Your Project Calculations

Project Name: St Gregorys 5x10 A

S3D Model Link:
https://platform.skyciv.com/structural?preload_name=St%20Gregorys%205x10%20A&preload_path=Shared%20Enterprise%20Folder/MT_Solar_Projects/6_2023

Public Model Link:
https://platform.skyciv.com/structural-viewer?project_id=dCI5cl6qpBsGOP0BdseOGWBStihRsnZvlSDuaSdo8x2z7YTwnyr5p5YizqlvyQU3

Array Specification

Product:	Beam
Unique ID:	4P-19.75-6TOP-XD-12-L-5Hx10W-IFA7
Duty Classification:	XD
Module Width:	40.90 in
Module Length:	82.44in
Number of Rows:	5
Number of Columns:	10
Total Number of Modules:	50
Desired Tilt Angle:	10
Front Edge Clearance:	7
Total Array Height at Tilt:	9.98 ft
Total Frame Length:	68.75 ft
Frame Weight:	3384 lbs
Array Dimensions N/S:	17.25 ft
Array Dimensions E/W:	69.53 ft
Rail Length:	207.00 in
Rail Spacing:	3.48 ft
Rail Check:	PASS (40% utilized)

Support Specifications

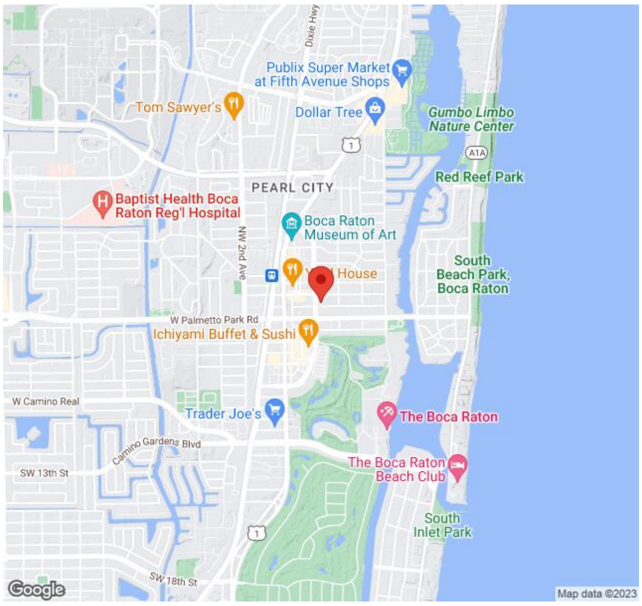
Pole Size:	6in Pipe Sch 40
Pole Length above Grade:	8.50 ft
Number of Poles:	4
Pole Spacing:	19.75 ft

Foundation Specifications

Foundation Type:	Square
Foundation Dimensions:	48 x 48 in
Foundation Depth (below grade):	Pile 1: 5.75 ft Pile 2: 6.00 ft Pile 3: 6.00 ft Pile 4: 5.75 ft
Foundation Volume:	13.926 y^3
Foundation Result:	PASSED
Mount Twist:	0.299975 kip

Site Info

Risk Category:	I
Exposure:	B
Soil Classification:	sand
Site Location:	100 NE Mizner Blvd, Boca Raton, FL 33432, USA
Wind Speed:	156 mph
Snow Load:	0 psf
Design Uplift Pressure:	Multiple pressures
Design Downforce Pressure:	Multiple pressures
Design Snow Pressure:	0.000000 ksf



Design Disclaimer

This software should be used for preliminary designs and should not be used as a final design unless reviewed, verified and designed by a qualified structural engineer.

AutoDesigner Input

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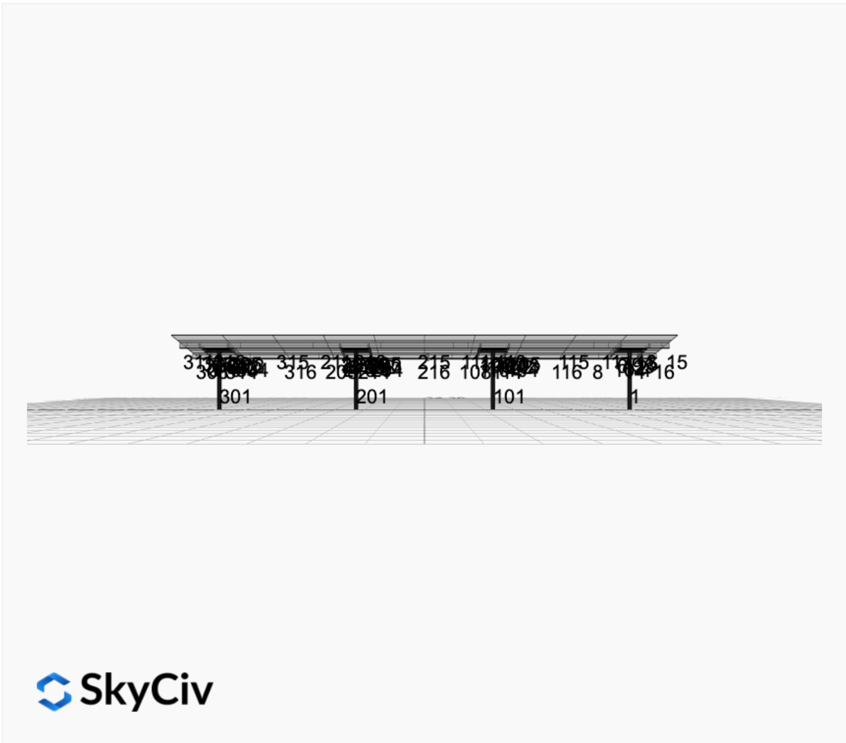
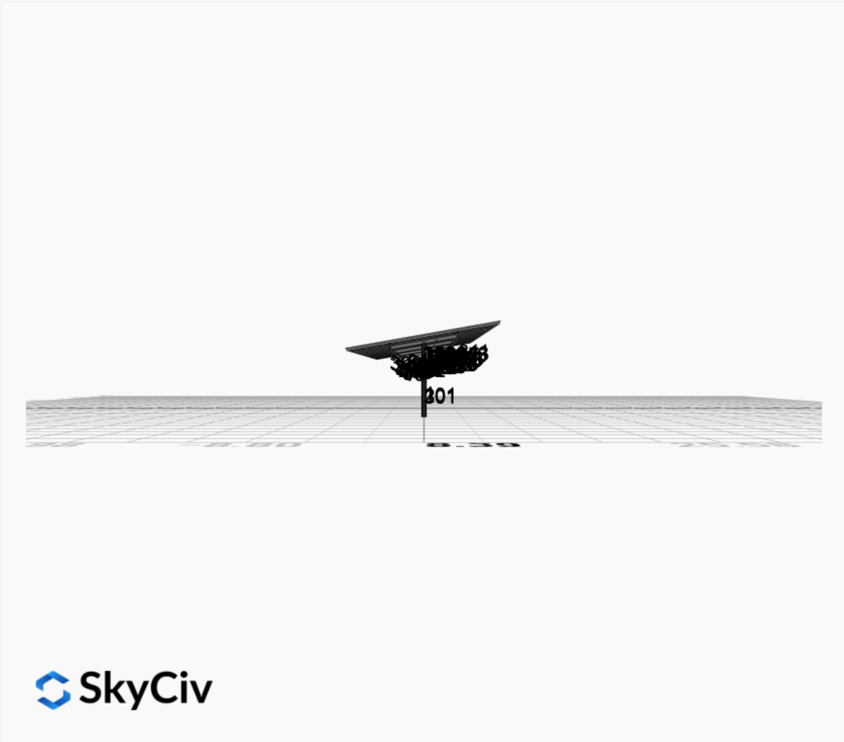
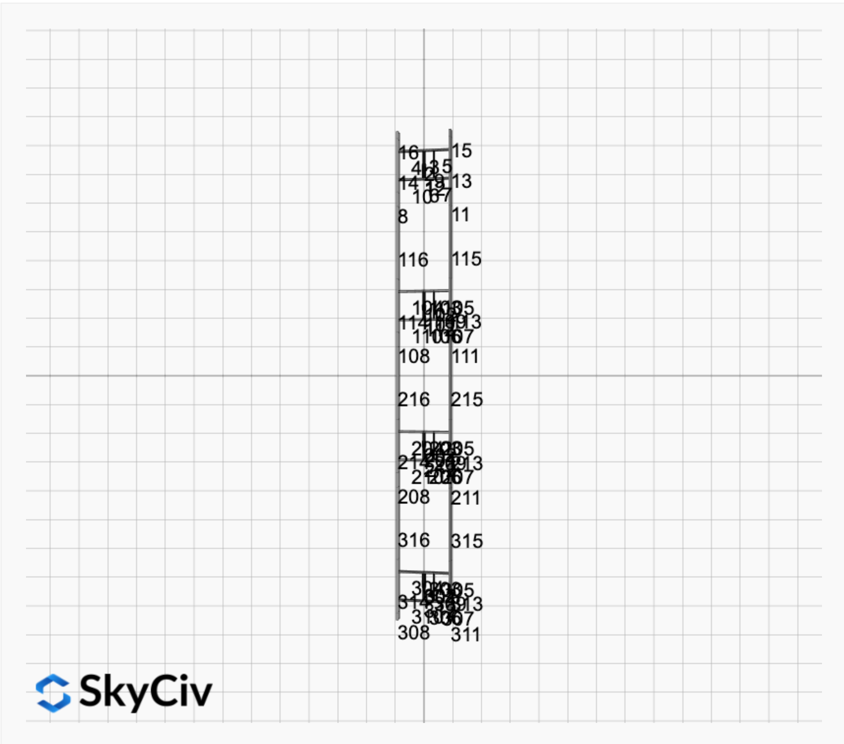
Design Notes:

- AISC Deflection checks are set to L/1 due to structure design intent



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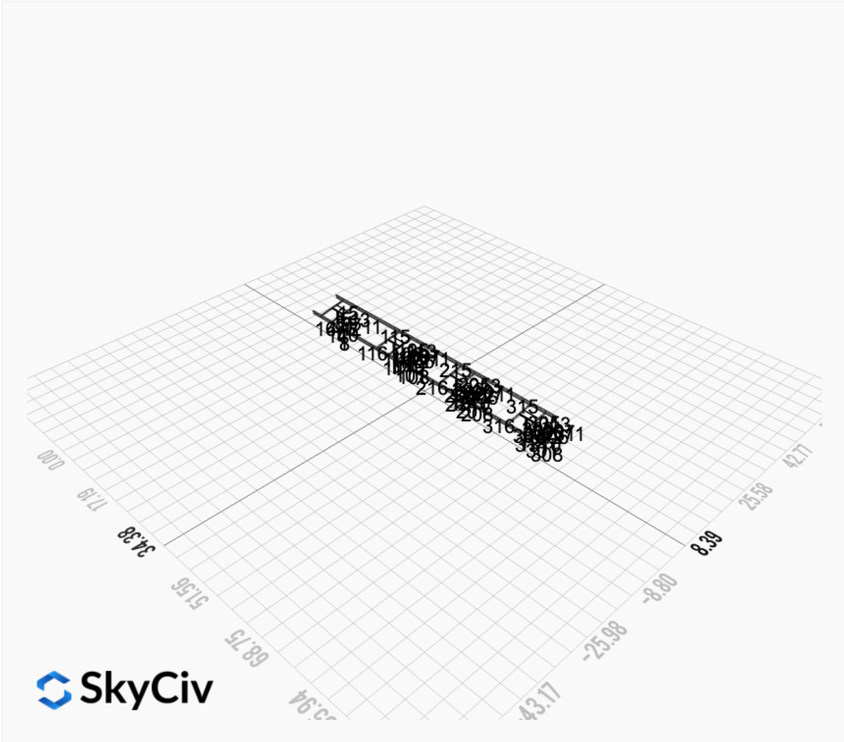
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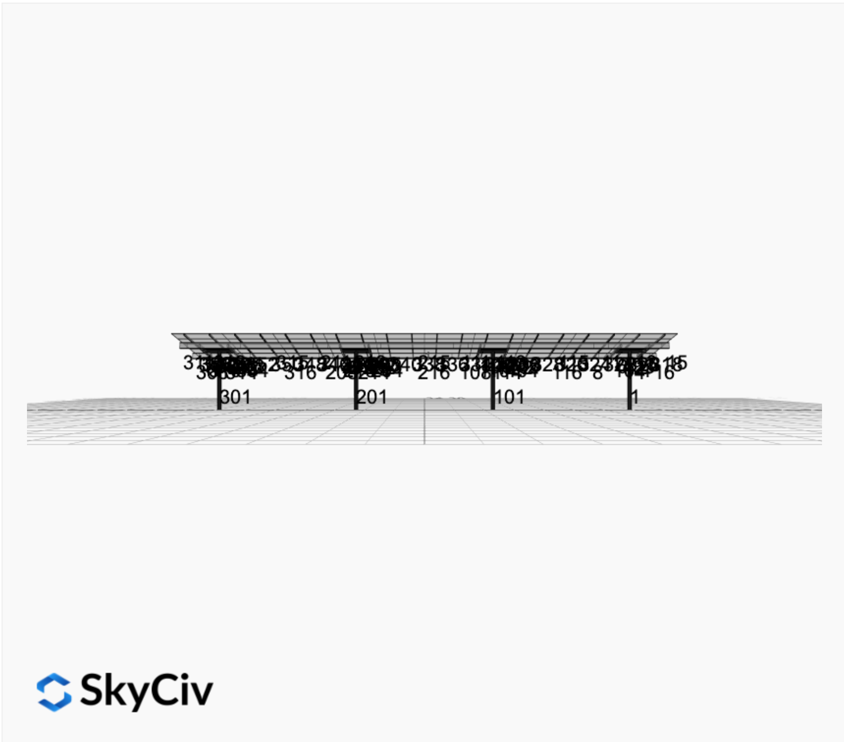
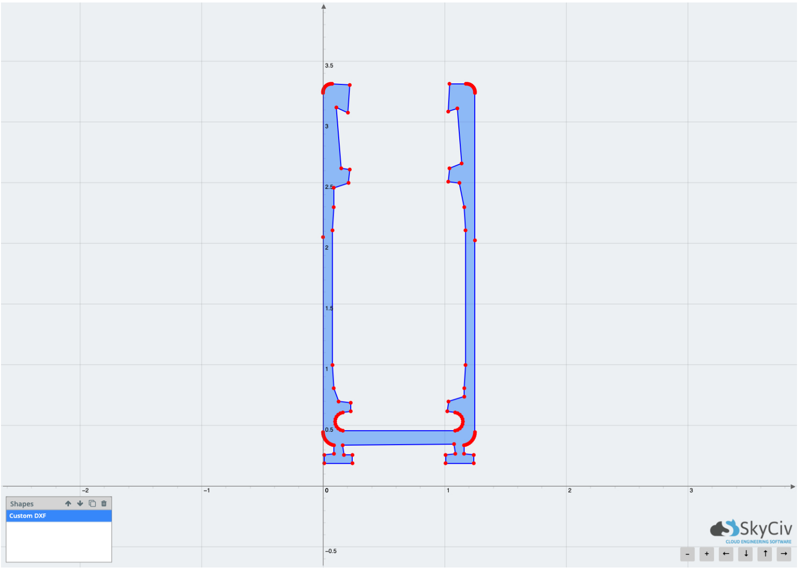
-8.80
-25.98
-43.17
25.58
42.77
34.38
51.56
68.75

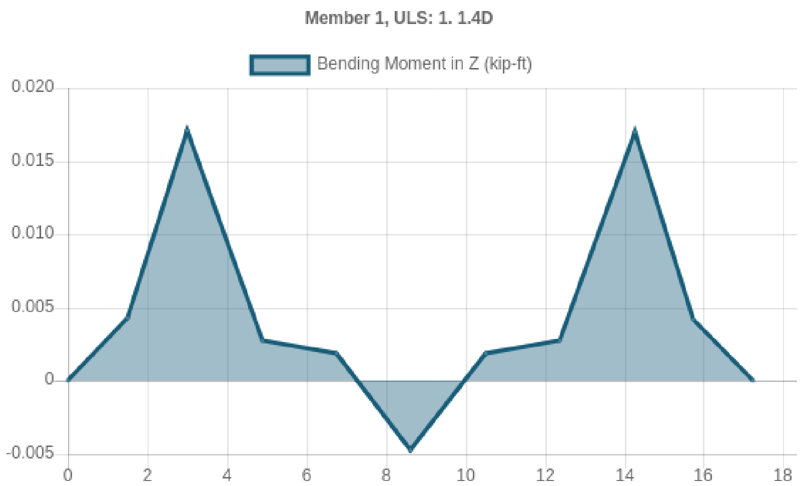

301
201
101
1

Rail Design Check

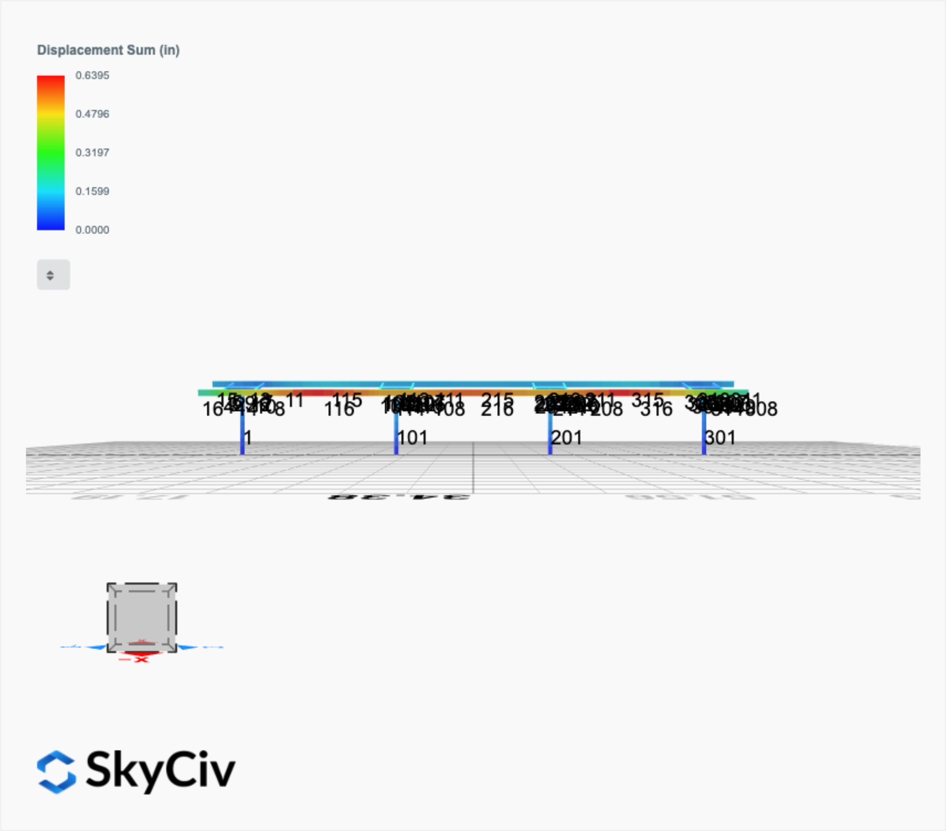
Rail Length: 17.25 ft
Additional Restraints Required: 4ft Spread Clamps
Tributary Width: 3.4766666666666666 ft
Material: Aluminium
Density: 169 lb/ft^3
Elasticity Modulus: 10000 ksi
Fy: 34.5 ksi
Fu: 37 ksi
Wind uplift Case A (X): 0.0000 kip/ft
Wind uplift Case A (Y): 0.0989 kip/ft
Wind uplift Case A: 0.0629 kip/ft
Wind uplift Case B: 0.0629 kip/ft
Wind uplift Case B (X): 0.0000 kip/ft
Wind uplift Case B (Y): 0.1408 kip/ft

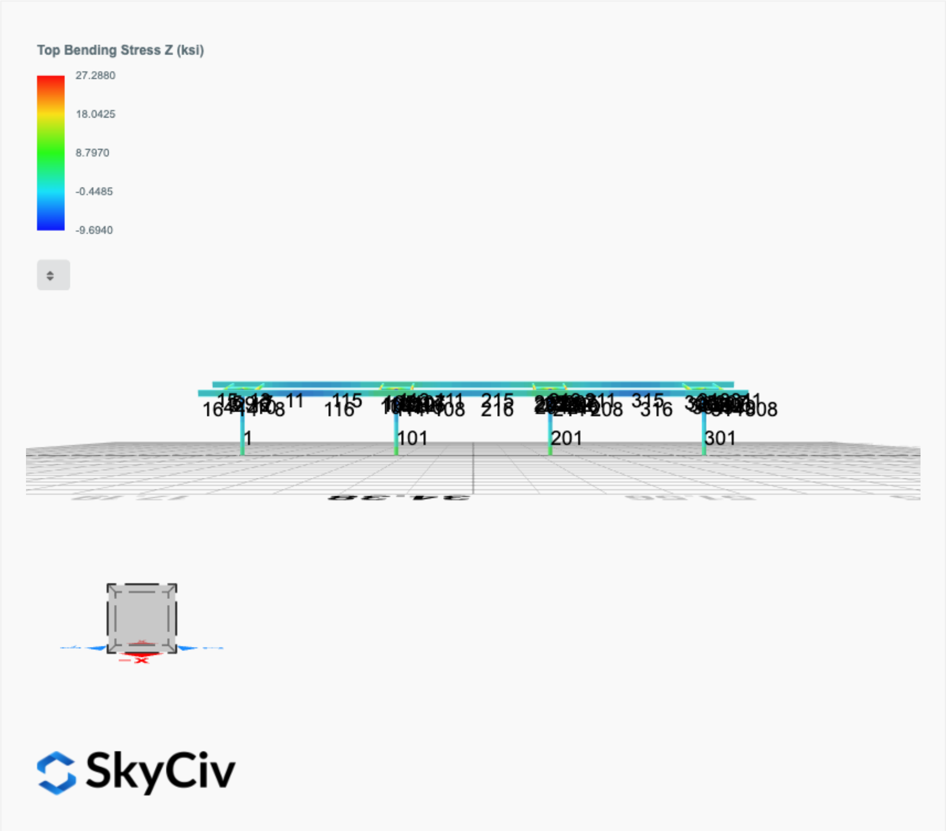


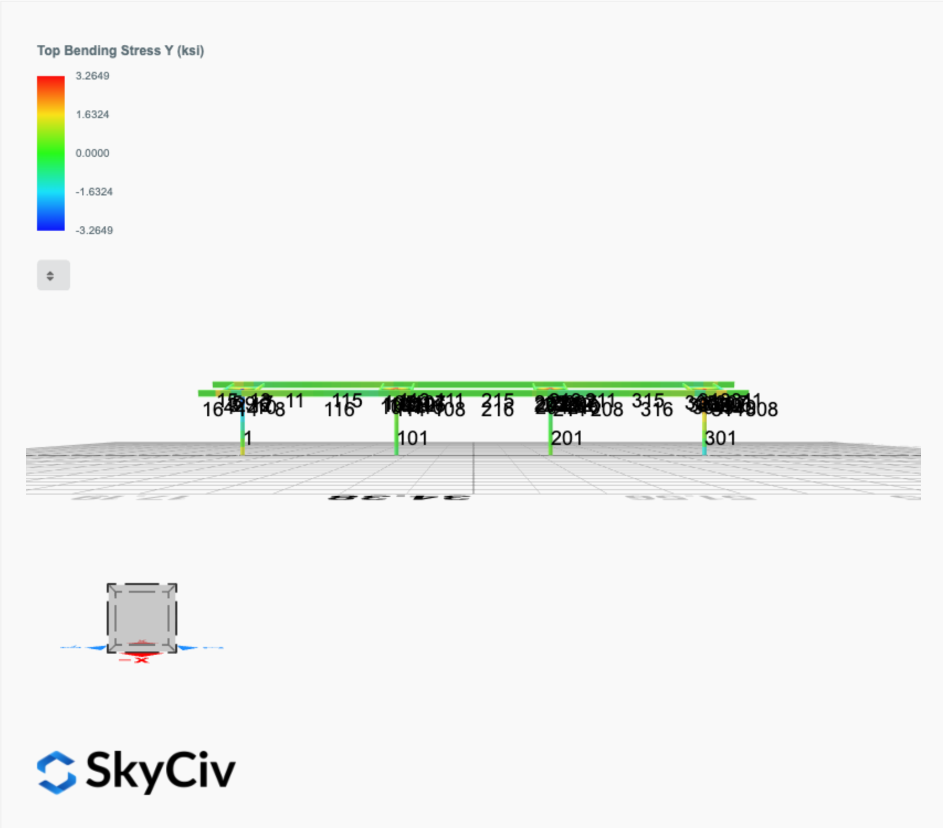
Result Check	Max Limit	Max Value	Utility	Status
Custom Stress Limit	34.5	13.73157853	0.398	PASS
Material Yield	34.5	13.73157853	0.398	PASS
Material Strength	37	13.73157853	0.371	PASS

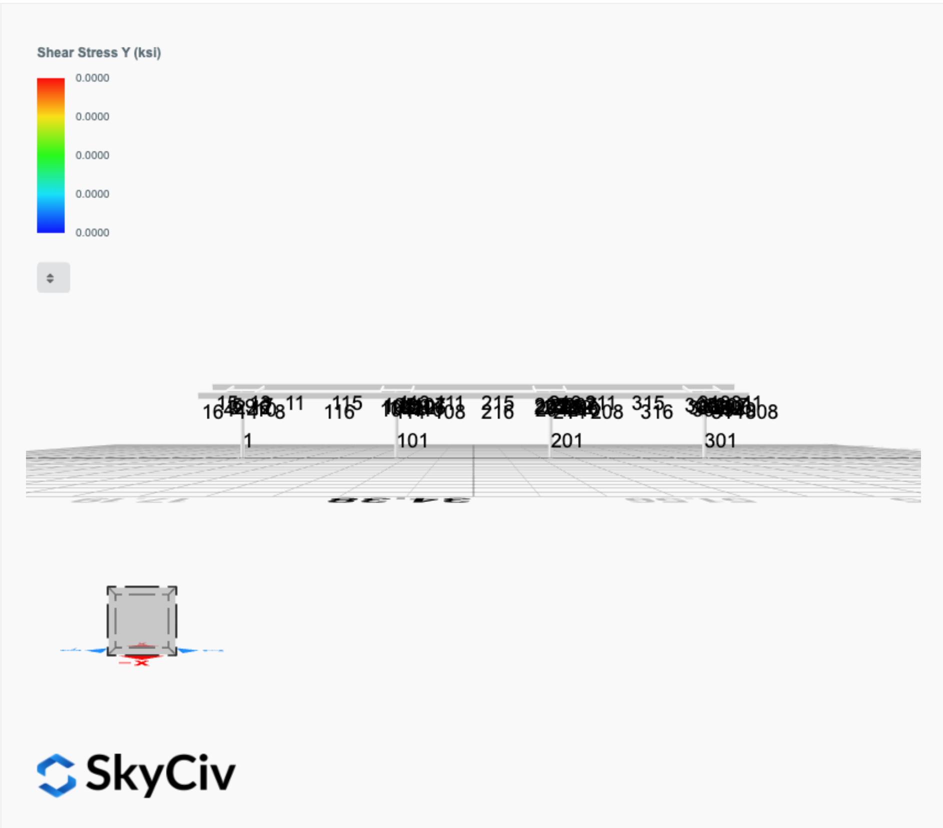


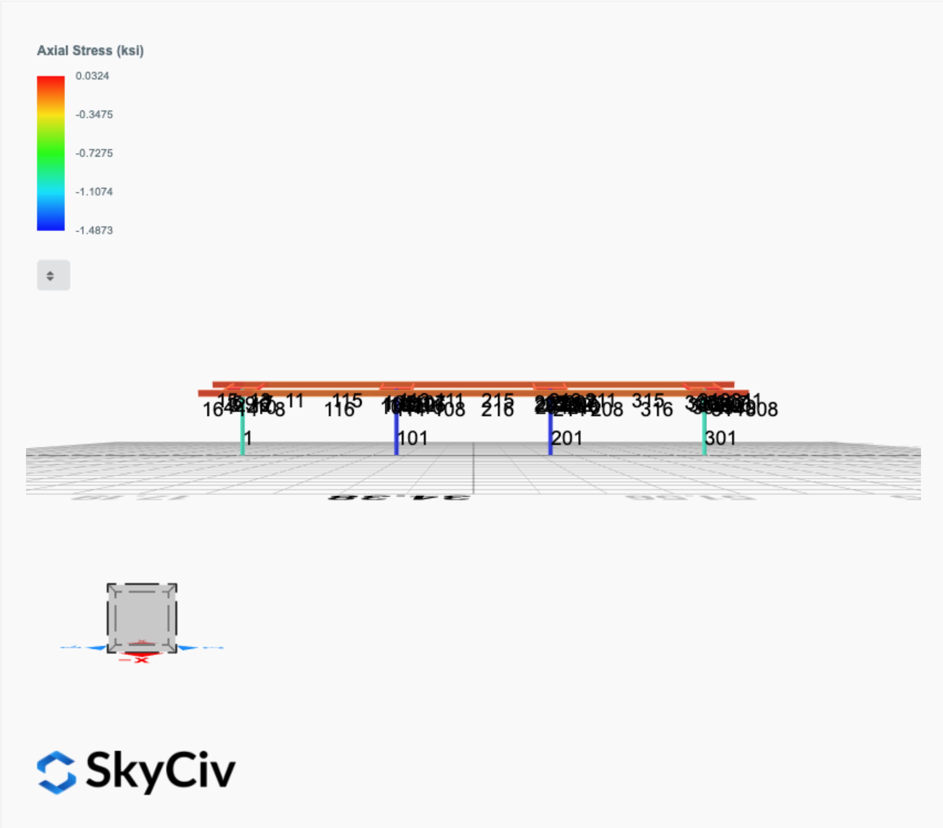
FEM Results (Envelope Worst Case for each member)






Top Bending Stress Y (ksi)
3.2649
1.6324
0.0000
-1.6324
-3.2649
115
116
215
216
315
316
1
101
201
301
SkyCiv


Shear Stress Y (ksi)
0.0000
0.0000
0.0000
0.0000
0.0000
115
116
215
216
315
316
1
101
201
301
SkyCiv


Axial Stress (ksi)
0.0324
-0.3475
-0.7275
-1.1074
-1.4873
1
101
201
301

Reaction Forces for Foundation 1 (Node ID#1), (kip, kip-ft)

ASD Load Combination Results

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 1. D	0.0154	1.8326	0.0987	0.2499	-0.0157	-0.0823
ULS: 2. D + L	0.0154	1.8326	0.0987	0.2499	-0.0157	-0.0823
ULS: 3. D + (S or Lr or R)	0.0154	1.8326	0.0987	0.2499	-0.0157	-0.0823
ULS: 3. D + (S or Lr or R)	0.0154	1.8326	0.0987	0.2499	-0.0157	-0.0823
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	0.0154	1.8326	0.0987	0.2499	-0.0157	-0.0823
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	0.0154	1.8326	0.0987	0.2499	-0.0157	-0.0823
ULS: 5b. D + 0.7E	0.0154	1.8326	0.0987	0.2499	-0.0157	-0.0823
ULS: 6b. D + 0.75L + 0.75(0.7)E + 0.75S	0.0154	1.8326	0.0987	0.2499	-0.0157	-0.0823
ULS: 8. 0.6D + 0.7E	0.0092	1.0996	0.0592	0.1499	-0.0094	-0.0494
ULS: 5a. D + 0.6W_Wind downforce Case A only	-0.8533	6.5038	0.4562	1.1465	-0.1835	11.7304
ULS: 5a. D + 0.6W_Wind downforce Case B only	-0.8533	6.5038	0.4562	1.1465	-0.1835	11.7304
ULS: 5a. D + 0.6W_Wind uplift Case A only	0.5964	-1.4331	-0.1322	-0.3276	0.0824	-2.1496
ULS: 5a. D + 0.6W_Wind uplift Case B only	0.5831	-1.0311	-0.1413	-0.3479	0.1112	-16.2732
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-0.6361	5.3360	0.3668	0.9223	-0.1416	8.7772
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-0.6361	5.3360	0.3668	0.9223	-0.1416	8.7772
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	0.4512	-0.6167	-0.0745	-0.1832	0.0579	-1.6328
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	0.4412	-0.3152	-0.0813	-0.1984	0.0795	-12.2255
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-0.6361	5.3360	0.3668	0.9223	-0.1416	8.7772
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-0.6361	5.3360	0.3668	0.9223	-0.1416	8.7772
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	0.4512	-0.6167	-0.0745	-0.1832	0.0579	-1.6328
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	0.4412	-0.3152	-0.0813	-0.1984	0.0795	-12.2255
ULS: 7. 0.6D + 0.6W_Wind downforce Case A only	-0.8595	5.7708	0.4167	1.0465	-0.1773	11.7633
ULS: 7. 0.6D + 0.6W_Wind downforce Case B only	-0.8595	5.7708	0.4167	1.0465	-0.1773	11.7633
ULS: 7. 0.6D + 0.6W_Wind uplift Case A only	0.5903	-2.1662	-0.1717	-0.4276	0.0887	-2.1167
ULS: 7. 0.6D + 0.6W_Wind uplift Case B only	0.5769	-1.7642	-0.1808	-0.4478	0.1174	-16.2403

Worst Case Reactions LRFD

These calculations are taken directly from the FEA via SkyCiv and are used in the Concrete Checks of the Foundatino Module. Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	9.9848
Shear X	-1.4479
Shear Z	0.7163
Moment X	1.8010
Moment Y (Twist)	0.2999
Moment Z	27.4598

Worst Case Reactions ASD

These results are taken from the worst case values in the above table and are used in the Soil Checks in the Foundation Module. Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	6.5038
Shear X	-0.8595
Shear Z	0.4562
Moment X	1.1465
Moment Y (Twist)	0.1835
Moment Z	16.2732

Reaction Forces for Foundation 2 (Node ID#101), (kip, kip-ft)

ASD Load Combination Results

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 1. D	-0.0154	2.5585	-0.0148	-0.0381	0.0055	0.1478
ULS: 2. D + L	-0.0154	2.5585	-0.0148	-0.0381	0.0055	0.1478
ULS: 3. D + (S or Lr or R)	-0.0154	2.5585	-0.0148	-0.0381	0.0055	0.1478
ULS: 3. D + (S or Lr or R)	-0.0154	2.5585	-0.0148	-0.0381	0.0055	0.1478
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	-0.0154	2.5585	-0.0148	-0.0381	0.0055	0.1478
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	-0.0154	2.5585	-0.0148	-0.0381	0.0055	0.1478
ULS: 5b. D + 0.7E	-0.0154	2.5585	-0.0148	-0.0381	0.0055	0.1478

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 6b. D + 0.75L + 0.75(0.7)E + 0.75S	-0.0154	2.5585	-0.0148	-0.0381	0.0055	0.1478
ULS: 8. 0.6D + 0.7E	-0.0092	1.5351	-0.0089	-0.0228	0.0033	0.0887
ULS: 5a. D + 0.6W_Wind downforce Case A only	-1.2201	9.6463	-0.0525	-0.1371	0.0019	16.0643
ULS: 5a. D + 0.6W_Wind downforce Case B only	-1.2201	9.6463	-0.0525	-0.1371	0.0019	16.0643
ULS: 5a. D + 0.6W_Wind uplift Case A only	0.8577	-2.4223	0.0212	0.0539	-0.0068	-3.1060
ULS: 5a. D + 0.6W_Wind uplift Case B only	0.6825	-1.7553	-0.0034	-0.0055	0.0266	-20.4433
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-0.9189	7.8744	-0.0431	-0.1123	0.0028	12.0852
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-0.9189	7.8744	-0.0431	-0.1123	0.0028	12.0852
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	0.6394	-1.1771	0.0122	0.0309	-0.0037	-2.2926
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	0.5080	-0.6769	-0.0063	-0.0137	0.0213	-15.2955
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-0.9189	7.8744	-0.0431	-0.1123	0.0028	12.0852
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-0.9189	7.8744	-0.0431	-0.1123	0.0028	12.0852
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	0.6394	-1.1771	0.0122	0.0309	-0.0037	-2.2926
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	0.5080	-0.6769	-0.0063	-0.0137	0.0213	-15.2955
ULS: 7. 0.6D + 0.6W_Wind downforce Case A only	-1.2140	8.6229	-0.0465	-0.1219	-0.0003	16.0052
ULS: 7. 0.6D + 0.6W_Wind downforce Case B only	-1.2140	8.6229	-0.0465	-0.1219	-0.0003	16.0052
ULS: 7. 0.6D + 0.6W_Wind uplift Case A only	0.8638	-3.4457	0.0272	0.0692	-0.0091	-3.1651
ULS: 7. 0.6D + 0.6W_Wind uplift Case B only	0.6887	-2.7787	0.0025	0.0097	0.0244	-20.5024

Worst Case Reactions LRFD

These calculations are taken directly from the FEA via SkyCiv and are used in the Concrete Checks of the Foundatino Module. Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	14.8830
Shear X	-2.0247
Shear Z	-0.0800
Moment X	-0.2099
Moment Y (Twist)	0.0430
Moment Z	34.7128

Worst Case Reactions ASD

These results are taken from the worst case values in the above table and are used in the Soil Checks in the Foundation Module. Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	9.6463
Shear X	-1.2201
Shear Z	-0.0525
Moment X	-0.1371
Moment Y (Twist)	0.0266
Moment Z	20.5024

Reaction Forces for Foundation 3 (Node ID#201), (kip, kip-ft)

ASD Load Combination Results

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 1. D	-0.0154	2.5585	0.0148	0.0381	-0.0055	0.1478
ULS: 2. D + L	-0.0154	2.5585	0.0148	0.0381	-0.0055	0.1478
ULS: 3. D + (S or Lr or R)	-0.0154	2.5585	0.0148	0.0381	-0.0055	0.1478
ULS: 3. D + (S or Lr or R)	-0.0154	2.5585	0.0148	0.0381	-0.0055	0.1478
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	-0.0154	2.5585	0.0148	0.0381	-0.0055	0.1478
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	-0.0154	2.5585	0.0148	0.0381	-0.0055	0.1478
ULS: 5b. D + 0.7E	-0.0154	2.5585	0.0148	0.0381	-0.0055	0.1478
ULS: 6b. D + 0.75L + 0.75(0.7)E + 0.75S	-0.0154	2.5585	0.0148	0.0381	-0.0055	0.1478
ULS: 8. 0.6D + 0.7E	-0.0092	1.5351	0.0089	0.0228	-0.0033	0.0887
ULS: 5a. D + 0.6W_Wind downforce Case A only	-1.2201	9.6463	0.0525	0.1371	-0.0019	16.0643
ULS: 5a. D + 0.6W_Wind downforce Case B only	-1.2201	9.6463	0.0525	0.1371	-0.0019	16.0643
ULS: 5a. D + 0.6W_Wind uplift Case A only	0.8577	-2.4223	-0.0212	-0.0539	0.0068	-3.1060
ULS: 5a. D + 0.6W_Wind uplift Case B only	0.6825	-1.7553	0.0034	0.0055	-0.0266	-20.4432
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-0.9189	7.8744	0.0431	0.1123	-0.0028	12.0852
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-0.9189	7.8744	0.0431	0.1123	-0.0028	12.0852
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	0.6394	-1.1771	-0.0122	-0.0309	0.0037	-2.2926
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	0.5080	-0.6769	0.0063	0.0137	-0.0213	-15.2955

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-0.9189	7.8744	0.0431	0.1123	-0.0028	12.0852
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-0.9189	7.8744	0.0431	0.1123	-0.0028	12.0852
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	0.6394	-1.1771	-0.0122	-0.0309	0.0037	-2.2926
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	0.5080	-0.6769	0.0063	0.0137	-0.0213	-15.2955
ULS: 7. 0.6D + 0.6W_Wind downforce Case A only	-1.2140	8.6229	0.0465	0.1219	0.0003	16.0052
ULS: 7. 0.6D + 0.6W_Wind downforce Case B only	-1.2140	8.6229	0.0465	0.1219	0.0003	16.0052
ULS: 7. 0.6D + 0.6W_Wind uplift Case A only	0.8638	-3.4457	-0.0272	-0.0692	0.0091	-3.1651
ULS: 7. 0.6D + 0.6W_Wind uplift Case B only	0.6887	-2.7787	-0.0025	-0.0097	-0.0244	-20.5024

Worst Case Reactions LRFD

These calculations are taken directly from the FEA via SkyCiv and are used in the Concrete Checks of the Foundatino Module.
Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	14.8830
Shear X	-2.0247
Shear Z	0.0800
Moment X	0.2099
Moment Y (Twist)	0.0430
Moment Z	34.7129

Worst Case Reactions ASD

These results are taken from the worst case values in the above table and are used in the Soil Checks in the Foundation Module.
Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	9.6463
Shear X	-1.2201
Shear Z	0.0525
Moment X	0.1371
Moment Y (Twist)	0.0266
Moment Z	20.5024

Reaction Forces for Foundation 4 (Node ID#301), (kip, kip-ft)

ASD Load Combination Results

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 1. D	0.0154	1.8326	-0.0987	-0.2499	0.0157	-0.0823
ULS: 2. D + L	0.0154	1.8326	-0.0987	-0.2499	0.0157	-0.0823
ULS: 3. D + (S or Lr or R)	0.0154	1.8326	-0.0987	-0.2499	0.0157	-0.0823
ULS: 3. D + (S or Lr or R)	0.0154	1.8326	-0.0987	-0.2499	0.0157	-0.0823
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	0.0154	1.8326	-0.0987	-0.2499	0.0157	-0.0823
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	0.0154	1.8326	-0.0987	-0.2499	0.0157	-0.0823
ULS: 5b. D + 0.7E	0.0154	1.8326	-0.0987	-0.2499	0.0157	-0.0823
ULS: 6b. D + 0.75L + 0.75(0.7)E + 0.75S	0.0154	1.8326	-0.0987	-0.2499	0.0157	-0.0823
ULS: 8. 0.6D + 0.7E	0.0092	1.0996	-0.0592	-0.1499	0.0094	-0.0494
ULS: 5a. D + 0.6W_Wind downforce Case A only	-0.8533	6.5038	-0.4562	-1.1465	0.1835	11.7304
ULS: 5a. D + 0.6W_Wind downforce Case B only	-0.8533	6.5038	-0.4562	-1.1465	0.1835	11.7304
ULS: 5a. D + 0.6W_Wind uplift Case A only	0.5964	-1.4331	0.1322	0.3276	-0.0824	-2.1496
ULS: 5a. D + 0.6W_Wind uplift Case B only	0.5831	-1.0311	0.1413	0.3479	-0.1112	-16.2732
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-0.6361	5.3360	-0.3668	-0.9223	0.1416	8.7772
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-0.6361	5.3360	-0.3668	-0.9223	0.1416	8.7772
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	0.4512	-0.6167	0.0745	0.1832	-0.0579	-1.6328
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	0.4412	-0.3152	0.0813	0.1984	-0.0794	-12.2255
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-0.6361	5.3360	-0.3668	-0.9223	0.1416	8.7772
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-0.6361	5.3360	-0.3668	-0.9223	0.1416	8.7772
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	0.4512	-0.6167	0.0745	0.1832	-0.0579	-1.6328
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	0.4412	-0.3152	0.0813	0.1984	-0.0794	-12.2255
ULS: 7. 0.6D + 0.6W_Wind downforce Case A only	-0.8595	5.7708	-0.4167	-1.0465	0.1773	11.7633
ULS: 7. 0.6D + 0.6W_Wind downforce Case B only	-0.8595	5.7708	-0.4167	-1.0465	0.1773	11.7633
ULS: 7. 0.6D + 0.6W_Wind uplift Case A only	0.5903	-2.1662	0.1717	0.4276	-0.0887	-2.1167
ULS: 7. 0.6D + 0.6W_Wind uplift Case B only	0.5769	-1.7642	0.1808	0.4478	-0.1174	-16.2403

Worst Case Reactions LRFD

Worst Case Reactions ASD

These calculations are taken directly from the FEA via SkyCiv and are used in the Concrete Checks of the Foundatino Module.
Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	9.9848
Shear X	-1.4479
Shear Z	-0.7163
Moment X	-1.8010
Moment Y (Twist)	0.3000
Moment Z	27.4602

These results are taken from the worst case values in the above table and are used in the Soil Checks in the Foundation Module.
Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	6.5038
Shear X	-0.8595
Shear Z	-0.4562
Moment X	-1.1465
Moment Y (Twist)	0.1835
Moment Z	16.2732

Project Details

Design Code: AISC 360-16 LRFD
Provision: LRFD
Country: United States

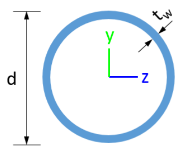
User Name: sales@mtsolar.us
Unit System: imperial

Design Input Information

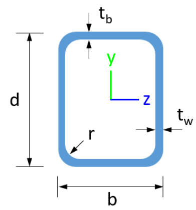
Design Factors			
Φ_t	Φ_c	Φ_b	Φ_v
0.9	0.9	0.9	0.9

Design Materials			
ID	E (ksi)	F_y (ksi)	F_u (ksi)
1	29000	50	65

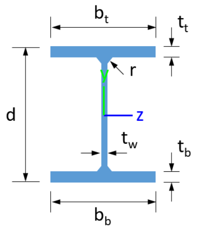
Section Dimensions



ID	Name	d (in)	t_w (in)					
3	2in Pipe Sch 120	2.38	0.25					
6	4in Pipe Sch 120	4.50	0.44					
7	6in Pipe Sch 40	6.63	0.28					



ID	Name	d (in)	b (in)	t_w (in)	t_b (in)	r (in)		
17	HSS5x3x1/4	5.00	3.00	0.23	0.23	0.23		



ID	Name	d (in)	t_w (in)	b_t (in)	b_b (in)	t_t (in)	t_b (in)	r (in)
20	W10x12	9.87	0.19	3.96	3.96	0.21	0.21	0.30

Section Properties								
ID	Name	A (in^2)	J (in^4)	I_{yp} (in^4)	I_{zp} (in^4)	I_w (in^6)	S_{yp} (in^3)	S_{zp} (in^3)
3	2in Pipe Sch 120	1.67	1.91	0.96	0.96	0.00	1.13	1.13
6	4in Pipe Sch 120	5.58	23.29	11.64	11.64	0.00	7.24	7.24
7	6in Pipe Sch 40	5.58	56.28	28.14	28.14	0.00	11.28	11.28

17	HSS5x3x1/4	3.37	11.00	4.81	10.70	62.42	3.77	5.38
20	W10x12	3.54	0.05	2.18	53.80	50.90	1.74	12.60

Member Properties								
Member ID	Section ID	K_zL (ft)	K_yL (ft)	L_b(ft)	C_b	LST	LSC	LD
1	7	17.85	17.85	8.50	-	300	200	1
2	6	1.30	1.30	2.00	-	300	200	1
3	17	0.92	0.92	1.42	1.19,1.19,1.19,1.19,1.19,1.19,1.17,1.17,1.16,1.17,1.17,1.17,1.16,1.17,1.18,1.18,1.09,1.16,1.18,1.18,1.09,1.16,1.17,1.17,1.17,1.17	300	200	1
4	17	2.44	2.44	3.75	1.70,1.70,1.70,1.70,1.70,1.70,1.67,1.67,1.66,1.69,1.67,1.67,1.66,1.69,1.68,1.68,1.63,1.69,1.68,1.68,1.63,1.69,1.67,1.67,1.66,1.69	300	200	1
5	17	1.52	1.52	2.33	1.69,1.69,1.69,1.69,1.69,1.67,1.67,1.65,1.66,1.67,1.67,1.65,1.66,1.67,1.67,1.58,1.65,1.67,1.67,1.58,1.65,1.67,1.67,1.66,1.66	300	200	1
6	17	0.92	0.92	1.42	1.19,1.19,1.19,1.19,1.19,1.19,1.18,1.18,1.17,1.18,1.18,1.18,1.17,1.18,1.19,1.19,1.13,1.18,1.19,1.19,1.13,1.18,1.18,1.18,1.18,1.18	300	200	1
7	17	1.52	1.52	2.33	1.68,1.68,1.68,1.68,1.68,1.68,1.67,1.67,1.66,1.66,1.67,1.67,1.66,1.66,1.67,1.67,1.61,1.66,1.67,1.67,1.61,1.66,1.67,1.67,1.66,1.67	300	200	1
8	20	1.33	1.33	2.05	1.20,1.20,1.20,1.20,1.20,1.17,1.17,1.22,2.14,1.17,1.17,1.22,2.14,1.18,1.18,1.23,1.50,1.18,1.18,1.23,1.50,1.17,1.17,1.22,2.22	300	200	1
9	3	2.60	2.60	4.00	-	300	200	1
10	17	2.44	2.44	3.75	1.69,1.69,1.69,1.69,1.69,1.69,1.67,1.67,1.66,1.69,1.67,1.67,1.66,1.69,1.68,1.68,1.64,1.69,1.68,1.68,1.64,1.69,1.67,1.67,1.66,1.69	300	200	1
11	20	1.33	1.33	2.05	1.21,1.21,1.21,1.21,1.21,1.21,1.24,1.24,1.18,1.31,1.24,1.24,1.18,1.31,1.24,1.24,1.13,1.35,1.24,1.24,1.13,1.35,1.24,1.24,1.18,1.31	300	200	1
12	6	1.30	1.30	2.00	-	300	200	1
13	20	4.88	4.00	7.50	1.88,1.88,1.88,1.88,1.88,1.88,1.59,1.59,2.31,1.45,1.59,1.59,2.31,1.45,1.64,1.64,3.07,1.43,1.64,1.64,3.07,1.43,1.58,1.58,2.25,1.45	300	200	1
14	20	4.88	4.00	7.50	1.96,1.96,1.96,1.96,1.96,1.96,2.46,2.46,1.93,1.99,2.46,2.46,1.93,1.99,2.35,2.35,1.92,1.63,2.35,2.35,1.92,1.63,2.49,2.49,1.94,2.11	300	200	1
15	20	2.10	2.10	1.00	2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33	300	200	1
16	20	2.10	2.10	1.00	2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33	300	200	1
101	7	17.85	17.85	8.50	-	300	200	1
102	6	1.30	1.30	2.00	-	300	200	1
103	17	0.92	0.92	1.42	1.19,1.19,1.19,1.19,1.19,1.19,1.18,1.18,1.17,1.17,1.18,1.18,1.17,1.17,1.18,1.18,1.15,1.17,1.18,1.18,1.15,1.17,1.18,1.18,1.18,1.18	300	200	1
104	17	2.44	2.44	3.75	1.69,1.69,1.69,1.69,1.69,1.69,1.67,1.67,1.66,1.69,1.67,1.67,1.66,1.69,1.67,1.67,1.65,1.69,1.67,1.67,1.65,1.69,1.67,1.67,1.66,1.70	300	200	1
105	17	1.52	1.52	2.33	1.68,1.68,1.68,1.68,1.68,1.68,1.67,1.67,1.66,1.66,1.67,1.67,1.66,1.66,1.67,1.67,1.63,1.66,1.67,1.67,1.63,1.66,1.67,1.67,1.66,1.67	300	200	1
106	17	0.92	0.92	1.42	1.19,1.19,1.19,1.19,1.19,1.19,1.18,1.18,1.17,1.18,1.18,1.18,1.17,1.18,1.18,1.18,1.14,1.17,1.18,1.18,1.14,1.17,1.18,1.18,1.17,1.18	300	200	1
107	17	1.52	1.52	2.33	1.68,1.68,1.68,1.68,1.68,1.68,1.67,1.67,1.66,1.66,1.67,1.67,1.66,1.66,1.67,1.67,1.62,1.66,1.67,1.67,1.62,1.66,1.67,1.67,1.66,1.67	300	200	1

108	20	1.33	1.33	2.05	2.35,2.35,2.35,2.35,2.35,2.35,2.22,2.22,2.16,1.18,2.22,2.22,2.16,1.18,2.25,2.25,2.10,1.35,2.25,2.25,2.10,1.35,2.19,2.19,2.19,1.14	300	200	1
109	3	2.60	2.60	4.00	-	300	200	1
110	17	2.44	2.44	3.75	1.69,1.69,1.69,1.69,1.69,1.69,1.67,1.67,1.66,1.69,1.67,1.67,1.66,1.69,1.67,1.67,1.65,1.69,1.67,1.67,1.65,1.69,1.67,1.67,1.66,1.69	300	200	1
111	20	1.33	1.33	2.05	2.35,2.35,2.35,2.35,2.35,2.35,2.07,2.07,2.27,1.60,2.07,2.07,2.27,1.60,2.08,2.08,1.78,1.48,2.08,2.08,1.78,1.48,2.07,2.07,2.28,1.63	300	200	1
112	6	1.30	1.30	2.00	-	300	200	1
113	20	4.88	4.00	7.50	1.06,1.06,1.06,1.06,1.06,1.06,1.07,1.07,1.05,1.13,1.07,1.07,1.05,1.13,1.07,1.07,1.05,1.22,1.07,1.07,1.05,1.22,1.07,1.07,1.06,1.11	300	200	1
114	20	4.88	4.00	7.50	1.06,1.06,1.06,1.06,1.06,1.06,1.06,1.06,1.07,1.42,1.06,1.06,1.07,1.42,1.06,1.06,1.07,1.83,1.06,1.06,1.07,1.83,1.06,1.06,1.07,1.14	300	200	1
115	20	6.63	6.63	10.20	1.16,1.16,1.16,1.16,1.16,1.16,1.13,1.13,1.19,1.10,1.13,1.13,1.19,1.10,1.14,1.14,1.25,1.09,1.14,1.14,1.25,1.09,1.13,1.13,1.19,1.10	300	200	1
116	20	6.63	6.63	10.20	1.17,1.17,1.17,1.17,1.17,1.17,1.21,1.21,1.15,1.08,1.21,1.21,1.15,1.08,1.20,1.20,1.14,1.07,1.20,1.20,1.14,1.07,1.21,1.21,1.15,1.12	300	200	1
201	7	17.85	17.85	8.50	-	300	200	1
202	6	1.30	1.30	2.00	-	300	200	1
203	17	0.92	0.92	1.42	1.19,1.19,1.19,1.19,1.19,1.19,1.18,1.18,1.17,1.18,1.18,1.18,1.17,1.18,1.18,1.18,1.14,1.17,1.18,1.18,1.14,1.17,1.18,1.18,1.17,1.18	300	200	1
204	17	2.44	2.44	3.75	1.69,1.69,1.69,1.69,1.69,1.69,1.67,1.67,1.66,1.69,1.67,1.67,1.66,1.69,1.67,1.67,1.65,1.69,1.67,1.67,1.65,1.69,1.67,1.67,1.66,1.69	300	200	1
205	17	1.52	1.52	2.33	1.68,1.68,1.68,1.68,1.68,1.68,1.67,1.67,1.66,1.66,1.67,1.67,1.66,1.66,1.67,1.67,1.62,1.66,1.67,1.67,1.62,1.66,1.67,1.67,1.66,1.67	300	200	1
206	17	0.92	0.92	1.42	1.19,1.19,1.19,1.19,1.19,1.19,1.18,1.18,1.17,1.17,1.18,1.18,1.17,1.17,1.18,1.18,1.15,1.17,1.18,1.18,1.15,1.17,1.18,1.18,1.18,1.18	300	200	1
207	17	1.52	1.52	2.33	1.68,1.68,1.68,1.68,1.68,1.68,1.67,1.67,1.66,1.66,1.67,1.67,1.66,1.66,1.67,1.67,1.63,1.66,1.67,1.67,1.63,1.66,1.67,1.67,1.66,1.67	300	200	1
208	20	1.33	1.33	2.05	2.03,2.03,2.03,2.03,2.03,2.03,2.12,2.12,1.85,1.09,2.12,2.12,1.85,1.09,2.10,2.10,1.78,1.24,2.10,2.10,1.78,1.24,2.12,2.12,1.86,1.04	300	200	1
209	3	2.60	2.60	4.00	-	300	200	1
210	17	2.44	2.44	3.75	1.69,1.69,1.69,1.69,1.69,1.69,1.67,1.67,1.66,1.69,1.67,1.67,1.66,1.69,1.67,1.67,1.65,1.69,1.67,1.67,1.65,1.69,1.67,1.67,1.66,1.70	300	200	1
211	20	1.33	1.33	2.05	1.91,1.91,1.91,1.91,1.91,1.91,1.63,1.63,2.09,1.40,1.63,1.63,2.09,1.40,1.66,1.66,2.36,1.33,1.66,1.66,2.36,1.33,1.62,1.62,2.09,1.41	300	200	1
212	6	1.30	1.30	2.00	-	300	200	1
213	20	4.88	4.00	7.50	1.06,1.06,1.06,1.06,1.06,1.06,1.07,1.07,1.05,1.13,1.07,1.07,1.05,1.13,1.07,1.07,1.05,1.22,1.07,1.07,1.05,1.22,1.07,1.07,1.06,1.12	300	200	1
214	20	4.88	4.00	7.50	1.06,1.06,1.06,1.06,1.06,1.06,1.06,1.06,1.07,1.42,1.06,1.06,1.07,1.42,1.06,1.06,1.07,1.83,1.06,1.06,1.07,1.83,1.06,1.06,1.07,1.14	300	200	1
215	20	6.63	6.63	10.20	1.18,1.18,1.18,1.18,1.18,1.18,1.15,1.15,1.21,1.12,1.15,1.15,1.21,1.12,1.15,1.15,1.52,1.11,1.15,1.15,1.52,1.11,1.15,1.15,1.20,1.12	300	200	1
216	20	6.63	6.63	10.20	1.19,1.19,1.19,1.19,1.19,1.19,1.23,1.23,1.17,1.06,1.23,1.23,1.17,1.06,1.22,1.22,1.16,1.09,1.22,1.22,1.16,1.09,1.23,1.23,1.17,1.05	300	200	1
301	7	17.85	17.85	8.50	-	300	200	1

302	6	1.30	1.30	2.00	-	300	200	1
303	17	0.92	0.92	1.42	1.19,1.19,1.19,1.19,1.19,1.19,1.18,1.18,1.17,1.18,1.18,1.18,1.17,1.18,1.19,1.19,1.13,1.18,1.19,1.19,1.13,1.18,1.18,1.18,1.18,1.18	300	200	1
304	17	2.44	2.44	3.75	1.69,1.69,1.69,1.69,1.69,1.69,1.67,1.67,1.66,1.69,1.67,1.67,1.66,1.69,1.68,1.68,1.64,1.69,1.68,1.68,1.64,1.69,1.67,1.67,1.66,1.69	300	200	1
305	17	1.52	1.52	2.33	1.68,1.68,1.68,1.68,1.68,1.68,1.67,1.67,1.66,1.66,1.67,1.67,1.66,1.66,1.67,1.67,1.61,1.66,1.67,1.67,1.61,1.66,1.67,1.67,1.66,1.67	300	200	1
306	17	0.92	0.92	1.42	1.19,1.19,1.19,1.19,1.19,1.19,1.17,1.17,1.16,1.17,1.17,1.17,1.16,1.17,1.18,1.18,1.09,1.16,1.18,1.18,1.09,1.16,1.17,1.17,1.17,1.17	300	200	1
307	17	1.52	1.52	2.33	1.69,1.69,1.69,1.69,1.69,1.69,1.67,1.67,1.65,1.66,1.67,1.67,1.65,1.66,1.67,1.67,1.58,1.65,1.67,1.67,1.58,1.65,1.67,1.67,1.66,1.66	300	200	1
308	20	2.10	2.10	1.00	2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33	300	200	1
309	3	2.60	2.60	4.00	-	300	200	1
310	17	2.44	2.44	3.75	1.70,1.70,1.70,1.70,1.70,1.70,1.67,1.67,1.66,1.69,1.67,1.67,1.66,1.69,1.68,1.68,1.63,1.69,1.68,1.68,1.63,1.69,1.67,1.67,1.66,1.69	300	200	1
311	20	2.10	2.10	1.00	2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33,2.33	300	200	1
312	6	1.30	1.30	2.00	-	300	200	1
313	20	4.88	4.00	7.50	1.88,1.88,1.88,1.88,1.88,1.88,1.59,1.59,2.31,1.45,1.59,1.59,2.31,1.45,1.64,1.64,3.07,1.43,1.64,1.64,3.07,1.43,1.58,1.58,2.25,1.45	300	200	1
314	20	4.88	4.00	7.50	1.96,1.96,1.96,1.96,1.96,1.96,2.46,2.46,1.93,1.99,2.46,2.46,1.93,1.99,2.35,2.35,1.92,1.63,2.35,2.35,1.92,1.63,2.49,2.49,1.94,2.11	300	200	1
315	20	6.63	6.63	10.20	1.08,1.08,1.08,1.08,1.08,1.08,1.07,1.07,1.07,1.08,1.07,1.07,1.07,1.08,1.07,1.07,1.08,1.09,1.07,1.07,1.08,1.09,1.07,1.07,1.07,1.08	300	200	1
316	20	6.63	6.63	10.20	1.08,1.08,1.08,1.08,1.08,1.08,1.08,1.08,1.08,1.25,1.08,1.08,1.08,1.25,1.08,1.08,1.12,1.08,1.08,1.08,1.12,1.08,1.08,1.08,1.38	300	200	1

Member Design Capacity

Member ID	$\Phi_t P_n$ (kip)	$\Phi_c P_n$ (kip)	$\Phi_b M_{zn}$ (k-ft)	$\Phi_b M_{yn}$ (k-ft)	$\Phi_v V_{yn}$ (kip)	$\Phi_v V_{zn}$ (kip)
1	251.16	129.17	42.30	42.30	75.35	75.35
2	251.01	248.88	27.16	27.16	75.30	75.30
3	151.65	150.70	20.17	14.14	54.12	28.95
4	151.65	145.15	20.17	14.14	54.12	28.95
5	151.65	149.10	20.17	14.14	54.12	28.95
6	151.65	150.70	20.17	14.14	54.12	28.95
7	151.65	149.10	20.17	14.14	54.12	28.95
8	159.30	142.47	46.90	6.46	56.26	44.91
9	75.10	66.32	4.25	4.25	22.53	22.53
10	151.65	145.15	20.17	14.14	54.12	28.95
11	159.30	142.47	46.90	6.46	56.26	44.91
12	251.01	248.88	27.16	27.16	75.30	75.30
13	159.30	116.35	43.70	6.46	56.26	44.91
14	159.30	116.35	46.90	6.46	56.26	44.91
15	159.30	137.23	46.90	6.46	56.26	44.91
16	159.30	137.23	46.90	6.46	56.26	44.91
101	251.16	129.17	42.30	42.30	75.35	75.35
102	251.01	248.88	27.16	27.16	75.30	75.30
103	151.65	150.70	20.17	14.14	54.12	28.95

103	151.65	150.70	20.17	14.14	54.12	28.95
104	151.65	145.15	20.17	14.14	54.12	28.95
105	151.65	149.10	20.17	14.14	54.12	28.95
106	151.65	150.70	20.17	14.14	54.12	28.95
107	151.65	149.10	20.17	14.14	54.12	28.95
108	159.30	142.47	46.90	6.46	56.26	44.91
109	75.10	66.32	4.25	4.25	22.53	22.53
110	151.65	145.15	20.17	14.14	54.12	28.95
111	159.30	142.47	46.90	6.46	56.26	44.91
112	251.01	248.88	27.16	27.16	75.30	75.30
113	159.30	116.35	32.09	6.46	56.26	44.91
114	159.30	116.35	32.39	6.46	56.26	44.91
115	159.30	75.13	20.99	6.46	56.26	44.91
116	159.30	75.13	20.61	6.46	56.26	44.91
201	251.16	129.17	42.30	42.30	75.35	75.35
202	251.01	248.88	27.16	27.16	75.30	75.30
203	151.65	150.70	20.17	14.14	54.12	28.95
204	151.65	145.15	20.17	14.14	54.12	28.95
205	151.65	149.10	20.17	14.14	54.12	28.95
206	151.65	150.70	20.17	14.14	54.12	28.95
207	151.65	149.10	20.17	14.14	54.12	28.95
208	159.30	142.47	46.90	6.46	56.26	44.91
209	75.10	66.32	4.25	4.25	22.53	22.53
210	151.65	145.15	20.17	14.14	54.12	28.95
211	159.30	142.47	46.90	6.46	56.26	44.91
212	251.01	248.88	27.16	27.16	75.30	75.30
213	159.30	116.35	32.09	6.46	56.26	44.91
214	159.30	116.35	32.39	6.46	56.26	44.91
215	159.30	75.13	21.38	6.46	56.26	44.91
216	159.30	75.13	20.22	6.46	56.26	44.91
301	251.16	129.17	42.30	42.30	75.35	75.35
302	251.01	248.88	27.16	27.16	75.30	75.30
303	151.65	150.70	20.17	14.14	54.12	28.95
304	151.65	145.15	20.17	14.14	54.12	28.95
305	151.65	149.10	20.17	14.14	54.12	28.95
306	151.65	150.70	20.17	14.14	54.12	28.95
307	151.65	149.10	20.17	14.14	54.12	28.95
308	159.30	137.23	46.90	6.46	56.26	44.91
309	75.10	66.32	4.25	4.25	22.53	22.53
310	151.65	145.15	20.17	14.14	54.12	28.95
311	159.30	137.23	46.90	6.46	56.26	44.91
312	251.01	248.88	27.16	27.16	75.30	75.30
313	159.30	116.35	43.70	6.46	56.26	44.91
314	159.30	116.35	46.90	6.46	56.26	44.91
315	159.30	75.13	20.61	6.46	56.26	44.91
316	159.30	75.13	20.80	6.46	56.26	44.91

Design Ratio

Member ID	P	M_z	M_y	V_y	V_z	(P,M_z,M_y)	Worst LC	KL/r	δ	Status
1	0.077	0.649	0.101	0.019	0.010	0.671	#32	0.477	Not Required	Pass
2	0.001	0.229	0.042	0.054	0.008	0.271	#13	0.054	Not Required	Pass
3	0.001	0.443	0.016	0.043	0.002	0.460	#13	0.046	Not Required	Pass
4	0.001	0.313	0.036	[illegible]	0.007	[illegible]	#13	[illegible]	Not Required	Pass

4	0.001	0.313	0.036	0.032	0.007	0.349	#13	0.122	Not Required	Pass
5	0.001	0.273	0.014	0.044	0.005	0.288	#13	0.076	Not Required	Pass
6	0.001	0.623	0.028	0.064	0.007	0.651	#13	0.046	Not Required	Pass
7	0.002	0.387	0.029	0.062	0.006	0.399	#13	0.076	Not Required	Pass
8	0.003	0.117	0.062	0.025	0.003	0.151	#13	0.102	Not Required	Pass
9	0.002	0.071	0.028	0.003	0.002	0.099	#13	0.206	Not Required	Pass
10	0.001	0.421	0.043	0.042	0.010	0.456	#13	0.082	Not Required	Pass
11	0.002	0.146	0.059	0.039	0.003	0.173	#13	0.102	Not Required	Pass
12	0.002	0.386	0.053	0.076	0.011	0.439	#13	0.054	Not Required	Pass
13	0.002	0.067	0.093	0.052	0.003	0.137	#16	0.306	Not Required	Pass
14	0.004	0.069	0.097	0.034	0.003	0.127	#16	0.204	Not Required	Pass
15	0.000	0.004	0.001	0.007	0.000	0.005	#13	Not Required	Not Required	Pass
16	0.000	0.003	0.001	0.005	0.000	0.004	#13	Not Required	Not Required	Pass
101	0.115	0.821	0.011	0.027	0.001	0.829	#16	0.477	Not Required	Pass
102	0.000	0.480	0.071	0.099	0.013	0.551	#13	0.054	Not Required	Pass
103	0.001	0.780	0.010	0.078	0.002	0.791	#13	0.046	Not Required	Pass
104	0.001	0.597	0.027	0.060	0.005	0.611	#13	0.082	Not Required	Pass
105	0.001	0.483	0.031	0.077	0.008	0.491	#13	0.076	Not Required	Pass
106	0.001	0.776	0.012	0.078	0.005	0.779	#13	0.046	Not Required	Pass
107	0.001	0.482	0.014	0.077	0.004	0.486	#13	0.076	Not Required	Pass
108	0.004	0.052	0.015	0.032	0.002	0.066	#13	0.102	Not Required	Pass
109	0.003	0.071	0.020	0.001	0.001	0.092	#13	0.206	Not Required	Pass
110	0.001	0.566	0.015	0.057	0.003	0.578	#13	0.082	Not Required	Pass
111	0.001	0.065	0.015	0.044	0.002	0.069	#32	0.102	Not Required	Pass
112	0.001	0.462	0.071	0.096	0.014	0.534	#13	0.054	Not Required	Pass
113	0.002	0.217	0.081	0.062	0.003	0.254	#13	0.306	Not Required	Pass
114	0.005	0.217	0.083	0.048	0.003	0.269	#13	0.306	Not Required	Pass
115	0.004	0.410	0.050	0.049	0.002	0.441	#13	0.507	Not Required	Pass
116	0.006	0.291	0.052	0.039	0.002	0.313	#13	0.507	Not Required	Pass
201	0.115	0.821	0.011	0.027	0.001	0.829	#16	0.477	Not Required	Pass
202	0.001	0.462	0.071	0.096	0.014	0.534	#13	0.054	Not Required	Pass
203	0.001	0.776	0.012	0.078	0.005	0.779	#13	0.046	Not Required	Pass
204	0.001	0.566	0.015	0.057	0.003	0.578	#13	0.082	Not Required	Pass
205	0.001	0.482	0.014	0.077	0.004	0.486	#13	0.076	Not Required	Pass
206	0.001	0.780	0.010	0.078	0.002	0.791	#13	0.046	Not Required	Pass
207	0.001	0.483	0.031	0.077	0.008	0.491	#13	0.076	Not Required	Pass
208	0.003	0.048	0.052	0.039	0.002	0.089	#13	0.102	Not Required	Pass
209	0.003	0.071	0.020	0.001	0.001	0.092	#13	0.206	Not Required	Pass
210	0.001	0.597	0.027	0.060	0.005	0.611	#13	0.122	Not Required	Pass
211	0.002	0.098	0.050	0.049	0.002	0.126	#32	0.102	Not Required	Pass
212	0.000	0.480	0.071	0.099	0.013	0.551	#13	0.054	Not Required	Pass
213	0.002	0.217	0.081	0.062	0.003	0.254	#13	0.306	Not Required	Pass
214	0.005	0.217	0.084	0.048	0.003	0.269	#13	0.306	Not Required	Pass
215	0.003	0.270	0.021	0.044	0.002	0.297	#13	0.507	Not Required	Pass
216	0.007	0.134	0.021	0.032	0.002	0.151	#13	0.507	Not Required	Pass
301	0.077	0.649	0.101	0.019	0.010	0.671	#32	0.477	Not Required	Pass
302	0.002	0.386	0.053	0.076	0.011	0.439	#13	0.054	Not Required	Pass
303	0.001	0.623	0.028	0.064	0.007	0.651	#13	0.046	Not Required	Pass
304	0.001	0.421	0.043	0.042	0.010	0.456	#13	0.082	Not Required	Pass
305	0.002	0.387	0.029	0.062	0.006	0.399	#13	0.076	Not Required	Pass
306	0.001	0.443	0.016	0.043	0.002	0.460	#13	0.046	Not Required	Pass
307	0.001	0.273	0.014	0.044	0.005	0.288	#13	0.076	Not Required	Pass
308	0.000	0.003	0.001	0.005	0.000	0.004	#13	Not Required	Not Required	Pass
309	0.002	0.071	0.028	0.003	0.002	0.099	#13	0.206	Not Required	Pass

310	0.001	0.313	0.036	0.032	0.007	0.349	#13	0.122	Not Required	Pass
311	0.000	0.004	0.001	0.007	0.000	0.005	#13	Not Required	Not Required	Pass
312	0.001	0.229	0.042	0.054	0.008	0.271	#13	0.054	Not Required	Pass
313	0.002	0.067	0.093	0.052	0.003	0.137	#16	0.204	Not Required	Pass
314	0.004	0.069	0.097	0.034	0.003	0.127	#16	0.306	Not Required	Pass
315	0.004	0.441	0.059	0.039	0.003	0.468	#13	0.507	Not Required	Pass
316	0.006	0.315	0.062	0.028	0.003	0.344	#13	0.507	Not Required	Pass

Definitions

Φ_t	Safety factor for tensile
Φ_c	Safety factor for compression
Φ_b	Safety factor for flexure
Φ_v	Safety factor for shear
E	Modulus of elasticity
F_y	Specified minimum yield stress
F_u	Specified minimum tensile strength
A	Cross-sectional area
J	Torsional constant
I_{yp}	Moment of inertia about the Y axes
I_{zp}	Moment of inertia about the Z axes
I_w	Warping constant
S_{yp}	Plastic section modulus about the Y axis
S_{zp}	Plastic section modulus about the Z axis
KL	Effective length
C_b	Buckling modification factor (from all load combinations)
L_b	Length between braced points
LST	Limited slenderness for tension
LSC	Limited slenderness for compression
LD	Limited deflection
P_n	Nominal axial strength (tension/comression)
M_n	Nominal flexural strength (about Z/Y axis)
V_n	Nominal shear strength (along Z/Y axis)
P	Design ratio in case of axial force
M_z	Design ratio in case of bending about Z axis
M_y	Design ratio in case of bending about Y axis
V_y	Design ratio in case of shear along Y axis
V_z	Design ratio in case of shear along Z axis
(P,M_z,M_y)	Design ratio in case of axial force and bending action
KL/r	Design ratio in case of section slenderness
δ	Design ratio in case of member deflection
OK	Capacity is provided
NG	Capacity is not provided

REFERENCES	CALCULATIONS	RESULTS

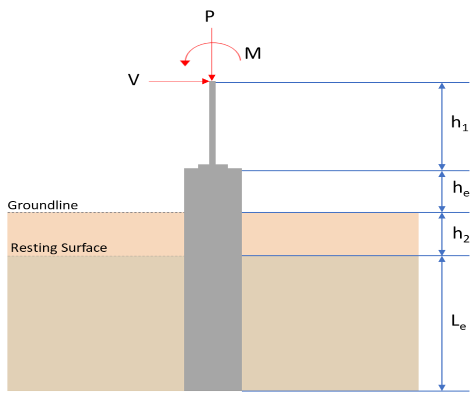
SkyCiv Foundation Design

Pile Foundation

Design Information :

Design code : IBC 2021 (International Building Code)
Unit System : Imperial

Pile Input



Geometry

Pile shape: rectangular
$b = 48\ \text{in}$ - Pile width
$D = 48\ \text{in}$ - Pile depth
$L = 5.75\ \text{ft}$ - Total pile length
$h_1 = 0\ \text{ft}$ - Lateral load height from the top of the pile,
$h_2 = 0\ \text{ft}$ - Depth to resisting surface
$h_e = 0\ \text{ft}$ - Length of pile above the ground

Tabulation of Soil Parameters

Layer	Label	Allowable Bearing Pressure (q_a) (psf)	Allowable Lateral Pressure (R) (psf/ft)
1	Sand, silty sand, clayey sand, silty gravel & clayey gravel	2000.000	150.000

Tabulation of Loads

Load Component	ASD	LRFD
P (kip)	6.504	9.985
V_x (kip)	-0.859	-1.448
V_z (kip)	0.456	0.716
M_x (kipft)	1.146	1.801
M_z (kipft)	16.273	27.460

Material Properties

$f'_{ck} = 3\ \text{ksi}$ - Concrete strength,

Required depth to resist lateral loads (ASD)

H - Point of application of the lateral load

$$H = h_1 + h_2 + h_e$$

$$H = (0\ \text{ft}) + (0\ \text{ft}) + (0\ \text{ft})$$

$$H = 0\ \text{ft}$$

Considering x-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\ D}$$

$$H_o = \frac{(-0.859\ \text{kip})}{1.57 \times (48\ \text{in})}$$

$$H_o = -0.13678\ \text{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\ H)}{1.57\ D}$$

$$M_o = \frac{(16.273\ \text{kipft}) + ((-0.859\ \text{kip}) \times (0\ \text{ft}))}{1.57 \times (48\ \text{in})}$$

$$M_o = 2.5912\ \text{kipft/ft}$$

Required depth of embedment in earth:

$$L_x^3 - \left(14.14 \times \frac{H_o \times L_x}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,x} = 5.4571\ \text{ft}$ - Required depth in x-direction,

Considering z-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\,b}$$

$$H_o = \frac{(0.456\ \text{kip})}{1.57 \times (48\ \text{in})}$$

$$H_o = 0.072611\ \text{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\,H)}{1.57\,b}$$

$$M_o = \frac{(1.146\ \text{kipft}) + ((0.456\ \text{kip}) \times (0\ \text{ft}))}{1.57 \times (48\ \text{in})}$$

$$M_o = 0.18248\ \text{kipft/ft}$$

Required depth of embedment in earth:

$$L_z^3 - \left(14.14 \times \frac{H_o \times L_z}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,z} = 3.0292\ \text{ft}$ - Required depth in z-direction,

Minimum embedded depth required:

$L_{e,req}$ - Depth of pile required,

$$L_{e,req} = MAX[L_{e,x}, L_{e,z}]$$

$$L_{e,req} = MAX[(5.4571\ \text{ft}), (3.0292\ \text{ft})]$$

$$L_{e,req} = 5.457\ \text{ft}$$

L_e - Actual embedded length of pile,

$$L_e = L - h_e - h_2$$

$$L_e = (5.75\ \text{ft}) - (0\ \text{ft}) - (0\ \text{ft})$$

$$L_e = 5.75\ \text{ft}$$

$Ratio$ - Embedded depth

$$Ratio = \frac{L_{e,req}}{L_e}$$

$$Ratio = \frac{(5.457\ \text{ft})}{(5.75\ \text{ft})}$$

$$Ratio = 0.94904$$

Status: **PASS**
Ratio: **0.950**

End-bearing Capacity (ASD)

A - Pile cross-section area

$$A = b\,D$$

$$A = (48\ \text{in}) \times (48\ \text{in})$$

$$A = 16\ \text{ft}^2$$

q - End-bearing pressure

$$q = \frac{P_v}{A}$$

$$q = \frac{(6.504\ \text{kip})}{(16\ \text{ft}^2)}$$

$$q = 0.4065\ \text{kip/ft}^2$$

$q = 0.4065 \text{ kip/ft}^2$

Check bearing capacity ratio:

$Ratio$ - Capacity

$$Ratio = \frac{q}{q_a}$$

$$Ratio = \frac{(0.4065 \text{ kip/ft}^2)}{(2000 \text{ psf})}$$

$$Ratio = 0.20325$$

Status: **PASS**
Ratio: **0.200**

Czerniak

Lateral Soil Pressure (ASD):

L/D - Length to least lateral dimension ratio,

$$L/D = \frac{L}{D}$$

$$L/D = \frac{(5.75 \text{ ft})}{(48 \text{ in})}$$

$$L/D = 1.4375$$

Since L/D $\leq$ 10,

Pile is short.

Considering x-direction:

$H_o = -0.13678 \text{ kip/ft}$ - Lateral force per length of pile,
$M_o = 2.5912 \text{ kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\, M_o\, L_e) + (3\, H_o\, L_e^2)}{(6\, M_o) + (4\, H_o\, L_e)}$$

$$a = \frac{(4 \times (2.5912 \text{ kipft/ft}) \times (5.75 \text{ ft})) + \left(3 \times (-0.13678 \text{ kip/ft}) \times (5.75 \text{ ft})^2\right)}{(6 \times (2.5912 \text{ kipft/ft})) + (4 \times (-0.13678 \text{ kip/ft}) \times (5.75 \text{ ft}))}$$

$$a = 3.914 \text{ ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\, [(4\, M_o) + (3\, H_o\, L_e)]^2}{L_e^2\, [(3\, M_o) + (2\, H_o\, L_e)]}$$

$$p = \frac{0.75 \times [(4 \times (2.5912 \text{ kipft/ft})) + (3 \times (-0.13678 \text{ kip/ft}) \times (5.75 \text{ ft}))]^2}{(5.75 \text{ ft})^2 \times [(3 \times (2.5912 \text{ kipft/ft})) + (2 \times (-0.13678 \text{ kip/ft}) \times (5.75 \text{ ft}))]}$$

$$p = 0.23445 \text{ kip/ft}^2$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\, [(2\, M_o) + (H_o\, L_e)]}{L_e^2}$$

$$s = \frac{6 \times [(2 \times (2.5912 \text{ kipft/ft})) + ((-0.13678 \text{ kip/ft}) \times (5.75 \text{ ft}))]}{(5.75 \text{ ft})^2}$$

$$s = 0.79776 \text{ kip/ft}^2$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\, \frac{a}{2}$$

$$p_a = (150 \text{ psf/ft}) \times \frac{(3.914 \text{ ft})}{2}$$

$$p_a = 0.29355 \text{ kip/ft}^2$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{(0.23445 \text{ kip/ft}^2)}{(0.29355 \text{ kip/ft}^2)}$$

$$Ratio = 0.79869$$

Status: **PASS**
Ratio: **0.800**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\,L_e$$

$$p_s = (150\ \mathrm{psf/ft}) \times (5.75\ \mathrm{ft})$$

$$p_s = 0.8625\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{\left(0.79776\ \mathrm{kip/ft^2}\right)}{\left(0.8625\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = 0.92494$$

Status: **PASS**
Ratio: **0.920**

Considering z-direction:

$H_o = 0.072611\ \mathrm{kip/ft}$ - Lateral force per length of pile,
$M_o = 0.18248\ \mathrm{kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (0.18248\ \mathrm{kipft/ft}) \times (5.75\ \mathrm{ft})) + \left(3 \times (0.072611\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft})^2\right)}{(6 \times (0.18248\ \mathrm{kipft/ft})) + (4 \times (0.072611\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))}$$

$$a = 4.1228\ \mathrm{ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\,[(4\,M_o) + (3\,H_o\,L_e)]^2}{L_e^2\,[(3\,M_o) + (2\,H_o\,L_e)]}$$

$$p = \frac{0.75 \times [(4 \times (0.18248\ \mathrm{kipft/ft})) + (3 \times (0.072611\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))]^{\ 2}}{(5.75\ \mathrm{ft})^2 \times [(3 \times (0.18248\ \mathrm{kipft/ft})) + (2 \times (0.072611\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))]}$$

$$p = 0.064489\ \mathrm{kip/ft^2}$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\,[(2\,M_o) + (H_o\,L_e)]}{L_e^2}$$

$$s = \frac{6 \times [(2 \times (0.18248\ \mathrm{kipft/ft})) + ((0.072611\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))]}{(5.75\ \mathrm{ft})^2}$$

$$s = 0.142\ \mathrm{kip/ft^2}$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\,\frac{a}{2}$$

$$p_a = (150\ \mathrm{psf/ft}) \times \frac{(4.1228\ \mathrm{ft})}{2}$$

$$p_a = 0.30921\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{\left(0.064489\ \mathrm{kip/ft^2}\right)}{\left(0.30921\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = 0.20856$$

Status: **PASS**
Ratio: **0.210**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\,L_e$$

$$p_s = (150\ \mathrm{psf/ft}) \times (5.75\ \mathrm{ft})$$

$$p_s = 0.8625\ \mathrm{kip/ft^2}$$

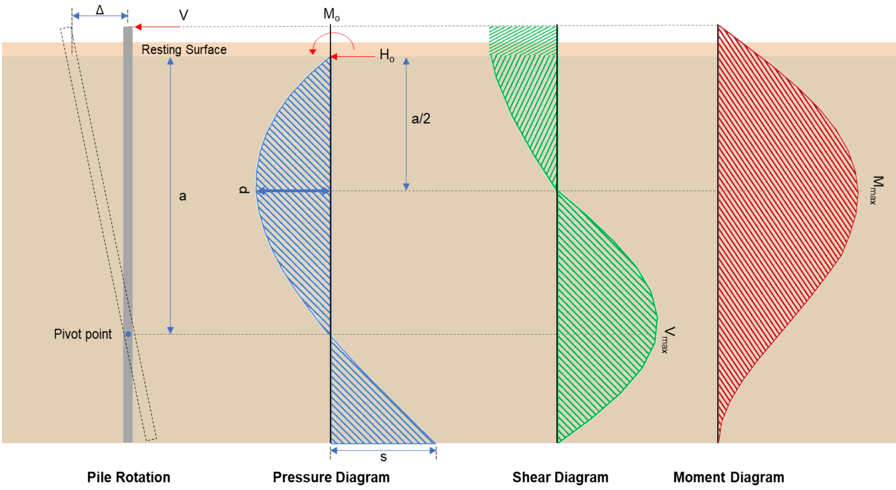
$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{(0.142\ \mathrm{kip/ft^2})}{(0.8625\ \mathrm{kip/ft^2})}$$

$$Ratio = 0.16464$$

Status: **PASS**
Ratio: **0.160**



Shear force and Bending moment (x-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\ D}$$

$$H_o = \frac{(-1.448\ \mathrm{kip})}{1.57 \times (48\ \mathrm{in})}$$

$$H_o = -0.23057\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\ H)}{1.57\ D}$$

$$M_o = \frac{(27.46\ \mathrm{kipft}) + ((-1.448\ \mathrm{kip}) \times (0\ \mathrm{ft}))}{1.57 \times (48\ \mathrm{in})}$$

$$M_o = 4.3726\ \mathrm{kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(4.3726\ \mathrm{kipft/ft})}{(-0.23057\ \mathrm{kip/ft})}$$

$$E = 18.964\ \mathrm{ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\ M_o\ L_e) + \left(3\ H_o\ L_e^2\right)}{(6\ M_o) + (4\ H_o\ L_e)}$$

$$a = \frac{(4 \times (4.3726\ \mathrm{kipft/ft}) \times (5.75\ \mathrm{ft})) + \left(3 \times (-0.23057\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft})^2\right)}{(6 \times (4.3726\ \mathrm{kipft/ft})) + (4 \times (-0.23057\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))}$$

$$a = 3.9139\ \mathrm{ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\ D) \left[1 - \left[3 \left(\frac{4\ E}{L_e} + 3\right) \left(\frac{a}{L_e}\right)^2\right] + \left[4 \left(\frac{3\ E}{L_e} + 2\right) \left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((-0.23057\ \mathrm{kip/ft}) \times (48\ \mathrm{in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (18.964\ \mathrm{ft})}{(5.75\ \mathrm{ft})} + 3\right) \times \left(\frac{(3.9139\ \mathrm{ft})}{(5.75\ \mathrm{ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (18.964\ \mathrm{ft})}{(5.75\ \mathrm{ft})} + 2\right) \times \left(\frac{(3.9139\ \mathrm{ft})}{(5.75\ \mathrm{ft})}\right)^3\right]\right]$$

$$V_{max} = 5.9971\ \mathrm{kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, D\, L_e) \left[\left(\frac{E}{L_e} + \frac{a}{2\, L_e} \right) - \left[\left(\frac{4\, E}{L_e} + 3 \right) \left(\frac{a}{2\, L_e} \right)^3 \right] + \left[\left(\frac{3\, E}{L_e} + 2 \right) \left(\frac{a}{2\, L_e} \right)^4 \right] \right]$$

$$M_{max} = ((-0.23057 \text{ kip/ft}) \times (48 \text{ in}) \times (5.75 \text{ ft})) \times \left[\left(\frac{(18.964 \text{ ft})}{(5.75 \text{ ft})} + \frac{(3.9139 \text{ ft})}{2 \times (5.75 \text{ ft})} \right) - \left[\left(\frac{4 \times (18.964 \text{ ft})}{(5.75 \text{ ft})} + 3 \right) \times \left(\frac{(3.9139 \text{ ft})}{2 \times (5.75 \text{ ft})} \right)^3 \right] + \left[\left(\frac{3 \times (18.964 \text{ ft})}{(5.75 \text{ ft})} + 2 \right) \times \left(\frac{(3.9139 \text{ ft})}{2 \times (5.75 \text{ ft})} \right)^4 \right] \right]$$

$$M_{max} = 16.756 \text{ kipft}$$

Shear force and Bending moment (z-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\, b}$$

$$H_o = \frac{(0.716 \text{ kip})}{1.57 \times (48 \text{ in})}$$

$$H_o = 0.11401 \text{ kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\, H)}{1.57\, b}$$

$$M_o = \frac{(1.801 \text{ kipft}) + ((0.716 \text{ kip}) \times (0 \text{ ft}))}{1.57 \times (48 \text{ in})}$$

$$M_o = 0.28678 \text{ kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(0.28678 \text{ kipft/ft})}{(0.11401 \text{ kip/ft})}$$

$$E = 2.5154 \text{ ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\, M_o\, L_e) + \left(3\, H_o\, L_e^2\right)}{(6\, M_o) + (4\, H_o\, L_e)}$$

$$a = \frac{(4 \times (0.28678 \text{ kipft/ft}) \times (5.75 \text{ ft})) + \left(3 \times (0.11401 \text{ kip/ft}) \times (5.75 \text{ ft})^2\right)}{(6 \times (0.28678 \text{ kipft/ft})) + (4 \times (0.11401 \text{ kip/ft}) \times (5.75 \text{ ft}))}$$

$$a = 4.1227 \text{ ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\, b) \left[1 - \left[3 \left(\frac{4\, E}{L_e} + 3 \right) \left(\frac{a}{L_e} \right)^2 \right] + \left[4 \left(\frac{3\, E}{L_e} + 2 \right) \left(\frac{a}{L_e} \right)^3 \right] \right]$$

$$V_{max} = ((0.11401 \text{ kip/ft}) \times (48 \text{ in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (2.5154 \text{ ft})}{(5.75 \text{ ft})} + 3 \right) \times \left(\frac{(4.1227 \text{ ft})}{(5.75 \text{ ft})} \right)^2 \right] + \left[4 \times \left(\frac{3 \times (2.5154 \text{ ft})}{(5.75 \text{ ft})} + 2 \right) \times \left(\frac{(4.1227 \text{ ft})}{(5.75 \text{ ft})} \right)^3 \right] \right]$$

$$V_{max} = 0.6575 \text{ kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, b\, L_e) \left[\left(\frac{E}{L_e} + \frac{a}{2\, L_e} \right) - \left[\left(\frac{4\, E}{L_e} + 3 \right) \left(\frac{a}{2\, L_e} \right)^3 \right] + \left[\left(\frac{3\, E}{L_e} + 2 \right) \left(\frac{a}{2\, L_e} \right)^4 \right] \right]$$

$$M_{max} = ((0.11401 \text{ kip/ft}) \times (48 \text{ in}) \times (5.75 \text{ ft})) \times \left[\left(\frac{(2.5154 \text{ ft})}{(5.75 \text{ ft})} + \frac{(4.1227 \text{ ft})}{2 \times (5.75 \text{ ft})} \right) - \left[\left(\frac{4 \times (2.5154 \text{ ft})}{(5.75 \text{ ft})} + 3 \right) \times \left(\frac{(4.1227 \text{ ft})}{2 \times (5.75 \text{ ft})} \right)^3 \right] + \left[\left(\frac{3 \times (2.5154 \text{ ft})}{(5.75 \text{ ft})} + 2 \right) \times \left(\frac{(4.1227 \text{ ft})}{2 \times (5.75 \text{ ft})} \right)^4 \right] \right]$$

$$M_{max} = 1.6568 \text{ kipft}$$

Minimum Reinforcement Check (LRFD)

Parameters:

$f'_{ck} = 3 \text{ ksi}$ - Concrete strength,
$f_{yk} = 60 \text{ ksi}$ - Longitudinal reinforcement strength,
$\phi = 0.65$ - Reduction factor for axial strength,
Table 22.4.2.1 $\alpha = 0.8$ - Alpha factor for axial strength,
$A_g = 2304 \text{ in}^2$ - Gross area of concrete,

Longitudinal reinforcement:

Required reinforcement due to axial load, $A_{st,required}$
22.4.2.2, 10.6.1.1 $A_{st,required}$

$$A_{st,required} = Min\left[\frac{\frac{P}{\phi\ \alpha} - (0.85\ f'_{ck}\ A_g)}{f_{yk} - (0.85\ f'_{ck})}, (0.08\ A_g)\right]$$

$$A_{st,required} = Min\left[\frac{\frac{(9.985 \text{ kip})}{(0.65)\times(0.8)} - \left(0.85 \times (3 \text{ ksi}) \times \left(2304 \text{ in}^2\right)\right)}{(60 \text{ ksi}) - (0.85 \times (3 \text{ ksi}))}, \left(0.08 \times \left(2304 \text{ in}^2\right)\right)\right]$$

$$A_{st,required} = -101.93 \text{ in}^2$$

A_{min} - Governing minimum reinforcement area,

$$A_{min} = Max\left[A_{st,required}, (0.0018\ A_g)\right]$$

$$A_{min} = Max\left[\left(-101.93 \text{ in}^2\right), \left(0.0018 \times \left(2304 \text{ in}^2\right)\right)\right]$$

$$A_{min} = 4.1472 \text{ in}^2$$

n_{rebar} - Required number of reinforcement,

$$n_{rebar} = \frac{A_{min}}{A_{rebar}}$$

$$n_{rebar} = \frac{\left(4.1472 \text{ in}^2\right)}{\left(0.3068 \text{ in}^2\right)}$$

$$n_{rebar} = 14$$

A_{st} - Actual total reinforcement area,

$$A_{st} = n_{rebar}\ \frac{\pi\ d_{bar}^2}{4}$$

$$A_{st} = (14) \times \frac{\pi \times (0.625 \text{ in})^2}{4}$$

$$A_{st} = 4.2951 \text{ in}^2$$

$Ratio$ - Capacity

$$Ratio = \frac{A_{min}}{A_{st}}$$

$$Ratio = \frac{\left(4.1472 \text{ in}^2\right)}{\left(4.2951 \text{ in}^2\right)}$$

$$Ratio = 0.96556$$

Status: **PASS**
Ratio: **0.970**

25.2.3 s_{rebar} - Minimum spacing of reinforcement,

$$s_{rebar} = Max\left[1.5, (1.5\ d_{bar})\right]$$

$$s_{rebar} = Max\left[1.5, (1.5 \times (0.625 \text{ in}))\right]$$

$$s_{rebar} = 1.5 \text{ in}$$

Ties:

25.7.2.2 Since longitudinal reinforcement is $\leq$ No. 10ø: Use #3(0.375 in)
25.7.2.1 s_{ties} - Maximum spacing of ties,

$$s_{ties} = Min\left[(16\ d_{bar}), (48\ d_{ties}), Min\,(D, b)\right]$$

$$s_{ties} = Min\left[(16 \times (0.625 \text{ in})), (48 \times (0.375 \text{ in})), Min\,((48 \text{ in}), (48 \text{ in}))\right]$$

$$s_{ties} = 10 \text{ in}$$

Summary:

Main reinforcement: **14 - #5 (0.625 in)**

Ties: **#3(0.375 in) - 10 in**

Axial Compression Strength (ACI 318-19, LRFD)

22.4.2.2 ϕP_N - Allowable axial compressive strength

$$\phi P_N = \phi\, 0.80\, \left[\left(0.85\, f'_{ck}\, [A_g - A_{st}]\right) + (f_{yk}\, A_{st})\right]$$

$$\phi P_N = (0.65) \times 0.80 \times \left[\left(0.85 \times (3\ \mathrm{ksi}) \times \left[\left(2304\ \mathrm{in}^2\right) - \left(4.2951\ \mathrm{in}^2\right)\right]\right) + \left((60\ \mathrm{ksi}) \times \left(4.2951\ \mathrm{in}^2\right)\right)\right]$$

$$\phi P_N = 3183.4\ \mathrm{kip}$$

$Ratio$ - Capacity

$$Ratio = \frac{P}{\phi P_N}$$

$$Ratio = \frac{(9.985\ \mathrm{kip})}{(3183.4\ \mathrm{kip})}$$

$$Ratio = 0.0031366$$

Status: **PASS**
Ratio: **0.000**

Shear Strength (ACI 318-19, LRFD)

Parameters:

$b_w = 48\ \mathrm{in}$ - Effective width,

22.5.2.2 d - Effective depth

$$d = 0.80\, D$$

$$d = 0.80 \times (48\ \mathrm{in})$$

$$d = 38.4\ \mathrm{in}$$

22.5.5.1.3 λ_s - size effect modification factor

$$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{d}{10}}}, 1\right]$$

$$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{(38.4\ \mathrm{in})}{10}}}, 1\right]$$

$$\lambda_s = 0.64282$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 3\ \mathrm{ksi} \rightarrow 3000\ \mathrm{psi}$,

22.5.5.1.1 $V_{c,max}$ - Max shear strength of concrete

$$V_{c,max} = 5\, \lambda_s\, \sqrt{f'_{ck}}\, b_w\, d$$

$$V_{c,max} = 5 \times (0.64282) \times \sqrt{(3000\ \mathrm{psi})} \times (48\ \mathrm{in}) \times (38.4\ \mathrm{in})$$

$$V_{c,max} = 324.49\ \mathrm{kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 3\ \mathrm{ksi} \rightarrow 3000\ \mathrm{psi}$, $P = 9.985\ \mathrm{kip} \rightarrow 9985\ \mathrm{lbf}$,

22.5.5.1.1(a) $V_{c,a}$ - Shear strength of concrete (a)

$$V_{c,a} = \left[2\, \lambda_s\, \sqrt{f'_{ck}} + \frac{P}{6\, A_g}\right]\, b_w\, d$$

$$V_{c,a} = \left[2 \times (0.64282) \times \sqrt{(3000\ \mathrm{psi})} + \frac{(9985\ \mathrm{lbf})}{6 \times (2304\ \mathrm{in}^2)}\right] \times (48\ \mathrm{in}) \times (38.4\ \mathrm{in})$$

$$V_{c,a} = 131.13\ \mathrm{kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 3\ \mathrm{ksi} \rightarrow 3000\ \mathrm{psi}$,

22.5.5.1.2 $V_{c,b}$ - Shear strength of concrete (b)

$$V_{c,b} = \left[2\, \lambda_s\, \sqrt{f'_{ck}} + \left(0.05\, f'_{ck}\right)\right]\, b_w\, d$$

$$V_{c,b} = \left[2 \times (0.64282) \times \sqrt{(3000\ \mathrm{psi})} + (0.05 \times (3000\ \mathrm{psi}))\right] \times (48\ \mathrm{in}) \times (38.4\ \mathrm{in})$$

$$V_{c,b} = 406.27\ \mathrm{kip}$$

V_c - Governing shear strength of concrete

$$V_c = Min\left[V_{c,max}, V_{c,a}, V_{c,b}\right]$$

$$V_c = Min\left[(324.49\ \mathrm{kip}), (131.13\ \mathrm{kip}), (406.27\ \mathrm{kip})\right]$$

$$V_c = 131.13\ \mathrm{kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 3\ \mathrm{ksi} \rightarrow 3000\ \mathrm{psi}$,

22.5.1.2 $V_{s,a}$ - Shear strength of steel (a)

$$V_{s,a} = 8\sqrt{f'_{ck}}\, b_w\, d$$

$$V_{s,a} = 8 \times \sqrt{(3000\ \mathrm{psi})} \times (48\ \mathrm{in}) \times (38.4\ \mathrm{in})$$

$$V_{s,a} = 807.65\ \mathrm{kip}$$

A_v - Ties rebar area,

$$A_v = \frac{\pi\, d_{ties}^2}{4}$$

$$A_v = \frac{\pi \times (0.375\ \mathrm{in})^2}{4}$$

$$A_v = 0.11045\ \mathrm{in}^2$$

22.5.8.5.3 $V_{s,b}$ - Shear strength of steel (b)

$$V_{s,b} = \frac{2\, A_v\, f_{ywk}\, d}{s_{ties}}$$

$$V_{s,b} = \frac{2 \times \left(0.11045\ \mathrm{in}^2\right) \times (60\ \mathrm{ksi}) \times (38.4\ \mathrm{in})}{(10\ \mathrm{in})}$$

$$V_{s,b} = 50.894\ \mathrm{kip}$$

V_s - Governing shear strength of steel

$$V_s = MIN\left[V_{s,a}, V_{s,b}\right]$$

$$V_s = MIN\left[(807.65\ \mathrm{kip}), (50.894\ \mathrm{kip})\right]$$

$$V_s = 50.894\ \mathrm{kip}$$

22.5.1.1 ϕV_n - Allowable shear strength

$$\phi V_n = \phi\ (V_c + V_s)$$

$$\phi V_n = (0.65) \times ((131.13\ \mathrm{kip}) + (50.894\ \mathrm{kip}))$$

$$\phi V_n = 118.31\ \mathrm{kip}$$

Considering x-direction:

$V_{max} = 5.9971\ \mathrm{kip}$ - Maximum shear force in the x-direction,
$Ratio$ - Capacity

$$Ratio = \frac{V_{max}}{\phi V_n}$$

$$Ratio = \frac{(5.9971\ \mathrm{kip})}{(118.31\ \mathrm{kip})}$$

$$Ratio = 0.050688$$

Status: **PASS**
Ratio: **0.050**

Considering z-direction:

$V_{max} = 0.6575\ \mathrm{kip}$ - Maximum shear force in the z-direction,
$Ratio$ - Capacity

$$Ratio = \frac{V_{max}}{\phi V_n}$$

$$Ratio = \frac{(0.6575\ \mathrm{kip})}{(118.31\ \mathrm{kip})}$$

$$Ratio = 0.0055573$$

Status: **PASS**
Ratio: **0.010**

Flexural Strength (ACI 318-19, LRFD)

S_m - Section modulus

$$S_m = \frac{b\, D^2}{6}$$

$$S_m = \frac{(48\ \mathrm{in}) \times (48\ \mathrm{in})^2}{6}$$

$$S_m = 18432\ \mathrm{in}^3$$

	$\lambda = 1$ - Concrete modification factor (Normal concrete), Allowable flexural strength: M_n shall be the lesser of: $\phi M_{n,1}$ $$\phi M_{n,1} = \phi \times 5 \times \lambda \times \sqrt{(f_c')} \times S_m$$ $$\phi M_{n,1} = 0.65 \times 5 \times 1 \times \sqrt{(3ksi)} \times 18432.001in^3$$ $$\phi M_{n,1} = 273.423kipft$$	
14.5.2.1b	$\phi M_{n,2}$ $$\phi M_{n,2} = \phi\, 0.85\, f_{ck}'\, S_m$$ $$\phi M_{n,2} = (0.65) \times 0.85 \times (3 \text{ ksi}) \times \left(18432 \text{ in}^3\right)$$ $$\phi M_{n,2} = 2545.9 \text{ kipft}$$ Therefore, ϕM_n - Allowable flexural strength, $$\phi M_n = MIN[\phi M_{n,1}, \phi M_{n,2}]$$ $$\phi M_n = MIN[(273.42 \text{ kipft}), (2545.9 \text{ kipft})]$$ $$\phi M_n = 273.42 \text{ kipft}$$ **Considering x-direction:** $M_{max} = 16.756 \text{ kipft}$ - Maximum moment in the x-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\phi M_n}$$ $$Ratio = \frac{(16.756 \text{ kipft})}{(273.42 \text{ kipft})}$$ $$Ratio = 0.061284$$	Status: **PASS** Ratio: **0.060**
	Considering z-direction: $M_{max} = 1.6568 \text{ kipft}$ - Maximum moment in the z-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\phi M_n}$$ $$Ratio = \frac{(1.6568 \text{ kipft})}{(273.42 \text{ kipft})}$$ $$Ratio = 0.0060595$$	Status: **PASS** Ratio: **0.010**

REFERENCES	CALCULATIONS	RESULTS

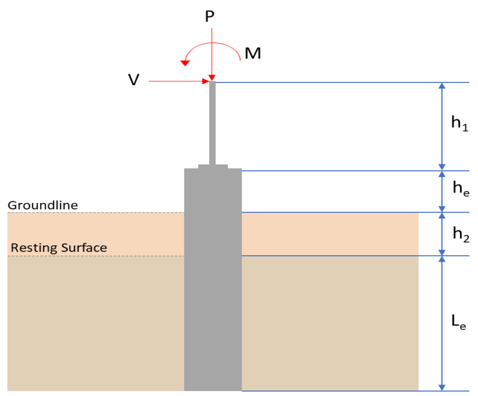
SkyCiv Foundation Design

Pile Foundation

Design Information :

Design code : IBC 2021 (International Building Code)
Unit System : Imperial

Pile Input



Geometry

Pile shape: rectangular
$b = 48\ \mathrm{in}$ - Pile width
$D = 48\ \mathrm{in}$ - Pile depth
$L = 5.75\ \mathrm{ft}$ - Total pile length
$h_1 = 0\ \mathrm{ft}$ - Lateral load height from the top of the pile,
$h_2 = 0\ \mathrm{ft}$ - Depth to resisting surface
$h_e = 0\ \mathrm{ft}$ - Length of pile above the ground

Tabulation of Soil Parameters

Layer	Label	Allowable Bearing Pressure (q_a) (psf)	Allowable Lateral Pressure (R) (psf/ft)
1	Sand, silty sand, clayey sand, silty gravel & clayey gravel	2000.000	150.000

Tabulation of Loads

Load Component	ASD	LRFD
P (kip)	6.504	9.985
V_x (kip)	-0.859	-1.448
V_z (kip)	-0.456	-0.716
M_x (kipft)	-1.146	-1.801
M_z (kipft)	16.273	27.460

Material Properties

$f'_{ck} = 3\ \mathrm{ksi}$ - Concrete strength,

Required depth to resist lateral loads (ASD)

H - Point of application of the lateral load

$$H = h_1 + h_2 + h_e$$

$$H = (0\ \mathrm{ft}) + (0\ \mathrm{ft}) + (0\ \mathrm{ft})$$

$$H = 0\ \mathrm{ft}$$

Considering x-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\,D}$$

$$H_o = \frac{(-0.859\ \mathrm{kip})}{1.57 \times (48\ \mathrm{in})}$$

$$H_o = -0.13678\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\,H)}{1.57\,D}$$

$$M_o = \frac{(16.273\ \text{kipft}) + ((-0.859\ \text{kip}) \times (0\ \text{ft}))}{1.57 \times (48\ \text{in})}$$

$$M_o = 2.5912\ \text{kipft/ft}$$

Required depth of embedment in earth:

$$L_x^3 - \left(14.14 \times \frac{H_o \times L_x}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,x} = 5.4571\ \text{ft}$ - Required depth in x-direction,

Considering z-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\,b}$$

$$H_o = \frac{(-0.456\ \text{kip})}{1.57 \times (48\ \text{in})}$$

$$H_o = -0.072611\ \text{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\,H)}{1.57\,b}$$

$$M_o = \frac{(1.146\ \text{kipft}) + ((-0.456\ \text{kip}) \times (0\ \text{ft}))}{1.57 \times (48\ \text{in})}$$

$$M_o = 0.18248\ \text{kipft/ft}$$

Required depth of embedment in earth:

$$L_z^3 - \left(14.14 \times \frac{H_o \times L_z}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,z} = 1.8641\ \text{ft}$ - Required depth in z-direction,

Minimum embedded depth required:

$L_{e,req}$ - Depth of pile required,

$$L_{e,req} = MAX[L_{e,x}, L_{e,z}]$$

$$L_{e,req} = MAX[(5.4571\ \text{ft}), (1.8641\ \text{ft})]$$

$$L_{e,req} = 5.457\ \text{ft}$$

L_e - Actual embedded length of pile,

$$L_e = L - h_e - h_2$$

$$L_e = (5.75\ \text{ft}) - (0\ \text{ft}) - (0\ \text{ft})$$

$$L_e = 5.75\ \text{ft}$$

$Ratio$ - Embedded depth

$$Ratio = \frac{L_{e,req}}{L_e}$$

$$Ratio = \frac{(5.457\ \text{ft})}{(5.75\ \text{ft})}$$

$$Ratio = 0.94904$$

Status: **PASS**
Ratio: **0.950**

End-bearing Capacity (ASD)

A - Pile cross-section area

$$A = b\,D$$

$$A = (48\ \text{in}) \times (48\ \text{in})$$

$$A = 16\ \text{ft}^2$$

q - End-bearing pressure

$$q = \frac{P_v}{A}$$

$$q = \frac{(6.504\ \text{kip})}{(16\ \text{ft}^2)}$$

$$q = 0.4065\ \text{kip/ft}^2$$

$q = 0.4065\ \text{kip/ft}^2$

Check bearing capacity ratio:

$Ratio$ - Capacity

$$Ratio = \frac{q}{q_a}$$

$$Ratio = \frac{\left(0.4065\ \text{kip/ft}^2\right)}{(2000\ \text{psf})}$$

$$Ratio = 0.20325$$

Status: **PASS**
Ratio: **0.200**

Czerniak

Lateral Soil Pressure (ASD):

L/D - Length to least lateral dimension ratio,

$$L/D = \frac{L}{D}$$

$$L/D = \frac{(5.75\ \text{ft})}{(48\ \text{in})}$$

$$L/D = 1.4375$$

Since L/D $\leq$ 10,

Pile is short.

Considering x-direction:

$H_o = -0.13678\ \text{kip/ft}$ - Lateral force per length of pile,
$M_o = 2.5912\ \text{kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (2.5912\ \text{kipft/ft}) \times (5.75\ \text{ft})) + \left(3 \times (-0.13678\ \text{kip/ft}) \times (5.75\ \text{ft})^2\right)}{(6 \times (2.5912\ \text{kipft/ft})) + (4 \times (-0.13678\ \text{kip/ft}) \times (5.75\ \text{ft}))}$$

$$a = 3.914\ \text{ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\,[(4\,M_o) + (3\,H_o\,L_e)]^2}{L_e^2\,[(3\,M_o) + (2\,H_o\,L_e)]}$$

$$p = \frac{0.75 \times [(4 \times (2.5912\ \text{kipft/ft})) + (3 \times (-0.13678\ \text{kip/ft}) \times (5.75\ \text{ft}))]^{2}}{(5.75\ \text{ft})^2 \times [(3 \times (2.5912\ \text{kipft/ft})) + (2 \times (-0.13678\ \text{kip/ft}) \times (5.75\ \text{ft}))]}$$

$$p = 0.23445\ \text{kip/ft}^2$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\,[(2\,M_o) + (H_o\,L_e)]}{L_e^2}$$

$$s = \frac{6 \times [(2 \times (2.5912\ \text{kipft/ft})) + ((-0.13678\ \text{kip/ft}) \times (5.75\ \text{ft}))]}{(5.75\ \text{ft})^2}$$

$$s = 0.79776\ \text{kip/ft}^2$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\,\frac{a}{2}$$

$$p_a = (150\ \text{psf/ft}) \times \frac{(3.914\ \text{ft})}{2}$$

$$p_a = 0.29355\ \text{kip/ft}^2$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{\left(0.23445\ \text{kip/ft}^2\right)}{\left(0.29355\ \text{kip/ft}^2\right)}$$

$$Ratio = 0.79869$$

Status: **PASS**
Ratio: **0.800**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\,L_e$$

$$p_s = (150\ \mathrm{psf/ft}) \times (5.75\ \mathrm{ft})$$

$$p_s = 0.8625\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{(0.79776\ \mathrm{kip/ft^2})}{(0.8625\ \mathrm{kip/ft^2})}$$

$$Ratio = 0.92494$$

Status: PASS
Ratio: 0.920

Considering z-direction:

$H_o = -0.072611\ \mathrm{kip/ft}$ - Lateral force per length of pile,
$M_o = 0.18248\ \mathrm{kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (0.18248\ \mathrm{kipft/ft}) \times (5.75\ \mathrm{ft})) + \left(3 \times (-0.072611\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft})^2\right)}{(6 \times (0.18248\ \mathrm{kipft/ft})) + (4 \times (-0.072611\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))}$$

$$a = 4.1228\ \mathrm{ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\,[(4\,M_o) + (3\,H_o\,L_e)]^2}{L_e^2\,[(3\,M_o) + (2\,H_o\,L_e)]}$$

$$p = \frac{0.75 \times [(4 \times (0.18248\ \mathrm{kipft/ft})) + (3 \times (-0.072611\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))]^2}{(5.75\ \mathrm{ft})^2 \times [(3 \times (0.18248\ \mathrm{kipft/ft})) + (2 \times (-0.072611\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))]}$$

$$p = -0.021544\ \mathrm{kip/ft^2}$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\,[(2\,M_o) + (H_o\,L_e)]}{L_e^2}$$

$$s = \frac{6 \times [(2 \times (0.18248\ \mathrm{kipft/ft})) + ((-0.072611\ \mathrm{kip/ft}) \times (5.75\ \mathrm{ft}))]}{(5.75\ \mathrm{ft})^2}$$

$$s = -0.0095361\ \mathrm{kip/ft^2}$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\,\frac{a}{2}$$

$$p_a = (150\ \mathrm{psf/ft}) \times \frac{(4.1228\ \mathrm{ft})}{2}$$

$$p_a = 0.30921\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{(-0.021544\ \mathrm{kip/ft^2})}{(0.30921\ \mathrm{kip/ft^2})}$$

$$Ratio = -0.069675$$

Status: PASS
Ratio: -0.070

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\,L_e$$

$$p_s = (150\ \mathrm{psf/ft}) \times (5.75\ \mathrm{ft})$$

$$p_s = 0.8625\ \mathrm{kip/ft^2}$$

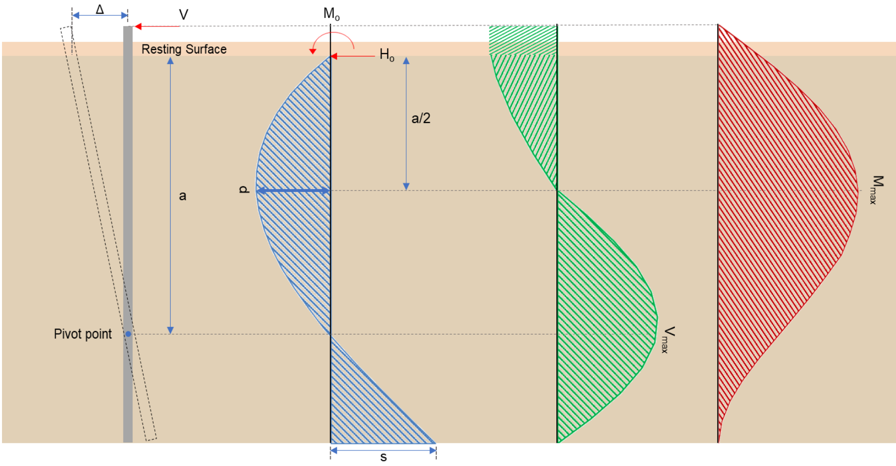
$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{(-0.0095361\ \text{kip/ft}^2)}{(0.8625\ \text{kip/ft}^2)}$$

$$Ratio = -0.011056$$

Status: **PASS**
Ratio: **-0.010**



Shear force and Bending moment (x-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\ D}$$

$$H_o = \frac{(-1.448\ \text{kip})}{1.57 \times (48\ \text{in})}$$

$$H_o = -0.23057\ \text{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\ H)}{1.57\ D}$$

$$M_o = \frac{(27.46\ \text{kipft}) + ((-1.448\ \text{kip}) \times (0\ \text{ft}))}{1.57 \times (48\ \text{in})}$$

$$M_o = 4.3726\ \text{kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(4.3726\ \text{kipft/ft})}{(-0.23057\ \text{kip/ft})}$$

$$E = 18.964\ \text{ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\ M_o\ L_e) + \left(3\ H_o\ L_e^2\right)}{(6\ M_o) + (4\ H_o\ L_e)}$$

$$a = \frac{(4 \times (4.3726\ \text{kipft/ft}) \times (5.75\ \text{ft})) + \left(3 \times (-0.23057\ \text{kip/ft}) \times (5.75\ \text{ft})^2\right)}{(6 \times (4.3726\ \text{kipft/ft})) + (4 \times (-0.23057\ \text{kip/ft}) \times (5.75\ \text{ft}))}$$

$$a = 3.9139\ \text{ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\ D) \left[1 - \left[3 \left(\frac{4\ E}{L_e} + 3\right) \left(\frac{a}{L_e}\right)^2\right] + \left[4 \left(\frac{3\ E}{L_e} + 2\right) \left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((-0.23057\ \text{kip/ft}) \times (48\ \text{in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (18.964\ \text{ft})}{(5.75\ \text{ft})} + 3\right) \times \left(\frac{(3.9139\ \text{ft})}{(5.75\ \text{ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (18.964\ \text{ft})}{(5.75\ \text{ft})} + 2\right) \times \left(\frac{(3.9139\ \text{ft})}{(5.75\ \text{ft})}\right)^3\right]\right]$$

$$V_{max} = 5.9971\ \text{kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, D\, L_e) \left[\left(\frac{E}{L_e} + \frac{a}{2\, L_e} \right) - \left[\left(\frac{4\, E}{L_e} + 3 \right) \left(\frac{a}{2\, L_e} \right)^3 \right] + \left[\left(\frac{3\, E}{L_e} + 2 \right) \left(\frac{a}{2\, L_e} \right)^4 \right] \right]$$

$$M_{max} = ((-0.23057 \text{ kip/ft}) \times (48 \text{ in}) \times (5.75 \text{ ft})) \times \left[\left(\frac{(18.964 \text{ ft})}{(5.75 \text{ ft})} + \frac{(3.9139 \text{ ft})}{2 \times (5.75 \text{ ft})} \right) - \left[\left(\frac{4 \times (18.964 \text{ ft})}{(5.75 \text{ ft})} + 3 \right) \times \left(\frac{(3.9139 \text{ ft})}{2 \times (5.75 \text{ ft})} \right)^3 \right] + \left[\left(\frac{3 \times (18.964 \text{ ft})}{(5.75 \text{ ft})} + 2 \right) \times \left(\frac{(3.9139 \text{ ft})}{2 \times (5.75 \text{ ft})} \right)^4 \right] \right]$$

$$M_{max} = 16.756 \text{ kipft}$$

Shear force and Bending moment (z-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\, b}$$

$$H_o = \frac{(-0.716 \text{ kip})}{1.57 \times (48 \text{ in})}$$

$$H_o = -0.11401 \text{ kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\, H)}{1.57\, b}$$

$$M_o = \frac{(1.801 \text{ kipft}) + ((-0.716 \text{ kip}) \times (0 \text{ ft}))}{1.57 \times (48 \text{ in})}$$

$$M_o = 0.28678 \text{ kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(0.28678 \text{ kipft/ft})}{(-0.11401 \text{ kip/ft})}$$

$$E = 2.5154 \text{ ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\, M_o\, L_e) + \left(3\, H_o\, L_e^2\right)}{(6\, M_o) + (4\, H_o\, L_e)}$$

$$a = \frac{(4 \times (0.28678 \text{ kipft/ft}) \times (5.75 \text{ ft})) + \left(3 \times (-0.11401 \text{ kip/ft}) \times (5.75 \text{ ft})^2\right)}{(6 \times (0.28678 \text{ kipft/ft})) + (4 \times (-0.11401 \text{ kip/ft}) \times (5.75 \text{ ft}))}$$

$$a = 4.1227 \text{ ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\, b) \left[1 - \left[3 \left(\frac{4\, E}{L_e} + 3 \right) \left(\frac{a}{L_e} \right)^2 \right] + \left[4 \left(\frac{3\, E}{L_e} + 2 \right) \left(\frac{a}{L_e} \right)^3 \right] \right]$$

$$V_{max} = ((-0.11401 \text{ kip/ft}) \times (48 \text{ in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (2.5154 \text{ ft})}{(5.75 \text{ ft})} + 3 \right) \times \left(\frac{(4.1227 \text{ ft})}{(5.75 \text{ ft})} \right)^2 \right] + \left[4 \times \left(\frac{3 \times (2.5154 \text{ ft})}{(5.75 \text{ ft})} + 2 \right) \times \left(\frac{(4.1227 \text{ ft})}{(5.75 \text{ ft})} \right)^3 \right] \right]$$

$$V_{max} = 0.6575 \text{ kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, b\, L_e) \left[\left(\frac{E}{L_e} + \frac{a}{2\, L_e} \right) - \left[\left(\frac{4\, E}{L_e} + 3 \right) \left(\frac{a}{2\, L_e} \right)^3 \right] + \left[\left(\frac{3\, E}{L_e} + 2 \right) \left(\frac{a}{2\, L_e} \right)^4 \right] \right]$$

$$M_{max} = ((-0.11401 \text{ kip/ft}) \times (48 \text{ in}) \times (5.75 \text{ ft})) \times \left[\left(\frac{(2.5154 \text{ ft})}{(5.75 \text{ ft})} + \frac{(4.1227 \text{ ft})}{2 \times (5.75 \text{ ft})} \right) - \left[\left(\frac{4 \times (2.5154 \text{ ft})}{(5.75 \text{ ft})} + 3 \right) \times \left(\frac{(4.1227 \text{ ft})}{2 \times (5.75 \text{ ft})} \right)^3 \right] + \left[\left(\frac{3 \times (2.5154 \text{ ft})}{(5.75 \text{ ft})} + 2 \right) \times \left(\frac{(4.1227 \text{ ft})}{2 \times (5.75 \text{ ft})} \right)^4 \right] \right]$$

$$M_{max} = 1.6568 \text{ kipft}$$

Minimum Reinforcement Check (LRFD)

Parameters:

$f'_{ck} = 3 \text{ ksi}$ - Concrete strength,
$f_{yk} = 60 \text{ ksi}$ - Longitudinal reinforcement strength,
$\phi = 0.65$ - Reduction factor for axial strength,
Table 22.4.2.1 $\alpha = 0.8$ - Alpha factor for axial strength,
$A_g = 2304 \text{ in}^2$ - Gross area of concrete,

Longitudinal reinforcement:

Required reinforcement due to axial load, $A_{st,required}$
22.4.2.2, 10.6.1.1 $A_{st,required}$

$$A_{st,required} = Min\left[\frac{\frac{P}{\phi\ \alpha} - \left(0.85\ f'_{ck}\ A_g\right)}{f_{yk} - \left(0.85\ f'_{ck}\right)}, \left(0.08\ A_g\right)\right]$$

$$A_{st,required} = Min\left[\frac{\frac{(9.985 \text{ kip})}{(0.65)\times(0.8)} - \left(0.85 \times (3 \text{ ksi}) \times \left(2304 \text{ in}^2\right)\right)}{(60 \text{ ksi}) - (0.85 \times (3 \text{ ksi}))}, \left(0.08 \times \left(2304 \text{ in}^2\right)\right)\right]$$

$$A_{st,required} = -101.93 \text{ in}^2$$

A_{min} - Governing minimum reinforcement area,

$$A_{min} = Max\left[A_{st,required}, \left(0.0018\ A_g\right)\right]$$

$$A_{min} = Max\left[\left(-101.93 \text{ in}^2\right), \left(0.0018 \times \left(2304 \text{ in}^2\right)\right)\right]$$

$$A_{min} = 4.1472 \text{ in}^2$$

n_{rebar} - Required number of reinforcement,

$$n_{rebar} = \frac{A_{min}}{A_{rebar}}$$

$$n_{rebar} = \frac{\left(4.1472 \text{ in}^2\right)}{\left(0.3068 \text{ in}^2\right)}$$

$$n_{rebar} = 14$$

A_{st} - Actual total reinforcement area,

$$A_{st} = n_{rebar}\ \frac{\pi\ d_{bar}^2}{4}$$

$$A_{st} = (14) \times \frac{\pi \times (0.625 \text{ in})^2}{4}$$

$$A_{st} = 4.2951 \text{ in}^2$$

$Ratio$ - Capacity

$$Ratio = \frac{A_{min}}{A_{st}}$$

$$Ratio = \frac{\left(4.1472 \text{ in}^2\right)}{\left(4.2951 \text{ in}^2\right)}$$

$$Ratio = 0.96556$$

Status: **PASS**
Ratio: **0.970**

25.2.3 s_{rebar} - Minimum spacing of reinforcement,

$$s_{rebar} = Max\left[1.5, \left(1.5\ d_{bar}\right)\right]$$

$$s_{rebar} = Max\left[1.5, \left(1.5 \times (0.625 \text{ in})\right)\right]$$

$$s_{rebar} = 1.5 \text{ in}$$

Ties:

25.7.2.2 Since longitudinal reinforcement is $\leq$ No. 10ø: Use #3(0.375 in)
25.7.2.1 s_{ties} - Maximum spacing of ties,

$$s_{ties} = Min\left[\left(16\ d_{bar}\right), \left(48\ d_{ties}\right), Min\left(D, b\right)\right]$$

$$s_{ties} = Min\left[\left(16 \times (0.625 \text{ in})\right), \left(48 \times (0.375 \text{ in})\right), Min\left((48 \text{ in}), (48 \text{ in})\right)\right]$$

$$s_{ties} = 10 \text{ in}$$

Summary:

Main reinforcement: **14 - #5 (0.625 in)**

	Ties: **#3(0.375 in) - 10 in**	

Axial Compression Strength (ACI 318-19, LRFD)

22.4.2.2 ϕP_N - Allowable axial compressive strength

$$\phi P_N = \phi\, 0.80\, \left[\left(0.85\, f'_{ck}\, [A_g - A_{st}]\right) + (f_{yk}\, A_{st})\right]$$

$$\phi P_N = (0.65) \times 0.80 \times \left[\left(0.85 \times (3\ \mathrm{ksi}) \times \left[\left(2304\ \mathrm{in}^2\right) - \left(4.2951\ \mathrm{in}^2\right)\right]\right) + \left((60\ \mathrm{ksi}) \times \left(4.2951\ \mathrm{in}^2\right)\right)\right]$$

$$\phi P_N = 3183.4\ \mathrm{kip}$$

$Ratio$ - Capacity

$$Ratio = \frac{P}{\phi P_N}$$

$$Ratio = \frac{(9.985\ \mathrm{kip})}{(3183.4\ \mathrm{kip})}$$

$$Ratio = 0.0031366$$

Status: **PASS**
Ratio: **0.000**

Shear Strength (ACI 318-19, LRFD)

Parameters:

$b_w = 48\ \mathrm{in}$ - Effective width,

22.5.2.2 d - Effective depth

$$d = 0.80\, D$$

$$d = 0.80 \times (48\ \mathrm{in})$$

$$d = 38.4\ \mathrm{in}$$

22.5.5.1.3 λ_s - size effect modification factor

$$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{d}{10}}}, 1\right]$$

$$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{(38.4\ \mathrm{in})}{10}}}, 1\right]$$

$$\lambda_s = 0.64282$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 3\ \mathrm{ksi} \rightarrow 3000\ \mathrm{psi}$,

22.5.5.1.1 $V_{c,max}$ - Max shear strength of concrete

$$V_{c,max} = 5\, \lambda_s\, \sqrt{f'_{ck}}\, b_w\, d$$

$$V_{c,max} = 5 \times (0.64282) \times \sqrt{(3000\ \mathrm{psi})} \times (48\ \mathrm{in}) \times (38.4\ \mathrm{in})$$

$$V_{c,max} = 324.49\ \mathrm{kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 3\ \mathrm{ksi} \rightarrow 3000\ \mathrm{psi}$, $P = 9.985\ \mathrm{kip} \rightarrow 9985\ \mathrm{lbf}$,

22.5.5.1.1(a) $V_{c,a}$ - Shear strength of concrete (a)

$$V_{c,a} = \left[2\, \lambda_s\, \sqrt{f'_{ck}} + \frac{P}{6\, A_g}\right]\, b_w\, d$$

$$V_{c,a} = \left[2 \times (0.64282) \times \sqrt{(3000\ \mathrm{psi})} + \frac{(9985\ \mathrm{lbf})}{6 \times \left(2304\ \mathrm{in}^2\right)}\right] \times (48\ \mathrm{in}) \times (38.4\ \mathrm{in})$$

$$V_{c,a} = 131.13\ \mathrm{kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 3\ \mathrm{ksi} \rightarrow 3000\ \mathrm{psi}$,

22.5.5.1.2 $V_{c,b}$ - Shear strength of concrete (b)

$$V_{c,b} = \left[2\, \lambda_s\, \sqrt{f'_{ck}} + \left(0.05\, f'_{ck}\right)\right]\, b_w\, d$$

$$V_{c,b} = \left[2 \times (0.64282) \times \sqrt{(3000\ \mathrm{psi})} + (0.05 \times (3000\ \mathrm{psi}))\right] \times (48\ \mathrm{in}) \times (38.4\ \mathrm{in})$$

$$V_{c,b} = 406.27\ \mathrm{kip}$$

V_c - Governing shear strength of concrete

$$V_c = Min\left[V_{c,max}, V_{c,a}, V_{c,b}\right]$$

$$V_c = Min\left[(324.49\ \mathrm{kip}), (131.13\ \mathrm{kip}), (406.27\ \mathrm{kip})\right]$$

$$V_c = 131.13\ \mathrm{kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 3\ \text{ksi} \rightarrow 3000\ \text{psi}$,

22.5.1.2 $V_{s,a}$ - Shear strength of steel (a)

$$V_{s,a} = 8\sqrt{f'_{ck}}\, b_w\, d$$

$$V_{s,a} = 8 \times \sqrt{(3000\ \text{psi})} \times (48\ \text{in}) \times (38.4\ \text{in})$$

$$V_{s,a} = 807.65\ \text{kip}$$

A_v - Ties rebar area,

$$A_v = \frac{\pi\, d_{ties}^2}{4}$$

$$A_v = \frac{\pi \times (0.375\ \text{in})^2}{4}$$

$$A_v = 0.11045\ \text{in}^2$$

22.5.8.5.3 $V_{s,b}$ - Shear strength of steel (b)

$$V_{s,b} = \frac{2\, A_v\, f_{ywk}\, d}{s_{ties}}$$

$$V_{s,b} = \frac{2 \times \left(0.11045\ \text{in}^2\right) \times (60\ \text{ksi}) \times (38.4\ \text{in})}{(10\ \text{in})}$$

$$V_{s,b} = 50.894\ \text{kip}$$

V_s - Governing shear strength of steel

$$V_s = MIN\left[V_{s,a}, V_{s,b}\right]$$

$$V_s = MIN\left[(807.65\ \text{kip}), (50.894\ \text{kip})\right]$$

$$V_s = 50.894\ \text{kip}$$

22.5.1.1 ϕV_n - Allowable shear strength

$$\phi V_n = \phi\ (V_c + V_s)$$

$$\phi V_n = (0.65) \times ((131.13\ \text{kip}) + (50.894\ \text{kip}))$$

$$\phi V_n = 118.31\ \text{kip}$$

Considering x-direction:

$V_{max} = 5.9971\ \text{kip}$ - Maximum shear force in the x-direction,
$Ratio$ - Capacity

$$Ratio = \frac{V_{max}}{\phi V_n}$$

$$Ratio = \frac{(5.9971\ \text{kip})}{(118.31\ \text{kip})}$$

$$Ratio = 0.050688$$

Status: **PASS**
Ratio: **0.050**

Considering z-direction:

$V_{max} = 0.6575\ \text{kip}$ - Maximum shear force in the z-direction,
$Ratio$ - Capacity

$$Ratio = \frac{V_{max}}{\phi V_n}$$

$$Ratio = \frac{(0.6575\ \text{kip})}{(118.31\ \text{kip})}$$

$$Ratio = 0.0055573$$

Status: **PASS**
Ratio: **0.010**

Flexural Strength (ACI 318-19, LRFD)

S_m - Section modulus

$$S_m = \frac{b\, D^2}{6}$$

$$S_m = \frac{(48\ \text{in}) \times (48\ \text{in})^2}{6}$$

$$S_m = 18432\ \text{in}^3$$

	$\lambda = 1$ - Concrete modification factor (Normal concrete), Allowable flexural strength: M_n shall be the lesser of: $\phi M_{n,1}$ $$\phi M_{n,1} = \phi \times 5 \times \lambda \times \sqrt{(f_c')} \times S_m$$ $$\phi M_{n,1} = 0.65 \times 5 \times 1 \times \sqrt{(3ksi)} \times 18432.001 in^3$$ $$\phi M_{n,1} = 273.423 kipft$$	
14.5.2.1b	$\phi M_{n,2}$ $$\phi M_{n,2} = \phi\, 0.85\, f_{ck}'\, S_m$$ $$\phi M_{n,2} = (0.65) \times 0.85 \times (3 \text{ ksi}) \times \left(18432 \text{ in}^3\right)$$ $$\phi M_{n,2} = 2545.9 \text{ kipft}$$ Therefore, ϕM_n - Allowable flexural strength, $$\phi M_n = MIN[\phi M_{n,1}, \phi M_{n,2}]$$ $$\phi M_n = MIN[(273.42 \text{ kipft}), (2545.9 \text{ kipft})]$$ $$\phi M_n = 273.42 \text{ kipft}$$ **Considering x-direction:** $M_{max} = 16.756 \text{ kipft}$ - Maximum moment in the x-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\phi M_n}$$ $$Ratio = \frac{(16.756 \text{ kipft})}{(273.42 \text{ kipft})}$$ $$Ratio = 0.061284$$	Status: **PASS** Ratio: **0.060**
	Considering z-direction: $M_{max} = 1.6568 \text{ kipft}$ - Maximum moment in the z-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\phi M_n}$$ $$Ratio = \frac{(1.6568 \text{ kipft})}{(273.42 \text{ kipft})}$$ $$Ratio = 0.0060595$$	Status: **PASS** Ratio: **0.010**

REFERENCES	CALCULATIONS	RESULTS

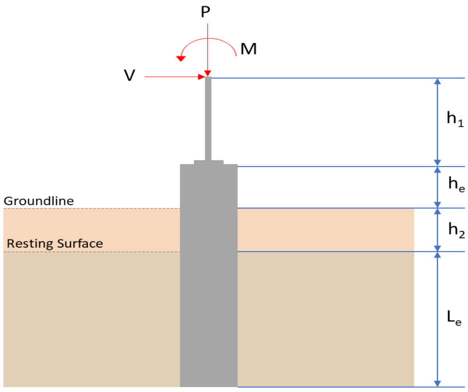
SkyCiv Foundation Design

Pile Foundation

Design Information :

Design code : IBC 2021 (International Building Code)
Unit System : Imperial

Pile Input



Geometry

Pile shape: rectangular
$b = 48$ in - Pile width
$D = 48$ in - Pile depth
$L = 6$ ft - Total pile length
$h_1 = 0$ ft - Lateral load height from the top of the pile,
$h_2 = 0$ ft - Depth to resisting surface
$h_e = 0$ ft - Length of pile above the ground

Tabulation of Soil Parameters

Layer	Label	Allowable Bearing Pressure (q_a) (psf)	Allowable Lateral Pressure (R) (psf/ft)
1	Sand, silty sand, clayey sand, silty gravel & clayey gravel	2000.000	150.000

Tabulation of Loads

Load Component	ASD	LRFD
P (kip)	9.646	14.883
V_x (kip)	-1.220	-2.025
V_z (kip)	-0.052	-0.080
M_x (kipft)	-0.137	-0.210
M_z (kipft)	20.502	34.713

Material Properties

$f'_{ck} = 3$ ksi - Concrete strength,

Required depth to resist lateral loads (ASD)

H - Point of application of the lateral load

$$H = h_1 + h_2 + h_e$$

$$H = (0\ \text{ft}) + (0\ \text{ft}) + (0\ \text{ft})$$

$$H = 0\ \text{ft}$$

Considering x-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\ D}$$

$$H_o = \frac{(-1.22\ \text{kip})}{1.57 \times (48\ \text{in})}$$

$$H_o = -0.19427\ \text{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\ H)}{1.57\ D}$$

$$M_o = \frac{(20.502 \text{ kipft}) + ((-1.22 \text{ kip}) \times (0 \text{ ft}))}{1.57 \times (48 \text{ in})}$$

$$M_o = 3.2646 \text{ kipft/ft}$$

Required depth of embedment in earth:

$$L_x^3 - \left(14.14 \times \frac{H_o \times L_x}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,x} = 5.7862 \text{ ft}$ - Required depth in x-direction,

Considering z-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\, b}$$

$$H_o = \frac{(-0.052 \text{ kip})}{1.57 \times (48 \text{ in})}$$

$$H_o = -0.0082803 \text{ kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\, H)}{1.57\, b}$$

$$M_o = \frac{(0.137 \text{ kipft}) + ((-0.052 \text{ kip}) \times (0 \text{ ft}))}{1.57 \times (48 \text{ in})}$$

$$M_o = 0.021815 \text{ kipft/ft}$$

Required depth of embedment in earth:

$$L_z^3 - \left(14.14 \times \frac{H_o \times L_z}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,z} = 1.0672 \text{ ft}$ - Required depth in z-direction,

Minimum embedded depth required:

$L_{e,req}$ - Depth of pile required,

$$L_{e,req} = MAX[L_{e,x}, L_{e,z}]$$

$$L_{e,req} = MAX[(5.7862 \text{ ft}), (1.0672 \text{ ft})]$$

$$L_{e,req} = 5.786 \text{ ft}$$

L_e - Actual embedded length of pile,

$$L_e = L - h_e - h_2$$

$$L_e = (6 \text{ ft}) - (0 \text{ ft}) - (0 \text{ ft})$$

$$L_e = 6 \text{ ft}$$

$Ratio$ - Embedded depth

$$Ratio = \frac{L_{e,req}}{L_e}$$

$$Ratio = \frac{(5.786 \text{ ft})}{(6 \text{ ft})}$$

$$Ratio = 0.96433$$

Status: **PASS**
Ratio: **0.960**

End-bearing Capacity (ASD)

A - Pile cross-section area

$$A = b\, D$$

$$A = (48 \text{ in}) \times (48 \text{ in})$$

$$A = 16 \text{ ft}^2$$

q - End-bearing pressure

$$q = \frac{P_v}{A}$$

$$q = \frac{(9.646 \text{ kip})}{(16 \text{ ft}^2)}$$

$$q = 0.60287 \text{ kip/ft}^2$$

$q = 0.60287\ \mathrm{kip/ft}$

Check bearing capacity ratio:

$Ratio$ - Capacity

$$Ratio = \frac{q}{q_a}$$

$$Ratio = \frac{\left(0.60287\ \mathrm{kip/ft^2}\right)}{\left(2000\ \mathrm{psf}\right)}$$

$$Ratio = 0.30144$$

Status: **PASS**
Ratio: **0.300**

Czerniak

Lateral Soil Pressure (ASD):

L/D - Length to least lateral dimension ratio,

$$L/D = \frac{L}{D}$$

$$L/D = \frac{\left(6\ \mathrm{ft}\right)}{\left(48\ \mathrm{in}\right)}$$

$$L/D = 1.5$$

Since L/D $\leq$ 10,

Pile is short.

Considering x-direction:

$H_o = -0.19427\ \mathrm{kip/ft}$ - Lateral force per length of pile,
$M_o = 3.2646\ \mathrm{kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{\left(4\ M_o\ L_e\right) + \left(3\ H_o\ L_e^2\right)}{\left(6\ M_o\right) + \left(4\ H_o\ L_e\right)}$$

$$a = \frac{\left(4 \times \left(3.2646\ \mathrm{kipft/ft}\right) \times \left(6\ \mathrm{ft}\right)\right) + \left(3 \times \left(-0.19427\ \mathrm{kip/ft}\right) \times \left(6\ \mathrm{ft}\right)^2\right)}{\left(6 \times \left(3.2646\ \mathrm{kipft/ft}\right)\right) + \left(4 \times \left(-0.19427\ \mathrm{kip/ft}\right) \times \left(6\ \mathrm{ft}\right)\right)}$$

$$a = 4.0961\ \mathrm{ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\ \left[\left(4\ M_o\right) + \left(3\ H_o\ L_e\right)\right]^2}{L_e^2\ \left[\left(3\ M_o\right) + \left(2\ H_o\ L_e\right)\right]}$$

$$p = \frac{0.75 \times \left[\left(4 \times \left(3.2646\ \mathrm{kipft/ft}\right)\right) + \left(3 \times \left(-0.19427\ \mathrm{kip/ft}\right) \times \left(6\ \mathrm{ft}\right)\right)\right]^2}{\left(6\ \mathrm{ft}\right)^2 \times \left[\left(3 \times \left(3.2646\ \mathrm{kipft/ft}\right)\right) + \left(2 \times \left(-0.19427\ \mathrm{kip/ft}\right) \times \left(6\ \mathrm{ft}\right)\right)\right]}$$

$$p = 0.25523\ \mathrm{kip/ft^2}$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\ \left[\left(2\ M_o\right) + \left(H_o\ L_e\right)\right]}{L_e^2}$$

$$s = \frac{6 \times \left[\left(2 \times \left(3.2646\ \mathrm{kipft/ft}\right)\right) + \left(\left(-0.19427\ \mathrm{kip/ft}\right) \times \left(6\ \mathrm{ft}\right)\right)\right]}{\left(6\ \mathrm{ft}\right)^2}$$

$$s = 0.89395\ \mathrm{kip/ft^2}$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\ \frac{a}{2}$$

$$p_a = \left(150\ \mathrm{psf/ft}\right) \times \frac{\left(4.0961\ \mathrm{ft}\right)}{2}$$

$$p_a = 0.30721\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{\left(0.25523\ \mathrm{kip/ft^2}\right)}{\left(0.30721\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = 0.83081$$

Status: **PASS**
Ratio: **0.830**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\,L_e$$

$$p_s = (150\ \mathrm{psf/ft}) \times (6\ \mathrm{ft})$$

$$p_s = 0.9\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{\left(0.89395\ \mathrm{kip/ft^2}\right)}{\left(0.9\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = 0.99328$$

Status: **PASS**
Ratio: **0.990**

Considering z-direction:

$H_o = -0.0082803\ \mathrm{kip/ft}$ - Lateral force per length of pile,
$M_o = 0.021815\ \mathrm{kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (0.021815\ \mathrm{kipft/ft}) \times (6\ \mathrm{ft})) + \left(3 \times (-0.0082803\ \mathrm{kip/ft}) \times (6\ \mathrm{ft})^2\right)}{(6 \times (0.021815\ \mathrm{kipft/ft})) + (4 \times (-0.0082803\ \mathrm{kip/ft}) \times (6\ \mathrm{ft}))}$$

$$a = 4.3014\ \mathrm{ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\,[(4\,M_o) + (3\,H_o\,L_e)]^2}{L_e^2\,[(3\,M_o) + (2\,H_o\,L_e)]}$$

$$p = \frac{0.75 \times [(4 \times (0.021815\ \mathrm{kipft/ft})) + (3 \times (-0.0082803\ \mathrm{kip/ft}) \times (6\ \mathrm{ft}))]^2}{(6\ \mathrm{ft})^2 \times [(3 \times (0.021815\ \mathrm{kipft/ft})) + (2 \times (-0.0082803\ \mathrm{kip/ft}) \times (6\ \mathrm{ft}))]}$$

$$p = -0.0023447\ \mathrm{kip/ft^2}$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\,[(2\,M_o) + (H_o\,L_e)]}{L_e^2}$$

$$s = \frac{6 \times [(2 \times (0.021815\ \mathrm{kipft/ft})) + ((-0.0082803\ \mathrm{kip/ft}) \times (6\ \mathrm{ft}))]}{(6\ \mathrm{ft})^2}$$

$$s = -0.0010085\ \mathrm{kip/ft^2}$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\,\frac{a}{2}$$

$$p_a = (150\ \mathrm{psf/ft}) \times \frac{(4.3014\ \mathrm{ft})}{2}$$

$$p_a = 0.32261\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{\left(-0.0023447\ \mathrm{kip/ft^2}\right)}{\left(0.32261\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = -0.0072679$$

Status: **PASS**
Ratio: **-0.010**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\,L_e$$

$$p_s = (150\ \mathrm{psf/ft}) \times (6\ \mathrm{ft})$$

$$p_s = 0.9\ \mathrm{kip/ft^2}$$

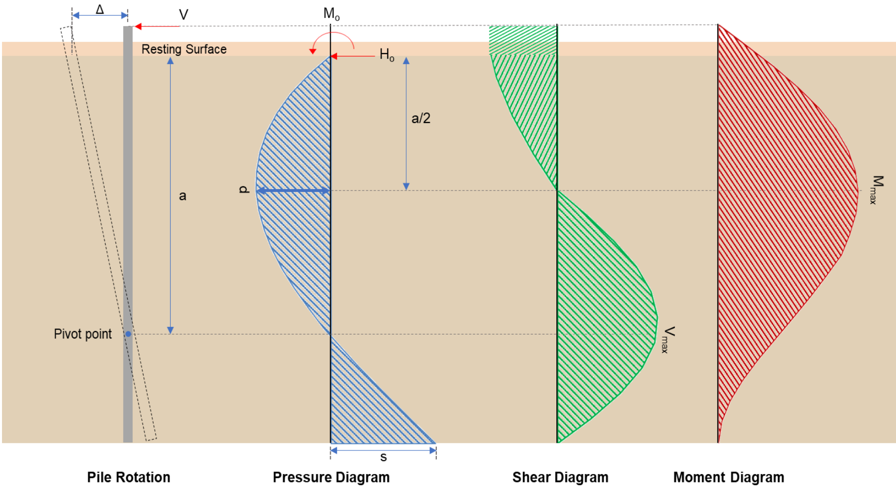
$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{(-0.0010085\ \text{kip/ft}^2)}{(0.9\ \text{kip/ft}^2)}$$

$$Ratio = -0.0011205$$

Status: **PASS**
Ratio: **0.000**



Shear force and Bending moment (x-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\,D}$$

$$H_o = \frac{(-2.025\ \text{kip})}{1.57 \times (48\ \text{in})}$$

$$H_o = -0.32245\ \text{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\,H)}{1.57\,D}$$

$$M_o = \frac{(34.713\ \text{kipft}) + ((-2.025\ \text{kip}) \times (0\ \text{ft}))}{1.57 \times (48\ \text{in})}$$

$$M_o = 5.5275\ \text{kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(5.5275\ \text{kipft/ft})}{(-0.32245\ \text{kip/ft})}$$

$$E = 17.142\ \text{ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (5.5275\ \text{kipft/ft}) \times (6\ \text{ft})) + \left(3 \times (-0.32245\ \text{kip/ft}) \times (6\ \text{ft})^2\right)}{(6 \times (5.5275\ \text{kipft/ft})) + (4 \times (-0.32245\ \text{kip/ft}) \times (6\ \text{ft}))}$$

$$a = 4.0946\ \text{ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\,D)\left[1 - \left[3\left(\frac{4\,E}{L_e} + 3\right)\left(\frac{a}{L_e}\right)^2\right] + \left[4\left(\frac{3\,E}{L_e} + 2\right)\left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((-0.32245\ \text{kip/ft}) \times (48\ \text{in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (17.142\ \text{ft})}{(6\ \text{ft})} + 3\right) \times \left(\frac{(4.0946\ \text{ft})}{(6\ \text{ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (17.142\ \text{ft})}{(6\ \text{ft})} + 2\right) \times \left(\frac{(4.0946\ \text{ft})}{(6\ \text{ft})}\right)^3\right]\right]$$

$$V_{max} = 7.3769\ \text{kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, D\, L_e) \left[\left(\frac{E}{L_e} + \frac{a}{2\, L_e}\right) - \left[\left(\frac{4\, E}{L_e} + 3\right) \left(\frac{a}{2\, L_e}\right)^3\right] + \left[\left(\frac{3\, E}{L_e} + 2\right) \left(\frac{a}{2\, L_e}\right)^4\right]\right]$$

$$M_{max} = ((-0.32245\ \text{kip/ft}) \times (48\ \text{in}) \times (6\ \text{ft})) \times \left[\left(\frac{(17.142\ \text{ft})}{(6\ \text{ft})} + \frac{(4.0946\ \text{ft})}{2 \times (6\ \text{ft})}\right) - \left[\left(\frac{4 \times (17.142\ \text{ft})}{(6\ \text{ft})} + 3\right) \times \left(\frac{(4.0946\ \text{ft})}{2 \times (6\ \text{ft})}\right)^3\right] + \left[\left(\frac{3 \times (17.142\ \text{ft})}{(6\ \text{ft})} + 2\right) \times \left(\frac{(4.0946\ \text{ft})}{2 \times (6\ \text{ft})}\right)^4\right]\right]$$

$$M_{max} = 21.424\ \text{kipft}$$

Shear force and Bending moment (z-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\, b}$$

$$H_o = \frac{(-0.08\ \text{kip})}{1.57 \times (48\ \text{in})}$$

$$H_o = -0.012739\ \text{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\, H)}{1.57\, b}$$

$$M_o = \frac{(0.21\ \text{kipft}) + ((-0.08\ \text{kip}) \times (0\ \text{ft}))}{1.57 \times (48\ \text{in})}$$

$$M_o = 0.033439\ \text{kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(0.033439\ \text{kipft/ft})}{(-0.012739\ \text{kip/ft})}$$

$$E = 2.625\ \text{ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\, M_o\, L_e) + \left(3\, H_o\, L_e^2\right)}{(6\, M_o) + (4\, H_o\, L_e)}$$

$$a = \frac{(4 \times (0.033439\ \text{kipft/ft}) \times (6\ \text{ft})) + \left(3 \times (-0.012739\ \text{kip/ft}) \times (6\ \text{ft})^2\right)}{(6 \times (0.033439\ \text{kipft/ft})) + (4 \times (-0.012739\ \text{kip/ft}) \times (6\ \text{ft}))}$$

$$a = 4.3019\ \text{ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\, b) \left[1 - \left[3 \left(\frac{4\, E}{L_e} + 3\right) \left(\frac{a}{L_e}\right)^2\right] + \left[4 \left(\frac{3\, E}{L_e} + 2\right) \left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((-0.012739\ \text{kip/ft}) \times (48\ \text{in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (2.625\ \text{ft})}{(6\ \text{ft})} + 3\right) \times \left(\frac{(4.3019\ \text{ft})}{(6\ \text{ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (2.625\ \text{ft})}{(6\ \text{ft})} + 2\right) \times \left(\frac{(4.3019\ \text{ft})}{(6\ \text{ft})}\right)^3\right]\right]$$

$$V_{max} = 0.073467\ \text{kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, b\, L_e) \left[\left(\frac{E}{L_e} + \frac{a}{2\, L_e}\right) - \left[\left(\frac{4\, E}{L_e} + 3\right) \left(\frac{a}{2\, L_e}\right)^3\right] + \left[\left(\frac{3\, E}{L_e} + 2\right) \left(\frac{a}{2\, L_e}\right)^4\right]\right]$$

$$M_{max} = ((-0.012739\ \text{kip/ft}) \times (48\ \text{in}) \times (6\ \text{ft})) \times \left[\left(\frac{(2.625\ \text{ft})}{(6\ \text{ft})} + \frac{(4.3019\ \text{ft})}{2 \times (6\ \text{ft})}\right) - \left[\left(\frac{4 \times (2.625\ \text{ft})}{(6\ \text{ft})} + 3\right) \times \left(\frac{(4.3019\ \text{ft})}{2 \times (6\ \text{ft})}\right)^3\right] + \left[\left(\frac{3 \times (2.625\ \text{ft})}{(6\ \text{ft})} + 2\right) \times \left(\frac{(4.3019\ \text{ft})}{2 \times (6\ \text{ft})}\right)^4\right]\right]$$

	$M_{max} = 0.19318$ kipft		

Minimum Reinforcement Check (LRFD)

Parameters:

$f'_{ck} = 3$ ksi - Concrete strength,
$f_{yk} = 60$ ksi - Longitudinal reinforcement strength,
$\phi = 0.65$ - Reduction factor for axial strength,
Table 22.4.2.1 $\alpha = 0.8$ - Alpha factor for axial strength,
$A_g = 2304\ \text{in}^2$ - Gross area of concrete,

Longitudinal reinforcement:

Required reinforcement due to axial load, $A_{st,required}$
22.4.2.2, 10.6.1.1 $A_{st,required}$

$$A_{st,required} = Min\left[\frac{\frac{P}{\phi\ \alpha} - (0.85\ f'_{ck}\ A_g)}{f_{yk} - (0.85\ f'_{ck})}, (0.08\ A_g)\right]$$

$$A_{st,required} = Min\left[\frac{\frac{(14.883\ \text{kip})}{(0.65)\times(0.8)} - \left(0.85 \times (3\ \text{ksi}) \times \left(2304\ \text{in}^2\right)\right)}{(60\ \text{ksi}) - (0.85 \times (3\ \text{ksi}))}, \left(0.08 \times \left(2304\ \text{in}^2\right)\right)\right]$$

$$A_{st,required} = -101.77\ \text{in}^2$$

A_{min} - Governing minimum reinforcement area,

$$A_{min} = Max\left[A_{st,required}, (0.0018\ A_g)\right]$$

$$A_{min} = Max\left[\left(-101.77\ \text{in}^2\right), \left(0.0018 \times \left(2304\ \text{in}^2\right)\right)\right]$$

$$A_{min} = 4.1472\ \text{in}^2$$

n_{rebar} - Required number of reinforcement,

$$n_{rebar} = \frac{A_{min}}{A_{rebar}}$$

$$n_{rebar} = \frac{\left(4.1472\ \text{in}^2\right)}{\left(0.3068\ \text{in}^2\right)}$$

$$n_{rebar} = 14$$

A_{st} - Actual total reinforcement area,

$$A_{st} = n_{rebar}\ \frac{\pi\ d_{bar}^2}{4}$$

$$A_{st} = (14) \times \frac{\pi \times (0.625\ \text{in})^2}{4}$$

$$A_{st} = 4.2951\ \text{in}^2$$

$Ratio$ - Capacity

$$Ratio = \frac{A_{min}}{A_{st}}$$

$$Ratio = \frac{\left(4.1472\ \text{in}^2\right)}{\left(4.2951\ \text{in}^2\right)}$$

$$Ratio = 0.96556$$

Status: **PASS**
Ratio: **0.970**

25.2.3 s_{rebar} - Minimum spacing of reinforcement,

$$s_{rebar} = Max\left[1.5, (1.5\ d_{bar})\right]$$

$$s_{rebar} = Max\left[1.5, (1.5 \times (0.625\ \text{in}))\right]$$

$$s_{rebar} = 1.5\ \text{in}$$

Ties:

25.7.2.2 Since longitudinal reinforcement is $\leq$ No. 10ø: Use #3(0.375 in)
25.7.2.1 s_{ties} - Maximum spacing of ties,

$$s_{ties} = Min\left[(16\ d_{bar}), (48\ d_{ties}), Min(D, b)\right]$$

$$s_{ties} = Min\left[(16 \times (0.625\ \text{in})), (48 \times (0.375\ \text{in})), Min((48\ \text{in}), (48\ \text{in}))\right]$$

$$s_{ties} = 10\ \text{in}$$

Summary:

Main reinforcement: **14 - #5 (0.625 in)**

	Ties: **#3(0.375 in) - 10 in**	

Axial Compression Strength (ACI 318-19, LRFD)

22.4.2.2 ϕP_N - Allowable axial compressive strength

$$\phi P_N = \phi\, 0.80\, \left[\left(0.85\, f'_{ck}\, \left[A_g - A_{st}\right]\right) + \left(f_{yk}\, A_{st}\right)\right]$$

$$\phi P_N = (0.65) \times 0.80 \times \left[\left(0.85 \times (3\text{ ksi}) \times \left[\left(2304\text{ in}^2\right) - \left(4.2951\text{ in}^2\right)\right]\right) + \left((60\text{ ksi}) \times \left(4.2951\text{ in}^2\right)\right)\right]$$

$$\phi P_N = 3183.4\text{ kip}$$

$Ratio$ - Capacity

$$Ratio = \frac{P}{\phi P_N}$$

$$Ratio = \frac{(14.883\text{ kip})}{(3183.4\text{ kip})}$$

$$Ratio = 0.0046752$$

Status: **PASS**
Ratio: **0.000**

Shear Strength (ACI 318-19, LRFD)

Parameters:

$b_w = 48\text{ in}$ - Effective width,

22.5.2.2 d - Effective depth

$$d = 0.80\, D$$

$$d = 0.80 \times (48\text{ in})$$

$$d = 38.4\text{ in}$$

22.5.5.1.3 λ_s - size effect modification factor

$$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{d}{10}}}, 1\right]$$

$$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{(38.4\text{ in})}{10}}}, 1\right]$$

$$\lambda_s = 0.64282$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 3\text{ ksi} \rightarrow 3000\text{ psi}$,

22.5.5.1.1 $V_{c,max}$ - Max shear strength of concrete

$$V_{c,max} = 5\, \lambda_s\, \sqrt{f'_{ck}}\, b_w\, d$$

$$V_{c,max} = 5 \times (0.64282) \times \sqrt{(3000\text{ psi})} \times (48\text{ in}) \times (38.4\text{ in})$$

$$V_{c,max} = 324.49\text{ kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 3\text{ ksi} \rightarrow 3000\text{ psi}$, $P = 14.883\text{ kip} \rightarrow 14883\text{ lbf}$,

22.5.5.1.1(a) $V_{c,a}$ - Shear strength of concrete (a)

$$V_{c,a} = \left[2\, \lambda_s\, \sqrt{f'_{ck}} + \frac{P}{6\, A_g}\right]\, b_w\, d$$

$$V_{c,a} = \left[2 \times (0.64282) \times \sqrt{(3000\text{ psi})} + \frac{(14883\text{ lbf})}{6 \times \left(2304\text{ in}^2\right)}\right] \times (48\text{ in}) \times (38.4\text{ in})$$

$$V_{c,a} = 131.78\text{ kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 3\text{ ksi} \rightarrow 3000\text{ psi}$,

22.5.5.1.2 $V_{c,b}$ - Shear strength of concrete (b)

$$V_{c,b} = \left[2\, \lambda_s\, \sqrt{f'_{ck}} + \left(0.05\, f'_{ck}\right)\right]\, b_w\, d$$

$$V_{c,b} = \left[2 \times (0.64282) \times \sqrt{(3000\text{ psi})} + (0.05 \times (3000\text{ psi}))\right] \times (48\text{ in}) \times (38.4\text{ in})$$

$$V_{c,b} = 406.27\text{ kip}$$

V_c - Governing shear strength of concrete

$$V_c = Min\left[V_{c,max}, V_{c,a}, V_{c,b}\right]$$

$$V_c = Min\left[(324.49\text{ kip}), (131.78\text{ kip}), (406.27\text{ kip})\right]$$

$$V_c = 131.78\text{ kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 3\text{ ksi} \rightarrow 3000\text{ psi}$,

22.5.1.2 $V_{s,a}$ - Shear strength of steel (a)

$$V_{s,a} = 8\sqrt{f'_{ck}}\,b_w\,d$$

$$V_{s,a} = 8 \times \sqrt{(3000\text{ psi})} \times (48\text{ in}) \times (38.4\text{ in})$$

$$V_{s,a} = 807.65\text{ kip}$$

A_v - Ties rebar area,

$$A_v = \frac{\pi\,d_{ties}^2}{4}$$

$$A_v = \frac{\pi \times (0.375\text{ in})^2}{4}$$

$$A_v = 0.11045\text{ in}^2$$

22.5.8.5.3 $V_{s,b}$ - Shear strength of steel (b)

$$V_{s,b} = \frac{2\,A_v\,f_{ywk}\,d}{s_{ties}}$$

$$V_{s,b} = \frac{2 \times \left(0.11045\text{ in}^2\right) \times (60\text{ ksi}) \times (38.4\text{ in})}{(10\text{ in})}$$

$$V_{s,b} = 50.894\text{ kip}$$

V_s - Governing shear strength of steel

$$V_s = MIN\left[V_{s,a}, V_{s,b}\right]$$

$$V_s = MIN\left[(807.65\text{ kip}), (50.894\text{ kip})\right]$$

$$V_s = 50.894\text{ kip}$$

22.5.1.1 ϕV_n - Allowable shear strength

$$\phi V_n = \phi\,(V_c + V_s)$$

$$\phi V_n = (0.65) \times ((131.78\text{ kip}) + (50.894\text{ kip}))$$

$$\phi V_n = 118.74\text{ kip}$$

Considering x-direction:

$V_{max} = 7.3769\text{ kip}$ - Maximum shear force in the x-direction,
$Ratio$ - Capacity

$$Ratio = \frac{V_{max}}{\phi V_n}$$

$$Ratio = \frac{(7.3769\text{ kip})}{(118.74\text{ kip})}$$

$$Ratio = 0.062128$$

Status: **PASS**
Ratio: **0.060**

Considering z-direction:

$V_{max} = 0.073467\text{ kip}$ - Maximum shear force in the z-direction,
$Ratio$ - Capacity

$$Ratio = \frac{V_{max}}{\phi V_n}$$

$$Ratio = \frac{(0.073467\text{ kip})}{(118.74\text{ kip})}$$

$$Ratio = 0.00061874$$

Status: **PASS**
Ratio: **0.000**

Flexural Strength (ACI 318-19, LRFD)

S_m - Section modulus

$$S_m = \frac{b\,D^2}{6}$$

$$S_m = \frac{(48\text{ in}) \times (48\text{ in})^2}{6}$$

$$S_m = 18432\text{ in}^3$$

$\lambda = 1$ - Concrete modification factor (Normal concrete),
Allowable flexural strength:
M_n shall be the lesser of:
$\phi M_{n,1}$

$$\phi M_{n,1} = \phi \times 5 \times \lambda \times \sqrt{(f_c')} \times S_m$$

$$\phi M_{n,1} = 0.65 \times 5 \times 1 \times \sqrt{(3ksi)} \times 18432.001in^3$$

$$\phi M_{n,1} = 273.423kipft$$

14.5.2.1b $\phi M_{n,2}$

$$\phi M_{n,2} = \phi \, 0.85 \, f_{ck}' \, S_m$$

$$\phi M_{n,2} = (0.65) \times 0.85 \times (3 \text{ ksi}) \times \left(18432 \text{ in}^3\right)$$

$$\phi M_{n,2} = 2545.9 \text{ kipft}$$

Therefore,
ϕM_n - Allowable flexural strength,

$$\phi M_n = MIN[\phi M_{n,1}, \phi M_{n,2}]$$

$$\phi M_n = MIN[(273.42 \text{ kipft}), (2545.9 \text{ kipft})]$$

$$\phi M_n = 273.42 \text{ kipft}$$

Considering x-direction:

$M_{max} = 21.424 \text{ kipft}$ - Maximum moment in the x-direction,
$Ratio$ - Capacity

$$Ratio = \frac{M_{max}}{\phi M_n}$$

$$Ratio = \frac{(21.424 \text{ kipft})}{(273.42 \text{ kipft})}$$

$$Ratio = 0.078354$$

Status: **PASS**
Ratio: **0.080**

Considering z-direction:

$M_{max} = 0.19318 \text{ kipft}$ - Maximum moment in the z-direction,
$Ratio$ - Capacity

$$Ratio = \frac{M_{max}}{\phi M_n}$$

$$Ratio = \frac{(0.19318 \text{ kipft})}{(273.42 \text{ kipft})}$$

$$Ratio = 0.00070653$$

Status: **PASS**
Ratio: **0.000**

REFERENCES	CALCULATIONS	RESULTS

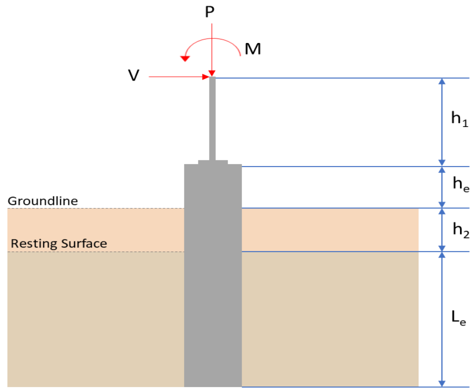
SkyCiv Foundation Design

Pile Foundation

Design Information :

Design code : IBC 2021 (International Building Code)
Unit System : Imperial

Pile Input



Geometry

Pile shape: rectangular
$b = 48\ \text{in}$ - Pile width
$D = 48\ \text{in}$ - Pile depth
$L = 6\ \text{ft}$ - Total pile length
$h_1 = 0\ \text{ft}$ - Lateral load height from the top of the pile,
$h_2 = 0\ \text{ft}$ - Depth to resisting surface
$h_e = 0\ \text{ft}$ - Length of pile above the ground

Tabulation of Soil Parameters

Layer	Label	Allowable Bearing Pressure (q_a) (psf)	Allowable Lateral Pressure (R) (psf/ft)
1	Sand, silty sand, clayey sand, silty gravel & clayey gravel	2000.000	150.000

Tabulation of Loads

Load Component	ASD	LRFD
P (kip)	9.646	14.883
V_x (kip)	-1.220	-2.025
V_z (kip)	0.052	0.080
M_x (kipft)	0.137	0.210
M_z (kipft)	20.502	34.713

Material Properties

$f'_{ck} = 3\ \text{ksi}$ - Concrete strength,

Required depth to resist lateral loads (ASD)

H - Point of application of the lateral load

$$H = h_1 + h_2 + h_e$$

$$H = (0\ \text{ft}) + (0\ \text{ft}) + (0\ \text{ft})$$

$$H = 0\ \text{ft}$$

Considering x-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\,D}$$

$$H_o = \frac{(-1.22\ \text{kip})}{1.57 \times (48\ \text{in})}$$

$$H_o = -0.19427\ \text{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\,H)}{1.57\,D}$$

$$M_o = \frac{(20.502\ \text{kipft}) + ((-1.22\ \text{kip}) \times (0\ \text{ft}))}{1.57 \times (48\ \text{in})}$$

$$M_o = 3.2646\ \text{kipft/ft}$$

Required depth of embedment in earth:

$$L_x^3 - \left(14.14 \times \frac{H_o \times L_x}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,x} = 5.7862\ \text{ft}$ - Required depth in x-direction,

Considering z-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\,b}$$

$$H_o = \frac{(0.052\ \text{kip})}{1.57 \times (48\ \text{in})}$$

$$H_o = 0.0082803\ \text{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\,H)}{1.57\,b}$$

$$M_o = \frac{(0.137\ \text{kipft}) + ((0.052\ \text{kip}) \times (0\ \text{ft}))}{1.57 \times (48\ \text{in})}$$

$$M_o = 0.021815\ \text{kipft/ft}$$

Required depth of embedment in earth:

$$L_z^3 - \left(14.14 \times \frac{H_o \times L_z}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,z} = 1.3411\ \text{ft}$ - Required depth in z-direction,

Minimum embedded depth required:

$L_{e,req}$ - Depth of pile required,

$$L_{e,req} = MAX[L_{e,x}, L_{e,z}]$$

$$L_{e,req} = MAX[(5.7862\ \text{ft}), (1.3411\ \text{ft})]$$

$$L_{e,req} = 5.786\ \text{ft}$$

L_e - Actual embedded length of pile,

$$L_e = L - h_e - h_2$$

$$L_e = (6\ \text{ft}) - (0\ \text{ft}) - (0\ \text{ft})$$

$$L_e = 6\ \text{ft}$$

$Ratio$ - Embedded depth

$$Ratio = \frac{L_{e,req}}{L_e}$$

$$Ratio = \frac{(5.786\ \text{ft})}{(6\ \text{ft})}$$

$$Ratio = 0.96433$$

Status: **PASS**
Ratio: **0.960**

End-bearing Capacity (ASD)

A - Pile cross-section area

$$A = b\,D$$

$$A = (48\ \text{in}) \times (48\ \text{in})$$

$$A = 16\ \text{ft}^2$$

q - End-bearing pressure

$$q = \frac{P_v}{A}$$

$$q = \frac{(9.646\ \text{kip})}{(16\ \text{ft}^2)}$$

$$q = 0.60287\ \text{kip/ft}^2$$

$q = 0.60287\ \mathrm{kip/ft}$

Check bearing capacity ratio:

$Ratio$ - Capacity

$$Ratio = \frac{q}{q_a}$$

$$Ratio = \frac{\left(0.60287\ \mathrm{kip/ft^2}\right)}{\left(2000\ \mathrm{psf}\right)}$$

$$Ratio = 0.30144$$

Status: **PASS**
Ratio: **0.300**

Czerniak

Lateral Soil Pressure (ASD):

L/D - Length to least lateral dimension ratio,

$$L/D = \frac{L}{D}$$

$$L/D = \frac{\left(6\ \mathrm{ft}\right)}{\left(48\ \mathrm{in}\right)}$$

$$L/D = 1.5$$

Since L/D $\leq$ 10,

Pile is short.

Considering x-direction:

$H_o = -0.19427\ \mathrm{kip/ft}$ - Lateral force per length of pile,
$M_o = 3.2646\ \mathrm{kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{\left(4\,M_o\,L_e\right) + \left(3\,H_o\,L_e^2\right)}{\left(6\,M_o\right) + \left(4\,H_o\,L_e\right)}$$

$$a = \frac{\left(4 \times \left(3.2646\ \mathrm{kipft/ft}\right) \times \left(6\ \mathrm{ft}\right)\right) + \left(3 \times \left(-0.19427\ \mathrm{kip/ft}\right) \times \left(6\ \mathrm{ft}\right)^2\right)}{\left(6 \times \left(3.2646\ \mathrm{kipft/ft}\right)\right) + \left(4 \times \left(-0.19427\ \mathrm{kip/ft}\right) \times \left(6\ \mathrm{ft}\right)\right)}$$

$$a = 4.0961\ \mathrm{ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\,\left[\left(4\,M_o\right) + \left(3\,H_o\,L_e\right)\right]^2}{L_e^2\,\left[\left(3\,M_o\right) + \left(2\,H_o\,L_e\right)\right]}$$

$$p = \frac{0.75 \times \left[\left(4 \times \left(3.2646\ \mathrm{kipft/ft}\right)\right) + \left(3 \times \left(-0.19427\ \mathrm{kip/ft}\right) \times \left(6\ \mathrm{ft}\right)\right)\right]^2}{\left(6\ \mathrm{ft}\right)^2 \times \left[\left(3 \times \left(3.2646\ \mathrm{kipft/ft}\right)\right) + \left(2 \times \left(-0.19427\ \mathrm{kip/ft}\right) \times \left(6\ \mathrm{ft}\right)\right)\right]}$$

$$p = 0.25523\ \mathrm{kip/ft^2}$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\,\left[\left(2\,M_o\right) + \left(H_o\,L_e\right)\right]}{L_e^2}$$

$$s = \frac{6 \times \left[\left(2 \times \left(3.2646\ \mathrm{kipft/ft}\right)\right) + \left(\left(-0.19427\ \mathrm{kip/ft}\right) \times \left(6\ \mathrm{ft}\right)\right)\right]}{\left(6\ \mathrm{ft}\right)^2}$$

$$s = 0.89395\ \mathrm{kip/ft^2}$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\,\frac{a}{2}$$

$$p_a = \left(150\ \mathrm{psf/ft}\right) \times \frac{\left(4.0961\ \mathrm{ft}\right)}{2}$$

$$p_a = 0.30721\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{\left(0.25523\ \mathrm{kip/ft^2}\right)}{\left(0.30721\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = 0.83081$$

Status: **PASS**
Ratio: **0.830**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\, L_e$$

$$p_s = (150 \text{ psf/ft}) \times (6 \text{ ft})$$

$$p_s = 0.9 \text{ kip/ft}^2$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{(0.89395 \text{ kip/ft}^2)}{(0.9 \text{ kip/ft}^2)}$$

$$Ratio = 0.99328$$

Status: **PASS**
Ratio: **0.990**

Considering z-direction:

$H_o = 0.0082803 \text{ kip/ft}$ - Lateral force per length of pile,
$M_o = 0.021815 \text{ kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\, M_o\, L_e) + (3\, H_o\, L_e^2)}{(6\, M_o) + (4\, H_o\, L_e)}$$

$$a = \frac{(4 \times (0.021815 \text{ kipft/ft}) \times (6 \text{ ft})) + \left(3 \times (0.0082803 \text{ kip/ft}) \times (6 \text{ ft})^2\right)}{(6 \times (0.021815 \text{ kipft/ft})) + (4 \times (0.0082803 \text{ kip/ft}) \times (6 \text{ ft}))}$$

$$a = 4.3014 \text{ ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\, [(4\, M_o) + (3\, H_o\, L_e)]^2}{L_e^2\, [(3\, M_o) + (2\, H_o\, L_e)]}$$

$$p = \frac{0.75 \times [(4 \times (0.021815 \text{ kipft/ft})) + (3 \times (0.0082803 \text{ kip/ft}) \times (6 \text{ ft}))]^{\,2}}{(6 \text{ ft})^2 \times [(3 \times (0.021815 \text{ kipft/ft})) + (2 \times (0.0082803 \text{ kip/ft}) \times (6 \text{ ft}))]}$$

$$p = 0.0070587 \text{ kip/ft}^2$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\, [(2\, M_o) + (H_o\, L_e)]}{L_e^2}$$

$$s = \frac{6 \times [(2 \times (0.021815 \text{ kipft/ft})) + ((0.0082803 \text{ kip/ft}) \times (6 \text{ ft}))]}{(6 \text{ ft})^2}$$

$$s = 0.015552 \text{ kip/ft}^2$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\, \frac{a}{2}$$

$$p_a = (150 \text{ psf/ft}) \times \frac{(4.3014 \text{ ft})}{2}$$

$$p_a = 0.32261 \text{ kip/ft}^2$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{(0.0070587 \text{ kip/ft}^2)}{(0.32261 \text{ kip/ft}^2)}$$

$$Ratio = 0.02188$$

Status: **PASS**
Ratio: **0.020**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\, L_e$$

$$p_s = (150 \text{ psf/ft}) \times (6 \text{ ft})$$

$$p_s = 0.9 \text{ kip/ft}^2$$

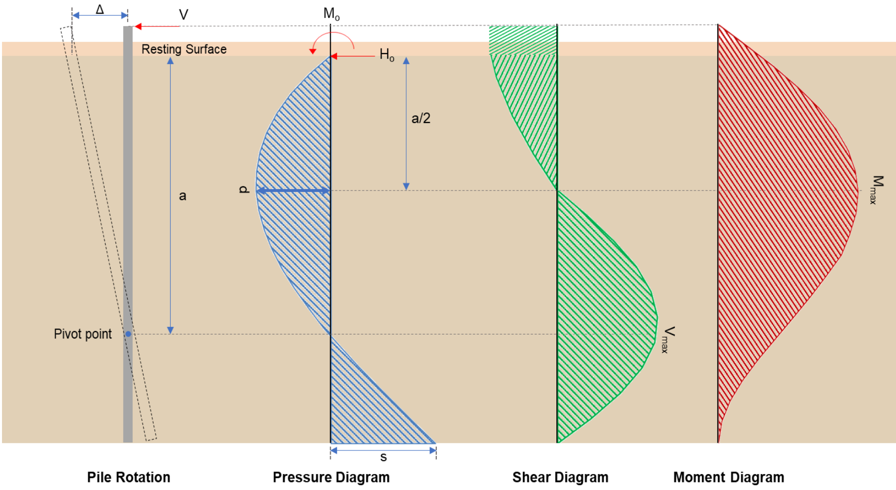
$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{(0.015552\ \mathrm{kip/ft^2})}{(0.9\ \mathrm{kip/ft^2})}$$

$$Ratio = 0.01728$$

Status: **PASS**
Ratio: **0.020**



Shear force and Bending moment (x-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\ D}$$

$$H_o = \frac{(-2.025\ \mathrm{kip})}{1.57 \times (48\ \mathrm{in})}$$

$$H_o = -0.32245\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\ H)}{1.57\ D}$$

$$M_o = \frac{(34.713\ \mathrm{kipft}) + ((-2.025\ \mathrm{kip}) \times (0\ \mathrm{ft}))}{1.57 \times (48\ \mathrm{in})}$$

$$M_o = 5.5275\ \mathrm{kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(5.5275\ \mathrm{kipft/ft})}{(-0.32245\ \mathrm{kip/ft})}$$

$$E = 17.142\ \mathrm{ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\ M_o\ L_e) + \left(3\ H_o\ L_e^2\right)}{(6\ M_o) + (4\ H_o\ L_e)}$$

$$a = \frac{(4 \times (5.5275\ \mathrm{kipft/ft}) \times (6\ \mathrm{ft})) + \left(3 \times (-0.32245\ \mathrm{kip/ft}) \times (6\ \mathrm{ft})^2\right)}{(6 \times (5.5275\ \mathrm{kipft/ft})) + (4 \times (-0.32245\ \mathrm{kip/ft}) \times (6\ \mathrm{ft}))}$$

$$a = 4.0946\ \mathrm{ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\ D)\left[1 - \left[3\left(\frac{4\ E}{L_e} + 3\right)\left(\frac{a}{L_e}\right)^2\right] + \left[4\left(\frac{3\ E}{L_e} + 2\right)\left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((-0.32245\ \mathrm{kip/ft}) \times (48\ \mathrm{in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (17.142\ \mathrm{ft})}{(6\ \mathrm{ft})} + 3\right) \times \left(\frac{(4.0946\ \mathrm{ft})}{(6\ \mathrm{ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (17.142\ \mathrm{ft})}{(6\ \mathrm{ft})} + 2\right) \times \left(\frac{(4.0946\ \mathrm{ft})}{(6\ \mathrm{ft})}\right)^3\right]\right]$$

$$V_{max} = 7.3769\ \mathrm{kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, D\, L_e) \left[\left(\frac{E}{L_e} + \frac{a}{2\, L_e} \right) - \left[\left(\frac{4\, E}{L_e} + 3 \right) \left(\frac{a}{2\, L_e} \right)^3 \right] + \left[\left(\frac{3\, E}{L_e} + 2 \right) \left(\frac{a}{2\, L_e} \right)^4 \right] \right]$$

$$M_{max} = ((-0.32245 \text{ kip/ft}) \times (48 \text{ in}) \times (6 \text{ ft})) \times \left[\left(\frac{(17.142 \text{ ft})}{(6 \text{ ft})} + \frac{(4.0946 \text{ ft})}{2 \times (6 \text{ ft})} \right) - \left[\left(\frac{4 \times (17.142 \text{ ft})}{(6 \text{ ft})} + 3 \right) \times \left(\frac{(4.0946 \text{ ft})}{2 \times (6 \text{ ft})} \right)^3 \right] + \left[\left(\frac{3 \times (17.142 \text{ ft})}{(6 \text{ ft})} + 2 \right) \times \left(\frac{(4.0946 \text{ ft})}{2 \times (6 \text{ ft})} \right)^4 \right] \right]$$

$$M_{max} = 21.424 \text{ kipft}$$

Shear force and Bending moment (z-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\, b}$$

$$H_o = \frac{(0.08 \text{ kip})}{1.57 \times (48 \text{ in})}$$

$$H_o = 0.012739 \text{ kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\, H)}{1.57\, b}$$

$$M_o = \frac{(0.21 \text{ kipft}) + ((0.08 \text{ kip}) \times (0 \text{ ft}))}{1.57 \times (48 \text{ in})}$$

$$M_o = 0.033439 \text{ kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(0.033439 \text{ kipft/ft})}{(0.012739 \text{ kip/ft})}$$

$$E = 2.625 \text{ ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\, M_o\, L_e) + \left(3\, H_o\, L_e^2\right)}{(6\, M_o) + (4\, H_o\, L_e)}$$

$$a = \frac{(4 \times (0.033439 \text{ kipft/ft}) \times (6 \text{ ft})) + \left(3 \times (0.012739 \text{ kip/ft}) \times (6 \text{ ft})^2\right)}{(6 \times (0.033439 \text{ kipft/ft})) + (4 \times (0.012739 \text{ kip/ft}) \times (6 \text{ ft}))}$$

$$a = 4.3019 \text{ ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\, b) \left[1 - \left[3 \left(\frac{4\, E}{L_e} + 3 \right) \left(\frac{a}{L_e} \right)^2 \right] + \left[4 \left(\frac{3\, E}{L_e} + 2 \right) \left(\frac{a}{L_e} \right)^3 \right] \right]$$

$$V_{max} = ((0.012739 \text{ kip/ft}) \times (48 \text{ in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (2.625 \text{ ft})}{(6 \text{ ft})} + 3 \right) \times \left(\frac{(4.3019 \text{ ft})}{(6 \text{ ft})} \right)^2 \right] + \left[4 \times \left(\frac{3 \times (2.625 \text{ ft})}{(6 \text{ ft})} + 2 \right) \times \left(\frac{(4.3019 \text{ ft})}{(6 \text{ ft})} \right)^3 \right] \right]$$

$$V_{max} = 0.073467 \text{ kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, b\, L_e) \left[\left(\frac{E}{L_e} + \frac{a}{2\, L_e} \right) - \left[\left(\frac{4\, E}{L_e} + 3 \right) \left(\frac{a}{2\, L_e} \right)^3 \right] + \left[\left(\frac{3\, E}{L_e} + 2 \right) \left(\frac{a}{2\, L_e} \right)^4 \right] \right]$$

$$M_{max} = ((0.012739 \text{ kip/ft}) \times (48 \text{ in}) \times (6 \text{ ft})) \times \left[\left(\frac{(2.625 \text{ ft})}{(6 \text{ ft})} + \frac{(4.3019 \text{ ft})}{2 \times (6 \text{ ft})} \right) - \left[\left(\frac{4 \times (2.625 \text{ ft})}{(6 \text{ ft})} + 3 \right) \times \left(\frac{(4.3019 \text{ ft})}{2 \times (6 \text{ ft})} \right)^3 \right] + \left[\left(\frac{3 \times (2.625 \text{ ft})}{(6 \text{ ft})} + 2 \right) \times \left(\frac{(4.3019 \text{ ft})}{2 \times (6 \text{ ft})} \right)^4 \right] \right]$$

$$M_{max} = 0.19318 \text{ kipft}$$

Minimum Reinforcement Check (LRFD)

Parameters:

$f'_{ck} = 3 \text{ ksi}$ - Concrete strength,
$f_{yk} = 60 \text{ ksi}$ - Longitudinal reinforcement strength,
$\phi = 0.65$ - Reduction factor for axial strength,
Table 22.4.2.1 $\alpha = 0.8$ - Alpha factor for axial strength,
$A_g = 2304 \text{ in}^2$ - Gross area of concrete,

Longitudinal reinforcement:

Required reinforcement due to axial load, $A_{st,required}$
22.4.2.2, 10.6.1.1 $A_{st,required}$

$$A_{st,required} = Min\left[\frac{\frac{P}{\phi\ \alpha} - (0.85\ f'_{ck}\ A_g)}{f_{yk} - (0.85\ f'_{ck})}, (0.08\ A_g)\right]$$

$$A_{st,required} = Min\left[\frac{\frac{(14.883 \text{ kip})}{(0.65)\times(0.8)} - \left(0.85 \times (3 \text{ ksi}) \times (2304 \text{ in}^2)\right)}{(60 \text{ ksi}) - (0.85 \times (3 \text{ ksi}))}, \left(0.08 \times (2304 \text{ in}^2)\right)\right]$$

$$A_{st,required} = -101.77 \text{ in}^2$$

A_{min} - Governing minimum reinforcement area,

$$A_{min} = Max\left[A_{st,required}, (0.0018\ A_g)\right]$$

$$A_{min} = Max\left[\left(-101.77 \text{ in}^2\right), \left(0.0018 \times (2304 \text{ in}^2)\right)\right]$$

$$A_{min} = 4.1472 \text{ in}^2$$

n_{rebar} - Required number of reinforcement,

$$n_{rebar} = \frac{A_{min}}{A_{rebar}}$$

$$n_{rebar} = \frac{(4.1472 \text{ in}^2)}{(0.3068 \text{ in}^2)}$$

$$n_{rebar} = 14$$

A_{st} - Actual total reinforcement area,

$$A_{st} = n_{rebar}\ \frac{\pi\ d_{bar}^2}{4}$$

$$A_{st} = (14) \times \frac{\pi \times (0.625 \text{ in})^2}{4}$$

$$A_{st} = 4.2951 \text{ in}^2$$

$Ratio$ - Capacity

$$Ratio = \frac{A_{min}}{A_{st}}$$

$$Ratio = \frac{(4.1472 \text{ in}^2)}{(4.2951 \text{ in}^2)}$$

$$Ratio = 0.96556$$

Status: **PASS**
Ratio: **0.970**

25.2.3 s_{rebar} - Minimum spacing of reinforcement,

$$s_{rebar} = Max\left[1.5, (1.5\ d_{bar})\right]$$

$$s_{rebar} = Max\left[1.5, (1.5 \times (0.625 \text{ in}))\right]$$

$$s_{rebar} = 1.5 \text{ in}$$

Ties:

25.7.2.2 Since longitudinal reinforcement is $\leq$ No. 10ø: Use #3(0.375 in)
25.7.2.1 s_{ties} - Maximum spacing of ties,

$$s_{ties} = Min\left[(16\ d_{bar}), (48\ d_{ties}), Min(D, b)\right]$$

$$s_{ties} = Min\left[(16 \times (0.625 \text{ in})), (48 \times (0.375 \text{ in})), Min((48 \text{ in}), (48 \text{ in}))\right]$$

$$s_{ties} = 10 \text{ in}$$

Summary:

Main reinforcement: **14 - #5 (0.625 in)**

	Ties: **#3(0.375 in) - 10 in**	

Axial Compression Strength (ACI 318-19, LRFD)

22.4.2.2 ϕP_N - Allowable axial compressive strength

$$\phi P_N = \phi \, 0.80 \, \left[\left(0.85 \, f'_{ck} \, \left[A_g - A_{st} \right] \right) + \left(f_{yk} \, A_{st} \right) \right]$$

$$\phi P_N = (0.65) \times 0.80 \times \left[\left(0.85 \times (3 \text{ ksi}) \times \left[\left(2304 \text{ in}^2 \right) - \left(4.2951 \text{ in}^2 \right) \right] \right) + \left((60 \text{ ksi}) \times \left(4.2951 \text{ in}^2 \right) \right) \right]$$

$$\phi P_N = 3183.4 \text{ kip}$$

$Ratio$ - Capacity

$$Ratio = \frac{P}{\phi P_N}$$

$$Ratio = \frac{(14.883 \text{ kip})}{(3183.4 \text{ kip})}$$

$$Ratio = 0.0046752$$

Status: **PASS**
Ratio: **0.000**

Shear Strength (ACI 318-19, LRFD)

Parameters:

$b_w = 48 \text{ in}$ - Effective width,

22.5.2.2 d - Effective depth

$$d = 0.80 \, D$$

$$d = 0.80 \times (48 \text{ in})$$

$$d = 38.4 \text{ in}$$

22.5.5.1.3 λ_s - size effect modification factor

$$\lambda_s = MIN \left[\sqrt{\frac{2}{1 + \frac{d}{10}}}, 1 \right]$$

$$\lambda_s = MIN \left[\sqrt{\frac{2}{1 + \frac{(38.4 \text{ in})}{10}}}, 1 \right]$$

$$\lambda_s = 0.64282$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 3 \text{ ksi} \rightarrow 3000 \text{ psi}$,

22.5.5.1.1 $V_{c,max}$ - Max shear strength of concrete

$$V_{c,max} = 5 \, \lambda_s \, \sqrt{f'_{ck}} \, b_w \, d$$

$$V_{c,max} = 5 \times (0.64282) \times \sqrt{(3000 \text{ psi})} \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{c,max} = 324.49 \text{ kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 3 \text{ ksi} \rightarrow 3000 \text{ psi}$,$P = 14.883 \text{ kip} \rightarrow 14883 \text{ lbf}$,

22.5.5.1.1(a) $V_{c,a}$ - Shear strength of concrete (a)

$$V_{c,a} = \left[2 \, \lambda_s \, \sqrt{f'_{ck}} + \frac{P}{6 \, A_g} \right] \, b_w \, d$$

$$V_{c,a} = \left[2 \times (0.64282) \times \sqrt{(3000 \text{ psi})} + \frac{(14883 \text{ lbf})}{6 \times \left(2304 \text{ in}^2 \right)} \right] \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{c,a} = 131.78 \text{ kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 3 \text{ ksi} \rightarrow 3000 \text{ psi}$,

22.5.5.1.2 $V_{c,b}$ - Shear strength of concrete (b)

$$V_{c,b} = \left[2 \, \lambda_s \, \sqrt{f'_{ck}} + \left(0.05 \, f'_{ck} \right) \right] \, b_w \, d$$

$$V_{c,b} = \left[2 \times (0.64282) \times \sqrt{(3000 \text{ psi})} + (0.05 \times (3000 \text{ psi})) \right] \times (48 \text{ in}) \times (38.4 \text{ in})$$

$$V_{c,b} = 406.27 \text{ kip}$$

V_c - Governing shear strength of concrete

$$V_c = Min \left[V_{c,max}, V_{c,a}, V_{c,b} \right]$$

$$V_c = Min \left[(324.49 \text{ kip}), (131.78 \text{ kip}), (406.27 \text{ kip}) \right]$$

$$V_c = 131.78 \text{ kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 3\ \text{ksi} \rightarrow 3000\ \text{psi}$,

22.5.1.2 $V_{s,a}$ - Shear strength of steel (a)

$$V_{s,a} = 8\sqrt{f'_{ck}}\, b_w\, d$$

$$V_{s,a} = 8 \times \sqrt{(3000\ \text{psi})} \times (48\ \text{in}) \times (38.4\ \text{in})$$

$$V_{s,a} = 807.65\ \text{kip}$$

A_v - Ties rebar area,

$$A_v = \frac{\pi\, d_{ties}^2}{4}$$

$$A_v = \frac{\pi \times (0.375\ \text{in})^2}{4}$$

$$A_v = 0.11045\ \text{in}^2$$

22.5.8.5.3 $V_{s,b}$ - Shear strength of steel (b)

$$V_{s,b} = \frac{2\, A_v\, f_{ywk}\, d}{s_{ties}}$$

$$V_{s,b} = \frac{2 \times \left(0.11045\ \text{in}^2\right) \times (60\ \text{ksi}) \times (38.4\ \text{in})}{(10\ \text{in})}$$

$$V_{s,b} = 50.894\ \text{kip}$$

V_s - Governing shear strength of steel

$$V_s = MIN\left[V_{s,a}, V_{s,b}\right]$$

$$V_s = MIN\left[(807.65\ \text{kip}), (50.894\ \text{kip})\right]$$

$$V_s = 50.894\ \text{kip}$$

22.5.1.1 ϕV_n - Allowable shear strength

$$\phi V_n = \phi\ (V_c + V_s)$$

$$\phi V_n = (0.65) \times ((131.78\ \text{kip}) + (50.894\ \text{kip}))$$

$$\phi V_n = 118.74\ \text{kip}$$

Considering x-direction:

$V_{max} = 7.3769\ \text{kip}$ - Maximum shear force in the x-direction,
$Ratio$ - Capacity

$$Ratio = \frac{V_{max}}{\phi V_n}$$

$$Ratio = \frac{(7.3769\ \text{kip})}{(118.74\ \text{kip})}$$

$$Ratio = 0.062128$$

Status: **PASS**
Ratio: **0.060**

Considering z-direction:

$V_{max} = 0.073467\ \text{kip}$ - Maximum shear force in the z-direction,
$Ratio$ - Capacity

$$Ratio = \frac{V_{max}}{\phi V_n}$$

$$Ratio = \frac{(0.073467\ \text{kip})}{(118.74\ \text{kip})}$$

$$Ratio = 0.00061874$$

Status: **PASS**
Ratio: **0.000**

Flexural Strength (ACI 318-19, LRFD)

S_m - Section modulus

$$S_m = \frac{b\, D^2}{6}$$

$$S_m = \frac{(48\ \text{in}) \times (48\ \text{in})^2}{6}$$

$$S_m = 18432\ \text{in}^3$$

	$\lambda = 1$ - Concrete modification factor (Normal concrete), Allowable flexural strength: M_n shall be the lesser of: $\phi M_{n,1}$ $$\phi M_{n,1} = \phi \times 5 \times \lambda \times \sqrt{(f_c')} \times S_m$$ $$\phi M_{n,1} = 0.65 \times 5 \times 1 \times \sqrt{(3ksi)} \times 18432.001in^3$$ $$\phi M_{n,1} = 273.423kipft$$	
14.5.2.1b	$\phi M_{n,2}$ $$\phi M_{n,2} = \phi\, 0.85\, f_{ck}'\, S_m$$ $$\phi M_{n,2} = (0.65) \times 0.85 \times (3 \text{ ksi}) \times \left(18432 \text{ in}^3\right)$$ $$\phi M_{n,2} = 2545.9 \text{ kipft}$$ Therefore, ϕM_n - Allowable flexural strength, $$\phi M_n = MIN[\phi M_{n,1}, \phi M_{n,2}]$$ $$\phi M_n = MIN[(273.42 \text{ kipft}), (2545.9 \text{ kipft})]$$ $$\phi M_n = 273.42 \text{ kipft}$$ **Considering x-direction:** $M_{max} = 21.424 \text{ kipft}$ - Maximum moment in the x-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\phi M_n}$$ $$Ratio = \frac{(21.424 \text{ kipft})}{(273.42 \text{ kipft})}$$ $$Ratio = 0.078354$$	Status: **PASS** Ratio: **0.080**
	Considering z-direction: $M_{max} = 0.19318 \text{ kipft}$ - Maximum moment in the z-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\phi M_n}$$ $$Ratio = \frac{(0.19318 \text{ kipft})}{(273.42 \text{ kipft})}$$ $$Ratio = 0.00070653$$	Status: **PASS** Ratio: **0.000**