Your Project Calculations

Project Name: 5x8-070824-struts-cu

S3D Model Link:
https://platform.skyciv.com/structural?preload_name=5x8-070824-struts-cu&preload_path=Shared%20Enterprise%20Folder/MT_Solar_Projects/7_2024

Public Model Link:
https://platform.skyciv.com/structural-viewer?project_id=gU1OO3dMHDMbrchUUNChCdP8EPSuC3phBFhkOPrtWQYbzAY6OGqtJ7xjYRBlLzky

Array Specification

Product:	Beam
Unique ID:	3P-17-8TOP-SD-45-L-5Hx8W-STRUTS-06F8
Duty Classification:	SD
Module Width:	41.10 in
Module Length:	74.00in
Number of Rows:	5
Number of Columns:	8
Total Number of Modules:	40
Desired Tilt Angle:	47
Front Edge Clearance:	10
Total Array Height at Tilt:	22.60 ft
Total Frame Length:	49.00 ft
Frame Weight:	2951 lbs
Array Dimensions N/S:	17.33 ft
Array Dimensions E/W:	50.00 ft
Rail Length:	208.00 in
Rail Spacing:	3.08 ft
Rail Check:	Not Checked

Support Specifications

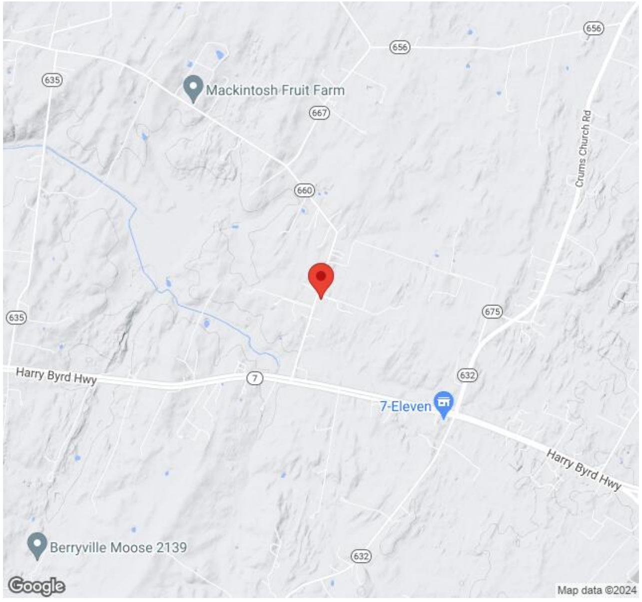
Pole Size:	8in Pipe Sch 40
Pole Length above Grade:	16.34 ft
Number of Poles:	3
Pole Spacing:	17 ft

Foundation Specifications

Foundation Type:	Square
Foundation Dimensions:	48 x 48 in
Foundation Depth (below grade):	Pile 1: 7.00 ft Pile 2: 7.00 ft Pile 3: 7.00 ft
Foundation Volume:	12.444 y^3
Foundation Result:	PASSED
Mount Twist:	0.145902 kip

Site Info

Risk Category:	I
Exposure:	B
Soil Classification:	sand
Site Location:	390 Russell Rd, Berryville, VA 22611, USA
Wind Speed:	100 mph
Snow Load:	30 psf
Design Uplift Pressure:	0.015295 ksf
Design Downforce Pressure:	-0.015295 ksf
Design Snow Pressure:	0.007587 ksf



Design Disclaimer

This software should be used for preliminary designs and should not be used as a final design unless reviewed, verified and designed by a qualified structural engineer.

AutoDesigner Input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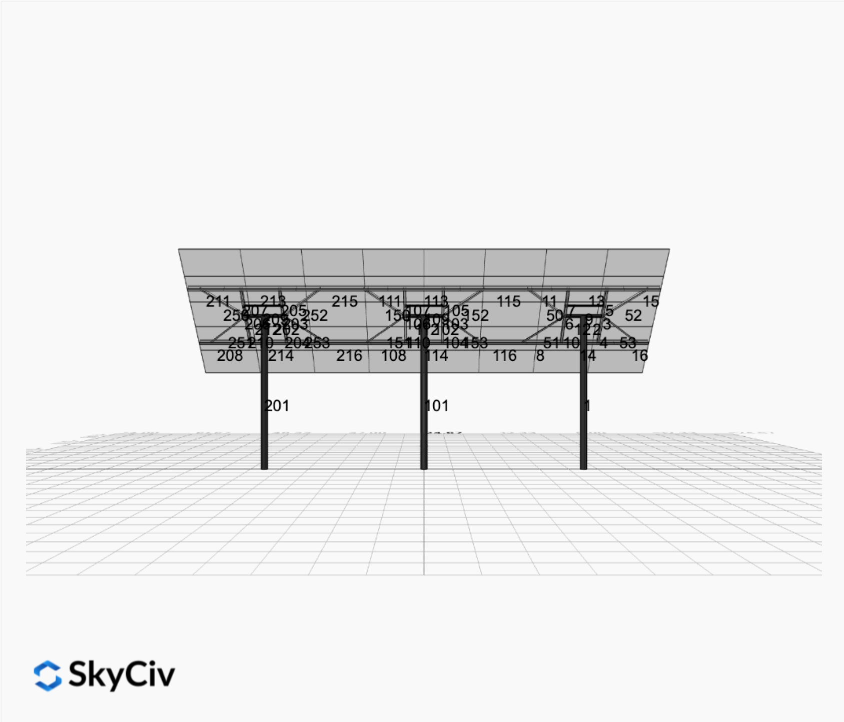
Design Notes:

- AISC Deflection checks are set to L/1 due to structure design intent
- Foundation Soil Parameters used in this Autodesign are all estimates, proper geotechnical reports are required to confirm soil profiles
- Wind speeds, snow loads and other site specific results are based on ASCE 7 2016
- Steel frame design checks are based on AISC 360 2016 (LRFD)
- Foundation Design and Sizing is approximate only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14
16
201
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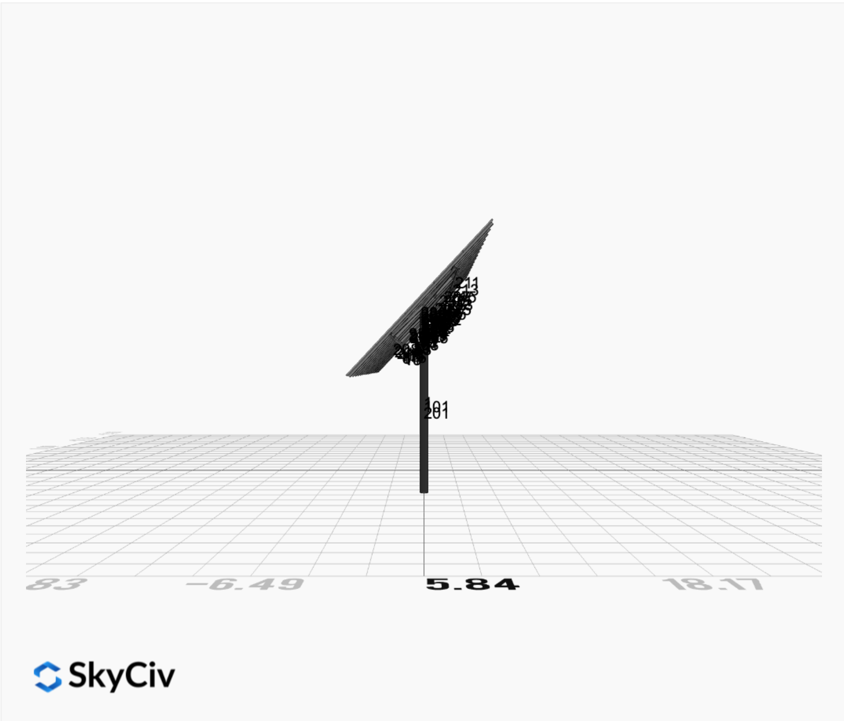
SkyCiv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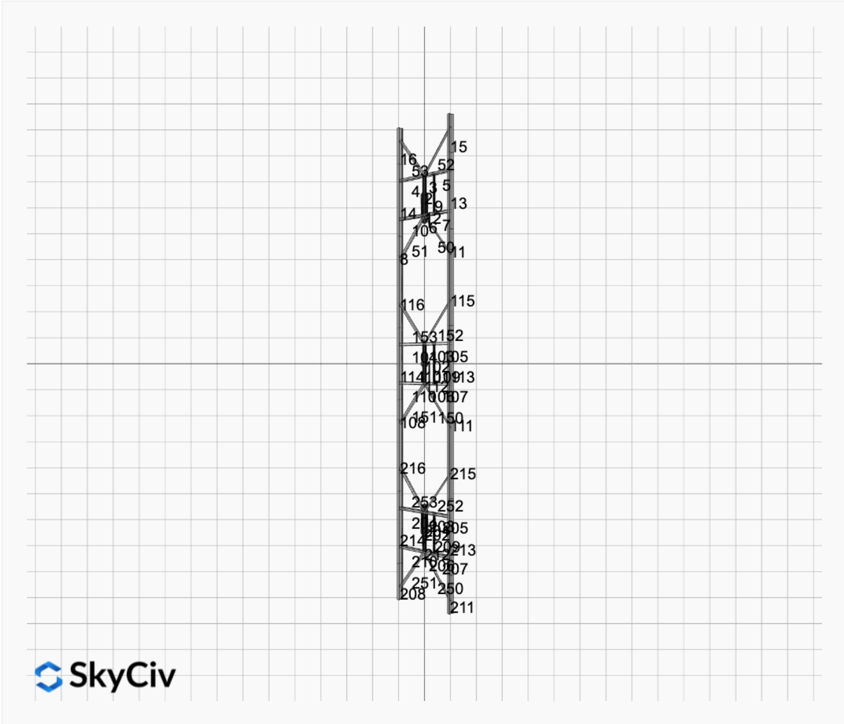
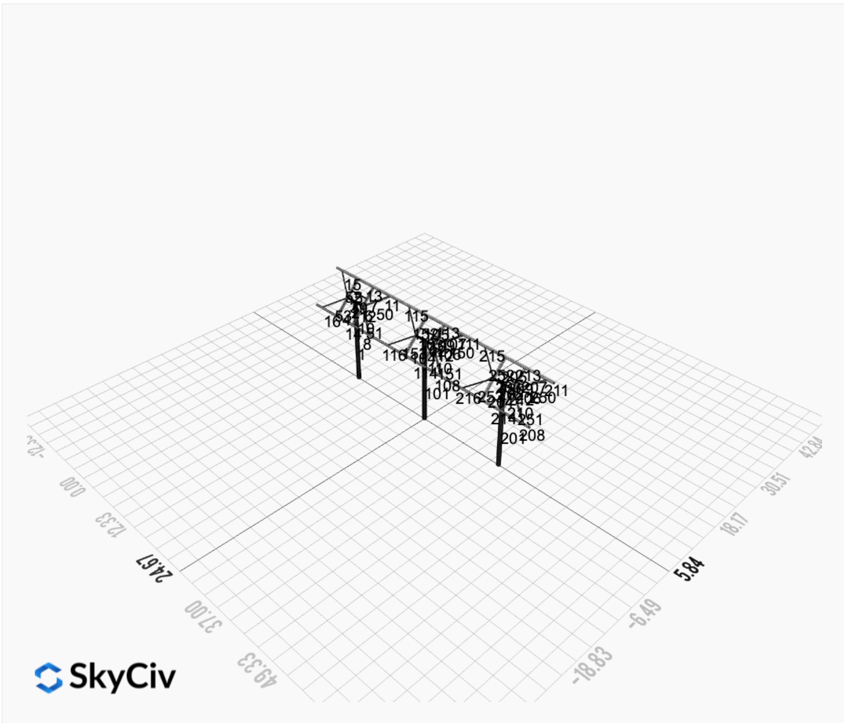
18.17
SkyCiv

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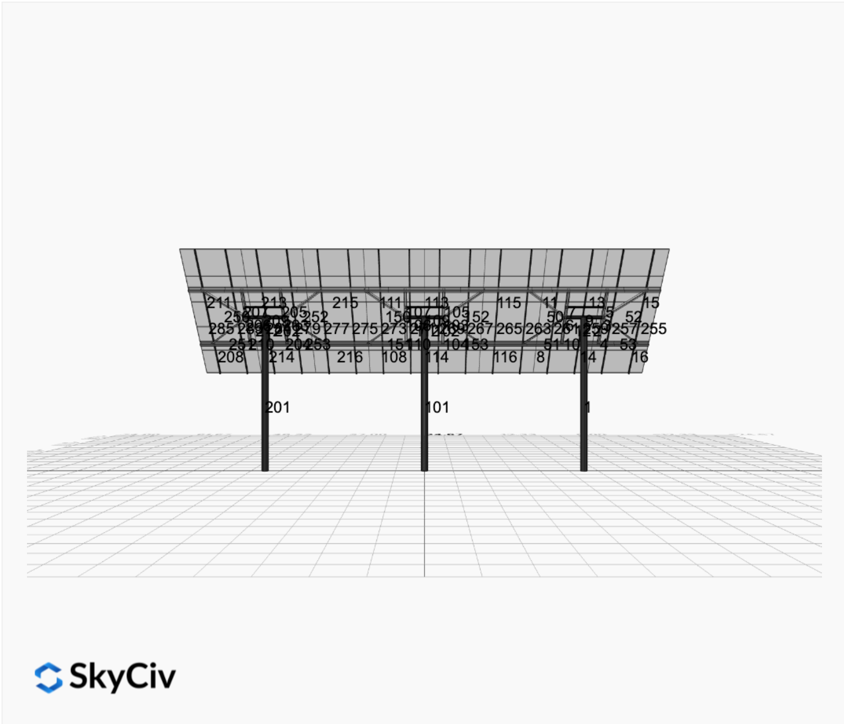
37.00
49.33
42.84
30.51
18.17
5.84
-6.49
-18.83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108
114
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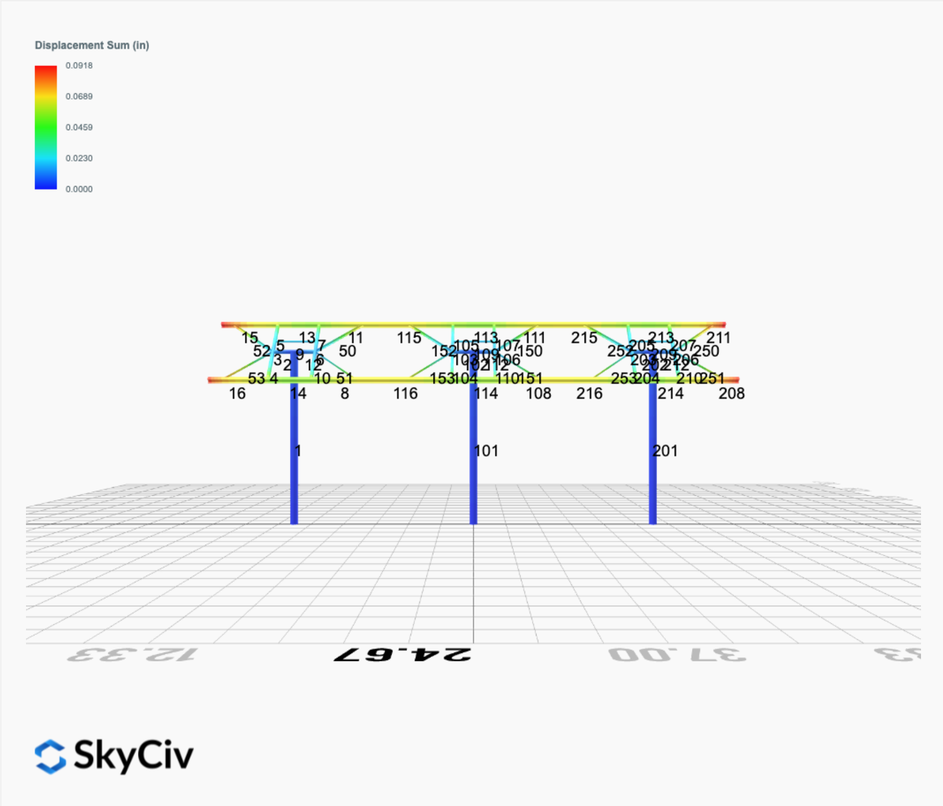
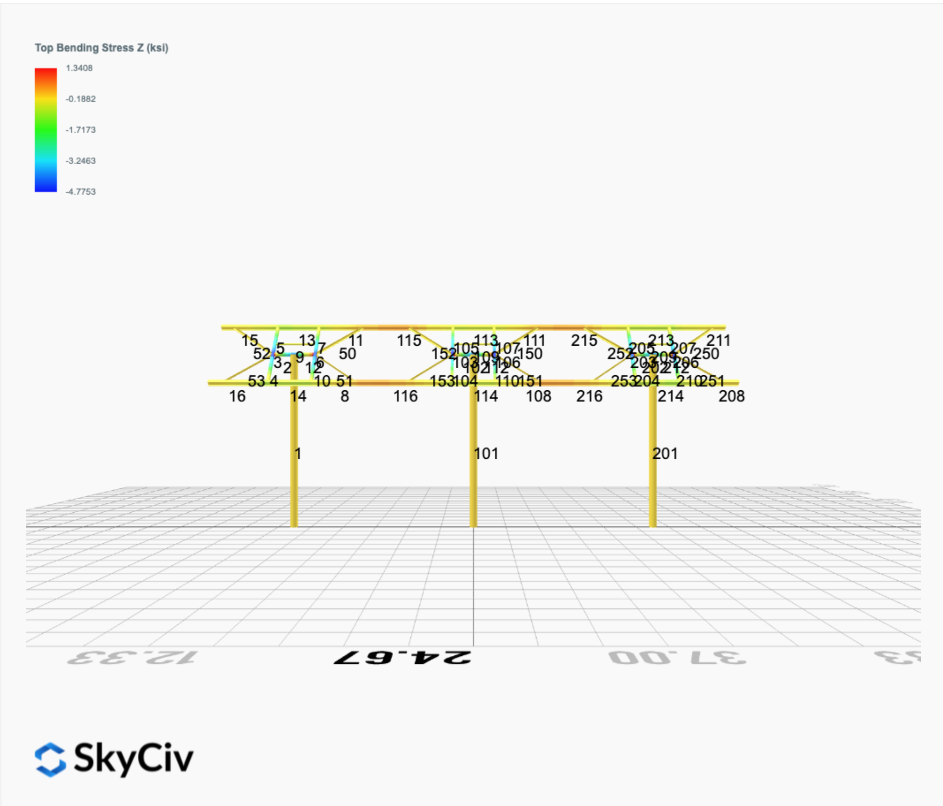
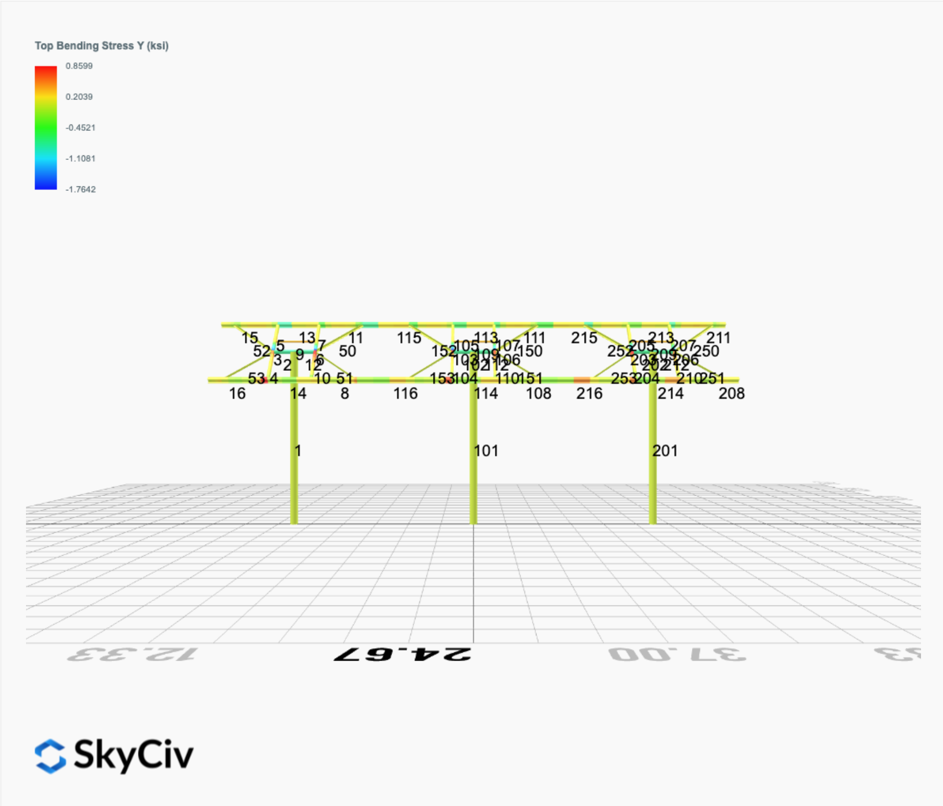
8
14
16
201
101
SkyCiv

FEM Results (Envelope Worst Case for each member)






Top Bending Stress Y (ksi)
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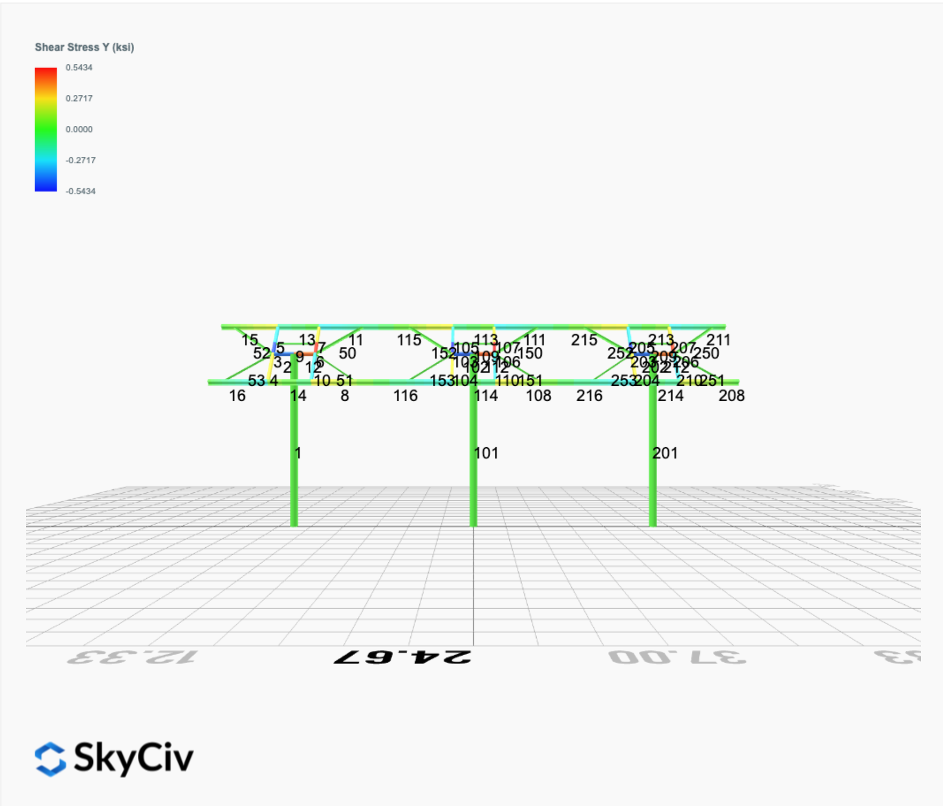
0.2039
-0.4521
-1.1081
-1.7642
SkyCiv


Shear Stress Y (ksi)
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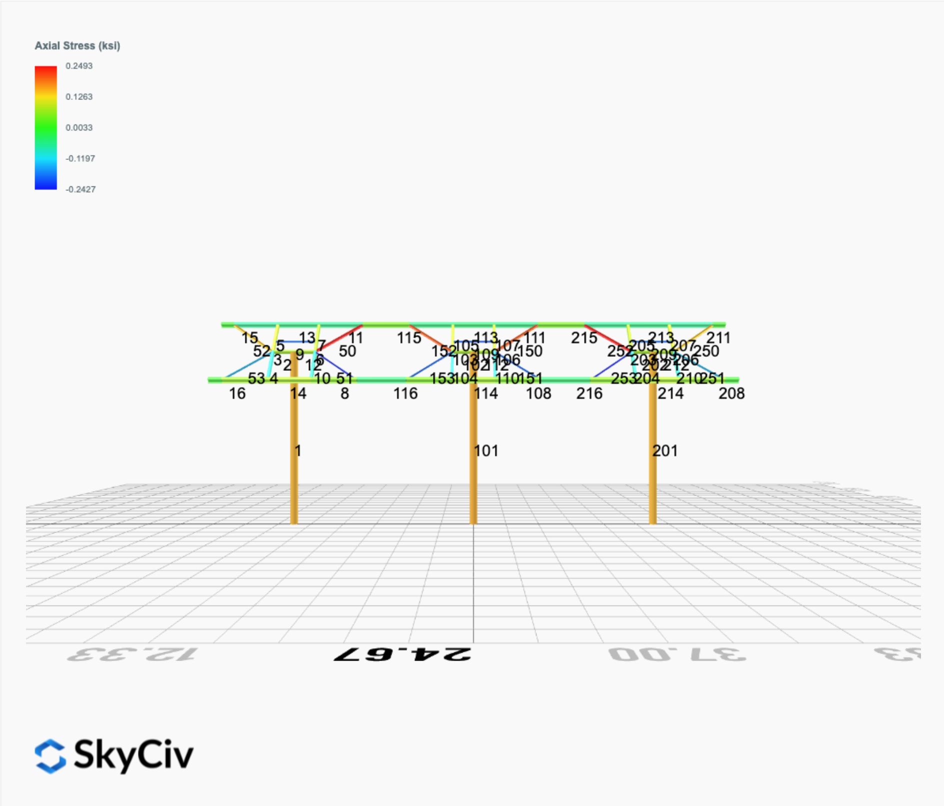
0.2717
0.0000
-0.2717
-0.5434
SkyCiv


Axial Stress (ksi)
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0.0033
-0.1197
-0.2427
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SkyCiv

Reaction Forces for Foundation 1 (Node ID#1), (kip, kip-ft)

ASD Load Combination Results

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 1. D	-0.0015	2.2818	-0.0027	-0.0130	0.0464	0.0399
ULS: 2. D + L	-0.0015	2.2818	-0.0027	-0.0130	0.0464	0.0399
ULS: 3. D + (S or Lr or R)	-0.0027	3.7292	-0.0044	-0.0208	0.0841	0.0617
ULS: 3. D + (S or Lr or R)	-0.0015	2.2818	-0.0027	-0.0130	0.0464	0.0399
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	-0.0024	3.3673	-0.0040	-0.0188	0.0747	0.0563
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	-0.0015	2.2818	-0.0027	-0.0130	0.0464	0.0399
ULS: 5b. D + 0.7E	-0.0015	2.2818	-0.0027	-0.0130	0.0464	0.0399
ULS: 6b. D + 0.75L + 0.75(0.7)E + 0.75S	-0.0024	3.3673	-0.0040	-0.0188	0.0747	0.0563
ULS: 8. 0.6D + 0.7E	-0.0009	1.3691	-0.0016	-0.0078	0.0278	0.0240
ULS: 5a. D + 0.6W_Wind downforce Case A only	-1.9382	4.0593	-0.0107	-0.0548	0.0800	32.2713
ULS: 5a. D + 0.6W_Wind downforce Case B only	-0.0015	2.2818	-0.0027	-0.0130	0.0464	0.0399
ULS: 5a. D + 0.6W_Wind uplift Case A only	1.9337	0.5046	0.0054	0.0292	0.0118	-31.0246
ULS: 5a. D + 0.6W_Wind uplift Case B only	-0.0015	2.2818	-0.0027	-0.0130	0.0464	0.0399
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-1.4549	4.7005	-0.0100	-0.0502	0.0999	24.2298
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-0.0024	3.3673	-0.0040	-0.0188	0.0747	0.0563
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	1.4490	2.0345	0.0021	0.0128	0.0488	-23.2421
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	-0.0024	3.3673	-0.0040	-0.0188	0.0747	0.0563
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-1.4540	3.6149	-0.0087	-0.0443	0.0716	24.2135
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-0.0015	2.2818	-0.0027	-0.0130	0.0464	0.0399
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	1.4499	0.9489	0.0034	0.0186	0.0205	-23.2584
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	-0.0015	2.2818	-0.0027	-0.0130	0.0464	0.0399
ULS: 7. 0.6D + 0.6W_Wind downforce Case A only	-1.9376	3.1466	-0.0096	-0.0496	0.0615	32.2554
ULS: 7. 0.6D + 0.6W_Wind downforce Case B only	-0.0009	1.3691	-0.0016	-0.0078	0.0278	0.0240
ULS: 7. 0.6D + 0.6W_Wind uplift Case A only	1.9343	-0.4081	0.0065	0.0344	-0.0067	-31.0405
ULS: 7. 0.6D + 0.6W_Wind uplift Case B only	-0.0009	1.3691	-0.0016	-0.0078	0.0278	0.0240

Worst Case Reactions LRFD

These calculations are taken directly from the FEA via SkyCiv and are used in the Concrete Checks of the Foundatino Module. Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	6.5351
Shear X	-3.2311
Shear Z	-0.0179
Moment X	-0.0916
Moment Y (Twist)	0.1459
Moment Z	55.0383

Worst Case Reactions ASD

These results are taken from the worst case values in the above table and are used in the Soil Checks in the Foundation Module. Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	4.7005
Shear X	-1.9382
Shear Z	-0.0107
Moment X	-0.0548
Moment Y (Twist)	0.0999
Moment Z	32.2713

Reaction Forces for Foundation 2 (Node ID#101), (kip, kip-ft)

ASD Load Combination Results

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 1. D	0.0030	2.3344	-0.0000	0.0000	0.0000	-0.0251
ULS: 2. D + L	0.0030	2.3344	-0.0000	0.0000	0.0000	-0.0251
ULS: 3. D + (S or Lr or R)	0.0055	3.8346	-0.0000	0.0000	0.0000	-0.0548
ULS: 3. D + (S or Lr or R)	0.0030	2.3344	-0.0000	0.0000	0.0000	-0.0251
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	0.0049	3.4596	-0.0000	0.0000	0.0000	-0.0474
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	0.0030	2.3344	-0.0000	0.0000	0.0000	-0.0251
ULS: 5b. D + 0.7E	0.0030	2.3344	-0.0000	0.0000	0.0000	-0.0251

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 6b. D + 0.75L + 0.75(0.7)E + 0.75S	0.0049	3.4596	-0.0000	0.0000	0.0000	-0.0474
ULS: 8. 0.6D + 0.7E	0.0018	1.4007	-0.0000	0.0000	0.0000	-0.0150
ULS: 5a. D + 0.6W_Wind downforce Case A only	-1.9403	4.2035	-0.0000	0.0000	0.0000	32.3333
ULS: 5a. D + 0.6W_Wind downforce Case B only	0.0030	2.3344	-0.0000	0.0000	0.0000	-0.0251
ULS: 5a. D + 0.6W_Wind uplift Case A only	1.9493	0.4646	-0.0000	0.0000	0.0000	-31.2244
ULS: 5a. D + 0.6W_Wind uplift Case B only	0.0030	2.3344	-0.0000	0.0000	0.0000	-0.0251
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-1.4527	4.8614	-0.0000	0.0000	0.0000	24.2214
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	0.0049	3.4596	-0.0000	0.0000	0.0000	-0.0474
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	1.4646	2.0572	-0.0000	0.0000	0.0000	-23.4468
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	0.0049	3.4596	-0.0000	0.0000	0.0000	-0.0474
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-1.4545	3.7363	-0.0000	0.0000	0.0000	24.2437
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	0.0030	2.3344	-0.0000	0.0000	0.0000	-0.0251
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	1.4627	0.9321	-0.0000	0.0000	0.0000	-23.4245
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	0.0030	2.3344	-0.0000	0.0000	0.0000	-0.0251
ULS: 7. 0.6D + 0.6W_Wind downforce Case A only	-1.9415	3.2698	-0.0000	0.0000	0.0000	32.3434
ULS: 7. 0.6D + 0.6W_Wind downforce Case B only	0.0018	1.4007	-0.0000	0.0000	0.0000	-0.0150
ULS: 7. 0.6D + 0.6W_Wind uplift Case A only	1.9481	-0.4692	-0.0000	0.0000	0.0000	-31.2143
ULS: 7. 0.6D + 0.6W_Wind uplift Case B only	0.0018	1.4007	-0.0000	0.0000	0.0000	-0.0150

Worst Case Reactions LRFD

These calculations are taken directly from the FEA via SkyCiv and are used in the Concrete Checks of the Foundatino Module. Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	6.7594
Shear X	-3.2473
Shear Z	-0.0000
Moment X	0.0001
Moment Y (Twist)	0.0000
Moment Z	55.1424

Worst Case Reactions ASD

These results are taken from the worst case values in the above table and are used in the Soil Checks in the Foundation Module. Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	4.8614
Shear X	-1.9493
Shear Z	-0.0000
Moment X	0.0000
Moment Y (Twist)	0.0000
Moment Z	32.3434

Reaction Forces for Foundation 3 (Node ID#201), (kip, kip-ft)

ASD Load Combination Results

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 1. D	-0.0015	2.2818	0.0027	0.0130	-0.0464	0.0399
ULS: 2. D + L	-0.0015	2.2818	0.0027	0.0130	-0.0464	0.0399
ULS: 3. D + (S or Lr or R)	-0.0027	3.7292	0.0044	0.0208	-0.0841	0.0617
ULS: 3. D + (S or Lr or R)	-0.0015	2.2818	0.0027	0.0130	-0.0464	0.0399
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	-0.0024	3.3673	0.0040	0.0189	-0.0747	0.0563
ULS: 4. D + 0.75L + 0.75(S or Lr or R)	-0.0015	2.2818	0.0027	0.0130	-0.0464	0.0399
ULS: 5b. D + 0.7E	-0.0015	2.2818	0.0027	0.0130	-0.0464	0.0399
ULS: 6b. D + 0.75L + 0.75(0.7)E + 0.75S	-0.0024	3.3673	0.0040	0.0189	-0.0747	0.0563
ULS: 8. 0.6D + 0.7E	-0.0009	1.3691	0.0016	0.0078	-0.0278	0.0240
ULS: 5a. D + 0.6W_Wind downforce Case A only	-1.9382	4.0593	0.0107	0.0548	-0.0800	32.2714
ULS: 5a. D + 0.6W_Wind downforce Case B only	-0.0015	2.2818	0.0027	0.0130	-0.0464	0.0399
ULS: 5a. D + 0.6W_Wind uplift Case A only	1.9337	0.5046	-0.0054	-0.0291	-0.0118	-31.0245
ULS: 5a. D + 0.6W_Wind uplift Case B only	-0.0015	2.2818	0.0027	0.0130	-0.0464	0.0399
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-1.4549	4.7005	0.0100	0.0502	-0.0999	24.2299
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-0.0024	3.3673	0.0040	0.0189	-0.0747	0.0563
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	1.4490	2.0345	-0.0021	-0.0128	-0.0488	-23.2421
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	-0.0024	3.3673	0.0040	0.0189	-0.0747	0.0563

Name	Fx	Fy	Fz	Mx	My	Mz
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case A only	-1.4540	3.6149	0.0087	0.0443	-0.0716	24.2135
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind downforce Case B only	-0.0015	2.2818	0.0027	0.0130	-0.0464	0.0399
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case A only	1.4499	0.9489	-0.0034	-0.0186	-0.0205	-23.2584
ULS: 6a. D + 0.75L + 0.75(0.6)W + 0.75(S or Lr or R)_Wind uplift Case B only	-0.0015	2.2818	0.0027	0.0130	-0.0464	0.0399
ULS: 7. 0.6D + 0.6W_Wind downforce Case A only	-1.9376	3.1466	0.0096	0.0496	-0.0615	32.2554
ULS: 7. 0.6D + 0.6W_Wind downforce Case B only	-0.0009	1.3691	0.0016	0.0078	-0.0278	0.0240
ULS: 7. 0.6D + 0.6W_Wind uplift Case A only	1.9343	-0.4081	-0.0065	-0.0343	0.0067	-31.0405
ULS: 7. 0.6D + 0.6W_Wind uplift Case B only	-0.0009	1.3691	0.0016	0.0078	-0.0278	0.0240

Worst Case Reactions LRFD

These calculations are taken directly from the FEA via SkyCiv and are used in the Concrete Checks of the Foundatino Module.
Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	6.5353
Shear X	-3.2309
Shear Z	0.0179
Moment X	0.0915
Moment Y (Twist)	0.1459
Moment Z	55.0374

Worst Case Reactions ASD

These results are taken from the worst case values in the above table and are used in the Soil Checks in the Foundation Module.
Note: Worst case values are assumed as downforce wind load cases.

Result	Value (kip, kip-ft)
Axial	4.7005
Shear X	-1.9382
Shear Z	0.0107
Moment X	0.0548
Moment Y (Twist)	0.0999
Moment Z	32.2714

Project Details

Design Code: AISC 360-16 LRFD
Provision: LRFD
Country: United States

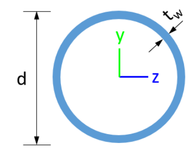
User Name: sales@mtsolar.us
Unit System: imperial

Design Input Information

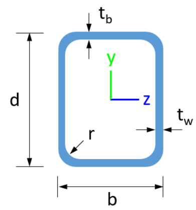
Design Factors			
Φ_t	Φ_c	Φ_b	Φ_v
0.9	0.9	0.9	0.9

Design Materials			
ID	E (ksi)	F_y (ksi)	F_u (ksi)
1	29000	50	65

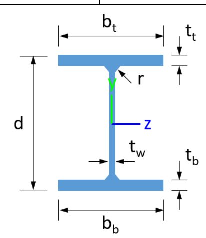
Section Dimensions



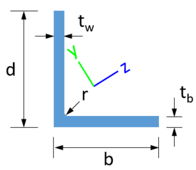
ID	Name	d (in)	t_w (in)					
1	2in Pipe Sch 40	2.38	0.15					
4	4in Pipe Sch 40	4.50	0.24					
9	8in Pipe Sch 40	8.63	0.32					



ID	Name	d (in)	b (in)	t_w (in)	t_b (in)	r (in)		
15	HSS5x3x1/8	5.00	3.00	0.12	0.12	0.12		



ID	Name	d (in)	t_w (in)	b_t (in)	b_b (in)	t_t (in)	t_b (in)	r (in)
18	W6x9	5.90	0.17	3.94	3.94	0.21	0.21	0.25



SkyCiv

ID	Name	d (in)	t_w (in)	b (in)	t_b (in)	r (in)		
34	L3x2x3/16	3.00	0.19	2.00	0.19	0.31		

Section Properties								
ID	Name	A (in^2)	J (in^4)	I_{yp} (in^4)	I_{zp} (in^4)	I_w (in^6)	S_{yp} (in^3)	S_{zp} (in^3)
1	2in Pipe Sch 40	1.07	1.33	0.67	0.67	0.00	0.76	0.76
4	4in Pipe Sch 40	3.17	14.47	7.23	7.23	0.00	4.31	4.31
9	8in Pipe Sch 40	8.40	144.98	72.49	72.49	0.00	22.21	22.21
15	HSS5x3x1/8	1.77	6.02	2.75	6.03	0.51	2.07	2.93
18	W6x9	2.68	0.04	2.20	16.40	17.70	1.72	6.23
34	L3x2x3/16	0.92	0.01	0.17	0.98	0.01	0.33	0.82

Member Properties								
Member ID	Section ID	K_zL (ft)	K_yL (ft)	L_b(ft)	C_b	LST	LSC	LD
1	9	34.31	34.31	16.34	-	300	200	1
2	4	1.30	1.30	2.00	-	300	200	1
3	15	0.92	0.92	1.42	1.19,1.18,1.19,1.18,1.18,1.19,1.18,1.18,1.17,1.18,1.18,1.19,1.18,1.19,1.18,1.18,1.19,1.18,1.18,1.19,1.17,1.19,1.18,1.19,1.18,1.19	300	200	1
4	15	2.44	2.44	3.75	1.68,1.68,1.68,1.67,1.68,1.68,1.67,1.68,1.66,1.68,1.67,1.68,1.66,1.68,1.67,1.67,1.69,1.67,1.67,1.68,1.65,1.68,1.67,1.68,1.66,1.68	300	200	1
5	15	1.52	1.52	2.33	1.67,1.67,1.67,1.67,1.67,1.67,1.67,1.67,1.66,1.67,1.67,1.67,1.66,1.67,1.67,1.67,1.68,1.67,1.67,1.67,1.65,1.67,1.67,1.67,1.66,1.67	300	200	1
6	15	0.92	0.92	1.42	1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.17,1.18,1.18,1.18,1.17,1.18,1.18,1.18,1.19,1.18,1.18,1.18,1.17,1.18,1.18,1.18,1.18,1.18	300	200	1
7	15	1.52	1.52	2.33	1.67,1.67,1.67,1.67,1.67,1.67,1.67,1.67,1.66,1.67,1.67,1.67,1.66,1.67,1.67,1.67,1.68,1.67,1.67,1.67,1.65,1.67,1.67,1.67,1.66,1.67	300	200	1
8	18	1.33	1.33	2.05	1.96,1.95,1.96,1.94,1.96,1.96,1.92,1.95,1.88,1.95,1.92,1.96,1.88,1.96,1.93,1.94,2.02,1.94,1.93,1.96,1.83,1.96,1.92,1.96,1.89,1.96	300	200	1
9	1	2.60	2.60	4.00	-	300	200	1
10	15	2.44	2.44	3.75	1.68,1.68,1.68,1.67,1.68,1.68,1.67,1.68,1.66,1.68,1.67,1.68,1.66,1.68,1.67,1.67,1.69,1.67,1.67,1.68,1.65,1.68,1.67,1.68,1.66,1.68	300	200	1
11	18	1.33	1.33	2.05	1.96,1.95,1.96,1.94,1.96,1.96,1.94,1.95,1.91,1.95,1.94,1.96,1.91,1.96,1.94,1.94,1.96,1.94,1.94,1.96,1.87,1.96,1.93,1.96,1.91,1.96	300	200	1
12	4	1.30	1.30	2.00	-	300	200	1
13	18	4.88	4.00	7.50	1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12	300	200	1
14	18	4.88	4.00	7.50	1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12	300	200	1
15	18	3.75	3.75	3.75	2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32	300	200	1
16	18	3.75	3.75	3.75	2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32	300	200	1
50	34	5.67	5.67	5.67	1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14	300	200	250
51	34	5.67	5.67	5.67	1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14	300	200	250

52	34	5.67	5.67	5.67	1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14	300	200	250
53	34	5.67	5.67	5.67	1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14	300	200	250
101	9	34.31	34.31	16.34	-	300	200	1
102	4	1.30	1.30	2.00	-	300	200	1
103	15	0.92	0.92	1.42	1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.17,1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.19,1.18,1.18,1.18,1.17,1.18,1.18,1.18,1.18,1.18	300	200	1
104	15	2.44	2.44	3.75	1.68,1.68,1.68,1.67,1.68,1.68,1.67,1.68,1.66,1.68,1.67,1.68,1.66,1.68,1.67,1.67,1.69,1.67,1.67,1.68,1.65,1.68,1.67,1.68,1.66,1.68	300	200	1
105	15	1.52	1.52	2.33	1.67,1.67,1.67,1.67,1.67,1.67,1.67,1.67,1.66,1.67,1.67,1.67,1.66,1.67,1.67,1.67,1.68,1.67,1.67,1.67,1.65,1.67,1.67,1.67,1.66,1.67	300	200	1
106	15	0.92	0.92	1.42	1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.17,1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.19,1.18,1.18,1.18,1.17,1.18,1.18,1.18,1.18,1.18	300	200	1
107	15	1.52	1.52	2.33	1.67,1.67,1.67,1.67,1.67,1.67,1.67,1.67,1.66,1.67,1.67,1.67,1.66,1.67,1.67,1.67,1.68,1.67,1.67,1.67,1.65,1.67,1.67,1.67,1.66,1.67	300	200	1
108	18	1.33	1.33	2.05	1.61,1.61,1.61,1.61,1.61,1.61,1.58,1.61,1.55,1.61,1.58,1.61,1.56,1.61,1.59,1.61,1.72,1.61,1.59,1.61,1.53,1.61,1.58,1.61,1.56,1.61	300	200	1
109	1	2.60	2.60	4.00	-	300	200	1
110	15	2.44	2.44	3.75	1.68,1.68,1.68,1.67,1.68,1.68,1.67,1.68,1.66,1.68,1.67,1.68,1.66,1.68,1.67,1.67,1.69,1.67,1.67,1.68,1.65,1.68,1.67,1.68,1.66,1.68	300	200	1
111	18	1.33	1.33	2.05	1.65,1.64,1.65,1.64,1.64,1.65,1.59,1.64,1.52,1.64,1.58,1.65,1.54,1.65,1.61,1.64,2.03,1.64,1.59,1.65,1.47,1.65,1.58,1.65,1.54,1.65	300	200	1
112	4	1.30	1.30	2.00	-	300	200	1
113	18	4.88	4.00	7.50	1.06,1.06,1.06,1.06,1.06,1.06,1.06,1.06,1.07,1.06,1.06,1.06,1.07,1.06,1.06,1.06,1.05,1.06,1.06,1.06,1.07,1.06,1.06,1.06,1.06,1.06	300	200	1
114	18	4.88	4.00	7.50	1.06,1.06,1.06,1.06,1.06,1.06,1.06,1.06,1.06,1.06,1.06,1.06,1.06,1.06,1.06,1.06,1.05,1.06,1.06,1.06,1.07,1.06,1.06,1.06,1.06,1.06	300	200	1
115	18	7.45	7.45	7.45	1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.15,1.12,1.12,1.12,1.11,1.12,1.12,1.12,1.12,1.12	300	200	1
116	18	7.45	7.45	7.45	1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.13,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12	300	200	1
150	34	5.67	5.67	5.67	1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14	300	200	250
151	34	5.67	5.67	5.67	1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14	300	200	250
152	34	5.67	5.67	5.67	1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14	300	200	250
153	34	5.67	5.67	5.67	1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14	300	200	250
201	9	34.31	34.31	16.34	-	300	200	1
202	4	1.30	1.30	2.00	-	300	200	1
203	15	0.92	0.92	1.42	1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.18,1.17,1.18,1.18,1.18,1.17,1.18,1.18,1.18,1.19,1.18,1.18,1.18,1.17,1.18,1.18,1.18,1.18,1.18	300	200	1
204	15	2.44	2.44	3.75	1.68,1.68,1.68,1.67,1.68,1.68,1.67,1.68,1.66,1.68,1.67,1.68,1.66,1.68,1.67,1.67,1.69,1.67,1.67,1.68,1.65,1.68,1.67,1.68,1.66,1.68	300	200	1
				2.3	[illegible]	3	2	

205	15	1.52	1.52	2.33	1.67,1.67,1.67,1.67,1.67,1.67,1.67,1.66,1.67,1.67,1.67,1.66,1.67,1.67,1.67,1.68,1.67,1.67,1.67,1.65,1.67,1.67,1.67,1.66,1.67	00	00	1
206	15	0.92	0.92	1.42	1.19,1.18,1.19,1.18,1.18,1.19,1.18,1.18,1.17,1.18,1.18,1.19,1.18,1.19,1.18,1.18,1.19,1.18,1.18,1.19,1.17,1.19,1.18,1.19,1.18,1.19	300	200	1
207	15	1.52	1.52	2.33	1.67,1.67,1.67,1.67,1.67,1.67,1.67,1.66,1.67,1.67,1.67,1.66,1.67,1.67,1.67,1.68,1.67,1.67,1.67,1.65,1.67,1.67,1.67,1.66,1.67	300	200	1
208	18	3.75	3.75	3.75	2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.31,2.32,2.32,2.32,2.32,2.32	300	200	1
209	1	2.60	2.60	4.00	-	300	200	1
210	15	2.44	2.44	3.75	1.68,1.68,1.68,1.67,1.68,1.67,1.68,1.66,1.68,1.67,1.68,1.66,1.68,1.67,1.67,1.69,1.67,1.67,1.68,1.65,1.68,1.67,1.68,1.66,1.68	300	200	1
211	18	3.75	3.75	3.75	2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32,2.32	300	200	1
212	4	1.30	1.30	2.00	-	300	200	1
213	18	4.88	4.00	7.50	1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.13,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12	300	200	1
214	18	4.88	4.00	7.50	1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12,1.12	300	200	1
215	18	7.45	7.45	7.45	1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.14,1.16,1.16,1.16,1.17,1.16,1.16,1.16,1.16,1.16	300	200	1
216	18	7.45	7.45	7.45	1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16,1.16	300	200	1
250	34	5.67	5.67	5.67	1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14	300	200	250
251	34	5.67	5.67	5.67	1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14	300	200	250
252	34	5.67	5.67	5.67	1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14	300	200	250
253	34	5.67	5.67	5.67	1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14,1.14	300	200	250

Member Design Capacity

Member ID	$\Phi_t P_n$ (kip)	$\Phi_c P_n$ (kip)	$\Phi_b M_{zn}$ (k-ft)	$\Phi_b M_{yn}$ (k-ft)	$\Phi_v V_{yn}$ (kip)	$\Phi_v V_{zn}$ (kip)
1	377.97	96.60	83.29	83.29	113.39	113.39
2	142.83	141.72	16.17	16.17	42.85	42.85
3	79.65	74.02	10.99	4.60	29.14	16.61
4	79.65	72.01	10.99	4.60	29.14	16.61
5	79.65	73.44	10.99	4.60	29.14	16.61
6	79.65	74.02	10.99	4.60	29.14	16.61
7	79.65	73.44	10.99	4.60	29.14	16.61
8	120.60	115.40	23.36	6.45	30.09	45.74
9	48.35	43.11	2.85	2.85	14.51	14.51
10	79.65	72.01	10.99	4.60	29.14	16.61
11	120.60	115.40	23.36	6.45	30.09	45.74
12	142.83	141.72	16.17	16.17	42.85	42.85
13	120.60	84.03	19.71	6.45	30.09	45.74
14	120.60	84.03	19.68	6.45	30.09	45.74
15	120.60	100.70	23.36	6.45	30.09	45.74
16	120.60	100.70	23.36	6.45	30.09	45.74
50	41.27	8.45	1.63	0.88	15.23	10.15

51	41.27	8.45	1.63	0.88	15.23	10.15
52	41.27	8.45	1.63	0.88	15.23	10.15
53	41.27	8.45	1.63	0.88	15.23	10.15
101	377.97	96.60	83.29	83.29	113.39	113.39
102	142.83	141.72	16.17	16.17	42.85	42.85
103	79.65	74.02	10.99	4.60	29.14	16.61
104	79.65	72.01	10.99	4.60	29.14	16.61
105	79.65	73.44	10.99	4.60	29.14	16.61
106	79.65	74.02	10.99	4.60	29.14	16.61
107	79.65	73.44	10.99	4.60	29.14	16.61
108	120.60	115.40	23.36	6.45	30.09	45.74
109	48.35	43.11	2.85	2.85	14.51	14.51
110	79.65	72.01	10.99	4.60	29.14	16.61
111	120.60	115.40	23.36	6.45	30.09	45.74
112	142.83	141.72	16.17	16.17	42.85	42.85
113	120.60	84.03	18.49	6.45	30.09	45.74
114	120.60	84.03	18.56	6.45	30.09	45.74
115	120.60	59.18	19.66	6.45	30.09	45.74
116	120.60	59.18	19.75	6.45	30.09	45.74
150	41.27	8.45	1.63	0.88	15.23	10.15
151	41.27	8.45	1.63	0.88	15.23	10.15
152	41.27	8.45	1.63	0.88	15.23	10.15
153	41.27	8.45	1.63	0.88	15.23	10.15
201	377.97	96.60	83.29	83.29	113.39	113.39
202	142.83	141.72	16.17	16.17	42.85	42.85
203	79.65	74.02	10.99	4.60	29.14	16.61
204	79.65	72.01	10.99	4.60	29.14	16.61
205	79.65	73.44	10.99	4.60	29.14	16.61
206	79.65	74.02	10.99	4.60	29.14	16.61
207	79.65	73.44	10.99	4.60	29.14	16.61
208	120.60	100.70	23.36	6.45	30.09	45.74
209	48.35	43.11	2.85	2.85	14.51	14.51
210	79.65	72.01	10.99	4.60	29.14	16.61
211	120.60	100.70	23.36	6.45	30.09	45.74
212	142.83	141.72	16.17	16.17	42.85	42.85
213	120.60	84.03	19.71	6.45	30.09	45.74
214	120.60	84.03	19.68	6.45	30.09	45.74
215	120.60	59.18	20.20	6.45	30.09	45.74
216	120.60	59.18	20.43	6.45	30.09	45.74
250	41.27	8.45	1.63	0.88	15.23	10.15
251	41.27	8.45	1.63	0.88	15.23	10.15
252	41.27	8.45	1.63	0.88	15.23	10.15
253	41.27	8.45	1.63	0.88	15.23	10.15

Design Ratio

Member ID	P	M_z	M_y	V_y	V_z	(P,M_z,M_y)	Worst LC	KL/r	δ	Status
1	0.068	0.661	0.002	0.028	0.000	0.695	#13	0.701	Not Required	Pass
2	0.004	0.309	0.207	0.070	0.039	0.518	#13	0.052	Not Required	Pass
3	0.006	0.547	0.166	0.055	0.038	0.654	#13	0.044	Not Required	Pass
4	0.006	0.544	0.067	0.055	0.006	0.587	#13	0.078	Not Required	Pass
5	0.006	0.339	0.032	0.055	0.004	0.359	#13	0.073	Not Required	Pass
6	0.005	0.534	0.181	0.054	0.041	0.649	#13	0.044	Not Required	Pass

7	0.005	0.331	0.022	0.053	0.002	0.347	#13	0.073	Not Required	Pass
8	0.003	0.050	0.017	0.032	0.004	0.054	#13	0.088	Not Required	Pass
9	0.015	0.045	0.045	0.001	0.000	0.094	#13	0.132	Not Required	Pass
10	0.005	0.530	0.087	0.053	0.009	0.588	#13	0.078	Not Required	Pass
11	0.003	0.049	0.016	0.032	0.004	0.053	#13	0.059	Not Required	Pass
12	0.004	0.297	0.199	0.070	0.038	0.497	#13	0.052	Not Required	Pass
13	0.003	0.158	0.030	0.043	0.005	0.177	#13	0.177	Not Required	Pass
14	0.005	0.161	0.037	0.043	0.005	0.178	#13	0.265	Not Required	Pass
15	0.003	0.059	0.015	0.024	0.004	0.068	#13	0.166	Not Required	Pass
16	0.003	0.059	0.012	0.024	0.004	0.064	#13	0.166	Not Required	Pass
50	0.084	0.009	0.004	0.001	0.001	0.054	#23	0.783	Not Required	Pass
51	0.017	0.005	0.015	0.001	0.001	0.024	#23	0.522	Not Required	Pass
52	0.056	0.009	0.004	0.001	0.001	0.039	#23	0.783	Not Required	Pass
53	0.011	0.005	0.015	0.001	0.001	0.022	#1	0.522	Not Required	Pass
101	0.070	0.662	0.000	0.029	0.000	0.697	#13	0.701	Not Required	Pass
102	0.004	0.311	0.202	0.072	0.038	0.514	#13	0.052	Not Required	Pass
103	0.006	0.555	0.181	0.056	0.041	0.676	#13	0.044	Not Required	Pass
104	0.006	0.552	0.080	0.056	0.008	0.605	#13	0.078	Not Required	Pass
105	0.006	0.344	0.028	0.055	0.003	0.365	#13	0.073	Not Required	Pass
106	0.006	0.555	0.181	0.056	0.041	0.676	#13	0.044	Not Required	Pass
107	0.006	0.344	0.028	0.055	0.003	0.365	#13	0.073	Not Required	Pass
108	0.002	0.058	0.010	0.029	0.003	0.067	#13	0.088	Not Required	Pass
109	0.015	0.037	0.042	0.001	0.000	0.085	#13	0.132	Not Required	Pass
110	0.006	0.552	0.080	0.056	0.008	0.605	#13	0.078	Not Required	Pass
111	0.003	0.058	0.014	0.030	0.004	0.069	#13	0.059	Not Required	Pass
112	0.004	0.311	0.202	0.072	0.038	0.514	#13	0.052	Not Required	Pass
113	0.003	0.116	0.024	0.041	0.005	0.130	#13	0.177	Not Required	Pass
114	0.005	0.118	0.035	0.041	0.005	0.128	#13	0.265	Not Required	Pass
115	0.004	0.097	0.031	0.030	0.005	0.103	#13	0.329	Not Required	Pass
116	0.005	0.097	0.029	0.029	0.005	0.104	#13	0.493	Not Required	Pass
150	0.077	0.009	0.004	0.001	0.001	0.050	#21	0.783	Not Required	Pass
151	0.015	0.005	0.015	0.001	0.001	0.023	#21	0.522	Not Required	Pass
152	0.077	0.009	0.004	0.001	0.001	0.050	#21	0.783	Not Required	Pass
153	0.015	0.005	0.015	0.001	0.001	0.023	#21	0.522	Not Required	Pass
201	0.068	0.661	0.002	0.028	0.000	0.695	#13	0.701	Not Required	Pass
202	0.004	0.297	0.199	0.070	0.038	0.497	#13	0.052	Not Required	Pass
203	0.005	0.534	0.181	0.054	0.041	0.649	#13	0.044	Not Required	Pass
204	0.005	0.530	0.087	0.053	0.009	0.588	#13	0.078	Not Required	Pass
205	0.005	0.331	0.022	0.053	0.002	0.347	#13	0.073	Not Required	Pass
206	0.006	0.547	0.166	0.055	0.038	0.654	#13	0.044	Not Required	Pass
207	0.006	0.339	0.032	0.055	0.004	0.360	#13	0.073	Not Required	Pass
208	0.003	0.059	0.012	0.024	0.004	0.063	#13	0.248	Not Required	Pass
209	0.015	0.045	0.045	0.001	0.000	0.094	#13	0.132	Not Required	Pass
210	0.006	0.544	0.067	0.055	0.006	0.586	#13	0.078	Not Required	Pass
211	0.003	0.059	0.015	0.024	0.004	0.068	#13	0.166	Not Required	Pass
212	0.004	0.309	0.207	0.070	0.039	0.518	#13	0.052	Not Required	Pass
213	0.003	0.158	0.030	0.043	0.005	0.177	#13	0.177	Not Required	Pass
214	0.005	0.161	0.037	0.043	0.005	0.178	#13	0.265	Not Required	Pass
215	0.004	0.095	0.031	0.032	0.005	0.100	#13	0.329	Not Required	Pass
216	0.005	0.094	0.029	0.032	0.005	0.101	#13	0.493	Not Required	Pass
250	0.056	0.009	0.004	0.001	0.001	0.039	#23	0.783	Not Required	Pass
251	0.011	0.005	0.015	0.001	0.001	0.022	#1	0.522	Not Required	Pass
252	0.084	0.009	0.004	0.001	0.001	0.054	#23	0.783	Not Required	Pass
253	0.017	0.005	0.015	0.001	0.001	0.024	#23	0.522	Not Required	Pass

255	0.017	0.005	0.015	0.001	0.001	0.024	#25	0.322	Not Required	Pass

Definitions

Φ_t	Safety factor for tensile
Φ_c	Safety factor for compression
Φ_b	Safety factor for flexure
Φ_v	Safety factor for shear
E	Modulus of elasticity
F_y	Specified minimum yield stress
F_u	Specified minimum tensile strength
A	Cross-sectional area
J	Torsional constant
I_{yp}	Moment of inertia about the Y axes
I_{zp}	Moment of inertia about the Z axes
I_w	Warping constant
S_{yp}	Plastic section modulus about the Y axis
S_{zp}	Plastic section modulus about the Z axis
KL	Effective length
C_b	Buckling modification factor (from all load combinations)
L_b	Length between braced points
LST	Limited slenderness for tension
LSC	Limited slenderness for compression
LD	Limited deflection
P_n	Nominal axial strength (tension/comression)
M_n	Nominal flexural strength (about Z/Y axis)
V_n	Nominal shear strength (along Z/Y axis)
P	Design ratio in case of axial force
M_z	Design ratio in case of bending about Z axis
M_y	Design ratio in case of bending about Y axis
V_y	Design ratio in case of shear along Y axis
V_z	Design ratio in case of shear along Z axis
(P, M_z, M_y)	Design ratio in case of axial force and bending action
KL/r	Design ratio in case of section slenderness
δ	Design ratio in case of member deflection
OK	Capacity is provided
NG	Capacity is not provided

REFERENCES	CALCULATIONS	RESULTS

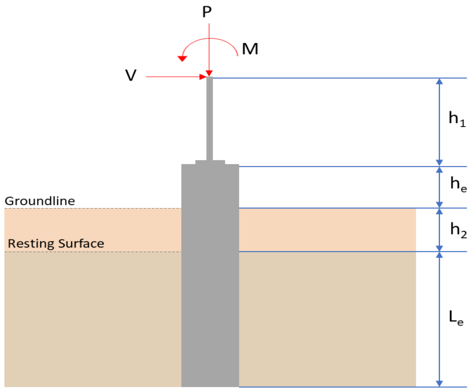
SkyCiv Foundation Design

Pile Foundation

Design Information :

Design code : IBC 2021 (International Building Code)
Unit System : Imperial

Pile Input



Geometry

Pile shape: rectangular
$b = 48\ \mathrm{in}$ - Pile width
$D = 48\ \mathrm{in}$ - Pile depth
$L = 7\ \mathrm{ft}$ - Total pile length
$h_1 = 0\ \mathrm{ft}$ - Lateral load height from the top of the pile,
$h_2 = 0\ \mathrm{ft}$ - Depth to resisting surface
$h_e = 0\ \mathrm{ft}$ - Length of pile above the ground

Tabulation of Soil Parameters

Layer	Label	Allowable Bearing Pressure (q_a) (psf)	Allowable Lateral Pressure (R) (psf/ft)
1	Sand, silty sand, clayey sand, silty gravel & clayey gravel	2000.000	150.000

Tabulation of Loads

Load Component	ASD	LRFD
P (kip)	4.701	6.535
V_x (kip)	-1.938	-3.231
V_z (kip)	-0.011	-0.018
M_x (kipft)	-0.055	-0.092
M_z (kipft)	32.271	55.038

Material Properties

$f'_{ck} = 2.5\ \mathrm{ksi}$ - Concrete strength,

Required depth to resist lateral loads (ASD)

H - Point of application of the lateral load

$$H = h_1 + h_2 + h_e$$

$$H = (0\ \mathrm{ft}) + (0\ \mathrm{ft}) + (0\ \mathrm{ft})$$

$$H = 0\ \mathrm{ft}$$

Considering x-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\ D}$$

$$H_o = \frac{(-1.938\ \mathrm{kip})}{1.57 \times (48\ \mathrm{in})}$$

$$H_o = -0.3086\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\ H)}{1.57\ D}$$

$$M_o = \frac{(32.271\ \text{kipft}) + ((-1.938\ \text{kip}) \times (0\ \text{ft}))}{1.57 \times (48\ \text{in})}$$

$$M_o = 5.1387\ \text{kipft/ft}$$

Required depth of embedment in earth:

$$L_x^3 - \left(14.14 \times \frac{H_o \times L_x}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,x} = 6.6093\ \text{ft}$ - Required depth in x-direction,

Considering z-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\, b}$$

$$H_o = \frac{(-0.011\ \text{kip})}{1.57 \times (48\ \text{in})}$$

$$H_o = -0.0017516\ \text{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\, H)}{1.57\, b}$$

$$M_o = \frac{(0.055\ \text{kipft}) + ((-0.011\ \text{kip}) \times (0\ \text{ft}))}{1.57 \times (48\ \text{in})}$$

$$M_o = 0.008758\ \text{kipft/ft}$$

Required depth of embedment in earth:

$$L_z^3 - \left(14.14 \times \frac{H_o \times L_z}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,z} = 0.84887\ \text{ft}$ - Required depth in z-direction,

Minimum embedded depth required:

$L_{e,req}$ - Depth of pile required,

$$L_{e,req} = MAX[L_{e,x}, L_{e,z}]$$

$$L_{e,req} = MAX[(6.6093\ \text{ft}), (0.84887\ \text{ft})]$$

$$L_{e,req} = 6.609\ \text{ft}$$

L_e - Actual embedded length of pile,

$$L_e = L - h_e - h_2$$

$$L_e = (7\ \text{ft}) - (0\ \text{ft}) - (0\ \text{ft})$$

$$L_e = 7\ \text{ft}$$

$Ratio$ - Embedded depth

$$Ratio = \frac{L_{e,req}}{L_e}$$

$$Ratio = \frac{(6.609\ \text{ft})}{(7\ \text{ft})}$$

$$Ratio = 0.94414$$

Status: **PASS**
Ratio: **0.940**

End-bearing Capacity (ASD)

A - Pile cross-section area

$$A = b\, D$$

$$A = (48\ \text{in}) \times (48\ \text{in})$$

$$A = 16\ \text{ft}^2$$

q - End-bearing pressure

$$q = \frac{P_v}{A}$$

$$q = \frac{(4.701\ \text{kip})}{(16\ \text{ft}^2)}$$

$$q = 0.29381\ \text{kip/ft}^2$$

$q = 0.29381\ \mathrm{kip/ft}^2$

Check bearing capacity ratio:

$Ratio$ - Capacity

$$Ratio = \frac{q}{q_a}$$

$$Ratio = \frac{\left(0.29381\ \mathrm{kip/ft}^2\right)}{(2000\ \mathrm{psf})}$$

$$Ratio = 0.14691$$

Status: **PASS**
Ratio: **0.150**

Czerniak

Lateral Soil Pressure (ASD):

L/D - Length to least lateral dimension ratio,

$$L/D = \frac{L}{D}$$

$$L/D = \frac{(7\ \mathrm{ft})}{(48\ \mathrm{in})}$$

$$L/D = 1.75$$

Since L/D $\leq$ 10,

Pile is short.

Considering x-direction:

$H_o = -0.3086\ \mathrm{kip/ft}$ - Lateral force per length of pile,
$M_o = 5.1387\ \mathrm{kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (5.1387\ \mathrm{kipft/ft}) \times (7\ \mathrm{ft})) + \left(3 \times (-0.3086\ \mathrm{kip/ft}) \times (7\ \mathrm{ft})^2\right)}{(6 \times (5.1387\ \mathrm{kipft/ft})) + (4 \times (-0.3086\ \mathrm{kip/ft}) \times (7\ \mathrm{ft}))}$$

$$a = 4.7944\ \mathrm{ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\ [(4\,M_o) + (3\,H_o\,L_e)]^2}{L_e^2\ [(3\,M_o) + (2\,H_o\,L_e)]}$$

$$p = \frac{0.75 \times [(4 \times (5.1387\ \mathrm{kipft/ft})) + (3 \times (-0.3086\ \mathrm{kip/ft}) \times (7\ \mathrm{ft}))]^{\ 2}}{(7\ \mathrm{ft})^2 \times [(3 \times (5.1387\ \mathrm{kipft/ft})) + (2 \times (-0.3086\ \mathrm{kip/ft}) \times (7\ \mathrm{ft}))]}$$

$$p = 0.27325\ \mathrm{kip/ft}^2$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\ [(2\,M_o) + (H_o\,L_e)]}{L_e^2}$$

$$s = \frac{6 \times [(2 \times (5.1387\ \mathrm{kipft/ft})) + ((-0.3086\ \mathrm{kip/ft}) \times (7\ \mathrm{ft}))]}{(7\ \mathrm{ft})^2}$$

$$s = 0.99394\ \mathrm{kip/ft}^2$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\,\frac{a}{2}$$

$$p_a = (150\ \mathrm{psf/ft}) \times \frac{(4.7944\ \mathrm{ft})}{2}$$

$$p_a = 0.35958\ \mathrm{kip/ft}^2$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{\left(0.27325\ \mathrm{kip/ft}^2\right)}{\left(0.35958\ \mathrm{kip/ft}^2\right)}$$

$$Ratio = 0.75992$$

Status: **PASS**
Ratio: **0.760**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\,L_e$$

$$p_s = (150\ \mathrm{psf/ft}) \times (7\ \mathrm{ft})$$

$$p_s = 1.05\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{\left(0.99394\ \mathrm{kip/ft^2}\right)}{\left(1.05\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = 0.94661$$

Status: **PASS**
Ratio: **0.950**

Considering z-direction:

$H_o = -0.0017516\ \mathrm{kip/ft}$ - Lateral force per length of pile,
$M_o = 0.008758\ \mathrm{kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (0.008758\ \mathrm{kipft/ft}) \times (7\ \mathrm{ft})) + \left(3 \times (-0.0017516\ \mathrm{kip/ft}) \times (7\ \mathrm{ft})^2\right)}{(6 \times (0.008758\ \mathrm{kipft/ft})) + (4 \times (-0.0017516\ \mathrm{kip/ft}) \times (7\ \mathrm{ft}))}$$

$$a = 4.9483\ \mathrm{ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\,[(4\,M_o) + (3\,H_o\,L_e)]^2}{L_e^2\,[(3\,M_o) + (2\,H_o\,L_e)]}$$

$$p = \frac{0.75 \times [(4 \times (0.008758\ \mathrm{kipft/ft})) + (3 \times (-0.0017516\ \mathrm{kip/ft}) \times (7\ \mathrm{ft}))]^{2}}{(7\ \mathrm{ft})^2 \times [(3 \times (0.008758\ \mathrm{kipft/ft})) + (2 \times (-0.0017516\ \mathrm{kip/ft}) \times (7\ \mathrm{ft}))]}$$

$$p = 0.00002681\ \mathrm{kip/ft^2}$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\,[(2\,M_o) + (H_o\,L_e)]}{L_e^2}$$

$$s = \frac{6 \times [(2 \times (0.008758\ \mathrm{kipft/ft})) + ((-0.0017516\ \mathrm{kip/ft}) \times (7\ \mathrm{ft}))]}{(7\ \mathrm{ft})^2}$$

$$s = 0.00064344\ \mathrm{kip/ft^2}$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\,\frac{a}{2}$$

$$p_a = (150\ \mathrm{psf/ft}) \times \frac{(4.9483\ \mathrm{ft})}{2}$$

$$p_a = 0.37112\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{\left(0.00002681\ \mathrm{kip/ft^2}\right)}{\left(0.37112\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = 0.000072241$$

Status: **PASS**
Ratio: **0.000**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\,L_e$$

$$p_s = (150\ \mathrm{psf/ft}) \times (7\ \mathrm{ft})$$

$$p_s = 1.05\ \mathrm{kip/ft^2}$$

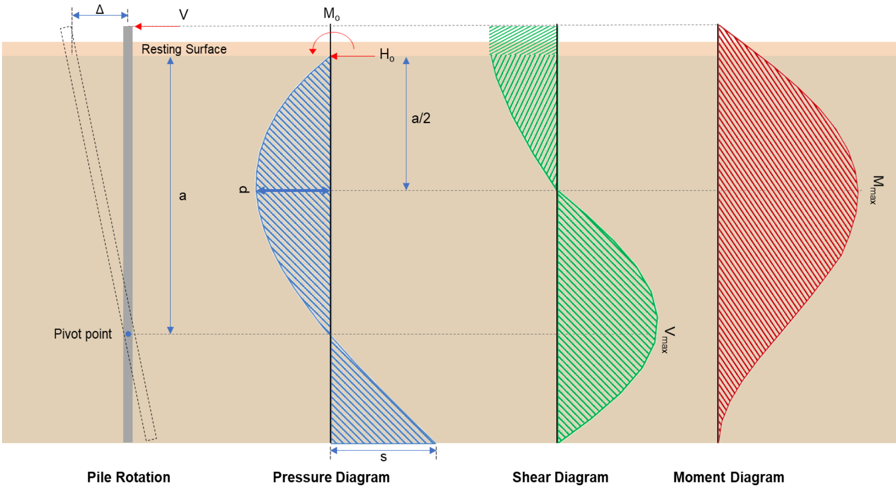
$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{\left(0.00064344\ \mathrm{kip/ft^2}\right)}{\left(1.05\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = 0.0006128$$

Status: **PASS**
Ratio: **0.000**



Shear force and Bending moment (x-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\,D}$$

$$H_o = \frac{(-3.231\ \mathrm{kip})}{1.57 \times (48\ \mathrm{in})}$$

$$H_o = -0.51449\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\,H)}{1.57\,D}$$

$$M_o = \frac{(55.038\ \mathrm{kipft}) + ((-3.231\ \mathrm{kip}) \times (0\ \mathrm{ft}))}{1.57 \times (48\ \mathrm{in})}$$

$$M_o = 8.764\ \mathrm{kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(8.764\ \mathrm{kipft/ft})}{(-0.51449\ \mathrm{kip/ft})}$$

$$E = 17.034\ \mathrm{ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (8.764\ \mathrm{kipft/ft}) \times (7\ \mathrm{ft})) + \left(3 \times (-0.51449\ \mathrm{kip/ft}) \times (7\ \mathrm{ft})^2\right)}{(6 \times (8.764\ \mathrm{kipft/ft})) + (4 \times (-0.51449\ \mathrm{kip/ft}) \times (7\ \mathrm{ft}))}$$

$$a = 4.7921\ \mathrm{ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\,D)\left[1 - \left[3\left(\frac{4\,E}{L_e} + 3\right)\left(\frac{a}{L_e}\right)^2\right] + \left[4\left(\frac{3\,E}{L_e} + 2\right)\left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((-0.51449\ \mathrm{kip/ft}) \times (48\ \mathrm{in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (17.034\ \mathrm{ft})}{(7\ \mathrm{ft})} + 3\right) \times \left(\frac{(4.7921\ \mathrm{ft})}{(7\ \mathrm{ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (17.034\ \mathrm{ft})}{(7\ \mathrm{ft})} + 2\right) \times \left(\frac{(4.7921\ \mathrm{ft})}{(7\ \mathrm{ft})}\right)^3\right]\right]$$

$$V_{max} = 10.224\ \mathrm{kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, D\, L_e) \left[\left(\frac{E}{L_e} + \frac{a}{2\,L_e}\right) - \left[\left(\frac{4\,E}{L_e} + 3\right) \left(\frac{a}{2\,L_e}\right)^3\right] + \left[\left(\frac{3\,E}{L_e} + 2\right) \left(\frac{a}{2\,L_e}\right)^4\right]\right]$$

$$M_{max} = ((-0.51449\ \text{kip/ft}) \times (48\ \text{in}) \times (7\ \text{ft})) \times \left[\left(\frac{(17.034\ \text{ft})}{(7\ \text{ft})} + \frac{(4.7921\ \text{ft})}{2 \times (7\ \text{ft})}\right) - \left[\left(\frac{4 \times (17.034\ \text{ft})}{(7\ \text{ft})} + 3\right) \times \left(\frac{(4.7921\ \text{ft})}{2 \times (7\ \text{ft})}\right)^3\right] + \left[\left(\frac{3 \times (17.034\ \text{ft})}{(7\ \text{ft})} + 2\right) \times \left(\frac{(4.7921\ \text{ft})}{2 \times (7\ \text{ft})}\right)^4\right]\right]$$

$$M_{max} = 34.469\ \text{kipft}$$

Shear force and Bending moment (z-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\, b}$$

$$H_o = \frac{(-0.018\ \text{kip})}{1.57 \times (48\ \text{in})}$$

$$H_o = -0.0028662\ \text{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\, H)}{1.57\, b}$$

$$M_o = \frac{(0.092\ \text{kipft}) + ((-0.018\ \text{kip}) \times (0\ \text{ft}))}{1.57 \times (48\ \text{in})}$$

$$M_o = 0.01465\ \text{kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(0.01465\ \text{kipft/ft})}{(-0.0028662\ \text{kip/ft})}$$

$$E = 5.1111\ \text{ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (0.01465\ \text{kipft/ft}) \times (7\ \text{ft})) + \left(3 \times (-0.0028662\ \text{kip/ft}) \times (7\ \text{ft})^2\right)}{(6 \times (0.01465\ \text{kipft/ft})) + (4 \times (-0.0028662\ \text{kip/ft}) \times (7\ \text{ft}))}$$

$$a = 4.9451\ \text{ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\, b) \left[1 - \left[3 \left(\frac{4\,E}{L_e} + 3\right) \left(\frac{a}{L_e}\right)^2\right] + \left[4 \left(\frac{3\,E}{L_e} + 2\right) \left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((-0.0028662\ \text{kip/ft}) \times (48\ \text{in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (5.1111\ \text{ft})}{(7\ \text{ft})} + 3\right) \times \left(\frac{(4.9451\ \text{ft})}{(7\ \text{ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (5.1111\ \text{ft})}{(7\ \text{ft})} + 2\right) \times \left(\frac{(4.9451\ \text{ft})}{(7\ \text{ft})}\right)^3\right]\right]$$

$$V_{max} = 0.022411\ \text{kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\, b\, L_e) \left[\left(\frac{E}{L_e} + \frac{a}{2\,L_e}\right) - \left[\left(\frac{4\,E}{L_e} + 3\right) \left(\frac{a}{2\,L_e}\right)^3\right] + \left[\left(\frac{3\,E}{L_e} + 2\right) \left(\frac{a}{2\,L_e}\right)^4\right]\right]$$

$$M_{max} = ((-0.0028662\ \text{kip/ft}) \times (48\ \text{in}) \times (7\ \text{ft})) \times \left[\left(\frac{(5.1111\ \text{ft})}{(7\ \text{ft})} + \frac{(4.9451\ \text{ft})}{2 \times (7\ \text{ft})}\right) - \left[\left(\frac{4 \times (5.1111\ \text{ft})}{(7\ \text{ft})} + 3\right) \times \left(\frac{(4.9451\ \text{ft})}{2 \times (7\ \text{ft})}\right)^3\right] + \left[\left(\frac{3 \times (5.1111\ \text{ft})}{(7\ \text{ft})} + 2\right) \times \left(\frac{(4.9451\ \text{ft})}{2 \times (7\ \text{ft})}\right)^4\right]\right]$$

$$M_{max} = 0.071241 \text{ kipft}$$

Minimum Reinforcement Check (LRFD)

Parameters:

$f'_{ck} = 2.5 \text{ ksi}$ - Concrete strength,
$f_{yk} = 60 \text{ ksi}$ - Longitudinal reinforcement strength,
$\phi = 0.65$ - Reduction factor for axial strength,
Table 22.4.2.1 $\alpha = 0.8$ - Alpha factor for axial strength,
$A_g = 2304 \text{ in}^2$ - Gross area of concrete,

Longitudinal reinforcement:

Required reinforcement due to axial load, $A_{st,required}$
22.4.2.2, 10.6.1.1 $A_{st,required}$

$$A_{st,required} = Min\left[\frac{\frac{P}{\phi\ \alpha} - (0.85\ f'_{ck}\ A_g)}{f_{yk} - (0.85\ f'_{ck})}, (0.08\ A_g)\right]$$

$$A_{st,required} = Min\left[\frac{\frac{(6.535 \text{ kip})}{(0.65)\times(0.8)} - \left(0.85 \times (2.5 \text{ ksi}) \times \left(2304 \text{ in}^2\right)\right)}{(60 \text{ ksi}) - (0.85 \times (2.5 \text{ ksi}))}, \left(0.08 \times \left(2304 \text{ in}^2\right)\right)\right]$$

$$A_{st,required} = -84.379 \text{ in}^2$$

A_{min} - Governing minimum reinforcement area,

$$A_{min} = Max\left[A_{st,required}, (0.0018\ A_g)\right]$$

$$A_{min} = Max\left[\left(-84.379 \text{ in}^2\right), \left(0.0018 \times \left(2304 \text{ in}^2\right)\right)\right]$$

$$A_{min} = 4.1472 \text{ in}^2$$

n_{rebar} - Required number of reinforcement,

$$n_{rebar} = \frac{A_{min}}{A_{rebar}}$$

$$n_{rebar} = \frac{\left(4.1472 \text{ in}^2\right)}{\left(0.3068 \text{ in}^2\right)}$$

$$n_{rebar} = 14$$

A_{st} - Actual total reinforcement area,

$$A_{st} = n_{rebar}\ \frac{\pi\ d_{bar}^2}{4}$$

$$A_{st} = (14) \times \frac{\pi \times (0.625 \text{ in})^2}{4}$$

$$A_{st} = 4.2951 \text{ in}^2$$

$Ratio$ - Capacity

$$Ratio = \frac{A_{min}}{A_{st}}$$

$$Ratio = \frac{\left(4.1472 \text{ in}^2\right)}{\left(4.2951 \text{ in}^2\right)}$$

$$Ratio = 0.96556$$

Status: **PASS**
Ratio: **0.970**

25.2.3 s_{rebar} - Minimum spacing of reinforcement,

$$s_{rebar} = Max\left[1.5, (1.5\ d_{bar})\right]$$

$$s_{rebar} = Max\left[1.5, (1.5 \times (0.625 \text{ in}))\right]$$

$$s_{rebar} = 1.5 \text{ in}$$

Ties:

25.7.2.2 Since longitudinal reinforcement is $\leq$ No. 10ø: Use #3(0.375 in)
25.7.2.1 s_{ties} - Maximum spacing of ties,

$$s_{ties} = Min\left[(16\ d_{bar}), (48\ d_{ties}), Min(D, b)\right]$$

$$s_{ties} = Min\left[(16 \times (0.625 \text{ in})), (48 \times (0.375 \text{ in})), Min((48 \text{ in}), (48 \text{ in}))\right]$$

$$s_{ties} = 10 \text{ in}$$

Summary:

Main reinforcement: **14 - #5 (0.625 in)**

	Ties: **#3(0.375 in) - 10 in**	

Axial Compression Strength (ACI 318-19, LRFD)

22.4.2.2 ϕP_N - Allowable axial compressive strength

$$\phi P_N = \phi\, 0.80\, \left[\left(0.85\, f'_{ck}\, \left[A_g - A_{st}\right]\right) + \left(f_{yk}\, A_{st}\right)\right]$$

$$\phi P_N = (0.65) \times 0.80 \times \left[\left(0.85 \times (2.5\ \mathrm{ksi}) \times \left[\left(2304\ \mathrm{in}^2\right) - \left(4.2951\ \mathrm{in}^2\right)\right]\right) + \left((60\ \mathrm{ksi}) \times \left(4.2951\ \mathrm{in}^2\right)\right)\right]$$

$$\phi P_N = 2675.2\ \mathrm{kip}$$

$Ratio$ - Capacity

$$Ratio = \frac{P}{\phi P_N}$$

$$Ratio = \frac{(6.535\ \mathrm{kip})}{(2675.2\ \mathrm{kip})}$$

$$Ratio = 0.0024428$$

Status: **PASS**
Ratio: **0.000**

Shear Strength (ACI 318-19, LRFD)

Parameters:

$b_w = 48\ \mathrm{in}$ - Effective width,

22.5.2.2 d - Effective depth

$$d = 0.80\, D$$

$$d = 0.80 \times (48\ \mathrm{in})$$

$$d = 38.4\ \mathrm{in}$$

22.5.5.1.3 λ_s - size effect modification factor

$$\lambda_s = MIN\left[\sqrt{\frac{2}{1+\frac{d}{10}}}, 1\right]$$

$$\lambda_s = MIN\left[\sqrt{\frac{2}{1+\frac{(38.4\ \mathrm{in})}{10}}}, 1\right]$$

$$\lambda_s = 0.64282$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5\ \mathrm{ksi} \rightarrow 2500\ \mathrm{psi}$,

22.5.5.1.1 $V_{c,max}$ - Max shear strength of concrete

$$V_{c,max} = 5\, \lambda_s\, \sqrt{f'_{ck}}\, b_w\, d$$

$$V_{c,max} = 5 \times (0.64282) \times \sqrt{(2500\ \mathrm{psi})} \times (48\ \mathrm{in}) \times (38.4\ \mathrm{in})$$

$$V_{c,max} = 296.21\ \mathrm{kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5\ \mathrm{ksi} \rightarrow 2500\ \mathrm{psi}$, $P = 6.535\ \mathrm{kip} \rightarrow 6535\ \mathrm{lbf}$,

22.5.5.1.1(a) $V_{c,a}$ - Shear strength of concrete (a)

$$V_{c,a} = \left[2\, \lambda_s\, \sqrt{f'_{ck}} + \frac{P}{6\, A_g}\right]\, b_w\, d$$

$$V_{c,a} = \left[2 \times (0.64282) \times \sqrt{(2500\ \mathrm{psi})} + \frac{(6535\ \mathrm{lbf})}{6 \times (2304\ \mathrm{in}^2)}\right] \times (48\ \mathrm{in}) \times (38.4\ \mathrm{in})$$

$$V_{c,a} = 119.36\ \mathrm{kip}$$

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5\ \mathrm{ksi} \rightarrow 2500\ \mathrm{psi}$,

22.5.5.1.2 $V_{c,b}$ - Shear strength of concrete (b)

$$V_{c,b} = \left[2\, \lambda_s\, \sqrt{f'_{ck}} + \left(0.05\, f'_{ck}\right)\right]\, b_w\, d$$

$$V_{c,b} = \left[2 \times (0.64282) \times \sqrt{(2500\ \mathrm{psi})} + (0.05 \times (2500\ \mathrm{psi}))\right] \times (48\ \mathrm{in}) \times (38.4\ \mathrm{in})$$

$$V_{c,b} = 348.89\ \mathrm{kip}$$

V_c - Governing shear strength of concrete

$$V_c = Min\left[V_{c,max}, V_{c,a}, V_{c,b}\right]$$

$$V_c = Min\left[(296.21\ \mathrm{kip}), (119.36\ \mathrm{kip}), (348.89\ \mathrm{kip})\right]$$

$$V_c = 119.36\ \mathrm{kip}$$

	The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5\ \mathrm{ksi} \rightarrow 2500\ \mathrm{psi}$,	
22.5.1.2	$V_{s,a}$ - Shear strength of steel (a) $$V_{s,a} = 8\sqrt{f'_{ck}}\, b_w\, d$$ $$V_{s,a} = 8 \times \sqrt{(2500\ \mathrm{psi})} \times (48\ \mathrm{in}) \times (38.4\ \mathrm{in})$$ $$V_{s,a} = 737.28\ \mathrm{kip}$$	
	A_v - Ties rebar area, $$A_v = \frac{\pi\, d_{ties}^2}{4}$$ $$A_v = \frac{\pi \times (0.375\ \mathrm{in})^2}{4}$$ $$A_v = 0.11045\ \mathrm{in}^2$$	
22.5.8.5.3	$V_{s,b}$ - Shear strength of steel (b) $$V_{s,b} = \frac{2\, A_v\, f_{ywk}\, d}{s_{ties}}$$ $$V_{s,b} = \frac{2 \times \left(0.11045\ \mathrm{in}^2\right) \times (60\ \mathrm{ksi}) \times (38.4\ \mathrm{in})}{(10\ \mathrm{in})}$$ $$V_{s,b} = 50.894\ \mathrm{kip}$$	
	V_s - Governing shear strength of steel $$V_s = MIN\left[V_{s,a}, V_{s,b}\right]$$ $$V_s = MIN\left[(737.28\ \mathrm{kip}), (50.894\ \mathrm{kip})\right]$$ $$V_s = 50.894\ \mathrm{kip}$$	
22.5.1.1	ϕV_n - Allowable shear strength $$\phi V_n = \phi\ (V_c + V_s)$$ $$\phi V_n = (0.65) \times ((119.36\ \mathrm{kip}) + (50.894\ \mathrm{kip}))$$ $$\phi V_n = 110.66\ \mathrm{kip}$$	
	Considering x-direction: $V_{max} = 10.224\ \mathrm{kip}$ - Maximum shear force in the x-direction, $Ratio$ - Capacity $$Ratio = \frac{V_{max}}{\phi V_n}$$ $$Ratio = \frac{(10.224\ \mathrm{kip})}{(110.66\ \mathrm{kip})}$$ $$Ratio = 0.092386$$	Status: **PASS** Ratio: **0.090**
	Considering z-direction: $V_{max} = 0.022411\ \mathrm{kip}$ - Maximum shear force in the z-direction, $Ratio$ - Capacity $$Ratio = \frac{V_{max}}{\phi V_n}$$ $$Ratio = \frac{(0.022411\ \mathrm{kip})}{(110.66\ \mathrm{kip})}$$ $$Ratio = 0.00020252$$	Status: **PASS** Ratio: **0.000**
	Flexural Strength (ACI 318-19, LRFD) S_m - Section modulus $$S_m = \frac{b\, D^2}{6}$$ $$S_m = \frac{(48\ \mathrm{in}) \times (48\ \mathrm{in})^2}{6}$$ $$S_m = 18432\ \mathrm{in}^3$$	

	$\lambda = 1$ - Concrete modification factor (Normal concrete), Allowable flexural strength: M_n shall be the lesser of: $\phi M_{n,1}$ $$\phi M_{n,1} = \phi \times 5 \times \lambda \times \sqrt{(f_c')} \times S_m$$ $$\phi M_{n,1} = 0.65 \times 5 \times 1 \times \sqrt{(2.5 ksi)} \times 18432.001 in^3$$ $$\phi M_{n,1} = 249.600 kipft$$	
14.5.2.1b	$\phi M_{n,2}$ $$\phi M_{n,2} = \phi \, 0.85 \, f_{ck}' \, S_m$$ $$\phi M_{n,2} = (0.65) \times 0.85 \times (2.5 \text{ ksi}) \times \left(18432 \text{ in}^3\right)$$ $$\phi M_{n,2} = 2121.6 \text{ kipft}$$ Therefore, ϕM_n - Allowable flexural strength, $$\phi M_n = MIN[\phi M_{n,1}, \phi M_{n,2}]$$ $$\phi M_n = MIN[(249.6 \text{ kipft}), (2121.6 \text{ kipft})]$$ $$\phi M_n = 249.6 \text{ kipft}$$ **Considering x-direction:** $M_{max} = 34.469 \text{ kipft}$ - Maximum moment in the x-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\phi M_n}$$ $$Ratio = \frac{(34.469 \text{ kipft})}{(249.6 \text{ kipft})}$$ $$Ratio = 0.1381$$	Status: **PASS** Ratio: **0.140**
	Considering z-direction: $M_{max} = 0.071241 \text{ kipft}$ - Maximum moment in the z-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\phi M_n}$$ $$Ratio = \frac{(0.071241 \text{ kipft})}{(249.6 \text{ kipft})}$$ $$Ratio = 0.00028542$$	Status: **PASS** Ratio: **0.000**

REFERENCES	CALCULATIONS	RESULTS

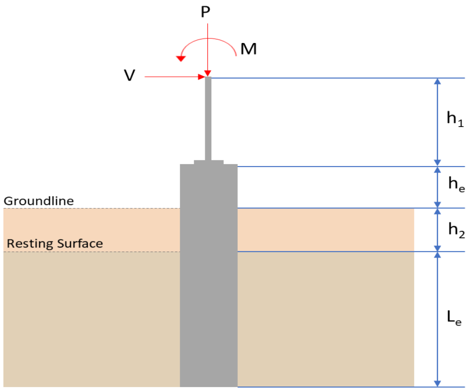
SkyCiv Foundation Design

Pile Foundation

Design Information :

Design code : IBC 2021 (International Building Code)
Unit System : Imperial

Pile Input



Geometry

Pile shape: rectangular
$b = 48\ \mathrm{in}$ - Pile width
$D = 48\ \mathrm{in}$ - Pile depth
$L = 7\ \mathrm{ft}$ - Total pile length
$h_1 = 0\ \mathrm{ft}$ - Lateral load height from the top of the pile,
$h_2 = 0\ \mathrm{ft}$ - Depth to resisting surface
$h_e = 0\ \mathrm{ft}$ - Length of pile above the ground

Tabulation of Soil Parameters

Layer	Label	Allowable Bearing Pressure (q_a) (psf)	Allowable Lateral Pressure (R) (psf/ft)
1	Sand, silty sand, clayey sand, silty gravel & clayey gravel	2000.000	150.000

Tabulation of Loads

Load Component	ASD	LRFD
P (kip)	4.861	6.759
V_x (kip)	-1.949	-3.247
V_z (kip)	0.000	0.000
M_x (kipft)	0.000	0.000
M_z (kipft)	32.343	55.142

Material Properties

$f'_{ck} = 2.5\ \mathrm{ksi}$ - Concrete strength,

Required depth to resist lateral loads (ASD)

H - Point of application of the lateral load

$$H = h_1 + h_2 + h_e$$

$$H = (0\ \mathrm{ft}) + (0\ \mathrm{ft}) + (0\ \mathrm{ft})$$

$$H = 0\ \mathrm{ft}$$

Considering x-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\,D}$$

$$H_o = \frac{(-1.949\ \mathrm{kip})}{1.57 \times (48\ \mathrm{in})}$$

$$H_o = -0.31035\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\,H)}{1.57\,D}$$

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$$M_o = \frac{(32.343\ \mathrm{kipft}) + ((-1.949\ \mathrm{kip}) \times (0\ \mathrm{ft}))}{1.57 \times (48\ \mathrm{in})}$$

$$M_o = 5.1502\ \mathrm{kipft/ft}$$

Required depth of embedment in earth:

$$L_x^3 - \left(14.14 \times \frac{H_o \times L_x}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,x} = 6.6108\ \mathrm{ft}$ - Required depth in x-direction,

Considering z-direction:

$L_{e,z} = 0\ \mathrm{ft}$ - Required depth in z-direction,

Minimum embedded depth required:

$L_{e,req}$ - Depth of pile required,

$$L_{e,req} = MAX[L_{e,x}, L_{e,z}]$$

$$L_{e,req} = MAX[(6.6108\ \mathrm{ft}), (0\ \mathrm{ft})]$$

$$L_{e,req} = 6.611\ \mathrm{ft}$$

L_e - Actual embedded length of pile,

$$L_e = L - h_e - h_2$$

$$L_e = (7\ \mathrm{ft}) - (0\ \mathrm{ft}) - (0\ \mathrm{ft})$$

$$L_e = 7\ \mathrm{ft}$$

$Ratio$ - Embedded depth

$$Ratio = \frac{L_{e,req}}{L_e}$$

$$Ratio = \frac{(6.611\ \mathrm{ft})}{(7\ \mathrm{ft})}$$

$$Ratio = 0.94443$$

Status: PASS
Ratio: 0.940

End-bearing Capacity (ASD)

A - Pile cross-section area

$$A = b\,D$$

$$A = (48\ \mathrm{in}) \times (48\ \mathrm{in})$$

$$A = 16\ \mathrm{ft}^2$$

q - End-bearing pressure

$$q = \frac{P_v}{A}$$

$$q = \frac{(4.861\ \mathrm{kip})}{(16\ \mathrm{ft}^2)}$$

$$q = 0.30381\ \mathrm{kip/ft}^2$$

Check bearing capacity ratio:

$Ratio$ - Capacity

$$Ratio = \frac{q}{q_a}$$

$$Ratio = \frac{(0.30381\ \mathrm{kip/ft}^2)}{(2000\ \mathrm{psf})}$$

$$Ratio = 0.15191$$

Status: PASS
Ratio: 0.150

Czerniak

Lateral Soil Pressure (ASD):

L/D - Length to least lateral dimension ratio,

$$L/D = \frac{L}{D}$$

$$L/D = \frac{(7\ \mathrm{ft})}{(48\ \mathrm{in})}$$

$$L/D = 1.75$$

Since L/D $\leq$ 10,

Pile is short.

Considering x-direction:

$H_o = -0.31035\ \text{kip/ft}$ - Lateral force per length of pile,
$M_o = 5.1502\ \text{kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (5.1502\ \text{kipft/ft}) \times (7\ \text{ft})) + \left(3 \times (-0.31035\ \text{kip/ft}) \times (7\ \text{ft})^2\right)}{(6 \times (5.1502\ \text{kipft/ft})) + (4 \times (-0.31035\ \text{kip/ft}) \times (7\ \text{ft}))}$$

$$a = 4.7947\ \text{ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\,[(4\,M_o) + (3\,H_o\,L_e)]^2}{L_e^2\,[(3\,M_o) + (2\,H_o\,L_e)]}$$

$$p = \frac{0.75 \times [(4 \times (5.1502\ \text{kipft/ft})) + (3 \times (-0.31035\ \text{kip/ft}) \times (7\ \text{ft}))]^{\ 2}}{(7\ \text{ft})^2 \times [(3 \times (5.1502\ \text{kipft/ft})) + (2 \times (-0.31035\ \text{kip/ft}) \times (7\ \text{ft}))]}$$

$$p = 0.27336\ \text{kip/ft}^2$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\,[(2\,M_o) + (H_o\,L_e)]}{L_e^2}$$

$$s = \frac{6 \times [(2 \times (5.1502\ \text{kipft/ft})) + ((-0.31035\ \text{kip/ft}) \times (7\ \text{ft}))]}{(7\ \text{ft})^2}$$

$$s = 0.99525\ \text{kip/ft}^2$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\,\frac{a}{2}$$

$$p_a = (150\ \text{psf/ft}) \times \frac{(4.7947\ \text{ft})}{2}$$

$$p_a = 0.3596\ \text{kip/ft}^2$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{\left(0.27336\ \text{kip/ft}^2\right)}{\left(0.3596\ \text{kip/ft}^2\right)}$$

$$Ratio = 0.76017$$

Status: **PASS**
Ratio: **0.760**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\,L_e$$

$$p_s = (150\ \text{psf/ft}) \times (7\ \text{ft})$$

$$p_s = 1.05\ \text{kip/ft}^2$$

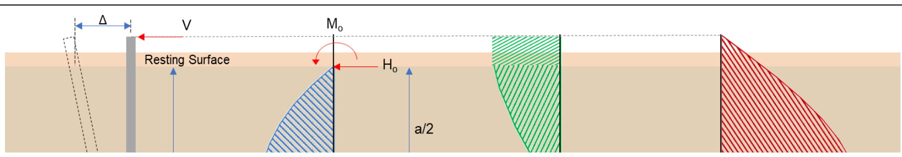
$Ratio$ - Lateral soil capacity

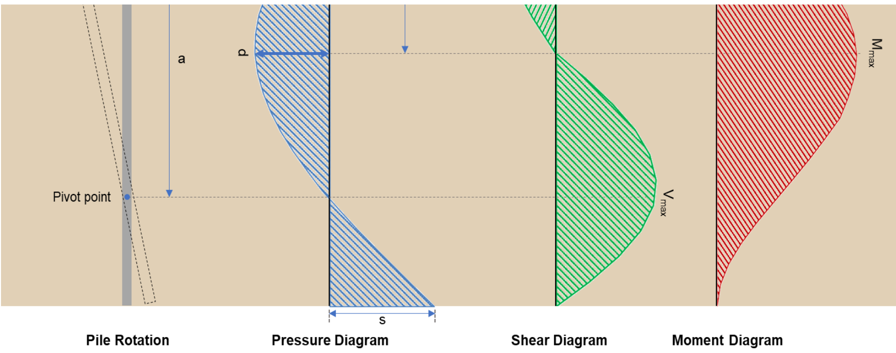
$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{\left(0.99525\ \text{kip/ft}^2\right)}{\left(1.05\ \text{kip/ft}^2\right)}$$

$$Ratio = 0.94786$$

Status: **PASS**
Ratio: **0.950**





Shear force and Bending moment (x-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\,D}$$

$$H_o = \frac{(-3.247\text{ kip})}{1.57 \times (48\text{ in})}$$

$$H_o = -0.51704\text{ kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\,H)}{1.57\,D}$$

$$M_o = \frac{(55.142\text{ kipft}) + ((-3.247\text{ kip}) \times (0\text{ ft}))}{1.57 \times (48\text{ in})}$$

$$M_o = 8.7806\text{ kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(8.7806\text{ kipft/ft})}{(-0.51704\text{ kip/ft})}$$

$$E = 16.982\text{ ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (8.7806\text{ kipft/ft}) \times (7\text{ ft})) + \left(3 \times (-0.51704\text{ kip/ft}) \times (7\text{ ft})^2\right)}{(6 \times (8.7806\text{ kipft/ft})) + (4 \times (-0.51704\text{ kip/ft}) \times (7\text{ ft}))}$$

$$a = 4.7924\text{ ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\,D)\left[1 - \left[3\left(\frac{4\,E}{L_e} + 3\right)\left(\frac{a}{L_e}\right)^2\right] + \left[4\left(\frac{3\,E}{L_e} + 2\right)\left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((-0.51704\text{ kip/ft}) \times (48\text{ in})) \times \left[1 - \left[3 \times \left(\frac{4 \times (16.982\text{ ft})}{(7\text{ ft})} + 3\right) \times \left(\frac{(4.7924\text{ ft})}{(7\text{ ft})}\right)^2\right] + \left[4 \times \left(\frac{3 \times (16.982\text{ ft})}{(7\text{ ft})} + 2\right) \times \left(\frac{(4.7924\text{ ft})}{(7\text{ ft})}\right)^3\right]\right]$$

$$V_{max} = 10.247\text{ kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\,D\,L_e)\left[\left(\frac{E}{L_e} + \frac{a}{2\,L_e}\right) - \left[\left(\frac{4\,E}{L_e} + 3\right)\left(\frac{a}{2\,L_e}\right)^3\right] + \left[\left(\frac{3\,E}{L_e} + 2\right)\left(\frac{a}{2\,L_e}\right)^4\right]\right]$$

$$M_{max} = ((-0.51704\text{ kip/ft}) \times (48\text{ in}) \times (7\text{ ft})) \times \left[\left(\frac{(16.982\text{ ft})}{(7\text{ ft})} + \frac{(4.7924\text{ ft})}{2 \times (7\text{ ft})}\right) - \left[\left(\frac{4 \times (16.982\text{ ft})}{(7\text{ ft})} + 3\right) \times \left(\frac{(4.7924\text{ ft})}{2 \times (7\text{ ft})}\right)^3\right] + \left[\left(\frac{3 \times (16.982\text{ ft})}{(7\text{ ft})} + 2\right) \times \left(\frac{(4.7924\text{ ft})}{2 \times (7\text{ ft})}\right)^4\right]\right]$$

$$M_{max} = 34.545 \text{ kipft}$$

Minimum Reinforcement Check (LRFD)

Parameters:

$f'_{ck} = 2.5 \text{ ksi}$ - Concrete strength,
$f_{yk} = 60 \text{ ksi}$ - Longitudinal reinforcement strength,
$\phi = 0.65$ - Reduction factor for axial strength,
Table 22.4.2.1 $\alpha = 0.8$ - Alpha factor for axial strength,
$A_g = 2304 \text{ in}^2$ - Gross area of concrete,

Longitudinal reinforcement:

Required reinforcement due to axial load, $A_{st,required}$
22.4.2.2, 10.6.1.1 $A_{st,required}$

$$A_{st,required} = Min\left[\frac{\frac{P}{\phi\ \alpha} - \left(0.85\ f'_{ck}\ A_g\right)}{f_{yk} - \left(0.85\ f'_{ck}\right)}, \left(0.08\ A_g\right)\right]$$

$$A_{st,required} = Min\left[\frac{\frac{(6.759 \text{ kip})}{(0.65)\times(0.8)} - \left(0.85 \times (2.5 \text{ ksi}) \times \left(2304 \text{ in}^2\right)\right)}{(60 \text{ ksi}) - (0.85 \times (2.5 \text{ ksi}))}, \left(0.08 \times \left(2304 \text{ in}^2\right)\right)\right]$$

$$A_{st,required} = -84.372 \text{ in}^2$$

A_{min} - Governing minimum reinforcement area,

$$A_{min} = Max\left[A_{st,required}, \left(0.0018\ A_g\right)\right]$$

$$A_{min} = Max\left[\left(-84.372 \text{ in}^2\right), \left(0.0018 \times \left(2304 \text{ in}^2\right)\right)\right]$$

$$A_{min} = 4.1472 \text{ in}^2$$

n_{rebar} - Required number of reinforcement,

$$n_{rebar} = \frac{A_{min}}{A_{rebar}}$$

$$n_{rebar} = \frac{\left(4.1472 \text{ in}^2\right)}{\left(0.3068 \text{ in}^2\right)}$$

$$n_{rebar} = 14$$

A_{st} - Actual total reinforcement area,

$$A_{st} = n_{rebar}\ \frac{\pi\ d_{bar}^2}{4}$$

$$A_{st} = (14) \times \frac{\pi \times (0.625 \text{ in})^2}{4}$$

$$A_{st} = 4.2951 \text{ in}^2$$

$Ratio$ - Capacity

$$Ratio = \frac{A_{min}}{A_{st}}$$

$$Ratio = \frac{\left(4.1472 \text{ in}^2\right)}{\left(4.2951 \text{ in}^2\right)}$$

$$Ratio = 0.96556$$

Status: **PASS**
Ratio: **0.970**

25.2.3 s_{rebar} - Minimum spacing of reinforcement,

$$s_{rebar} = Max\left[1.5, \left(1.5\ d_{bar}\right)\right]$$

$$s_{rebar} = Max\left[1.5, (1.5 \times (0.625 \text{ in}))\right]$$

$$s_{rebar} = 1.5 \text{ in}$$

Ties:

25.7.2.2 Since longitudinal reinforcement is $\leq$ No. 10ø: Use #3(0.375 in)
25.7.2.1 s_{ties} - Maximum spacing of ties,

$$s_{ties} = Min\left[\left(16\ d_{bar}\right), \left(48\ d_{ties}\right), Min\left(D, b\right)\right]$$

$$s_{ties} = Min\left[(16 \times (0.625 \text{ in})), (48 \times (0.375 \text{ in})), Min\left((48 \text{ in}), (48 \text{ in})\right)\right]$$

$$s_{ties} = 10 \text{ in}$$

Summary:

Main reinforcement: **14 - #5 (0.625 in)**

	Ties: **#3(0.375 in) - 10 in**	
22.4.2.2	**Axial Compression Strength (ACI 318-19, LRFD)** ϕP_N - Allowable axial compressive strength $$\phi P_N = \phi\ 0.80\ \left[\left(0.85\ f'_{ck}\ \left[A_g - A_{st}\right]\right) + \left(f_{yk}\ A_{st}\right)\right]$$ $$\phi P_N = (0.65) \times 0.80 \times \left[\left(0.85 \times (2.5\ \mathrm{ksi}) \times \left[\left(2304\ \mathrm{in}^2\right) - \left(4.2951\ \mathrm{in}^2\right)\right]\right) + \left((60\ \mathrm{ksi}) \times \left(4.2951\ \mathrm{in}^2\right)\right)\right]$$ $$\phi P_N = 2675.2\ \mathrm{kip}$$ $Ratio$ - Capacity $$Ratio = \frac{P}{\phi P_N}$$ $$Ratio = \frac{(6.759\ \mathrm{kip})}{(2675.2\ \mathrm{kip})}$$ $$Ratio = 0.0025266$$	Status: **PASS** Ratio: **0.000**
22.5.2.2 22.5.5.1.3 22.5.5.1.1 22.5.5.1.1(a) 22.5.5.1.2	**Shear Strength (ACI 318-19, LRFD)** **Parameters:** $b_w = 48\ \mathrm{in}$ - Effective width, d - Effective depth $$d = 0.80\ D$$ $$d = 0.80 \times (48\ \mathrm{in})$$ $$d = 38.4\ \mathrm{in}$$ λ_s - size effect modification factor $$\lambda_s = MIN\left[\sqrt{\frac{2}{1+\frac{d}{10}}}, 1\right]$$ $$\lambda_s = MIN\left[\sqrt{\frac{2}{1+\frac{(38.4\ \mathrm{in})}{10}}}, 1\right]$$ $$\lambda_s = 0.64282$$ The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5\ \mathrm{ksi} \rightarrow 2500\ \mathrm{psi}$, $V_{c,max}$ - Max shear strength of concrete $$V_{c,max} = 5\ \lambda_s\ \sqrt{f'_{ck}}\ b_w\ d$$ $$V_{c,max} = 5 \times (0.64282) \times \sqrt{(2500\ \mathrm{psi})} \times (48\ \mathrm{in}) \times (38.4\ \mathrm{in})$$ $$V_{c,max} = 296.21\ \mathrm{kip}$$ The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5\ \mathrm{ksi} \rightarrow 2500\ \mathrm{psi}$, $P = 6.759\ \mathrm{kip} \rightarrow 6759\ \mathrm{lbf}$, $V_{c,a}$ - Shear strength of concrete (a) $$V_{c,a} = \left[2\ \lambda_s\ \sqrt{f'_{ck}} + \frac{P}{6\ A_g}\right]\ b_w\ d$$ $$V_{c,a} = \left[2 \times (0.64282) \times \sqrt{(2500\ \mathrm{psi})} + \frac{(6759\ \mathrm{lbf})}{6 \times \left(2304\ \mathrm{in}^2\right)}\right] \times (48\ \mathrm{in}) \times (38.4\ \mathrm{in})$$ $$V_{c,a} = 119.39\ \mathrm{kip}$$ The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5\ \mathrm{ksi} \rightarrow 2500\ \mathrm{psi}$, $V_{c,b}$ - Shear strength of concrete (b) $$V_{c,b} = \left[2\ \lambda_s\ \sqrt{f'_{ck}} + \left(0.05\ f'_{ck}\right)\right]\ b_w\ d$$ $$V_{c,b} = \left[2 \times (0.64282) \times \sqrt{(2500\ \mathrm{psi})} + (0.05 \times (2500\ \mathrm{psi}))\right] \times (48\ \mathrm{in}) \times (38.4\ \mathrm{in})$$ $$V_{c,b} = 348.89\ \mathrm{kip}$$ V_c - Governing shear strength of concrete $$V_c = Min\left[V_{c,max}, V_{c,a}, V_{c,b}\right]$$ $$V_c = Min\left[(296.21\ \mathrm{kip}), (119.39\ \mathrm{kip}), (348.89\ \mathrm{kip})\right]$$ $$V_c = 119.39\ \mathrm{kip}$$	

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5\ \text{ksi} \rightarrow 2500\ \text{psi}$,

22.5.1.2 $V_{s,a}$ - Shear strength of steel (a)

$$V_{s,a} = 8\sqrt{f'_{ck}}\, b_w\, d$$

$$V_{s,a} = 8 \times \sqrt{(2500\ \text{psi})} \times (48\ \text{in}) \times (38.4\ \text{in})$$

$$V_{s,a} = 737.28\ \text{kip}$$

A_v - Ties rebar area,

$$A_v = \frac{\pi\, d_{ties}^2}{4}$$

$$A_v = \frac{\pi \times (0.375\ \text{in})^2}{4}$$

$$A_v = 0.11045\ \text{in}^2$$

22.5.8.5.3 $V_{s,b}$ - Shear strength of steel (b)

$$V_{s,b} = \frac{2\, A_v\, f_{ywk}\, d}{s_{ties}}$$

$$V_{s,b} = \frac{2 \times \left(0.11045\ \text{in}^2\right) \times (60\ \text{ksi}) \times (38.4\ \text{in})}{(10\ \text{in})}$$

$$V_{s,b} = 50.894\ \text{kip}$$

V_s - Governing shear strength of steel

$$V_s = MIN\left[V_{s,a}, V_{s,b}\right]$$

$$V_s = MIN\left[(737.28\ \text{kip}), (50.894\ \text{kip})\right]$$

$$V_s = 50.894\ \text{kip}$$

22.5.1.1 ϕV_n - Allowable shear strength

$$\phi V_n = \phi\ (V_c + V_s)$$

$$\phi V_n = (0.65) \times ((119.39\ \text{kip}) + (50.894\ \text{kip}))$$

$$\phi V_n = 110.68\ \text{kip}$$

Considering x-direction:

$V_{max} = 10.247\ \text{kip}$ - Maximum shear force in the x-direction,
$Ratio$ - Capacity

$$Ratio = \frac{V_{max}}{\phi V_n}$$

$$Ratio = \frac{(10.247\ \text{kip})}{(110.68\ \text{kip})}$$

$$Ratio = 0.092581$$

Status: **PASS**
Ratio: **0.090**

Flexural Strength (ACI 318-19, LRFD)

S_m - Section modulus

$$S_m = \frac{b\, D^2}{6}$$

$$S_m = \frac{(48\ \text{in}) \times (48\ \text{in})^2}{6}$$

$$S_m = 18432\ \text{in}^3$$

$\lambda = 1$ - Concrete modification factor (Normal concrete),
Allowable flexural strength:
M_n shall be the lesser of:
$\phi M_{n,1}$

$$\phi M_{n,1} = \phi \times 5 \times \lambda \times \sqrt{(f'_c)} \times S_m$$

$$\phi M_{n,1} = 0.65 \times 5 \times 1 \times \sqrt{(2.5ksi)} \times 18432.001in^3$$

$$\phi M_{n,1} = 249.600kipft$$

14.5.2.1b $\phi M_{n,2}$

$$\phi M_{n,2} = \phi\, 0.85\, f'_c\, S_m$$

$$\phi M_{n,2} = \phi\, 0.85\, f_{ck}\, S_m$$

$$\phi M_{n,2} = (0.65) \times 0.85 \times (2.5 \text{ ksi}) \times \left(18432 \text{ in}^3\right)$$

$$\phi M_{n,2} = 2121.6 \text{ kipft}$$

Therefore,
ϕM_n - Allowable flexural strength,

$$\phi M_n = MIN[\phi M_{n,1}, \phi M_{n,2}]$$

$$\phi M_n = MIN[(249.6 \text{ kipft}), (2121.6 \text{ kipft})]$$

$$\phi M_n = 249.6 \text{ kipft}$$

Considering x-direction:

$M_{max} = 34.545 \text{ kipft}$ - Maximum moment in the x-direction,
$Ratio$ - Capacity

$$Ratio = \frac{M_{max}}{\phi M_n}$$

$$Ratio = \frac{(34.545 \text{ kipft})}{(249.6 \text{ kipft})}$$

$$Ratio = 0.1384$$

Status: **PASS**
Ratio: **0.140**

REFERENCES	CALCULATIONS	RESULTS

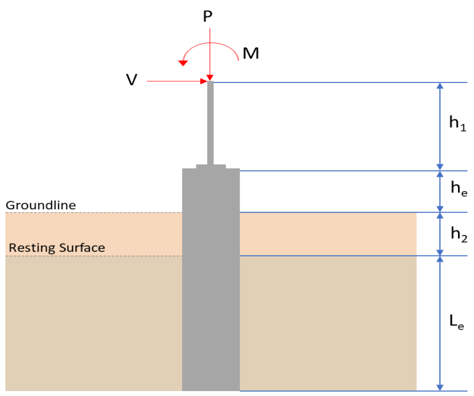
SkyCiv Foundation Design

Pile Foundation

Design Information :

Design code : IBC 2021 (International Building Code)
Unit System : Imperial

Pile Input



Geometry

Pile shape: rectangular
$b = 48\ \mathrm{in}$ - Pile width
$D = 48\ \mathrm{in}$ - Pile depth
$L = 7\ \mathrm{ft}$ - Total pile length
$h_1 = 0\ \mathrm{ft}$ - Lateral load height from the top of the pile,
$h_2 = 0\ \mathrm{ft}$ - Depth to resisting surface
$h_e = 0\ \mathrm{ft}$ - Length of pile above the ground

Tabulation of Soil Parameters

Layer	Label	Allowable Bearing Pressure (q_a) (psf)	Allowable Lateral Pressure (R) (psf/ft)
1	Sand, silty sand, clayey sand, silty gravel & clayey gravel	2000.000	150.000

Tabulation of Loads

Load Component	ASD	LRFD
P (kip)	4.701	6.535
V_x (kip)	-1.938	-3.231
V_z (kip)	0.011	0.018
M_x (kipft)	0.055	0.092
M_z (kipft)	32.271	55.037

Material Properties

$f'_{ck} = 2.5\ \mathrm{ksi}$ - Concrete strength,

Required depth to resist lateral loads (ASD)

H - Point of application of the lateral load

$$H = h_1 + h_2 + h_e$$

$$H = (0\ \mathrm{ft}) + (0\ \mathrm{ft}) + (0\ \mathrm{ft})$$

$$H = 0\ \mathrm{ft}$$

Considering x-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\ D}$$

$$H_o = \frac{(-1.938\ \mathrm{kip})}{1.57 \times (48\ \mathrm{in})}$$

$$H_o = -0.3086\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + (V_x\ H)}{1.57\ D}$$

$$M_o = \frac{(32.271\ \mathrm{kipft}) + ((-1.938\ \mathrm{kip}) \times (0\ \mathrm{ft}))}{1.57 \times (48\ \mathrm{in})}$$

$$M_o = 5.1387\ \mathrm{kipft/ft}$$

Required depth of embedment in earth:

$$L_x^3 - \left(14.14 \times \frac{H_o \times L_x}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,x} = 6.6093\ \mathrm{ft}$ - Required depth in x-direction,

Considering z-direction:

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\, b}$$

$$H_o = \frac{(0.011\ \mathrm{kip})}{1.57 \times (48\ \mathrm{in})}$$

$$H_o = 0.0017516\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\, H)}{1.57\, b}$$

$$M_o = \frac{(0.055\ \mathrm{kipft}) + ((0.011\ \mathrm{kip}) \times (0\ \mathrm{ft}))}{1.57 \times (48\ \mathrm{in})}$$

$$M_o = 0.008758\ \mathrm{kipft/ft}$$

Required depth of embedment in earth:

$$L_z^3 - \left(14.14 \times \frac{H_o \times L_z}{R}\right) - \left(18.85 \times \frac{M_o}{R}\right) = 0$$

Solving the cubic equation:
$L_{e,z} = 0.92762\ \mathrm{ft}$ - Required depth in z-direction,

Minimum embedded depth required:

$L_{e,req}$ - Depth of pile required,

$$L_{e,req} = MAX[L_{e,x}, L_{e,z}]$$

$$L_{e,req} = MAX[(6.6093\ \mathrm{ft}), (0.92762\ \mathrm{ft})]$$

$$L_{e,req} = 6.609\ \mathrm{ft}$$

L_e - Actual embedded length of pile,

$$L_e = L - h_e - h_2$$

$$L_e = (7\ \mathrm{ft}) - (0\ \mathrm{ft}) - (0\ \mathrm{ft})$$

$$L_e = 7\ \mathrm{ft}$$

$Ratio$ - Embedded depth

$$Ratio = \frac{L_{e,req}}{L_e}$$

$$Ratio = \frac{(6.609\ \mathrm{ft})}{(7\ \mathrm{ft})}$$

$$Ratio = 0.94414$$

Status: **PASS**
Ratio: **0.940**

End-bearing Capacity (ASD)

A - Pile cross-section area

$$A = b\, D$$

$$A = (48\ \mathrm{in}) \times (48\ \mathrm{in})$$

$$A = 16\ \mathrm{ft}^2$$

q - End-bearing pressure

$$q = \frac{P_v}{A}$$

$$q = \frac{(4.701\ \mathrm{kip})}{(16\ \mathrm{ft}^2)}$$

$$q = 0.29381\ \mathrm{kip/ft}^2$$

$$q = 0.29381\ \mathrm{kip/ft^2}$$

Check bearing capacity ratio:

$Ratio$ - Capacity

$$Ratio = \frac{q}{q_a}$$

$$Ratio = \frac{\left(0.29381\ \mathrm{kip/ft^2}\right)}{\left(2000\ \mathrm{psf}\right)}$$

$$Ratio = 0.14691$$

Status: **PASS**
Ratio: **0.150**

Czerniak

Lateral Soil Pressure (ASD):

L/D - Length to least lateral dimension ratio,

$$L/D = \frac{L}{D}$$

$$L/D = \frac{(7\ \mathrm{ft})}{(48\ \mathrm{in})}$$

$$L/D = 1.75$$

Since L/D $\leq$ 10,

Pile is short.

Considering x-direction:

$H_o = -0.3086\ \mathrm{kip/ft}$ - Lateral force per length of pile,
$M_o = 5.1387\ \mathrm{kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\, M_o\, L_e) + \left(3\, H_o\, L_e^2\right)}{(6\, M_o) + (4\, H_o\, L_e)}$$

$$a = \frac{(4 \times (5.1387\ \mathrm{kipft/ft}) \times (7\ \mathrm{ft})) + \left(3 \times (-0.3086\ \mathrm{kip/ft}) \times (7\ \mathrm{ft})^2\right)}{(6 \times (5.1387\ \mathrm{kipft/ft})) + (4 \times (-0.3086\ \mathrm{kip/ft}) \times (7\ \mathrm{ft}))}$$

$$a = 4.7944\ \mathrm{ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\ [(4\, M_o) + (3\, H_o\, L_e)]^2}{L_e^2\ [(3\, M_o) + (2\, H_o\, L_e)]}$$

$$p = \frac{0.75 \times [(4 \times (5.1387\ \mathrm{kipft/ft})) + (3 \times (-0.3086\ \mathrm{kip/ft}) \times (7\ \mathrm{ft}))]^{\ 2}}{(7\ \mathrm{ft})^2 \times [(3 \times (5.1387\ \mathrm{kipft/ft})) + (2 \times (-0.3086\ \mathrm{kip/ft}) \times (7\ \mathrm{ft}))]}$$

$$p = 0.27325\ \mathrm{kip/ft^2}$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\ [(2\, M_o) + (H_o\, L_e)]}{L_e^2}$$

$$s = \frac{6 \times [(2 \times (5.1387\ \mathrm{kipft/ft})) + ((-0.3086\ \mathrm{kip/ft}) \times (7\ \mathrm{ft}))]}{(7\ \mathrm{ft})^2}$$

$$s = 0.99394\ \mathrm{kip/ft^2}$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\, \frac{a}{2}$$

$$p_a = (150\ \mathrm{psf/ft}) \times \frac{(4.7944\ \mathrm{ft})}{2}$$

$$p_a = 0.35958\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{\left(0.27325\ \mathrm{kip/ft^2}\right)}{\left(0.35958\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = 0.75992$$

Status: **PASS**
Ratio: **0.760**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\,L_e$$

$$p_s = (150\ \mathrm{psf/ft}) \times (7\ \mathrm{ft})$$

$$p_s = 1.05\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{\left(0.99394\ \mathrm{kip/ft^2}\right)}{\left(1.05\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = 0.94661$$

Status: **PASS**
Ratio: **0.950**

Considering z-direction:

$H_o = 0.0017516\ \mathrm{kip/ft}$ - Lateral force per length of pile,
$M_o = 0.008758\ \mathrm{kipft/ft}$ - Overturning moment per length of pile,
a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4 \times (0.008758\ \mathrm{kipft/ft}) \times (7\ \mathrm{ft})) + \left(3 \times (0.0017516\ \mathrm{kip/ft}) \times (7\ \mathrm{ft})^2\right)}{(6 \times (0.008758\ \mathrm{kipft/ft})) + (4 \times (0.0017516\ \mathrm{kip/ft}) \times (7\ \mathrm{ft}))}$$

$$a = 4.9483\ \mathrm{ft}$$

p - Earth pressure against the pile at distance a/2 from resting surface,

$$p = \frac{0.75\,[(4\,M_o) + (3\,H_o\,L_e)]^2}{L_e^2\,[(3\,M_o) + (2\,H_o\,L_e)]}$$

$$p = \frac{0.75 \times [(4 \times (0.008758\ \mathrm{kipft/ft})) + (3 \times (0.0017516\ \mathrm{kip/ft}) \times (7\ \mathrm{ft}))]^{\ 2}}{(7\ \mathrm{ft})^2 \times [(3 \times (0.008758\ \mathrm{kipft/ft})) + (2 \times (0.0017516\ \mathrm{kip/ft}) \times (7\ \mathrm{ft}))]}$$

$$p = 0.0015541\ \mathrm{kip/ft^2}$$

s - Earth pressure against the pile at distance L_e,

$$s = \frac{6\,[(2\,M_o) + (H_o\,L_e)]}{L_e^2}$$

$$s = \frac{6 \times [(2 \times (0.008758\ \mathrm{kipft/ft})) + ((0.0017516\ \mathrm{kip/ft}) \times (7\ \mathrm{ft}))]}{(7\ \mathrm{ft})^2}$$

$$s = 0.0036462\ \mathrm{kip/ft^2}$$

Check lateral soil pressure capacity:

p_a - Allowable lateral soil pressure at depth a/2,

$$p_a = R\,\frac{a}{2}$$

$$p_a = (150\ \mathrm{psf/ft}) \times \frac{(4.9483\ \mathrm{ft})}{2}$$

$$p_a = 0.37112\ \mathrm{kip/ft^2}$$

$Ratio$ - Lateral soil capacity

$$Ratio = \frac{p}{p_a}$$

$$Ratio = \frac{\left(0.0015541\ \mathrm{kip/ft^2}\right)}{\left(0.37112\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = 0.0041875$$

Status: **PASS**
Ratio: **0.000**

p_s - Allowable lateral soil pressure at depth L_e,

$$p_s = R\,L_e$$

$$p_s = (150\ \mathrm{psf/ft}) \times (7\ \mathrm{ft})$$

$$p_s = 1.05\ \mathrm{kip/ft^2}$$

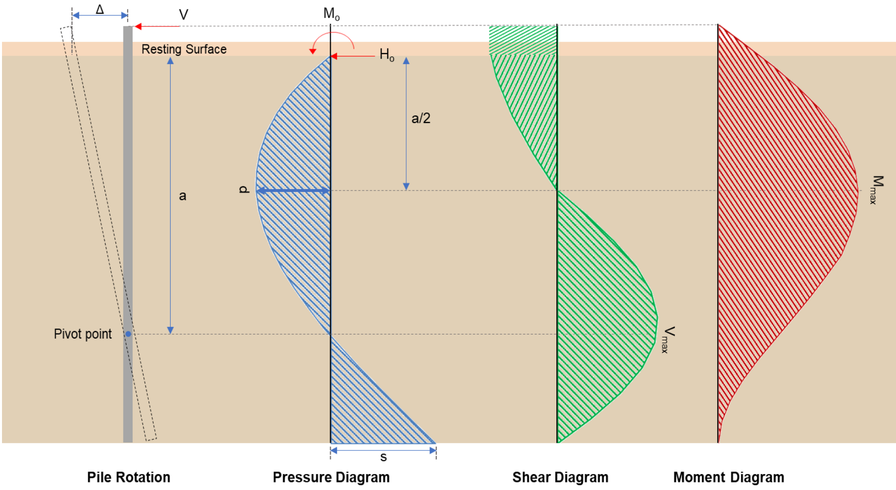
$Ratio$ - Lateral soil capacity

$$Ratio = \frac{s}{p_s}$$

$$Ratio = \frac{\left(0.0036462\ \mathrm{kip/ft^2}\right)}{\left(1.05\ \mathrm{kip/ft^2}\right)}$$

$$Ratio = 0.0034725$$

Status: **PASS**
Ratio: **0.000**



Shear force and Bending moment (x-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_x}{1.57\ D}$$

$$H_o = \frac{\left(-3.231\ \mathrm{kip}\right)}{1.57 \times \left(48\ \mathrm{in}\right)}$$

$$H_o = -0.51449\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_z + \left(V_x\ H\right)}{1.57\ D}$$

$$M_o = \frac{\left(55.037\ \mathrm{kipft}\right) + \left(\left(-3.231\ \mathrm{kip}\right) \times \left(0\ \mathrm{ft}\right)\right)}{1.57 \times \left(48\ \mathrm{in}\right)}$$

$$M_o = 8.7639\ \mathrm{kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{\left(8.7639\ \mathrm{kipft/ft}\right)}{\left(-0.51449\ \mathrm{kip/ft}\right)}$$

$$E = 17.034\ \mathrm{ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{\left(4\ M_o\ L_e\right) + \left(3\ H_o\ L_e^2\right)}{\left(6\ M_o\right) + \left(4\ H_o\ L_e\right)}$$

$$a = \frac{\left(4 \times \left(8.7639\ \mathrm{kipft/ft}\right) \times \left(7\ \mathrm{ft}\right)\right) + \left(3 \times \left(-0.51449\ \mathrm{kip/ft}\right) \times \left(7\ \mathrm{ft}\right)^2\right)}{\left(6 \times \left(8.7639\ \mathrm{kipft/ft}\right)\right) + \left(4 \times \left(-0.51449\ \mathrm{kip/ft}\right) \times \left(7\ \mathrm{ft}\right)\right)}$$

$$a = 4.7921\ \mathrm{ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = \left(H_o\ D\right) \left[1 - \left[3 \left(\frac{4\ E}{L_e} + 3\right) \left(\frac{a}{L_e}\right)^2\right] + \left[4 \left(\frac{3\ E}{L_e} + 2\right) \left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = \left(\left(-0.51449\ \mathrm{kip/ft}\right) \times \left(48\ \mathrm{in}\right)\right) \times \left[1 - \left[3 \times \left(\frac{4 \times \left(17.034\ \mathrm{ft}\right)}{\left(7\ \mathrm{ft}\right)} + 3\right) \times \left(\frac{\left(4.7921\ \mathrm{ft}\right)}{\left(7\ \mathrm{ft}\right)}\right)^2\right] + \left[4 \times \left(\frac{3 \times \left(17.034\ \mathrm{ft}\right)}{\left(7\ \mathrm{ft}\right)} + 2\right) \times \left(\frac{\left(4.7921\ \mathrm{ft}\right)}{\left(7\ \mathrm{ft}\right)}\right)^3\right]\right]$$

$$V_{max} = 10.224\ \mathrm{kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\,D\,L_e)\left[\left(\frac{E}{L_e}+\frac{a}{2\,L_e}\right) - \left[\left(\frac{4\,E}{L_e}+3\right)\left(\frac{a}{2\,L_e}\right)^3\right] + \left[\left(\frac{3\,E}{L_e}+2\right)\left(\frac{a}{2\,L_e}\right)^4\right]\right]$$

$$M_{max} = ((-0.51449\ \mathrm{kip/ft})\times(48\ \mathrm{in})\times(7\ \mathrm{ft}))\times\left[\left(\frac{(17.034\ \mathrm{ft})}{(7\ \mathrm{ft})}+\frac{(4.7921\ \mathrm{ft})}{2\times(7\ \mathrm{ft})}\right) - \left[\left(\frac{4\times(17.034\ \mathrm{ft})}{(7\ \mathrm{ft})}+3\right)\times\left(\frac{(4.7921\ \mathrm{ft})}{2\times(7\ \mathrm{ft})}\right)^3\right] + \left[\left(\frac{3\times(17.034\ \mathrm{ft})}{(7\ \mathrm{ft})}+2\right)\times\left(\frac{(4.7921\ \mathrm{ft})}{2\times(7\ \mathrm{ft})}\right)^4\right]\right]$$

$$M_{max} = 34.469\ \mathrm{kipft}$$

Shear force and Bending moment (z-direction, LRFD)

H_o - Lateral force per length of pile,

$$H_o = \frac{V_z}{1.57\,b}$$

$$H_o = \frac{(0.018\ \mathrm{kip})}{1.57\times(48\ \mathrm{in})}$$

$$H_o = 0.0028662\ \mathrm{kip/ft}$$

M_o - Moment per length of pile,

$$M_o = \frac{M_x + (V_z\,H)}{1.57\,b}$$

$$M_o = \frac{(0.092\ \mathrm{kipft}) + ((0.018\ \mathrm{kip})\times(0\ \mathrm{ft}))}{1.57\times(48\ \mathrm{in})}$$

$$M_o = 0.01465\ \mathrm{kipft/ft}$$

E - Distance from lateral load to resisting surface,

$$E = \frac{M_o}{H_o}$$

$$E = \frac{(0.01465\ \mathrm{kipft/ft})}{(0.0028662\ \mathrm{kip/ft})}$$

$$E = 5.1111\ \mathrm{ft}$$

a - Distance from resting surface to pivot point,

$$a = \frac{(4\,M_o\,L_e) + \left(3\,H_o\,L_e^2\right)}{(6\,M_o) + (4\,H_o\,L_e)}$$

$$a = \frac{(4\times(0.01465\ \mathrm{kipft/ft})\times(7\ \mathrm{ft})) + \left(3\times(0.0028662\ \mathrm{kip/ft})\times(7\ \mathrm{ft})^2\right)}{(6\times(0.01465\ \mathrm{kipft/ft})) + (4\times(0.0028662\ \mathrm{kip/ft})\times(7\ \mathrm{ft}))}$$

$$a = 4.9451\ \mathrm{ft}$$

V_{max} - Max shear force located at depth a,

$$V_{max} = (H_o\,b)\left[1 - \left[3\left(\frac{4\,E}{L_e}+3\right)\left(\frac{a}{L_e}\right)^2\right] + \left[4\left(\frac{3\,E}{L_e}+2\right)\left(\frac{a}{L_e}\right)^3\right]\right]$$

$$V_{max} = ((0.0028662\ \mathrm{kip/ft})\times(48\ \mathrm{in}))\times\left[1 - \left[3\times\left(\frac{4\times(5.1111\ \mathrm{ft})}{(7\ \mathrm{ft})}+3\right)\times\left(\frac{(4.9451\ \mathrm{ft})}{(7\ \mathrm{ft})}\right)^2\right] + \left[4\times\left(\frac{3\times(5.1111\ \mathrm{ft})}{(7\ \mathrm{ft})}+2\right)\times\left(\frac{(4.9451\ \mathrm{ft})}{(7\ \mathrm{ft})}\right)^3\right]\right]$$

$$V_{max} = 0.022411\ \mathrm{kip}$$

M_{max} - Max bending moment located at depth a/2,

$$M_{max} = (H_o\,b\,L_e)\left[\left(\frac{E}{L_e}+\frac{a}{2\,L_e}\right) - \left[\left(\frac{4\,E}{L_e}+3\right)\left(\frac{a}{2\,L_e}\right)^3\right] + \left[\left(\frac{3\,E}{L_e}+2\right)\left(\frac{a}{2\,L_e}\right)^4\right]\right]$$

$$M_{max} = ((0.0028662\ \mathrm{kip/ft})\times(48\ \mathrm{in})\times(7\ \mathrm{ft}))\times\left[\left(\frac{(5.1111\ \mathrm{ft})}{(7\ \mathrm{ft})}+\frac{(4.9451\ \mathrm{ft})}{2\times(7\ \mathrm{ft})}\right) - \left[\left(\frac{4\times(5.1111\ \mathrm{ft})}{(7\ \mathrm{ft})}+3\right)\times\left(\frac{(4.9451\ \mathrm{ft})}{2\times(7\ \mathrm{ft})}\right)^3\right] + \left[\left(\frac{3\times(5.1111\ \mathrm{ft})}{(7\ \mathrm{ft})}+2\right)\times\left(\frac{(4.9451\ \mathrm{ft})}{2\times(7\ \mathrm{ft})}\right)^4\right]\right]$$

	$$M_{max} = 0.071241 \text{ kipft}$$	

Minimum Reinforcement Check (LRFD)

Parameters:

$f'_{ck} = 2.5 \text{ ksi}$ - Concrete strength,
$f_{yk} = 60 \text{ ksi}$ - Longitudinal reinforcement strength,
$\phi = 0.65$ - Reduction factor for axial strength,
Table 22.4.2.1 $\alpha = 0.8$ - Alpha factor for axial strength,
$A_g = 2304 \text{ in}^2$ - Gross area of concrete,

Longitudinal reinforcement:

Required reinforcement due to axial load, $A_{st,required}$
22.4.2.2, 10.6.1.1 $A_{st,required}$

$$A_{st,required} = Min\left[\frac{\frac{P}{\phi\ \alpha} - (0.85\ f'_{ck}\ A_g)}{f_{yk} - (0.85\ f'_{ck})}, (0.08\ A_g)\right]$$

$$A_{st,required} = Min\left[\frac{\frac{(6.535 \text{ kip})}{(0.65)\times(0.8)} - \left(0.85 \times (2.5 \text{ ksi}) \times \left(2304 \text{ in}^2\right)\right)}{(60 \text{ ksi}) - (0.85 \times (2.5 \text{ ksi}))}, \left(0.08 \times \left(2304 \text{ in}^2\right)\right)\right]$$

$$A_{st,required} = -84.379 \text{ in}^2$$

A_{min} - Governing minimum reinforcement area,

$$A_{min} = Max\left[A_{st,required}, (0.0018\ A_g)\right]$$

$$A_{min} = Max\left[\left(-84.379 \text{ in}^2\right), \left(0.0018 \times \left(2304 \text{ in}^2\right)\right)\right]$$

$$A_{min} = 4.1472 \text{ in}^2$$

n_{rebar} - Required number of reinforcement,

$$n_{rebar} = \frac{A_{min}}{A_{rebar}}$$

$$n_{rebar} = \frac{\left(4.1472 \text{ in}^2\right)}{\left(0.3068 \text{ in}^2\right)}$$

$$n_{rebar} = 14$$

A_{st} - Actual total reinforcement area,

$$A_{st} = n_{rebar}\ \frac{\pi\ d_{bar}^2}{4}$$

$$A_{st} = (14) \times \frac{\pi \times (0.625 \text{ in})^2}{4}$$

$$A_{st} = 4.2951 \text{ in}^2$$

$Ratio$ - Capacity

$$Ratio = \frac{A_{min}}{A_{st}}$$

$$Ratio = \frac{\left(4.1472 \text{ in}^2\right)}{\left(4.2951 \text{ in}^2\right)}$$

$$Ratio = 0.96556$$

Status: **PASS**
Ratio: **0.970**

25.2.3 s_{rebar} - Minimum spacing of reinforcement,

$$s_{rebar} = Max\left[1.5, (1.5\ d_{bar})\right]$$

$$s_{rebar} = Max\left[1.5, (1.5 \times (0.625 \text{ in}))\right]$$

$$s_{rebar} = 1.5 \text{ in}$$

Ties:

25.7.2.2 Since longitudinal reinforcement is $\leq$ No. 10ø: Use #3(0.375 in)
25.7.2.1 s_{ties} - Maximum spacing of ties,

$$s_{ties} = Min\left[(16\ d_{bar}), (48\ d_{ties}), Min\,(D, b)\right]$$

$$s_{ties} = Min\left[(16 \times (0.625 \text{ in})), (48 \times (0.375 \text{ in})), Min\,((48 \text{ in}), (48 \text{ in}))\right]$$

$$s_{ties} = 10 \text{ in}$$

Summary:

Main reinforcement: **14 - #5 (0.625 in)**

	Ties: **#3(0.375 in) - 10 in**	

	Axial Compression Strength (ACI 318-19, LRFD)	
22.4.2.2	ϕP_N - Allowable axial compressive strength $$\phi P_N = \phi\, 0.80\, \left[\left(0.85\, f'_{ck}\, \left[A_g - A_{st}\right]\right) + \left(f_{yk}\, A_{st}\right)\right]$$ $$\phi P_N = (0.65) \times 0.80 \times \left[\left(0.85 \times (2.5\ \mathrm{ksi}) \times \left[\left(2304\ \mathrm{in}^2\right) - \left(4.2951\ \mathrm{in}^2\right)\right]\right) + \left((60\ \mathrm{ksi}) \times \left(4.2951\ \mathrm{in}^2\right)\right)\right]$$ $$\phi P_N = 2675.2\ \mathrm{kip}$$ $Ratio$ - Capacity $$Ratio = \frac{P}{\phi P_N}$$ $$Ratio = \frac{(6.535\ \mathrm{kip})}{(2675.2\ \mathrm{kip})}$$ $$Ratio = 0.0024428$$	Status: **PASS** Ratio: **0.000**

	Shear Strength (ACI 318-19, LRFD) **Parameters:** $b_w = 48\ \mathrm{in}$ - Effective width,	
22.5.2.2	d - Effective depth $$d = 0.80\, D$$ $$d = 0.80 \times (48\ \mathrm{in})$$ $$d = 38.4\ \mathrm{in}$$	
22.5.5.1.3	λ_s - size effect modification factor $$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{d}{10}}}, 1\right]$$ $$\lambda_s = MIN\left[\sqrt{\frac{2}{1 + \frac{(38.4\ \mathrm{in})}{10}}}, 1\right]$$ $$\lambda_s = 0.64282$$ The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5\ \mathrm{ksi} \rightarrow 2500\ \mathrm{psi}$,	
22.5.5.1.1	$V_{c,max}$ - Max shear strength of concrete $$V_{c,max} = 5\, \lambda_s\, \sqrt{f'_{ck}}\, b_w\, d$$ $$V_{c,max} = 5 \times (0.64282) \times \sqrt{(2500\ \mathrm{psi})} \times (48\ \mathrm{in}) \times (38.4\ \mathrm{in})$$ $$V_{c,max} = 296.21\ \mathrm{kip}$$ The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5\ \mathrm{ksi} \rightarrow 2500\ \mathrm{psi}$, $P = 6.535\ \mathrm{kip} \rightarrow 6535\ \mathrm{lbf}$,	
22.5.5.1.1(a)	$V_{c,a}$ - Shear strength of concrete (a) $$V_{c,a} = \left[2\, \lambda_s\, \sqrt{f'_{ck}} + \frac{P}{6\, A_g}\right]\, b_w\, d$$ $$V_{c,a} = \left[2 \times (0.64282) \times \sqrt{(2500\ \mathrm{psi})} + \frac{(6535\ \mathrm{lbf})}{6 \times \left(2304\ \mathrm{in}^2\right)}\right] \times (48\ \mathrm{in}) \times (38.4\ \mathrm{in})$$ $$V_{c,a} = 119.36\ \mathrm{kip}$$ The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5\ \mathrm{ksi} \rightarrow 2500\ \mathrm{psi}$,	
22.5.5.1.2	$V_{c,b}$ - Shear strength of concrete (b) $$V_{c,b} = \left[2\, \lambda_s\, \sqrt{f'_{ck}} + \left(0.05\, f'_{ck}\right)\right]\, b_w\, d$$ $$V_{c,b} = \left[2 \times (0.64282) \times \sqrt{(2500\ \mathrm{psi})} + (0.05 \times (2500\ \mathrm{psi}))\right] \times (48\ \mathrm{in}) \times (38.4\ \mathrm{in})$$ $$V_{c,b} = 348.89\ \mathrm{kip}$$ V_c - Governing shear strength of concrete $$V_c = Min\left[V_{c,max}, V_{c,a}, V_{c,b}\right]$$ $$V_c = Min\left[(296.21\ \mathrm{kip}), (119.36\ \mathrm{kip}), (348.89\ \mathrm{kip})\right]$$ $$V_c = 119.36\ \mathrm{kip}$$	

The following variables were converted to be consistent with empirical formula $f'_{ck} = 2.5\ \text{ksi} \rightarrow 2500\ \text{psi}$,

22.5.1.2 $V_{s,a}$ - Shear strength of steel (a)

$$V_{s,a} = 8\sqrt{f'_{ck}}\, b_w\, d$$

$$V_{s,a} = 8 \times \sqrt{(2500\ \text{psi})} \times (48\ \text{in}) \times (38.4\ \text{in})$$

$$V_{s,a} = 737.28\ \text{kip}$$

A_v - Ties rebar area,

$$A_v = \frac{\pi\, d_{ties}^2}{4}$$

$$A_v = \frac{\pi \times (0.375\ \text{in})^2}{4}$$

$$A_v = 0.11045\ \text{in}^2$$

22.5.8.5.3 $V_{s,b}$ - Shear strength of steel (b)

$$V_{s,b} = \frac{2\, A_v\, f_{ywk}\, d}{s_{ties}}$$

$$V_{s,b} = \frac{2 \times \left(0.11045\ \text{in}^2\right) \times (60\ \text{ksi}) \times (38.4\ \text{in})}{(10\ \text{in})}$$

$$V_{s,b} = 50.894\ \text{kip}$$

V_s - Governing shear strength of steel

$$V_s = MIN\left[V_{s,a}, V_{s,b}\right]$$

$$V_s = MIN\left[(737.28\ \text{kip}), (50.894\ \text{kip})\right]$$

$$V_s = 50.894\ \text{kip}$$

22.5.1.1 ϕV_n - Allowable shear strength

$$\phi V_n = \phi\ (V_c + V_s)$$

$$\phi V_n = (0.65) \times ((119.36\ \text{kip}) + (50.894\ \text{kip}))$$

$$\phi V_n = 110.66\ \text{kip}$$

Considering x-direction:

$V_{max} = 10.224\ \text{kip}$ - Maximum shear force in the x-direction,
$Ratio$ - Capacity

$$Ratio = \frac{V_{max}}{\phi V_n}$$

$$Ratio = \frac{(10.224\ \text{kip})}{(110.66\ \text{kip})}$$

$$Ratio = 0.092384$$

Status: **PASS**
Ratio: **0.090**

Considering z-direction:

$V_{max} = 0.022411\ \text{kip}$ - Maximum shear force in the z-direction,
$Ratio$ - Capacity

$$Ratio = \frac{V_{max}}{\phi V_n}$$

$$Ratio = \frac{(0.022411\ \text{kip})}{(110.66\ \text{kip})}$$

$$Ratio = 0.00020252$$

Status: **PASS**
Ratio: **0.000**

Flexural Strength (ACI 318-19, LRFD)

S_m - Section modulus

$$S_m = \frac{b\, D^2}{6}$$

$$S_m = \frac{(48\ \text{in}) \times (48\ \text{in})^2}{6}$$

$$S_m = 18432\ \text{in}^3$$

	$\lambda = 1$ - Concrete modification factor (Normal concrete), Allowable flexural strength: M_n shall be the lesser of: $\phi M_{n,1}$ $$\phi M_{n,1} = \phi \times 5 \times \lambda \times \sqrt{(f_c')} \times S_m$$ $$\phi M_{n,1} = 0.65 \times 5 \times 1 \times \sqrt{(2.5ksi)} \times 18432.001in^3$$ $$\phi M_{n,1} = 249.600kipft$$	
14.5.2.1b	$\phi M_{n,2}$ $$\phi M_{n,2} = \phi\, 0.85\, f_{ck}'\, S_m$$ $$\phi M_{n,2} = (0.65) \times 0.85 \times (2.5 \text{ ksi}) \times \left(18432 \text{ in}^3\right)$$ $$\phi M_{n,2} = 2121.6 \text{ kipft}$$ Therefore, ϕM_n - Allowable flexural strength, $$\phi M_n = MIN[\phi M_{n,1}, \phi M_{n,2}]$$ $$\phi M_n = MIN[(249.6 \text{ kipft}), (2121.6 \text{ kipft})]$$ $$\phi M_n = 249.6 \text{ kipft}$$ **Considering x-direction:** $M_{max} = 34.469 \text{ kipft}$ - Maximum moment in the x-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\phi M_n}$$ $$Ratio = \frac{(34.469 \text{ kipft})}{(249.6 \text{ kipft})}$$ $$Ratio = 0.1381$$	Status: **PASS** Ratio: **0.140**
	Considering z-direction: $M_{max} = 0.071241 \text{ kipft}$ - Maximum moment in the z-direction, $Ratio$ - Capacity $$Ratio = \frac{M_{max}}{\phi M_n}$$ $$Ratio = \frac{(0.071241 \text{ kipft})}{(249.6 \text{ kipft})}$$ $$Ratio = 0.00028542$$	Status: **PASS** Ratio: **0.000**