))}{(60\ \text{ksi}) - (0.85 \times (2.5\ \text{ksi}))}, (0.08 \times (1017.9\ \text{in}^2))\right]$$

$$A_{st,required} = -36.931\ \text{in}^2$$

A_{min} - Governing minimum reinforcement area,

$$A_{min} = Max\left[A_{st,required}, (0.0018\, A_g)\right]$$

$$A_{min} = Max\left[(-36.931\ \text{in}^2), (0.0018 \times (1017.9\ \text{in}^2))\right]$$

$$A_{min} = 1.8322\ \text{in}^2$$

n_{rebar} - Required number of reinforcement,

$$n_{rebar} = \frac{A_{min}}{A_{rebar}}$$

$$n_{rebar} = \frac{(1.8322\ \text{in}^2)}{(0.3068\ \text{in}^2)}$$

$$n_{rebar} = 6$$

A_{st} - Actual total reinforcement area,

$$A_{st} = n_{rebar}\,\frac{\pi\, d_{bar}^2}{4}$$

$$A_{st} = (6) \times \frac{\pi \times (0.625\ \text{in})^2}{4}$$

$$A_{st} = 1.8408\ \text{in}^2$$

$Ratio$ - Capacity

$$Ratio = \frac{A_{min}}{A_{st}}$$

$$(1.8322\ \text{in}^2)$$

$$Ratio = \frac{}{(1.8408\ \text{in}^2)}$$

$$Ratio = 0.99533$$

Status: **PASS**
Ratio: **1.000**

25.2.3 s_{rebar} - Minimum spacing of reinforcement,

$$s_{rebar} = Max\left[1.5, \left(1.5\ d_{bar}\right)\right]$$

$$s_{rebar} = Max\left[1.5, \left(1.5 \times (0.625\ \text{in})\right)\right]$$

$$s_{rebar} = 1.5\ \text{in}$$

Ties:

25.7.2.2 Since longitudinal reinforcement is $\leq$ No. 10ø: Use #3(0.375 in)

25.7.2.1 s_{ties} - Maximum center-to-center spacing of ties,

$$s_{ties} = Min\left[\left(16\ d_{bar}\right), \left(48\ d_{ties}\right), D\right]$$

$$s_{ties} = Min\left[\left(16 \times (0.625\ \text{in})\right), \left(48 \times (0.375\ \text{in})\right), (36\ \text{in})\right]$$

$$s_{ties} = 10\ \text{in}$$

Summary:

Main reinforcement: **6 - #5 (0.625 in)**
Ties: **#3(0.375 in) - 10 in**

Axial Compression Strength (ACI 318-19, LRFD)

22.4.2.2 ϕP_N - Allowable axial compressive strength

$$\phi P_N = \phi\ 0.85\ \left[\left(0.85\ f'_{ck}\ \left[A_g - A_{st}\right]\right) + \left(f_{yk}\ A_{st}\right)\right]$$

$$\phi P_N = (0.65) \times 0.85 \times \left[\left(0.85 \times (2.5\ \text{ksi}) \times \left[\left(1017.9\ \text{in}^2\right) - \left(1.8408\ \text{in}^2\right)\right]\right) + \left((60\ \text{ksi}) \times \left(1.8408\ \text{in}^2\right)\right)\right]$$

$$\phi P_N = 1253.9\ \text{kip}$$

$Ratio$ - Capacity

$$Ratio = \frac{P}{\phi P_N}$$

$$Ratio = \frac{(14.162\ \text{kip})}{(1253.9\ \text{kip})}$$

$$Ratio = 0.011294$$

Status: **PASS**
Ratio: **0.010**

Shear Strength (ACI 318-19, LRFD)

Parameters:

$b_w = 36\ \text{in}$ - Effective width,

22.5.2.2 d - Effective depth

$$d = 0.80\ D$$

$$d = 0.80 \times (36\ \text{in})$$

$$d = 28.8\ \text{in}$$

22.5.5.1.3 λ_s - size effect modification factor

$$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{d}{10}}}, 1\right]$$

$$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{(28.8\ \text{in})}{10}}}, 1\right]$$

$$\lambda_s = 0.71796$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5\ \text{ksi} \rightarrow 2500\ \text{psi}$,

22.5.5.1.1 $V_{c,max}$ - Max shear strength of concrete

$$V_{c,max} = 5\ \lambda_s\ \sqrt{f'_{ck}}\ b_w\ d$$

$$V_{c,max} = 5 \times (0.71796) \times \sqrt{(2500\ \text{psi})} \times (36\ \text{in}) \times (28.8\ \text{in})$$

$$V_{c,max} = 186.09 \text{ kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$, $P = 14.162 \text{ kip} \rightarrow 14162 \text{ lbf}$,

22.5.5.1.1(a) $V_{c,a}$ - Shear strength of concrete (a)

$$V_{c,a} = \left[2\, \lambda_s\, \sqrt{f'_{ck}} + \frac{P}{6\, A_g}\right]\, b_w\, d$$

$$V_{c,a} = \left[2 \times (0.71796) \times \sqrt{(2500 \text{ psi})} + \frac{(14162 \text{ lbf})}{6 \times (1017.9 \text{ in}^2)}\right] \times (36 \text{ in}) \times (28.8 \text{ in})$$

$$V_{c,a} = 76.842 \text{ kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$,

22.5.5.1.2 $V_{c,b}$ - Shear strength of concrete (b)

$$V_{c,b} = \left[2\, \lambda_s\, \sqrt{f'_{ck}} + \left(0.05\, f'_{ck}\right)\right]\, b_w\, d$$

$$V_{c,b} = \left[2 \times (0.71796) \times \sqrt{(2500 \text{ psi})} + (0.05 \times (2500 \text{ psi}))\right] \times (36 \text{ in}) \times (28.8 \text{ in})$$

$$V_{c,b} = 204.04 \text{ kip}$$

V_c - Governing shear strength of concrete

$$V_c = Min\left[V_{c,max}, V_{c,a}, V_{c,b}\right]$$

$$V_c = Min\left[(186.09 \text{ kip}), (76.842 \text{ kip}), (204.04 \text{ kip})\right]$$

$$V_c = 76.842 \text{ kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$,

22.5.1.2 $V_{s,a}$ - Shear strength of steel (a)

$$V_{s,a} = 8\, \sqrt{f'_{ck}}\, b_w\, d$$

$$V_{s,a} = 8 \times \sqrt{(2500 \text{ psi})} \times (36 \text{ in}) \times (28.8 \text{ in})$$

$$V_{s,a} = 414.72 \text{ kip}$$

A_v - Ties rebar area,

$$A_v = \frac{\pi\, d_{ties}^2}{4}$$

$$A_v = \frac{\pi \times (0.375 \text{ in})^2}{4}$$

$$A_v = 0.11045 \text{ in}^2$$

22.5.8.5.3 $V_{s,b}$ - Shear strength of steel (b)

$$V_{s,b} = \frac{2\, A_v\, f_{ywk}\, d}{s_{ties}}$$

$$V_{s,b} = \frac{2 \times \left(0.11045 \text{ in}^2\right) \times (60 \text{ ksi}) \times (28.8 \text{ in})}{(10 \text{ in})}$$

$$V_{s,b} = 38.17 \text{ kip}$$

V_s - Governing shear strength of steel

$$V_s = MIN\left[V_{s,a}, V_{s,b}\right]$$

$$V_s = MIN\left[(414.72 \text{ kip}), (38.17 \text{ kip})\right]$$

$$V_s = 38.17 \text{ kip}$$

22.5.1.1 ϕV_n - Allowable shear strength

$$\phi V_n = \phi\, \left(V_c + V_s\right)$$

$$\phi V_n = (0.65) \times ((76.842 \text{ kip}) + (38.17 \text{ kip}))$$

$$\phi V_n = 74.758 \text{ kip}$$

Considering x-direction:

$V_{max} = 2.4145 \text{ kip}$ - Maximum shear force in the x-direction,
$Ratio$ - Capacity

$$Ratio = \frac{V_{max}}{\phi V_n}$$

	$$Ratio = \frac{(2.4145\ \mathrm{kip})}{(74.758\ \mathrm{kip})}$$ $$Ratio = 0.032298$$	Status: **PASS** Ratio: **0.030**
	Considering z-direction: $V_{max} = 0.9367\ \mathrm{kip}$ - Maximum shear force in the z-direction, $Ratio$ - Capacity $$Ratio = \frac{V_{max}}{\phi V_n}$$ $$Ratio = \frac{(0.9367\ \mathrm{kip})}{(74.758\ \mathrm{kip})}$$ $$Ratio = 0.01253$$	Status: **PASS** Ratio: **0.010**
14.5.2.1b	**Flexural Strength (ACI 318-19, LRFD)** S_m - Section modulus $$S_m = \frac{\pi\, D^3}{32}$$ $$S_m = \frac{\pi \times (36\ \mathrm{in})^3}{32}$$ $$S_m = 4580.4\ \mathrm{in}^3$$ $\lambda = 1$ - Concrete modification factor (Normal concrete), Allowable flexural strength: M_n shall be the lesser of: $\phi M_{n,1}$ $$\phi M_{n,1} = \phi \times 5 \times \lambda \times \sqrt{(f_c')} \times S_m$$ $$\phi M_{n,1} = 0.65 \times 5 \times 1 \times \sqrt{(2.5ksi)} \times 4580.442 in^3$$ $$\phi M_{n,1} = 62.027 kipft$$ $\phi M_{n,2}$ $$\phi M_{n,2} = \phi\, 0.85\, f_{ck}'\, S_m$$ $$\phi M_{n,2} = (0.65) \times 0.85 \times (2.5\ \mathrm{ksi}) \times \left(4580.4\ \mathrm{in}^3\right)$$ $$\phi M_{n,2} = 527.23\ \mathrm{kipft}$$ Therefore, ϕM_n - Allowable flexural strength, $$\phi M_n = MIN[\phi M_{n,1}, \phi M_{n,2}]$$ $$\phi M_n = MIN[(62.027\ \mathrm{kipft}), (527.23\ \mathrm{kipft})]$$ $$\phi M_n = 62.027\ \mathrm{kipft}$$ **Considering x-direction:** $M_{max} = 6.5418\ \mathrm{kipft}$ - Maximum moment in the x-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\phi M_n}$$ $$Ratio = \frac{(6.5418\ \mathrm{kipft})}{(62.027\ \mathrm{kipft})}$$ $$Ratio = 0.10547$$	Status: **PASS** Ratio: **0.110**
	Considering z-direction: $M_{max} = 2.3132\ \mathrm{kipft}$ - Maximum moment in the z-direction, $Ratio$ - Capacity	

	$$Ratio = \frac{}{\phi M_n}$$ $$Ratio = \frac{(2.3132 \text{ kipft})}{(62.027 \text{ kipft})}$$ $$Ratio = 0.037294$$	Status: **PASS** Ratio: **0.040**

REFERENCES	CALCULATIONS	RESULTS

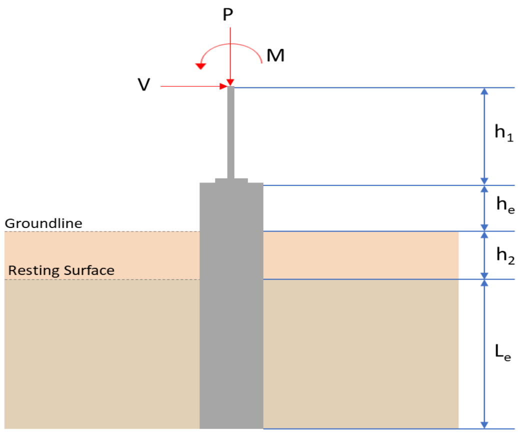
SkyCiv Foundation Design

Pile Foundation

Design Information :

Design code : IBC 2021 (International Building Code)
Unit System : Imperial

Pile Input



Geometry

Pile shape: round
$D = 36\ \mathrm{in}$ - Pile diameter
$L = 5.75\ \mathrm{ft}$ - Total pile length
$h_1 = 0\ \mathrm{ft}$ - Lateral load height from the top of the pile,
$h_2 = 0\ \mathrm{ft}$ - Depth to resisting surface
$h_e = 0\ \mathrm{ft}$ - Length of pile above the ground

Tabulation of Soil Parameters

Layer	Label	Allowable Bearing Pressure (q_a) (psf)	Allowable Lateral Pressure (R) (psf/ft)
1	Sand, silty sand, clayey sand, silty gravel & clayey gravel	2000.000	150.000

Tabulation of Loads

Load Component	ASD	LRFD
P (kip)	11.797	19.387
V_x (kip)	-0.152	-0.263
V_z (kip)	-0.051	-0.084
M_x (kipft)	-0.174	-0.291
M_z (kipft)	4.517	7.911

Material Properties

$f'_{ck} = 2.5\ \mathrm{ksi}$ - Concrete strength,

Required depth to resist lateral loads (ASD)

H - Point of application of the lateral load

$$H = h_1 + h_2 + h_e$$

$$H = (0\ \mathrm{ft}) + (0\ \mathrm{ft}) + (0\ \mathrm{ft})$$

$$H = 0\ \mathrm{ft}$$

Considering x-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{D}$$

$$H_o = \frac{(-0.152\ \mathrm{kip})}{(36\ \mathrm{in})}$$

$$H_o = -0.050667\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\ H)}{D}$$

$$M_o = \frac{(4.517\ \mathrm{kipft}) + ((-0.152\ \mathrm{kip}) \times (0\ \mathrm{ft}))}{(36\ \mathrm{in})}$$

$$M_o = 1.5057\ \mathrm{kipft/ft}$$

Required depth of embedment in earth:

$$L_x^3 - \left(14.14 \times \frac{H_o \times L_x}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,x} = 5.4638\ \mathrm{ft}$ - Required depth in x-direction,

Considering z-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{D}$$

$$H_o = \frac{(-0.051\ \mathrm{kip})}{(36\ \mathrm{in})}$$

$$H_o = -0.017\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\ H)}{D}$$

$$M_o = \frac{(0.174\ \mathrm{kipft}) + ((-0.051\ \mathrm{kip}) \times (0\ \mathrm{ft}))}{(36\ \mathrm{in})}$$

$$M_o = 0.058\ \mathrm{kipft/ft}$$

Required depth of embedment in earth:

$$L_z^3 - \left(14.14 \times \frac{H_o \times L_z}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,z} = 1.6655\ \mathrm{ft}$ - Required depth in z-direction,

Minimum embedded depth required:

$L_{e,req}$ - Depth of pile required,

$$L_{e,req} = MAX[L_{e,x}, L_{e,z}]$$

$$L_{e,req} = MAX[(5.4638\ \mathrm{ft}), (1.6655\ \mathrm{ft})]$$

$$L_{e,req} = 5.464\ \mathrm{ft}$$

L_e - Actual embedded length of pile,

$$L_e = L - h_e - h_2$$

$$L_e = (5.75\ \mathrm{ft}) - (0\ \mathrm{ft}) - (0\ \mathrm{ft})$$

$$L_e = 5.75\ \mathrm{ft}$$

$Ratio$ - Embedded depth

$$Ratio = \frac{L_{e,req}}{L_e}$$

$$Ratio = \frac{(5.464\ \mathrm{ft})}{(5.75\ \mathrm{ft})}$$

$$Ratio = 0.95026$$

Status: **PASS**
Ratio: **0.950**

End-bearing Capacity (ASD)

A - Pile cros-section area

$$A = \pi \left(\frac{D}{2}\right)^2$$

$$A = \pi \times \left(\frac{(36\ \mathrm{in})}{2}\right)^2$$

$$A = 7.0686\ \mathrm{ft}^2$$

q - End-bearing pressure

$$q = \frac{P_v}{A}$$

$$q = \frac{(11.797\ \mathrm{kip})}{(7.0686\ \mathrm{ft}^2)}$$

$$q = 1.6689\ \mathrm{kip/ft}^2$$

Check bearing capacity ratio:

$Ratio$ - Capacity

$$Ratio = \frac{q}{q_a}$$

$$Ratio = \frac{(1.6689\ \mathrm{kip/ft}^2)}{(2000\ \mathrm{psf})}$$

$$Ratio = 0.83447$$

Status: **PASS**
Ratio: **0.830**

Czerniak

Lateral Soil Pressure (ASD):

L/D - Length to least lateral dimension ratio,

$$L/D = \frac{L}{D}$$

$$L/D = \frac{(5.75\ \mathrm{ft})}{(36\ \mathrm{in})}$$

$$L/D = 1.9167$$

Since L/D $\leq$ 10,

Pile is short.

Considering x-direction:

$H_o = -0.050667\ \mathrm{kip/ft}$ - Lateral force per length of pile,
$M_o = 1.5057\ \mathrm{kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\, M_o\, L_e) + \left(3\, H_o\, L_e^2\right)}{(6\, M_o) + (4\, H_o\, L_e)}$$

$$a = \frac{(4 \times (1.5057\ \mathrm{kipft/ft}) \times (5.75\ \mathrm{ft})) + \left(3 \times (-0.050667\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft})^2\right)}{(6 \times (1.5057\ \mathrm{kipft/ft})) + (4 \times (-0.050667\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))}$$

$$a = 3.8881\ \mathrm{ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{1.178\ [(4\, M_o) + (3\, H_o\, L_e)]^2}{L_e^2\ [(3\, M_o) + (2\, H_o\, L_e)]}$$

$$p = \frac{1.178 \times [(4 \times (1.5057\ \mathrm{kipft/ft})) + (3 \times (-0.050667\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))]^{\ 2}}{(5.75\ \mathrm{ft})^2 \times [(3 \times (1.5057\ \mathrm{kipft/ft})) + (2 \times (-0.050667\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))]}$$

$$p = 0.24006\ \mathrm{kip/ft}^2$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{9.425\ [(2\, M_o) + (H_o\, L_e)]}{L_e^2}$$

$$s = \frac{9.425 \times [(2 \times (1.5057\ \mathrm{kipft/ft})) + ((-0.050667\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))]}{(5.75\ \mathrm{ft})^2}$$

$$s = 0.77538\ \mathrm{kip/ft}^2$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\,\frac{a}{2}$$

$$p_a = (150\ \mathrm{psf/ft}) \times \frac{(3.8881\ \mathrm{ft})}{2}$$

$$p_a = 0.29161\ \mathrm{kip/ft}^2$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{\left(0.24006\ \mathrm{kip/ft^2}\right)}{\left(0.29161\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = 0.82325$$

Status: **PASS**
Ratio: **0.820**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\, L_e$$

$$p_s = (150\ \mathrm{psf/ft}) \times (5.75\ \mathrm{ft})$$

$$p_s = 0.8625\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{\left(0.77538\ \mathrm{kip/ft^2}\right)}{\left(0.8625\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = 0.899$$

Status: **PASS**
Ratio: **0.900**

Considering z-direction:

$H_o = -0.017\ \mathrm{kip/ft}$ - Lateral force per length of pile,
$M_o = 0.058\ \mathrm{kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\, M_o\, L_e) + \left(3\, H_o\, L_e^2\right)}{(6\, M_o) + (4\, H_o\, L_e)}$$

$$a = \frac{(4 \times (0.058\ \mathrm{kipft/ft}) \times (5.75\ \mathrm{ft})) + \left(3 \times (-0.017\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft})^2\right)}{(6 \times (0.058\ \mathrm{kipft/ft})) + (4 \times (-0.017\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))}$$

$$a = 4.0869\ \mathrm{ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{1.178\ [(4\, M_o) + (3\, H_o\, L_e)]^2}{L_e^2\ [(3\, M_o) + (2\, H_o\, L_e)]}$$

$$p = \frac{1.178 \times [(4 \times (0.058\ \mathrm{kipft/ft})) + (3 \times (-0.017\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))]^{\ 2}}{(5.75\ \mathrm{ft})^2 \times [(3 \times (0.058\ \mathrm{kipft/ft})) + (2 \times (-0.017\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))]}$$

$$p = -0.006217\ \mathrm{kip/ft^2}$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{9.425\ [(2\, M_o) + (H_o\, L_e)]}{L_e^2}$$

$$s = \frac{9.425 \times [(2 \times (0.058\ \mathrm{kipft/ft})) + ((-0.017\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))]}{(5.75\ \mathrm{ft})^2}$$

$$s = 0.0052025\ \mathrm{kip/ft^2}$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\, \frac{a}{2}$$

$$p_a = (150\ \mathrm{psf/ft}) \times \frac{(4.0869\ \mathrm{ft})}{2}$$

$$p_a = 0.30651\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{\left(-0.006217\ \mathrm{kip/ft^2}\right)}{\left(0.30651\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = -0.020283$$

Status: **PASS**
Ratio: **-0.020**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\,L_e$$

$$p_s = (150\ \mathrm{psf/ft}) \times (5.75\ \mathrm{ft})$$

$$p_s = 0.8625\ \mathrm{kip/ft^2}$$

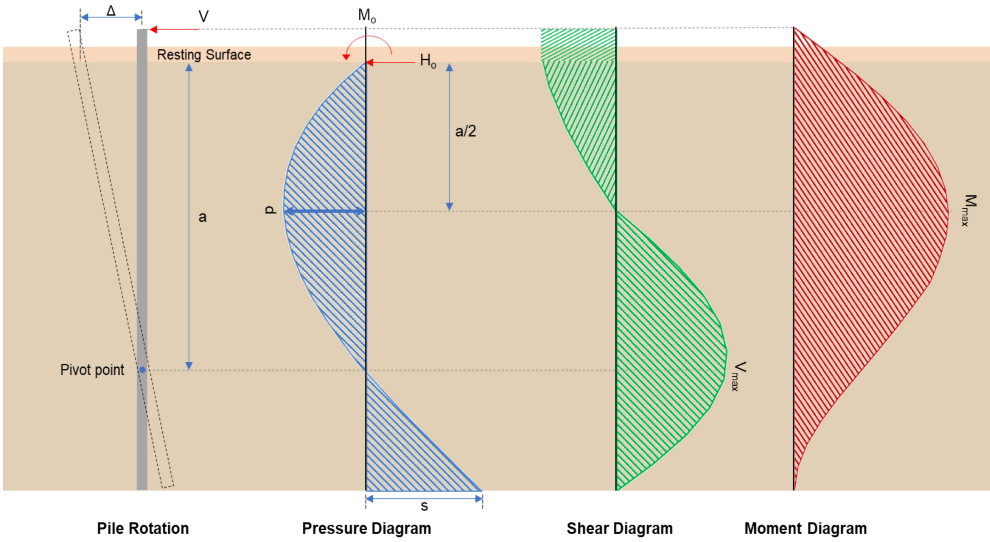
$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{(0.0052025\ \mathrm{kip/ft^2})}{(0.8625\ \mathrm{kip/ft^2})}$$

$$Ratio = 0.0060318$$

Status: **PASS**
Ratio: **0.010**



Shear force and Bending moment (x-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{D}$$

$$H_o = \frac{(-0.263\ \mathrm{kip})}{(36\ \mathrm{in})}$$

$$H_o = -0.087667\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\,H)}{D}$$

$$M_o = \frac{(7.911\ \mathrm{kipft}) + ((-0.263\ \mathrm{kip}) \times (0\ \mathrm{ft}))}{(36\ \mathrm{in})}$$

$$M_o = 2.637\ \mathrm{kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(2.637\ \mathrm{kipft/ft})}{(-0.087667\ \mathrm{kip/ft})}$$

$$E = 30.08\ \mathrm{ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (2.637\ \mathrm{kipft/ft}) \times (5.75\ \mathrm{ft})) + \left(3 \times (-0.087667\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft})^2\right)}{(6 \times (2.637\ \mathrm{kipft/ft})) + (4 \times (-0.087667\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))}$$

$$a = 3.8875\ \mathrm{ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\, D) \left[1 - \left[3 \left(\frac{4\, E}{L_e} + 3\right) \left(\frac{a}{L_e}\right)^2\right] + \left[4 \left(\frac{3\, E}{L_e} + 2\right) \left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((-0.087667\ \mathrm{kip/ft}) \times (36\ \mathrm{in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (30.08\ \mathrm{ft})}{(5.75\ \mathrm{ft})} + 3\right) \times \left(\frac{(3.8875\ \mathrm{ft})}{(5.75\ \mathrm{ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (30.08\ \mathrm{ft})}{(5.75\ \mathrm{ft})} + 2\right) \times \left(\frac{(3.8875\ \mathrm{ft})}{(5.75\ \mathrm{ft})}\right)^3\right]\right]$$

$$V_{max} = 2.6132\ \mathrm{kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, D\, L_e) \left[\left(\frac{E}{L_e} + \frac{a}{2\, L_e}\right) - \left[\left(\frac{4\, E}{L_e} + 3\right) \left(\frac{a}{2\, L_e}\right)^3\right] + \left[\left(\frac{3\, E}{L_e} + 2\right) \left(\frac{a}{2\, L_e}\right)^4\right]\right]$$

$$M_{max} = ((-0.087667\ \mathrm{kip/ft}) \times (36\ \mathrm{in}) \times (5.75\ \mathrm{ft})) \times \left[\left(\frac{(30.08\ \mathrm{ft})}{(5.75\ \mathrm{ft})} + \frac{(3.8875\ \mathrm{ft})}{2 \times (5.75\ \mathrm{ft})}\right) - \left[\left(\frac{4 \times (30.08\ \mathrm{ft})}{(5.75\ \mathrm{ft})} + 3\right) \times \left(\frac{(3.8875\ \mathrm{ft})}{2 \times (5.75\ \mathrm{ft})}\right)^3\right] + \left[\left(\frac{3 \times (30.08\ \mathrm{ft})}{(5.75\ \mathrm{ft})} + 2\right) \times \left(\frac{(3.8875\ \mathrm{ft})}{2 \times (5.75\ \mathrm{ft})}\right)^4\right]\right]$$

$$M_{max} = 7.374\ \mathrm{kipft}$$

Shear force and Bending moment (z-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{D}$$

$$H_o = \frac{(-0.084\ \mathrm{kip})}{(36\ \mathrm{in})}$$

$$H_o = -0.028\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\, H)}{D}$$

$$M_o = \frac{(0.291\ \mathrm{kipft}) + ((-0.084\ \mathrm{kip}) \times (0\ \mathrm{ft}))}{(36\ \mathrm{in})}$$

$$M_o = 0.097\ \mathrm{kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(0.097\ \mathrm{kipft/ft})}{(-0.028\ \mathrm{kip/ft})}$$

$$E = 3.4643\ \mathrm{ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\, M_o\, L_e) + \left(3\, H_o\, L_e^2\right)}{(6\, M_o) + (4\, H_o\, L_e)}$$

$$a = \frac{(4 \times (0.097\ \mathrm{kipft/ft}) \times (5.75\ \mathrm{ft})) + \left(3 \times (-0.028\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft})^2\right)}{(6 \times (0.097\ \mathrm{kipft/ft})) + (4 \times (-0.028\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))}$$

$$a = 4.085\ \mathrm{ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\, b) \left[1 - \left[3 \left(\frac{4\, E}{L_e} + 3\right) \left(\frac{a}{L_e}\right)^2\right] + \left[4 \left(\frac{3\, E}{L_e} + 2\right) \left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((-0.028\ \mathrm{kip/ft}) \times (36\ \mathrm{in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (3.4643\ \mathrm{ft})}{(5.75\ \mathrm{ft})} + 3\right) \times \left(\frac{(4.085\ \mathrm{ft})}{(5.75\ \mathrm{ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (3.4643\ \mathrm{ft})}{(5.75\ \mathrm{ft})} + 2\right) \times \left(\frac{(4.085\ \mathrm{ft})}{(5.75\ \mathrm{ft})}\right)^3\right]\right]$$

$$V_{max} = 0.14536\ \mathrm{kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, b\, L_e) \left[\left(\frac{E}{L_e} + \frac{a}{2\,L_e}\right) - \left[\left(\frac{4\,E}{L_e} + 3\right)\left(\frac{a}{2\,L_e}\right)^3\right] + \left[\left(\frac{3\,E}{L_e} + 2\right)\left(\frac{a}{2\,L_e}\right)^4\right]\right]$$

$$M_{max} = ((-0.028\ \mathrm{kip/ft}) \times (36\ \mathrm{in}) \times (5.75\ \mathrm{ft})) \times \left[\left(\frac{(3.4643\ \mathrm{ft})}{(5.75\ \mathrm{ft})} + \frac{(4.085\ \mathrm{ft})}{2 \times (5.75\ \mathrm{ft})}\right) - \left[\left(\frac{4 \times (3.4643\ \mathrm{ft})}{(5.75\ \mathrm{ft})} + 3\right) \times \left(\frac{(4.085\ \mathrm{ft})}{2 \times (5.75\ \mathrm{ft})}\right)^3\right] + \left[\left(\frac{3 \times (3.4643\ \mathrm{ft})}{(5.75\ \mathrm{ft})} + 2\right) \times \left(\frac{(4.085\ \mathrm{ft})}{2 \times (5.75\ \mathrm{ft})}\right)^4\right]\right]$$

$$M_{max} = 0.37473\ \mathrm{kipft}$$

Minimum Reinforcement Check (LRFD)

Parameters:

$f'_{ck} = 2.5\ \mathrm{ksi}$ - Concrete strength,
$f_{yk} = 60\ \mathrm{ksi}$ - Longitudinal reinforcement strength,
$\phi = 0.65$ - Reduction factor for axial strength,
Table 22.4.2.1 — $\alpha = 0.85$ - Alpha factor for axial strength,
$A_g = 1017.9\ \mathrm{in}^2$ - Gross area of concrete,

Longitudinal reinforcement:

Required reinforcement due to axial load, $A_{st,required}$

22.4.2.2, 10.6.1.1 — $A_{st,required}$

$$A_{st,required} = Min\left[\frac{\frac{P}{\phi\,\alpha} - (0.85\, f'_{ck}\, A_g)}{f_{yk} - (0.85\, f'_{ck})}, (0.08\, A_g)\right]$$

$$A_{st,required} = Min\left[\frac{\frac{(19.387\ \mathrm{kip})}{(0.65)\times(0.85)} - (0.85 \times (2.5\ \mathrm{ksi}) \times (1017.9\ \mathrm{in}^2))}{(60\ \mathrm{ksi}) - (0.85 \times (2.5\ \mathrm{ksi}))}, (0.08 \times (1017.9\ \mathrm{in}^2))\right]$$

$$A_{st,required} = -36.767\ \mathrm{in}^2$$

A_{min} - Governing minimum reinforcement area,

$$A_{min} = Max\left[A_{st,required}, (0.0018\, A_g)\right]$$

$$A_{min} = Max\left[(-36.767\ \mathrm{in}^2), (0.0018 \times (1017.9\ \mathrm{in}^2))\right]$$

$$A_{min} = 1.8322\ \mathrm{in}^2$$

n_{rebar} - Required number of reinforcement,

$$n_{rebar} = \frac{A_{min}}{A_{rebar}}$$

$$n_{rebar} = \frac{(1.8322\ \mathrm{in}^2)}{(0.3068\ \mathrm{in}^2)}$$

$$n_{rebar} = 6$$

A_{st} - Actual total reinforcement area,

$$A_{st} = n_{rebar}\,\frac{\pi\, d_{bar}^2}{4}$$

$$A_{st} = (6) \times \frac{\pi \times (0.625\ \mathrm{in})^2}{4}$$

$$A_{st} = 1.8408\ \mathrm{in}^2$$

$Ratio$ - Capacity

$$Ratio = \frac{A_{min}}{A_{st}}$$

$$(1.8322\ \mathrm{in}^2)$$

	$$Ratio = \frac{\ }{(1.8408\ \text{in}^2)}$$ $$Ratio = 0.99533$$	Status: **PASS** Ratio: **1.000**
25.2.3	s_{rebar} - Minimum spacing of reinforcement, $$s_{rebar} = Max\left[1.5, (1.5\ d_{bar})\right]$$ $$s_{rebar} = Max\left[1.5, (1.5 \times (0.625\ \text{in}))\right]$$ $$s_{rebar} = 1.5\ \text{in}$$ **Ties:**	
25.7.2.2 *25.7.2.1*	Since longitudinal reinforcement is $\leq$ No. 10ø: Use #3(0.375 in) s_{ties} - Maximum center-to-center spacing of ties, $$s_{ties} = Min\left[(16\ d_{bar}), (48\ d_{ties}), D\right]$$ $$s_{ties} = Min\left[(16 \times (0.625\ \text{in})), (48 \times (0.375\ \text{in})), (36\ \text{in})\right]$$ $$s_{ties} = 10\ \text{in}$$ **Summary:** Main reinforcement: **6 - #5 (0.625 in)** Ties: **#3(0.375 in) - 10 in**	
22.4.2.2	**Axial Compression Strength (ACI 318-19, LRFD)** ϕP_N - Allowable axial compressive strength $$\phi P_N = \phi\ 0.85\ \left[\left(0.85\ f'_{ck}\ [A_g - A_{st}]\right) + (f_{yk}\ A_{st})\right]$$ $$\phi P_N = (0.65) \times 0.85 \times \left[\left(0.85 \times (2.5\ \text{ksi}) \times \left[\left(1017.9\ \text{in}^2\right) - \left(1.8408\ \text{in}^2\right)\right]\right) + \left((60\ \text{ksi}) \times \left(1.8408\ \text{in}^2\right)\right)\right]$$ $$\phi P_N = 1253.9\ \text{kip}$$ $Ratio$ - Capacity $$Ratio = \frac{P}{\phi P_N}$$ $$Ratio = \frac{(19.387\ \text{kip})}{(1253.9\ \text{kip})}$$ $$Ratio = 0.015461$$	Status: **PASS** Ratio: **0.020**
  22.5.2.2	**Shear Strength (ACI 318-19, LRFD)** **Parameters:** $b_w = 36\ \text{in}$ - Effective width, d - Effective depth $$d = 0.80\ D$$ $$d = 0.80 \times (36\ \text{in})$$ $$d = 28.8\ \text{in}$$	
22.5.5.1.3	λ_s - size effect modification factor $$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{d}{10}}}, 1\right]$$ $$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{(28.8\ \text{in})}{10}}}, 1\right]$$ $$\lambda_s = 0.71796$$ The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5\ \text{ksi} \rightarrow 2500\ \text{psi}$,	
22.5.5.1.1	$V_{c,max}$ - Max shear strength of concrete $$V_{c,max} = 5\ \lambda_s\ \sqrt{f'_{ck}}\ b_w\ d$$ $$V_{c,max} = 5 \times (0.71796) \times \sqrt{(2500\ \text{psi})} \times (36\ \text{in}) \times (28.8\ \text{in})$$	

$$V_{c,max} = 186.09 \text{ kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$, $P = 19.387 \text{ kip} \rightarrow 19387 \text{ lbf}$,

22.5.5.1.1(a) $V_{c,a}$ - Shear strength of concrete (a)

$$V_{c,a} = \left[2\,\lambda_s\,\sqrt{f'_{ck}} + \frac{P}{6\,A_g}\right]\,b_w\,d$$

$$V_{c,a} = \left[2 \times (0.71796) \times \sqrt{(2500 \text{ psi})} + \frac{(19387 \text{ lbf})}{6 \times (1017.9 \text{ in}^2)}\right] \times (36 \text{ in}) \times (28.8 \text{ in})$$

$$V_{c,a} = 77.729 \text{ kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$,

22.5.5.1.2 $V_{c,b}$ - Shear strength of concrete (b)

$$V_{c,b} = \left[2\,\lambda_s\,\sqrt{f'_{ck}} + \left(0.05\,f'_{ck}\right)\right]\,b_w\,d$$

$$V_{c,b} = \left[2 \times (0.71796) \times \sqrt{(2500 \text{ psi})} + (0.05 \times (2500 \text{ psi}))\right] \times (36 \text{ in}) \times (28.8 \text{ in})$$

$$V_{c,b} = 204.04 \text{ kip}$$

V_c - Governing shear strength of concrete

$$V_c = Min\left[V_{c,max}, V_{c,a}, V_{c,b}\right]$$

$$V_c = Min\left[(186.09 \text{ kip}), (77.729 \text{ kip}), (204.04 \text{ kip})\right]$$

$$V_c = 77.729 \text{ kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5 \text{ ksi} \rightarrow 2500 \text{ psi}$,

22.5.1.2 $V_{s,a}$ - Shear strength of steel (a)

$$V_{s,a} = 8\,\sqrt{f'_{ck}}\,b_w\,d$$

$$V_{s,a} = 8 \times \sqrt{(2500 \text{ psi})} \times (36 \text{ in}) \times (28.8 \text{ in})$$

$$V_{s,a} = 414.72 \text{ kip}$$

A_v - Ties rebar area,

$$A_v = \frac{\pi\,d_{ties}^2}{4}$$

$$A_v = \frac{\pi \times (0.375 \text{ in})^2}{4}$$

$$A_v = 0.11045 \text{ in}^2$$

22.5.8.5.3 $V_{s,b}$ - Shear strength of steel (b)

$$V_{s,b} = \frac{2\,A_v\,f_{ywk}\,d}{s_{ties}}$$

$$V_{s,b} = \frac{2 \times \left(0.11045 \text{ in}^2\right) \times (60 \text{ ksi}) \times (28.8 \text{ in})}{(10 \text{ in})}$$

$$V_{s,b} = 38.17 \text{ kip}$$

V_s - Governing shear strength of steel

$$V_s = MIN\left[V_{s,a}, V_{s,b}\right]$$

$$V_s = MIN\left[(414.72 \text{ kip}), (38.17 \text{ kip})\right]$$

$$V_s = 38.17 \text{ kip}$$

22.5.1.1 ϕV_n - Allowable shear strength

$$\phi V_n = \phi\,\left(V_c + V_s\right)$$

$$\phi V_n = (0.65) \times ((77.729 \text{ kip}) + (38.17 \text{ kip}))$$

$$\phi V_n = 75.335 \text{ kip}$$

Considering x-direction:

$V_{max} = 2.6132 \text{ kip}$ - Maximum shear force in the x-direction,
$Ratio$ - Capacity

$$Ratio = \frac{V_{max}}{\phi V_n}$$

	$$Ratio = \frac{(2.6132\ \mathrm{kip})}{(75.335\ \mathrm{kip})}$$ $$Ratio = 0.034687$$	Status: **PASS** Ratio: **0.030**
	Considering z-direction: $V_{max} = 0.14536\ \mathrm{kip}$ - Maximum shear force in the z-direction, $Ratio$ - Capacity $$Ratio = \frac{V_{max}}{\phi V_n}$$ $$Ratio = \frac{(0.14536\ \mathrm{kip})}{(75.335\ \mathrm{kip})}$$ $$Ratio = 0.0019296$$	Status: **PASS** Ratio: **0.000**
14.5.2.1b	**Flexural Strength (ACI 318-19, LRFD)** S_m - Section modulus $$S_m = \frac{\pi\, D^3}{32}$$ $$S_m = \frac{\pi \times (36\ \mathrm{in})^3}{32}$$ $$S_m = 4580.4\ \mathrm{in}^3$$ $\lambda = 1$ - Concrete modification factor (Normal concrete), Allowable flexural strength: M_n shall be the lesser of: $\phi M_{n,1}$ $$\phi M_{n,1} = \phi \times 5 \times \lambda \times \sqrt{(f'_c)} \times S_m$$ $$\phi M_{n,1} = 0.65 \times 5 \times 1 \times \sqrt{(2.5ksi)} \times 4580.442in^3$$ $$\phi M_{n,1} = 62.027kipft$$ $\phi M_{n,2}$ $$\phi M_{n,2} = \phi\, 0.85\, f'_{ck}\, S_m$$ $$\phi M_{n,2} = (0.65) \times 0.85 \times (2.5\ \mathrm{ksi}) \times \left(4580.4\ \mathrm{in}^3\right)$$ $$\phi M_{n,2} = 527.23\ \mathrm{kipft}$$ Therefore, ϕM_n - Allowable flexural strength, $$\phi M_n = MIN[\phi M_{n,1}, \phi M_{n,2}]$$ $$\phi M_n = MIN[(62.027\ \mathrm{kipft}), (527.23\ \mathrm{kipft})]$$ $$\phi M_n = 62.027\ \mathrm{kipft}$$ **Considering x-direction:** $M_{max} = 7.374\ \mathrm{kipft}$ - Maximum moment in the x-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\phi M_n}$$ $$Ratio = \frac{(7.374\ \mathrm{kipft})}{(62.027\ \mathrm{kipft})}$$ $$Ratio = 0.11888$$	Status: **PASS** Ratio: **0.120**
	Considering z-direction: $M_{max} = 0.37473\ \mathrm{kipft}$ - Maximum moment in the z-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\cdots}$$	

	$$Ratio = \frac{}{\phi M_n}$$ $$Ratio = \frac{(0.37473 \text{ kipft})}{(62.027 \text{ kipft})}$$ $$Ratio = 0.0060414$$	Status: **PASS** Ratio: **0.010**

REFERENCES	CALCULATIONS	RESULTS

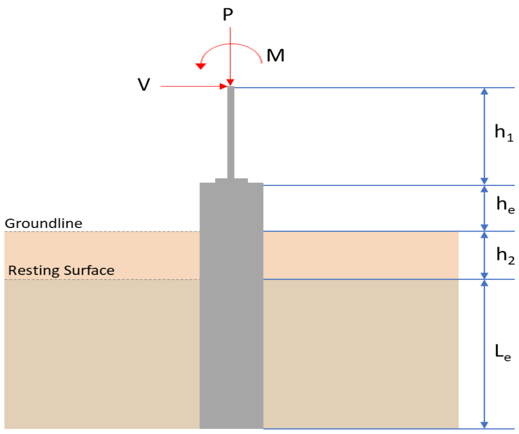
SkyCiv Foundation Design

Pile Foundation

Design Information :

Design code : IBC 2021 (International Building Code)
Unit System : Imperial

Pile Input



Geometry

Pile shape: round
$D = 36\ \mathrm{in}$ - Pile diameter
$L = 5.75\ \mathrm{ft}$ - Total pile length
$h_1 = 0\ \mathrm{ft}$ - Lateral load height from the top of the pile,
$h_2 = 0\ \mathrm{ft}$ - Depth to resisting surface
$h_e = 0\ \mathrm{ft}$ - Length of pile above the ground

Tabulation of Soil Parameters

Layer	Label	Allowable Bearing Pressure (q_a) (psf)	Allowable Lateral Pressure (R) (psf/ft)
1	Sand, silty sand, clayey sand, silty gravel & clayey gravel	2000.000	150.000

Tabulation of Loads

Load Component	ASD	LRFD
P (kip)	11.798	19.388
V_x (kip)	-0.152	-0.263
V_z (kip)	0.051	0.085
M_x (kipft)	0.173	0.289
M_z (kipft)	4.517	7.911

Material Properties

$f'_{ck} = 2.5\ \mathrm{ksi}$ - Concrete strength,

Required depth to resist lateral loads (ASD)

H - Point of application of the lateral load

$$H = h_1 + h_2 + h_e$$

$$H = (0\ \mathrm{ft}) + (0\ \mathrm{ft}) + (0\ \mathrm{ft})$$

$$H = 0\ \mathrm{ft}$$

Considering x-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{D}$$

$$H_o = \frac{(-0.152\ \mathrm{kip})}{(36\ \mathrm{in})}$$

$$H_o = -0.050667\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\ H)}{D}$$

$$M_o = \frac{(4.517\ \text{kipft}) + ((-0.152\ \text{kip}) \times (0\ \text{ft}))}{(36\ \text{in})}$$

$$M_o = 1.5057\ \text{kipft/ft}$$

Required depth of embedment in earth:

$$L_x^3 - \left(14.14 \times \frac{H_o \times L_x}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,x} = 5.4638\ \text{ft}$ - Required depth in x-direction,

Considering z-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{D}$$

$$H_o = \frac{(0.051\ \text{kip})}{(36\ \text{in})}$$

$$H_o = 0.017\ \text{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\ H)}{D}$$

$$M_o = \frac{(0.173\ \text{kipft}) + ((0.051\ \text{kip}) \times (0\ \text{ft}))}{(36\ \text{in})}$$

$$M_o = 0.057667\ \text{kipft/ft}$$

Required depth of embedment in earth:

$$L_z^3 - \left(14.14 \times \frac{H_o \times L_z}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,z} = 2.2096\ \text{ft}$ - Required depth in z-direction,

Minimum embedded depth required:

$L_{e,req}$ - Depth of pile required,

$$L_{e,req} = MAX[L_{e,x}, L_{e,z}]$$

$$L_{e,req} = MAX[(5.4638\ \text{ft}), (2.2096\ \text{ft})]$$

$$L_{e,req} = 5.464\ \text{ft}$$

L_e - Actual embedded length of pile,

$$L_e = L - h_e - h_2$$

$$L_e = (5.75\ \text{ft}) - (0\ \text{ft}) - (0\ \text{ft})$$

$$L_e = 5.75\ \text{ft}$$

$Ratio$ - Embedded depth

$$Ratio = \frac{L_{e,req}}{L_e}$$

$$Ratio = \frac{(5.464\ \text{ft})}{(5.75\ \text{ft})}$$

$$Ratio = 0.95026$$

Status: **PASS**
Ratio: **0.950**

End-bearing Capacity (ASD)

A - Pile cros-section area

$$A = \pi \left(\frac{D}{2}\right)^2$$

$$A = \pi \times \left(\frac{(36\ \text{in})}{2}\right)^2$$

$$A = 7.0686\ \text{ft}^2$$

q - End-bearing pressure

$$q = \frac{P_v}{A}$$

$$q = \frac{(11.798\ \mathrm{kip})}{(7.0686\ \mathrm{ft^2})}$$

$$q = 1.6691\ \mathrm{kip/ft^2}$$

Check bearing capacity ratio:

$Ratio$ - Capacity

$$Ratio = \frac{q}{q_a}$$

$$Ratio = \frac{(1.6691\ \mathrm{kip/ft^2})}{(2000\ \mathrm{psf})}$$

$$Ratio = 0.83454$$

Status: **PASS**
Ratio: **0.830**

Czerniak

Lateral Soil Pressure (ASD):

L/D - Length to least lateral dimension ratio,

$$L/D = \frac{L}{D}$$

$$L/D = \frac{(5.75\ \mathrm{ft})}{(36\ \mathrm{in})}$$

$$L/D = 1.9167$$

Since L/D $\leq$ 10,

Pile is short.

Considering x-direction:

$H_o = -0.050667\ \mathrm{kip/ft}$ - Lateral force per length of pile,
$M_o = 1.5057\ \mathrm{kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (1.5057\ \mathrm{kipft/ft}) \times (5.75\ \mathrm{ft})) + \left(3 \times (-0.050667\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft})^2\right)}{(6 \times (1.5057\ \mathrm{kipft/ft})) + (4 \times (-0.050667\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))}$$

$$a = 3.8881\ \mathrm{ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{1.178\ [(4\,M_o) + (3\,H_o\,L_e)]^2}{L_e^2\ [(3\,M_o) + (2\,H_o\,L_e)]}$$

$$p = \frac{1.178 \times [(4 \times (1.5057\ \mathrm{kipft/ft})) + (3 \times (-0.050667\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))]^{\ 2}}{(5.75\ \mathrm{ft})^2 \times [(3 \times (1.5057\ \mathrm{kipft/ft})) + (2 \times (-0.050667\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))]}$$

$$p = 0.24006\ \mathrm{kip/ft^2}$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{9.425\ [(2\,M_o) + (H_o\,L_e)]}{L_e^2}$$

$$s = \frac{9.425 \times [(2 \times (1.5057\ \mathrm{kipft/ft})) + ((-0.050667\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))]}{(5.75\ \mathrm{ft})^2}$$

$$s = 0.77538\ \mathrm{kip/ft^2}$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\,\frac{a}{2}$$

$$p_a = (150\ \mathrm{psf/ft}) \times \frac{(3.8881\ \mathrm{ft})}{2}$$

$$p_a = 0.29161\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{\left(0.24006\ \mathrm{kip/ft^2}\right)}{\left(0.29161\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = 0.82325$$

Status: **PASS**
Ratio: **0.820**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\,L_e$$

$$p_s = (150\ \mathrm{psf/ft}) \times (5.75\ \mathrm{ft})$$

$$p_s = 0.8625\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{\left(0.77538\ \mathrm{kip/ft^2}\right)}{\left(0.8625\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = 0.899$$

Status: **PASS**
Ratio: **0.900**

Considering z-direction:

$H_o = 0.017\ \mathrm{kip/ft}$ - Lateral force per length of pile,
$M_o = 0.057667\ \mathrm{kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (0.057667\ \mathrm{kipft/ft}) \times (5.75\ \mathrm{ft})) + \left(3 \times (0.017\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft})^2\right)}{(6 \times (0.057667\ \mathrm{kipft/ft})) + (4 \times (0.017\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))}$$

$$a = 4.0875\ \mathrm{ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{1.178\ [(4\,M_o) + (3\,H_o\,L_e)]^2}{L_e^2\ [(3\,M_o) + (2\,H_o\,L_e)]}$$

$$p = \frac{1.178 \times [(4 \times (0.057667\ \mathrm{kipft/ft})) + (3 \times (0.017\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))]^{\ 2}}{(5.75\ \mathrm{ft})^2 \times [(3 \times (0.057667\ \mathrm{kipft/ft})) + (2 \times (0.017\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))]}$$

$$p = 0.02654\ \mathrm{kip/ft^2}$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{9.425\ [(2\,M_o) + (H_o\,L_e)]}{L_e^2}$$

$$s = \frac{9.425 \times [(2 \times (0.057667\ \mathrm{kipft/ft})) + ((0.017\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))]}{(5.75\ \mathrm{ft})^2}$$

$$s = 0.060743\ \mathrm{kip/ft^2}$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\,\frac{a}{2}$$

$$p_a = (150\ \mathrm{psf/ft}) \times \frac{(4.0875\ \mathrm{ft})}{2}$$

$$p_a = 0.30657\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{\left(0.02654\ \mathrm{kip/ft^2}\right)}{\left(0.30657\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = 0.086571$$

Status: **PASS**
Ratio: **0.090**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\,L_e$$

$$p_s = (150\ \mathrm{psf/ft}) \times (5.75\ \mathrm{ft})$$

$$p_s = 0.8625\ \mathrm{kip/ft^2}$$

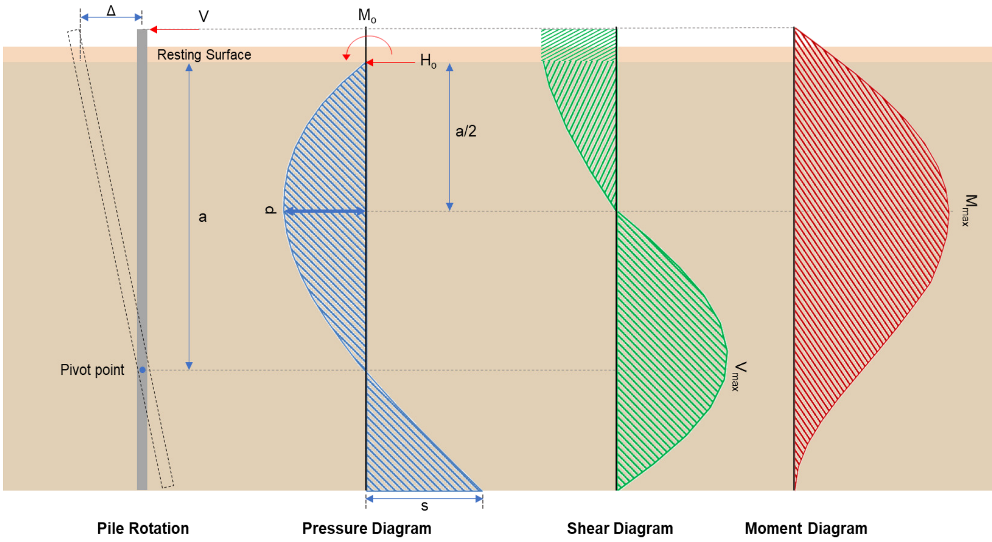
$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{(0.060743\ \mathrm{kip/ft^2})}{(0.8625\ \mathrm{kip/ft^2})}$$

$$Ratio = 0.070426$$

Status: **PASS**
Ratio: **0.070**



Shear force and Bending moment (x-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{D}$$

$$H_o = \frac{(-0.263\ \mathrm{kip})}{(36\ \mathrm{in})}$$

$$H_o = -0.087667\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\,H)}{D}$$

$$M_o = \frac{(7.911\ \mathrm{kipft}) + ((-0.263\ \mathrm{kip}) \times (0\ \mathrm{ft}))}{(36\ \mathrm{in})}$$

$$M_o = 2.637\ \mathrm{kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(2.637\ \mathrm{kipft/ft})}{(-0.087667\ \mathrm{kip/ft})}$$

$$E = 30.08\ \mathrm{ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (2.637\ \mathrm{kipft/ft}) \times (5.75\ \mathrm{ft})) + \left(3 \times (-0.087667\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft})^2\right)}{(6 \times (2.637\ \mathrm{kipft/ft})) + (4 \times (-0.087667\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))}$$

$$a = 3.8875\ \mathrm{ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\, D) \left[1 - \left[3 \left(\frac{4\,E}{L_e} + 3\right) \left(\frac{a}{L_e}\right)^2\right] + \left[4 \left(\frac{3\,E}{L_e} + 2\right) \left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((-0.087667\ \mathrm{kip/ft}) \times (36\ \mathrm{in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (30.08\ \mathrm{ft})}{(5.75\ \mathrm{ft})} + 3\right) \times \left(\frac{(3.8875\ \mathrm{ft})}{(5.75\ \mathrm{ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (30.08\ \mathrm{ft})}{(5.75\ \mathrm{ft})} + 2\right) \times \left(\frac{(3.8875\ \mathrm{ft})}{(5.75\ \mathrm{ft})}\right)^3\right]\right]$$

$$V_{max} = 2.6132\ \mathrm{kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, D\, L_e) \left[\left(\frac{E}{L_e} + \frac{a}{2\,L_e}\right) - \left[\left(\frac{4\,E}{L_e} + 3\right) \left(\frac{a}{2\,L_e}\right)^3\right] + \left[\left(\frac{3\,E}{L_e} + 2\right) \left(\frac{a}{2\,L_e}\right)^4\right]\right]$$

$$M_{max} = ((-0.087667\ \mathrm{kip/ft}) \times (36\ \mathrm{in}) \times (5.75\ \mathrm{ft})) \times \left[\left(\frac{(30.08\ \mathrm{ft})}{(5.75\ \mathrm{ft})} + \frac{(3.8875\ \mathrm{ft})}{2 \times (5.75\ \mathrm{ft})}\right) - \left[\left(\frac{4 \times (30.08\ \mathrm{ft})}{(5.75\ \mathrm{ft})} + 3\right) \times \left(\frac{(3.8875\ \mathrm{ft})}{2 \times (5.75\ \mathrm{ft})}\right)^3\right] + \left[\left(\frac{3 \times (30.08\ \mathrm{ft})}{(5.75\ \mathrm{ft})} + 2\right) \times \left(\frac{(3.8875\ \mathrm{ft})}{2 \times (5.75\ \mathrm{ft})}\right)^4\right]\right]$$

$$M_{max} = 7.374\ \mathrm{kipft}$$

Shear force and Bending moment (z-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{D}$$

$$H_o = \frac{(0.085\ \mathrm{kip})}{(36\ \mathrm{in})}$$

$$H_o = 0.028333\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\, H)}{D}$$

$$M_o = \frac{(0.289\ \mathrm{kipft}) + ((0.085\ \mathrm{kip}) \times (0\ \mathrm{ft}))}{(36\ \mathrm{in})}$$

$$M_o = 0.096333\ \mathrm{kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(0.096333\ \mathrm{kipft/ft})}{(0.028333\ \mathrm{kip/ft})}$$

$$E = 3.4\ \mathrm{ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\, M_o\, L_e) + \left(3\, H_o\, L_e^2\right)}{(6\, M_o) + (4\, H_o\, L_e)}$$

$$a = \frac{(4 \times (0.096333\ \mathrm{kipft/ft}) \times (5.75\ \mathrm{ft})) + \left(3 \times (0.028333\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft})^2\right)}{(6 \times (0.096333\ \mathrm{kipft/ft})) + (4 \times (0.028333\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))}$$

$$a = 4.0873\ \mathrm{ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\, b) \left[1 - \left[3 \left(\frac{4\,E}{L_e} + 3\right) \left(\frac{a}{L_e}\right)^2\right] + \left[4 \left(\frac{3\,E}{L_e} + 2\right) \left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((0.028333\ \text{kip/ft}) \times (36\ \text{in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (3.4\ \text{ft})}{(5.75\ \text{ft})} + 3\right) \times \left(\frac{(4.0873\ \text{ft})}{(5.75\ \text{ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (3.4\ \text{ft})}{(5.75\ \text{ft})} + 2\right) \times \left(\frac{(4.0873\ \text{ft})}{(5.75\ \text{ft})}\right)^3\right]\right]$$

$$V_{max} = 0.14543\ \text{kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, b\, L_e) \left[\left(\frac{E}{L_e} + \frac{a}{2\, L_e}\right) - \left[\left(\frac{4\, E}{L_e} + 3\right) \left(\frac{a}{2\, L_e}\right)^3\right] + \left[\left(\frac{3\, E}{L_e} + 2\right) \left(\frac{a}{2\, L_e}\right)^4\right]\right]$$

$$M_{max} = ((0.028333\ \text{kip/ft}) \times (36\ \text{in}) \times (5.75\ \text{ft})) \times \left[\left(\frac{(3.4\ \text{ft})}{(5.75\ \text{ft})} + \frac{(4.0873\ \text{ft})}{2 \times (5.75\ \text{ft})}\right) - \left[\left(\frac{4 \times (3.4\ \text{ft})}{(5.75\ \text{ft})} + 3\right) \times \left(\frac{(4.0873\ \text{ft})}{2 \times (5.75\ \text{ft})}\right)^3\right] + \left[\left(\frac{3 \times (3.4\ \text{ft})}{(5.75\ \text{ft})} + 2\right) \times \left(\frac{(4.0873\ \text{ft})}{2 \times (5.75\ \text{ft})}\right)^4\right]\right]$$

$$M_{max} = 0.37441\ \text{kipft}$$

Minimum Reinforcement Check (LRFD)

Parameters:

$f'_{ck} = 2.5\ \text{ksi}$ - Concrete strength,
$f_{yk} = 60\ \text{ksi}$ - Longitudinal reinforcement strength,
$\phi = 0.65$ - Reduction factor for axial strength,
Table 22.4.2.1 $\alpha = 0.85$ - Alpha factor for axial strength,
$A_g = 1017.9\ \text{in}^2$ - Gross area of concrete,

Longitudinal reinforcement:

Required reinforcement due to axial load, $A_{st,required}$
22.4.2.2, 10.6.1.1 $A_{st,required}$

$$A_{st,required} = Min\left[\frac{\frac{P}{\phi\, \alpha} - (0.85\, f'_{ck}\, A_g)}{f_{yk} - (0.85\, f'_{ck})}, (0.08\, A_g)\right]$$

$$A_{st,required} = Min\left[\frac{\frac{(19.388\ \text{kip})}{(0.65)\times(0.85)} - (0.85 \times (2.5\ \text{ksi}) \times (1017.9\ \text{in}^2))}{(60\ \text{ksi}) - (0.85 \times (2.5\ \text{ksi}))}, (0.08 \times (1017.9\ \text{in}^2))\right]$$

$$A_{st,required} = -36.767\ \text{in}^2$$

A_{min} - Governing minimum reinforcement area,

$$A_{min} = Max\left[A_{st,required}, (0.0018\, A_g)\right]$$

$$A_{min} = Max\left[(-36.767\ \text{in}^2), (0.0018 \times (1017.9\ \text{in}^2))\right]$$

$$A_{min} = 1.8322\ \text{in}^2$$

n_{rebar} - Required number of reinforcement,

$$n_{rebar} = \frac{A_{min}}{A_{rebar}}$$

$$n_{rebar} = \frac{(1.8322\ \text{in}^2)}{(0.3068\ \text{in}^2)}$$

$$n_{rebar} = 6$$

A_{st} - Actual total reinforcement area,

$$A_{st} = n_{rebar}\, \frac{\pi\, d_{bar}^2}{4}$$

$$A_{st} = (6) \times \frac{\pi \times (0.625\ \text{in})^2}{4}$$

$$A_{st} = 1.8408\ \text{in}^2$$

$Ratio$ - Capacity

$$Ratio = \frac{A_{min}}{A_{st}}$$

$$(1.8322\ \text{in}^2)$$

$$Ratio = \frac{}{(1.8408\ \mathrm{in}^2)}$$

$$Ratio = 0.99533$$

Status: **PASS**
Ratio: **1.000**

25.2.3 s_{rebar} - Minimum spacing of reinforcement,

$$s_{rebar} = Max\left[1.5, \left(1.5\ d_{bar}\right)\right]$$

$$s_{rebar} = Max\left[1.5, \left(1.5 \times (0.625\ \mathrm{in})\right)\right]$$

$$s_{rebar} = 1.5\ \mathrm{in}$$

Ties:

25.7.2.2 Since longitudinal reinforcement is $\leq$ No. 10ø: Use #3(0.375 in)

25.7.2.1 s_{ties} - Maximum center-to-center spacing of ties,

$$s_{ties} = Min\left[\left(16\ d_{bar}\right), \left(48\ d_{ties}\right), D\right]$$

$$s_{ties} = Min\left[\left(16 \times (0.625\ \mathrm{in})\right), \left(48 \times (0.375\ \mathrm{in})\right), (36\ \mathrm{in})\right]$$

$$s_{ties} = 10\ \mathrm{in}$$

Summary:

Main reinforcement: **6 - #5 (0.625 in)**
Ties: **#3(0.375 in) - 10 in**

Axial Compression Strength (ACI 318-19, LRFD)

22.4.2.2 ϕP_N - Allowable axial compressive strength

$$\phi P_N = \phi\, 0.85\ \left[\left(0.85\ f'_{ck}\ \left[A_g - A_{st}\right]\right) + \left(f_{yk}\ A_{st}\right)\right]$$

$$\phi P_N = (0.65) \times 0.85 \times \left[\left(0.85 \times (2.5\ \mathrm{ksi}) \times \left[\left(1017.9\ \mathrm{in}^2\right) - \left(1.8408\ \mathrm{in}^2\right)\right]\right) + \left((60\ \mathrm{ksi}) \times \left(1.8408\ \mathrm{in}^2\right)\right)\right]$$

$$\phi P_N = 1253.9\ \mathrm{kip}$$

$Ratio$ - Capacity

$$Ratio = \frac{P}{\phi P_N}$$

$$Ratio = \frac{(19.388\ \mathrm{kip})}{(1253.9\ \mathrm{kip})}$$

$$Ratio = 0.015462$$

Status: **PASS**
Ratio: **0.020**

Shear Strength (ACI 318-19, LRFD)

Parameters:

$b_w = 36\ \mathrm{in}$ - Effective width,

22.5.2.2 d - Effective depth

$$d = 0.80\ D$$

$$d = 0.80 \times (36\ \mathrm{in})$$

$$d = 28.8\ \mathrm{in}$$

22.5.5.1.3 λ_s - size effect modification factor

$$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{d}{10}}}, 1\right]$$

$$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{(28.8\ \mathrm{in})}{10}}}, 1\right]$$

$$\lambda_s = 0.71796$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5\ \mathrm{ksi} \rightarrow 2500\ \mathrm{psi}$,

22.5.5.1.1 $V_{c,max}$ - Max shear strength of concrete

$$V_{c,max} = 5\ \lambda_s\ \sqrt{f'_{ck}}\ b_w\ d$$

$$V_{c,max} = 5 \times (0.71796) \times \sqrt{(2500\ \mathrm{psi})} \times (36\ \mathrm{in}) \times (28.8\ \mathrm{in})$$

$$V_{c,max} = 186.09\ \text{kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5\ \text{ksi} \rightarrow 2500\ \text{psi}$, $P = 19.388\ \text{kip} \rightarrow 19388\ \text{lbf}$,

22.5.5.1.1(a) $V_{c,a}$ - Shear strength of concrete (a)

$$V_{c,a} = \left[2\ \lambda_s\ \sqrt{f'_{ck}} + \frac{P}{6\ A_g}\right]\ b_w\ d$$

$$V_{c,a} = \left[2 \times (0.71796) \times \sqrt{(2500\ \text{psi})} + \frac{(19388\ \text{lbf})}{6 \times (1017.9\ \text{in}^2)}\right] \times (36\ \text{in}) \times (28.8\ \text{in})$$

$$V_{c,a} = 77.729\ \text{kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5\ \text{ksi} \rightarrow 2500\ \text{psi}$,

22.5.5.1.2 $V_{c,b}$ - Shear strength of concrete (b)

$$V_{c,b} = \left[2\ \lambda_s\ \sqrt{f'_{ck}} + \left(0.05\ f'_{ck}\right)\right]\ b_w\ d$$

$$V_{c,b} = \left[2 \times (0.71796) \times \sqrt{(2500\ \text{psi})} + (0.05 \times (2500\ \text{psi}))\right] \times (36\ \text{in}) \times (28.8\ \text{in})$$

$$V_{c,b} = 204.04\ \text{kip}$$

V_c - Governing shear strength of concrete

$$V_c = Min\left[V_{c,max}, V_{c,a}, V_{c,b}\right]$$

$$V_c = Min\left[(186.09\ \text{kip}), (77.729\ \text{kip}), (204.04\ \text{kip})\right]$$

$$V_c = 77.729\ \text{kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5\ \text{ksi} \rightarrow 2500\ \text{psi}$,

22.5.1.2 $V_{s,a}$ - Shear strength of steel (a)

$$V_{s,a} = 8\ \sqrt{f'_{ck}}\ b_w\ d$$

$$V_{s,a} = 8 \times \sqrt{(2500\ \text{psi})} \times (36\ \text{in}) \times (28.8\ \text{in})$$

$$V_{s,a} = 414.72\ \text{kip}$$

A_v - Ties rebar area,

$$A_v = \frac{\pi\ d_{ties}^2}{4}$$

$$A_v = \frac{\pi \times (0.375\ \text{in})^2}{4}$$

$$A_v = 0.11045\ \text{in}^2$$

22.5.8.5.3 $V_{s,b}$ - Shear strength of steel (b)

$$V_{s,b} = \frac{2\ A_v\ f_{ywk}\ d}{s_{ties}}$$

$$V_{s,b} = \frac{2 \times \left(0.11045\ \text{in}^2\right) \times (60\ \text{ksi}) \times (28.8\ \text{in})}{(10\ \text{in})}$$

$$V_{s,b} = 38.17\ \text{kip}$$

V_s - Governing shear strength of steel

$$V_s = MIN\left[V_{s,a}, V_{s,b}\right]$$

$$V_s = MIN\left[(414.72\ \text{kip}), (38.17\ \text{kip})\right]$$

$$V_s = 38.17\ \text{kip}$$

22.5.1.1 ϕV_n - Allowable shear strength

$$\phi V_n = \phi\ \left(V_c + V_s\right)$$

$$\phi V_n = (0.65) \times ((77.729\ \text{kip}) + (38.17\ \text{kip}))$$

$$\phi V_n = 75.335\ \text{kip}$$

Considering x-direction:

$V_{max} = 2.6132\ \text{kip}$ - Maximum shear force in the x-direction,
$Ratio$ - Capacity

$$Ratio = \frac{V_{max}}{\phi V_n}$$

$$Ratio = \frac{(2.6132\ \mathrm{kip})}{(75.335\ \mathrm{kip})}$$

$$Ratio = 0.034687$$

Status: **PASS**
Ratio: **0.030**

Considering z-direction:

$V_{max} = 0.14543\ \mathrm{kip}$ - Maximum shear force in the z-direction,
$Ratio$ - Capacity

$$Ratio = \frac{V_{max}}{\phi V_n}$$

$$Ratio = \frac{(0.14543\ \mathrm{kip})}{(75.335\ \mathrm{kip})}$$

$$Ratio = 0.0019304$$

Status: **PASS**
Ratio: **0.000**

Flexural Strength (ACI 318-19, LRFD)

S_m - Section modulus

$$S_m = \frac{\pi\, D^3}{32}$$

$$S_m = \frac{\pi \times (36\ \mathrm{in})^3}{32}$$

$$S_m = 4580.4\ \mathrm{in}^3$$

$\lambda = 1$ - Concrete modification factor (Normal concrete),
Allowable flexural strength:
M_n shall be the lesser of:
$\phi M_{n,1}$

$$\phi M_{n,1} = \phi \times 5 \times \lambda \times \sqrt{(f_c')} \times S_m$$

$$\phi M_{n,1} = 0.65 \times 5 \times 1 \times \sqrt{(2.5 ksi)} \times 4580.442 in^3$$

$$\phi M_{n,1} = 62.027 kipft$$

14.5.2.1b $\phi M_{n,2}$

$$\phi M_{n,2} = \phi\, 0.85\, f_{ck}'\, S_m$$

$$\phi M_{n,2} = (0.65) \times 0.85 \times (2.5\ \mathrm{ksi}) \times \left(4580.4\ \mathrm{in}^3\right)$$

$$\phi M_{n,2} = 527.23\ \mathrm{kipft}$$

Therefore,
ϕM_n - Allowable flexural strength,

$$\phi M_n = MIN[\phi M_{n,1}, \phi M_{n,2}]$$

$$\phi M_n = MIN[(62.027\ \mathrm{kipft}), (527.23\ \mathrm{kipft})]$$

$$\phi M_n = 62.027\ \mathrm{kipft}$$

Considering x-direction:

$M_{max} = 7.374\ \mathrm{kipft}$ - Maximum moment in the x-direction,
$Ratio$ - Capacity

$$Ratio = \frac{M_{max}}{\phi M_n}$$

$$Ratio = \frac{(7.374\ \mathrm{kipft})}{(62.027\ \mathrm{kipft})}$$

$$Ratio = 0.11888$$

Status: **PASS**
Ratio: **0.120**

Considering z-direction:

$M_{max} = 0.37441\ \mathrm{kipft}$ - Maximum moment in the z-direction,
$Ratio$ - Capacity

	$Ratio = \frac{}{\phi M_n}$ $Ratio = \frac{(0.37441 \text{ kipft})}{(62.027 \text{ kipft})}$ $Ratio = 0.0060363$	Status: **PASS** Ratio: **0.010**